

# Nonlinear Legendre Spectral Finite Elements for Wind Turbine Blade Dynamics



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# Motivation

- ▶ Beam model currently used in FAST
  - Euler-Bernoulli beam model with shortening effect
  - Two degree-of-freedom
  - Assumed-mode method
- ▶ Beam models used in other wind turbine tools
  - Multibody-formulation
  - Linear beam models
  - Constraints introduced between linear beams to describe large deflections and rotations
  - Finite element method

# Objective

- ▶ Objective: create efficient high-fidelity beam models for wind turbine blade analysis that can
  - Capture geometrical nonlinearity systematically
  - Capture anisotropic and heterogeneous behavior of composite materials rigorously
  - Modeling moving beams (translation and rotation)
  - Achieve the speed of computational design without significant loss of accuracy comparing to the ultimate accuracy obtained by 3D nonlinear FEA



NWTC static and dynamic blade tests showing typical large, elastic deflections.

# Approach

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- ▶ Implementation
  - Geometrically Exact Beam Theory (GEBT)

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    - ▶ Mixed implementation (Yu and Blair, 2012)
  - Legendre Spectral Finite Element (LSFE) (Patera, 1984)



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    - ▶ Limited usage in structural dynamics
  - FAST Modularization Framework (Jonkman, 2013)
- ▶ Result: BeamDyn, which can be used as a structural module of FAST

## ► Governing Equation

$$\begin{aligned}\dot{\underline{h}} - \underline{F}' &= \underline{f} \\ \underline{\dot{g}} + \tilde{u}\underline{h} - \underline{M}' - (\tilde{x}'_0 + \tilde{u}')\underline{F} &= \underline{m}\end{aligned}$$

## ► Constitutive Equation

$$\begin{aligned}\begin{Bmatrix} \underline{h} \\ \underline{g} \end{Bmatrix} &= \underline{\underline{\mathcal{M}}} \begin{Bmatrix} \underline{\dot{u}} \\ \underline{\omega} \end{Bmatrix} \\ \begin{Bmatrix} \underline{F} \\ \underline{M} \end{Bmatrix} &= \underline{\underline{\mathcal{C}}} \begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix}\end{aligned}$$

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- Elastic couplings are captured

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- Elastic couplings are captured
- Timoshenko-like beam model

## ► Strain Measures

$$\begin{Bmatrix} \underline{\epsilon} \\ \underline{\kappa} \end{Bmatrix} = \begin{Bmatrix} \underline{x}'_0 + \underline{u}' - (\underline{\underline{R}} \underline{\underline{R}}_0) \bar{\underline{l}}_1 \\ \underline{k} + \underline{\underline{R}} \underline{\underline{k}}_i \end{Bmatrix}$$

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- Timoshenko-like beam model
- Geometrically exact: deformed beam geometry is represented exactly



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- $\mathcal{M}$  and  $\mathcal{C}$  are  $6 \times 6$  sectional mass and stiffness matrices, respctively
- Elastic couplings are captured
- Timoshenko-like beam model
- Geometrically exact: deformed beam geometry is represented exactly
- Small strains

## ▶ Wiener-Milenković Rotation Parameters

$$\underline{p} = 4 \tan \left( \frac{\phi}{4} \right) \bar{n}$$

## ▶ Interpolation of 3D Finite Rotation Field (Jelenić and Crisfield, 1999)

- Rotations do not form a linear space so that they must be “composed” rather than added
- A rescaling operation is needed to eliminate the singularity existing in the vectorial rotation parameters
- Rotation field lacks “objectivity”

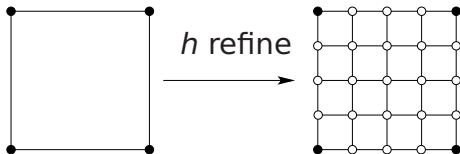
## ▶ Generalized- $\alpha$ Time Integration Scheme

- Unconditionally stable
- Second-order accurate
- High frequency numerical dissipation can be achieved by choosing proper parameters

# *h*-type Finite Elements

FEs in commercial codes (e.g., ABAQUS, ANSYS) typically employ Lagrangian-interpolant shape functions of fixed polynomial order  $N$

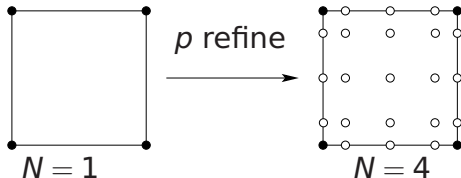
- ▶ Typically low order; linear ( $N = 1$ ) or quadratic ( $N = 2$ )
- ▶ Solution improved through  $h$ -refinement; increasing number of elements in the domain
- ▶ Exhibit *algebraic* convergence for sufficiently smooth problems; error  $\sim O(h^{N+1})$



# Legendre Spectral Finite Elements

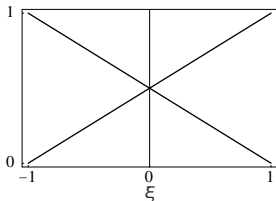
LSFE methods combine the geometric flexibility of the FE method with the accuracy of global spectral methods.

- ▶ Solution improved through increased basis polynomial order ( $p$ -refinement)
- ▶ LSFEs employ Lagrangian interpolant shape functions with nodes at Gauss-Lobatto-Legendre (GLL) points
- ▶ *Exponential* convergence rates for sufficiently smooth solutions

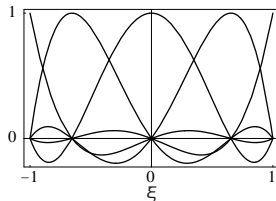


# LSFE: Example 1D Basis Functions

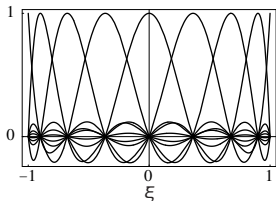
$(N + 1)$  LSFE nodes located in  $[-1, 1]$  at zeros of  $(1 - \xi^2)P'_N(\xi)$ , where  $P_N(\xi)$  is the  $N$ -degree Legendre polynomial



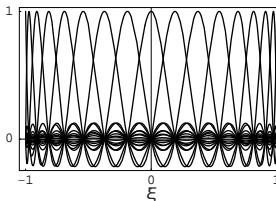
(a)  $N = 1$



(b)  $N = 4$



(c)  $N = 8$



(d)  $N = 16$

# Example 1: A Cantilevered Beam

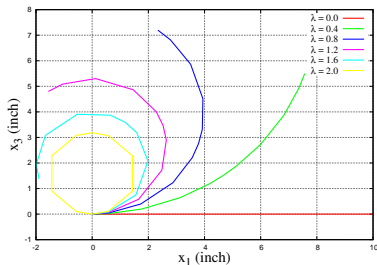
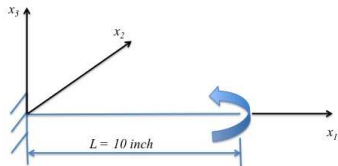
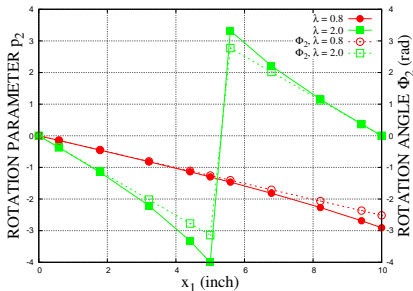


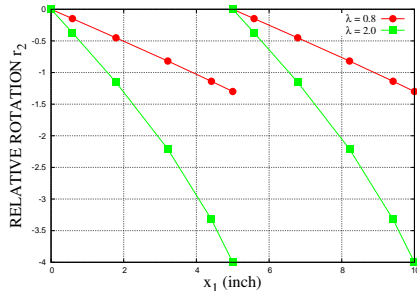
Table: Comparison of analytical and BeamDyn-calculated tip displacements  $u_1$  and  $u_3$  (in inches) of a cantilever beam subjected to a constant moment; the BeamDyn model was composed of two 5<sup>th</sup>-order LSFES.

$\lambda$	Analytical ( $u_1$ )	BeamDyn ( $u_1$ )	Analytical ( $u_3$ )	BeamDyn ( $u_3$ )
0.4	-2.4317	-2.4317	5.4987	5.4987
0.8	-7.6613	-7.6613	7.1978	7.1979
1.2	-11.5591	-11.5591	4.7986	4.7986
1.6	-11.8921	-11.8921	1.3747	1.3747
2.0	-10.0000	-10.0000	0.0000	0.0000

# Example 1: 3D Rotations



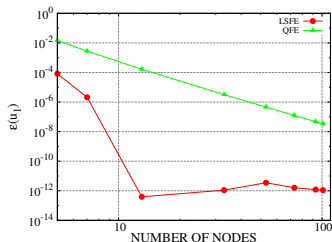
(a) Rotation parameter  $p_2$  and rotation angle  $\phi_2$



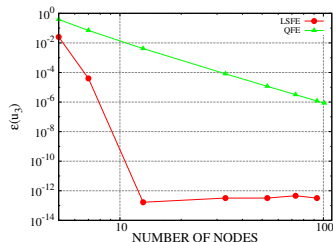
(b) Relative rotation  $r_2$

Figure: (a) Wiener-Milenković rotation parameters and rotation angles along the beam axis  $x_1$  as calculated by BeamDyn for two tip moments; (b) relative rotations in the two elements.

# Example 1: Convergence study



(a)  $u_1$



(b)  $u_3$

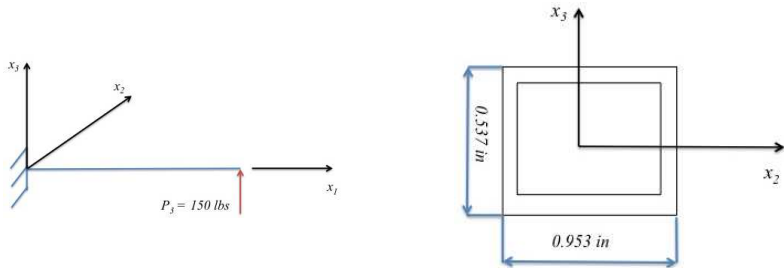
Figure: Normalized error of the (a)  $u_1$  and (b)  $u_3$  tip displacements of a cantilever beam under constant tip moment ( $\lambda = 1.0$ ) as a function of the total number of nodes. Results were calculated with BeamDyn (LSFE) and Dymore (QFE).

- ▶ Dymore is a well-known FE based multibody dynamics code for the comprehensive modeling of flexible multibody systems
- ▶ Conventional quadratic elements are limited to algebraic convergence.
- ▶ LSFES (with  $p$ -refinement) exhibit highly desirable exponential convergence to machine-precision error.
- ▶ The total numbers of iterations in Newton-Raphson method are the same.



# Example 2: Composite beam

## ► Sketch of Example 2

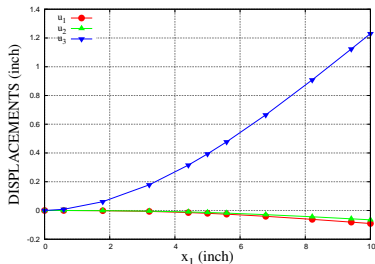


## ► Sectional stiffness matrix

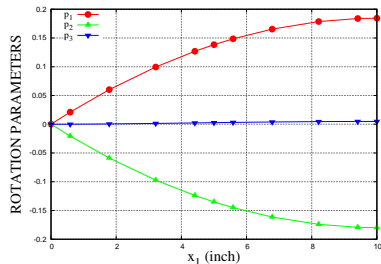
$$C^* = 10^3 \times \begin{bmatrix} 1368.17 & 0 & 0 & 0 & 0 & 0 \\ 0 & 88.56 & 0 & 0 & 0 & 0 \\ 0 & 0 & 38.78 & 0 & 0 & 0 \\ 0 & 0 & 0 & 16.96 & 17.61 & -0.351 \\ 0 & 0 & 0 & 17.61 & 59.12 & -0.370 \\ 0 & 0 & 0 & -0.351 & -0.370 & 141.47 \end{bmatrix}$$

- A composite boxed-beam.
- Units of  $C_{ij}^*$  (lb),  $C_{i,j+3}^*$  (lb.in), and  $C_{i+3,j+3}^*$  (lb.in<sup>2</sup>) for  $i, j = 1, 2, 3$ ; these units are adopted for consistency with those used in (Yu et al., 2002).

## Example 2: Results



(a) *Displacements*



(b) *Rotations*

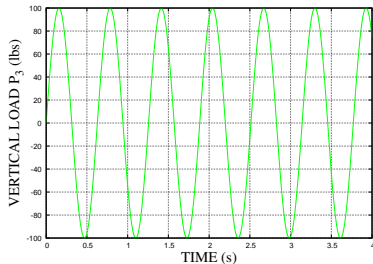
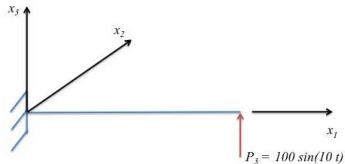
Figure: Displacements and rotation parameters along beam axis for Example 2.

Table: Numerically determined tip displacements and rotation parameters of a composite beam in Example 2 as calculated by BeamDyn (LSFE) and Dymore (QFE)

	$u_1$ (inch)	$u_2$ (inch)	$u_3$ (inch)	$p_1$	$p_2$	$p_3$
BeamDyn	-0.09064	-0.06484	1.22998	0.18445	-0.17985	0.00488
Dymore	-0.09064	-0.06483	1.22999	0.18443	-0.17985	0.00488

# Example 3: A composite beam - Dynamics

## ► Sketch of Example 3



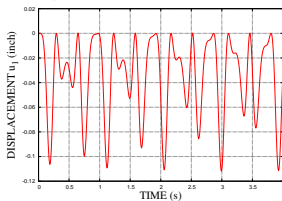
## ► Sectional mass matrix

$$M^* = 10^{-2} \times \begin{bmatrix} 8.538 & 0 & 0 & 0 & 0 & 0 \\ 0 & 8.538 & 0 & 0 & 0 & 0 \\ 0 & 0 & 8.538 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1.4433 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0.40972 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1.0336 \end{bmatrix}$$

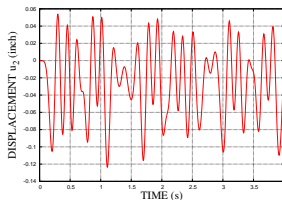
- A composite boxed-beam under applied sinusoidal force
- The units associated with the mass matrix values are  $M_{ii}^*$  (lb.s<sup>2</sup>/in<sup>2</sup>) and  $M_{i+3,i+3}^*$  (lb.s<sup>2</sup>) for  $i = 1, 2, 3$ .

# Example 3: Tip displacement field

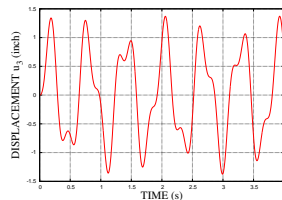
► Displacement  $u_1$



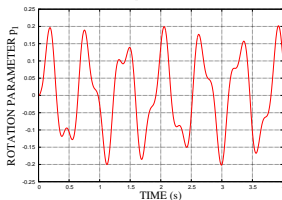
► Displacement  $u_2$



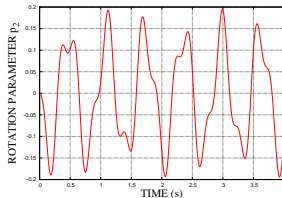
► Displacement  $u_3$



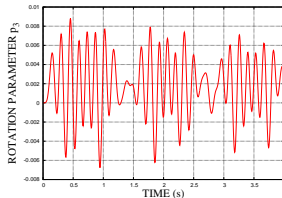
► Rotation  $p_1$



► Rotation  $p_2$

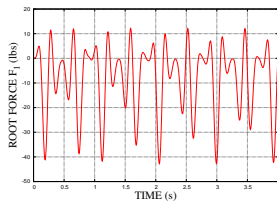


► Rotation  $p_3$

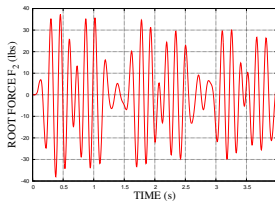


# Example 3: Root forces and moments

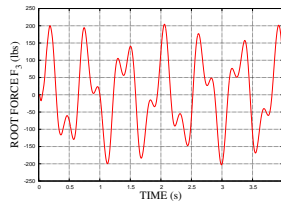
► Force  $F_1$



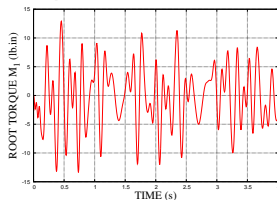
► Force  $F_2$



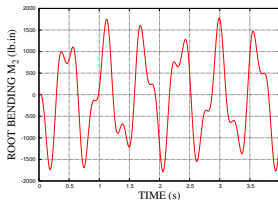
► Force  $F_3$



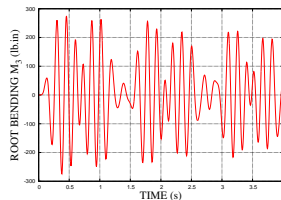
► Torque  $M_1$



► Moment  $M_2$



► Moment  $M_3$



## Example 3: Convergence study

- ▶ Normalized Root-Mean-Square (RMS) error

$$\varepsilon_{\text{RMS}}(u_1) = \sqrt{\frac{\sum_{k=0}^{n_{\max}} [u_1^k - u_b(t^k)]^2}{\sum_{k=0}^{n_{\max}} [u_b(t^k)]^2}}$$

- $u_b(t)$  is the benchmark solution: a highly resolved numerical solution obtained by BeamDyn with one  $20^{\text{th}}$ -order element
- Time step size:  $\Delta t_b = 1.0 \times 10^{-4} \text{ s}$
- ▶ Test calculations
  - Case 1:  $\Delta t_1 = 5.0 \times 10^{-3} \text{ s}$
  - Case 2:  $\Delta t_2 = \frac{\Delta t_1}{2} = 2.5 \times 10^{-3} \text{ s}$

## Example 3: Convergence study (continued)

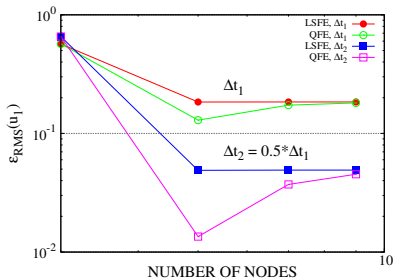


Figure: Normalized RMS error of tip displacement  $u_1$  histories over  $0 \leq t \leq 4$  as a function of number of nodes as calculated by BeamDyn (LSFs) and Dymore (QFEs).

- ▶ For a fixed  $\Delta t$ , both Dymore (QFEs) and BeamDyn (LSFs) converge with spatial refinement to the same error level. BeamDyn is converged with only five nodes, whereas Dymore requires at least nine nodes.
- ▶ The converged error levels are due exclusively to time-discretization error. We note that the converged error for  $\Delta t_2 = \Delta t_1/2$  is one-fourth that for  $\Delta t_1$ , which is expected for the second-order-accurate time integrator.

## ► Conclusion

- Implemented Legendre spectral finite elements based on geometrically exact beam theory
- Exponential convergence rates observed
- Verified against analytical solution, numerical solution for composite beams with elastic couplings

## ► Future Work

- Extend to model moving beams: translation and rotation
- Integrate into FAST for wind turbine analysis
- Full-blade verification and validation



# Questions?

## Acknowledgments

- ▶ Funded by the U.S. Department of Energy under Contract No. DE-AC36-08-GO28308 with the National Renewable Energy Laboratory.
- ▶ Support was provided through an NREL Laboratory Directed Research and Development grant *High-Fidelity Computational Modeling of Wind-Turbine Structural Dynamics*

## References

Bauchau, O. A., 2013. Dymore User's Manual.

[Http://dymoresolutions.com/dymore4\\_0/UsersManual/UsersManual.html](http://dymoresolutions.com/dymore4_0/UsersManual/UsersManual.html).

Hodges, D. H., 2006. Nonlinear Composite Beam Theory. AIAA.

Jelenić, G., Crisfield, M. A., 1999. Geometrically exact 3d beam theory: implementation of a strain-invariant finite element for statics and dynamics. *Computer Methods in Applied Mechanics and Engineering* 171, 141–171.

Jonkman, J. M., January 2013. The new modularization framework for the FAST wind turbine CAE tool. In: *Proceedings of the 51st AIAA Aerospace Sciences Meeting including the New Horizons Forum and Aerospace Exposition*. Grapevine, Texas.

Patera, A. T., 1984. A spectral element method for fluid dynamics: Laminar flow in a channel expansion. *Journal of Computational Physics* 54, 468–488.

Reissner, E., 1973. On one-dimensional large-displacement finite-strain beam theory. *Studies in Applied Mathematics LII*, 87–95.

Sprague, M. A., Geers, T. L., 2003. Spectral elements and field separation for an acoustic fluid subject to cavitation. *Journal of Computational Physics* 184, 149–162.