Transformations and correlations among some clay parameters - the global database -些黏土参数之间的转换和相关性 - 全球数据库

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Abstract 摘要

This study compiles a large database of 10 clay parameters (labeled as CLAY/10/7490) from 251 studies, covering clay data from 30 regions or countries worldwide. Hence, the range of data covered by this "global" database is broader than that underlying the calibration of existing transformation models in the literature. These transformation models relate test measurements (e.g., cone tip resistance) to appropriate design parameters (e.g., undrained shear strength). The correlation behaviours exhibited by the database among the 10 clay parameters are consistent with those exhibited by existing transformation models in the literature. The biases and transformation uncertainties of these transformation models with respect to the global database are calibrated. It is found that more recent transformation models are less biased and that the transformation uncertainties are typically fairly large. Such large transformation uncertainties are further reduced by incorporating secondary input parameters, such as plasticity index or sensitivity. In a companion paper written by the same authors, a 10dimensional multivariate probability distribution coupling these clay parameters is constructed from CLAY/10/7490 and a useful application involving updating the entire bivariate probability distribution of two design parameters from three separate measurements is presented.

Key words: clay properties, correlations, transformation models, database, statistics.

该研究汇编了来自 251 个研究的 10 个 粘土参数(标记为 CLAY/10/7490)的大型 数据库,涵盖了来自全球 30 个地区或国家 的粘土数据。因此,该"全球"数据库所涵盖 的数据范围比文献中现有转换模型的标定所 依据的范围要广。这些转换模型将测试测量 值(例如,锥头阻力)与适当的设计参数(例 如,不排水的剪切强度)相关联。数据库显示 的 10 个黏土参数之间的相关行为与文献中现 有的转换模型所显示的相关行为一致。这些 转换模型相对于全局数据库的偏差和转换不 确定性已得到校准。结果发现, 更新的转换 模型偏差较小,并且转换不确定性通常相当 大。通过合并次级输入参数(例如可塑性指数 或灵敏度),可以进一步降低如此大的变换不 确定性。在同一位作者撰写的伴随论文中,从 CLAY/10/7490 构建了耦合这些黏土参数的 10 维多元概率分布,并提出了一个有用的应 用程序,其中涉及从三个独立的测量值更新两 个设计参数的整个双变量概率分布。

关键词: 黏土特性,相关性,转换模型, 数据库,统计数据。

Introduction 简介

Geotechnical variability is a complex attribute that needs careful evaluation. Phoon and Kulhawy (1999a) demonstrated using fairly 评估。Phoon and Kulhawy (1999a) 使用相

岩土变异性是一个复杂的属性,需要仔细

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extensive soil statistics that geotechnical variability depends on the site condition, measurement error associated with a field test, and quality of the correlation model adopted to relate the field test to a design property. The first component refers to inherent soil variability, which is customarily categorized as aleatoric in nature because it cannot be reduced by performing more tests. The second and third components, namely measurement error inevitably introduced in a test procedure and data scatter about a mean correlation trend (typically in the form of a linear regression equation), are customarily categorized epistemic in nature. They can be reduced by gathering more data or building better models. While there are merits to categorizing uncertainties as aleatoric or epistemic, one should be mindful that this demarcation is in part a modeler's choice (Kiureghian and Ditlevsen, 2007). From a practical perspective, it is perhaps more important to align tatistical characterization to the property evaluation procedure already embedded in our geotechnical engineering practice. In recognition of the need to respect sound geotechnical engineering practice, Phoon and Kulhawy (1999b) presented guidelines for coefficients of variation (COVs) of some soil properties as a function of the test method, correlation equation, and soil type. The key conclusion in this study is that it is not possible to assign a single coefficient of variation (COV) to a design property, such as the undrained shear strength. Geotechnical reliability-based design (RBD) equations that are calibrated using this single COV assumption are too simplistic, because diverse methodologies in estimating soil properties are ignored. This diversity is actually good practice, because there is a need to accommodate diverse site conditions. Phoon and Kulhawy (1999a,b) advocated that the calibration of geotechnical RBD equations should be carried out in explicit recognition of property variability and in full compliance with how soil properties are physically evaluated in practice. The framework recommended by Phoon and Kulhawy (1999a,b) and the ensuing three-tier classification scheme of soil property variability (Phoon and Kulhawy, 2008) should be viewed as the minimum requirements in variability characterization. The third edition of ISO 2394 (General principles on reliability for structures; to be published in 2015) includes a new Annex D on "Reliability of geotechnical structures" where the importance of respecting sound geotechnical engineering practices in variability characterization and reliability calibration is strongly emphasized.

The characterization of geotechnical variability is far from being a mature area in research. An astute practitioner would readily point out that multiple tests are commonly conducted in a site investigation and it is common practice to estimate a design property from these tests, either by straightforward averaging or picking a credible worst-case value from the range of values produced by different tests. The information collected in a site investigation programme is fundamentally

当广泛的土壤统计数据证明, 岩土工程变异 性取决于现场条件,与现场测试相关的测量 误差以及将现场测试与设计属性相关联的相 关模型的质量。第一部分是土壤固有的变异 性,由于无法通过执行更多的测试来减少,因 此习惯上被归类为无酸。第二和第三部分,即 不可避免地在测试过程中引入的测量误差和 关于平均相关趋势的数据散布(通常以线性 回归方程的形式),本质上通常被归类为认识 论。可以通过收集更多数据或建立更好的模 型来减少它们。尽管将不确定性归类为偶然 性或认知性是有好处的, 但应该记住, 这种划 分在某种程度上是建模者的选择 (Kiureghian and Ditlevsen, 2007)。从实践的角度来看,将 地物特征与已经嵌入到我们的岩土工程实践 中的属性评估程序对齐可能更为重要。考虑 到需要尊重合理的岩土工程实践, Phoon and Kulhawy (1999b) 提出了一些土壤特性的变 异系数 (COV) 的准则,这些准则是测试方 法,相关方程和土壤类型的函数。这项研究的 关键结论是,不可能为设计特性(例如不排水 的剪切强度)分配单个变异系数 (COV)。使 用这种单一 COV 假设进行校准的基于岩土 工程的基于可靠性的设计(RBD)方程过于 简单,因为忽略了估算土壤性质的多种方法。 实际上,这种多样性是一种好习惯,因为有必 要适应各种现场条件。Phoon and Kulhawy (1999a,b) 提倡,对岩土工程 RBD 方程的 标定应在明确认识到物性变异性的前提下进 行,并完全符合实际中对土壤物性的评估方 法。Phoon and Kulhawy (1999a,b) 建议的 框架和随后的三层土壤性质变异性分类方案 (Phoon and Kulhawy, 2008) 应被视为变异性 表征的最低要求。ISO 2394 第三版(关于结 构可靠性的一般原则;将于2015年发布)包 括有关"岩土结构可靠性"的新附件 D, 在该 附录 D 中,强烈强调了在变异性表征和可靠 性校准中尊重岩土工程实践的重要性。

岩土工程变异性的表征还远远不是研究的成熟领域。精明的从业人员会很容易地指出,在现场调查中通常会进行多次测试,并且通常的做法是根据这些测试来评估设计属性,方法是直接取平均值,或者从测试产生的值的范围内选择可信的最坏情况下的值。不同的测试。在现场调查程序中收集的信息本质上

multivariate in nature and this aspect has not been considered in the earlier studies mentioned above. The purpose of this paper is to develop unbiased transformation models and to quantify their associated uncertainties for 10 common clay parameters. A companion paper (Ching and Phoon, 2014) develops a multivariate probability model coupling these clay parameters. The supporting database contains information from multiple tests that are collected in close proximity. Inother words, each data point records soil information at a specific location and depth, i.e., at a specific sampling point. Note that measurement error is present, but it is not possible to isolate measurement error from transformation uncertainty in conventional site investigation programs. Hence, the transformation uncertainties presented in this study include some measurement errors, but these errors are relatively minor for cone penetration testing with pore pressure measurement (CPTU) (Phoon and Kulhawy, 1999a). Inherent soil variability is clearly not considered in this study. In principle, inherent soil variability can be incorporated by extending the multivariate probability model (whichapplies to a sampling "point") to a vector random field covering the threedimensional (3D) spatial domain of the entire site. The outcomes of this study are thus incomplete in this sense, but they can be viewed as paving the way for characterization of geotechnical variability to advance beyond univariate data and to achieve closer alignment to how soil properties are estimated in actual practice from site investigation programs

As noted by Phoon and Kulhawy (1999a), the measurement from a geotechnical test is typically not directly applicable to design. Instead, a transformation model is needed to relate the test measurement to an appropriate design parameter. Most transformation models in geotechnical engineering are obtained by empirical or semi-empirical data-fitting using regression analyses. These transformation models are widely adopted in geotechnical engineering practice as a matter of practical expediency. Useful compilations of these models (mostly pairwise correlations) are available in the literature (e.g., Kulhawy and Mayne, 1990; Mayne et al., 2001). A cursory review of these compilations would reveal a rather bewildering variety and number of models. Most models were developed for a specific geomaterial type and (or) a specific locale.

It is not judicious to apply these models in discriminately to other sites without a proper appreciation of geomaterial behaviour and geology. This "site-specific" limitation is a distinctive and fundamental feature of geotechnical engineering practice. Geotechnical design must take cognizance of this limitation to avoid gross oversimplification of "ground truths." As opposed to site-specific models, Ching and Phoon (2012a) demonstrated the construction of "global" models. Global models are calibrated from global databases covering many

本质上是多元的,并且在上述早期研究中并 未考虑到这方面。本文的目的是为 10 个常 见黏土参数建立无偏转换模型并量化其相关 的不确定性。伴随论文 (Ching and Phoon, 2014) 开发了耦合这些粘土参数的多元概率 模型。支持数据库包含来自紧密收集的多个 测试的信息。换句话说,每个数据点记录特 定位置和深度,即特定采样点的土壤信息。请 注意,存在测量误差,但是在传统的站点调查 程序中无法将测量误差与变换不确定性区分 开。因此,本研究中提出的变换不确定性包 括一些测量误差,但是对于使用孔隙压力测 量(CPTU)进行的圆锥渗透测试,这些误差 相对较小 (Phoon and Kulhawy, 1999a)。这 项研究显然没有考虑土壤固有的变异性。原 则上,可以通过将多元概率模型(适用于采样 "点")扩展到覆盖整个站点的三维(3D)空间 域的向量随机字段来合并固有的土壤可变性。 因此,从这个意义上说,这项研究的结果是不 完整的,但可以认为它们为表征岩土工程变异 性铺平了道路,使岩土变异性超越了单变量数 据,并与现场调查程序在实际实践中对土壤特 性的估算更加接近。

正如Phoon and Kulhawy (1999a) 指出的那样,岩土试验的测量结果通常不适用于设计。取而代之的是,需要一个转换模型来将测试度量与适当的设计参数相关联。岩土工程中的大多数转换模型都是通过使用回归分析的经验或半经验数据拟合获得的。出于实际考虑,这些转换模型已在岩土工程实践中广泛采用。这些模型的有用汇编(主要是成对相关)可在文献中找到(例如Kulhawy and Mayne, 1990; Mayne et al., 2001)。粗略地回顾一下这些汇编,将会发现令人困惑的各种模型。大多数模型是针对特定的土工材料类型和(或)特定的区域设置而开发的。

在没有适当了解地球物质行为和地质的情况下,不加选择地将这些模型应用于其他地点并不明智。这种"特定于地点"的限制是岩土工程实践的独特和基本特征。岩土设计必须意识到这一局限性,以避免对"地面真相"的过分简化。与特定地点的模型相反,Ching and Phoon (2012a) 演示了"全局"模型的构建。全局模型是根据覆盖许多站点和岩土材

sites and geomaterial types. Ching and Phoon (2012a) observed that site-specific models are generally more precise than global models, but they can be significantly biased when applied to another site. On the contrary, global models are less precise than site-specific models, but they are less biased. Their observations are already well appreciated by engineers. The key contribution from Ching and Phoon (2012a) was to demonstrate these observations with statistical rigor using a sizeable global database.

Because most transformation models were built based on their own databases, their ranges of application are, in principle, limited to the range of characteristics contained in the databases, e.g., certain soil types, certain range of soil properties (e.g., insensitive clays), and certain geographic locations. It is important to assess their biases and the uncertainties when these models are applied globally (i.e., applying these models outside their range of calibration). In the current paper, a global clay database is compiled and presented. This database consists of data points from 251 studies, covering clay data from 30 regions or countries worldwide. Hence, the range of data covered by this "global" database is broader than that underlying the calibration of existing transformation models in the literature. Ten parameters of clays are of main interest, including three index properties (i.e., Atterberg' s limits); four parameters for effective stresses, shear strength, and sensitivity; and three parameters from piezocone tests (CPTU). This global database is the largest database compiled by the authors thus far in terms of number of data points and number of parameters of interest. Table 1 shows the databases compiled by the authors, labeled as (soil type)/(number of parameters of interest)/(number of data points). The current global database is CLAY/10/7490. The first purpose of this paper is to present this large database and verify whether the correlation behaviours in the data points are consistent with those exhibited by existing transformation models in the literature. Most of these models are sitespecific models. The biases and uncertainties in these models will be estimated using the global database. The sitespecific models can be applied to a wider range of conditions when their biases are corrected and their transformation uncertainties are uitably revised.

The uncertainties of the resulting global models are inevitably large because they are required to accommodate a broader range of clays, e.g., insensitive, sensitive, and quick clays. It is possible to reduce the transformation uncertainty by considering soil index properties (such as plasticity index and sensitivity) as secondary input (explanatory) parameters. Most of the existing transformation models do not include such secondary parameters. Nonetheless, with the global database compiled in this study, it is possible to augment the existing transformation models with secondary input parameters. The resulting

料类型的全局数据库进行校准的。Ching and Phoon (2012a) 观察到,特定于站点的模型通常比全局模型更为精确,但将其应用于其他站点时可能会出现明显偏差。相反,全局模型不如特定于站点的模型精确,但它们的偏向性较小。他们的观察已经得到工程师的赞赏。Ching and Phoon (2012a) 的主要贡献是通过使用

由于大多数转化模型都是基于其自身的 数据库构建的,因此,其应用范围原则上仅 限于数据库中包含的特征范围,例如某些土 壤类型,某些土壤特性范围(例如不敏感的粘 土),和某些地理位置。在全球范围内应用这 些模型时(即在校准范围之外应用这些模型), 评估它们的偏差和不确定性很重要。在当前 的论文中,一个全球粘土数据库被编译并呈现 出来。该数据库包含来自 251 个研究的数据 点,涵盖了来自全球 30 个地区或国家的粘土 数据。因此,该"全球"数据库所涵盖的数据范 围比文献中现有转换模型的标定所依据的范 围要广。十个主要参数是黏土,其中包括三个 指数属性(即,阿特伯格极限);有效应力,剪 切强度和灵敏度的四个参数;以及压电锥测试 (CPTU)的三个参数。就数据点的数量和感兴 趣的参数的数量而言,该全局数据库是迄今为 止作者编写的最大的数据库。表1显示了作者 编译的数据库,标记为(土壤类型)/(感兴趣 的参数数量)/(数据点数量)。当前的全局数 据库是 CLAY/10/7490。本文的首要目的是介 绍这个大型数据库,并验证数据点中的相关行 为是否与文献中现有转换模型所显示的行为 一致。这些模型大多数是特定于站点的模型。 这些模型中的偏差和不确定性将使用全球数 据库进行估算。当特定位置的模型的偏差得 到纠正并且适当地修改了转换不确定性时,可 以将其应用于更广泛的条件。

所产生的全局模型的不确定性不可避免 地很大,因为需要它们适应更大范围的粘土, 例如不敏感,敏感和快速的粘土。通过将土壤 指数属性(例如可塑性指数和敏感性)视为辅 助输入(解释性)参数,可以减少变换不确定 性。现有的大多数转换模型都不包含此类辅 助参数。但是,通过本研究中编译的全局数 据库,可以使用辅助输入参数来扩展现有的 转换模型。所得模型仍将是全局模型,但不

No. of No. of Range of Properties Reference Parameters of interest sites or Database data OCR PI S_t studies points Sensitive to CLAY/5/345 Ching and Phoon (2012b) $LI, s_u, s_u^{re}, \sigma_p', \sigma_v'$ 37 sites $1\sim4$ 345 quick clays s_u/σ'_v , OCR, $(q_t - s_v)/\sigma'_v$, Low to very Insensitive to CLAY/6/535 Ching et al. (2014) 40 sites $1\sim6$ 535 $(q_t - u_2)/\sigma'_v, (u_2 - u_0)/\sigma'_v, B_q$ high plasticity quick clavs s_u under seven different $\mathbf{s_u}$ Low to very Insensitive to CLAY/7/6310 Ching and Phoon (2013) 6310 164 studies $1 \sim \! 10$ high plasticity quick clays test types $LL, PI, LI, \sigma'_{v}/P_{a}, \sigma'_{p}/P_{a},$ Low to very Insensitive to CLAY/10/749(This paper $s_u/\sigma'_v, S_t, (q_t - \sigma_v)/\sigma'_v,$ 7490 251 studies high plasticity quick clays $(q_t - u_2)/\sigma'_v, B_q$

Table 1: Databases compiled by the authors.

表 1: 作者编译的数据库。

models will still be global, but with less uncertainties. This is the second purpose of this paper. In a companion paper (Ching and Phoon, 2014), a 10-dimensional multivariate probability distribution coupling these clay parameters is constructed from $\rm CLAY/10/7490$ and a useful application involving updating the entire bivariate probability distribution of two design parameters from three separate measurements is presented.

确定性较小。这是本文的第二个目的。在随附的论文中(Ching and Phoon, 2014),从 CLAY/10/7490构建了耦合这些黏土参数的10维多元概率分布,并呈现了一个有用的从三个独立的测量值中更新两个设计参数的整体二元概率分布的应用。

Database CLAY/10/7490 数据库 CLAY/10/7490

This study compiles a clay database (CLAY/10/7490) from the literature consisting of a large number of data points. This database consists of data points from 251 studies. The number of data points associated with each study varies from 1 to 419 with an average 30 data points per study. The geographical regions cover Australia, Austria, Brazil, Canada, China, England, Finland, France, Germany, Hong Kong, India, Iraq, Italy, Japan, Korea, Malaysia, Mexico, New Zealand, Northern Ireland, Norway, Poland, Singapore, South Africa, Spain, Sweden, Taiwan, Thailand, United Kingdom, United States, and Venezuela. The clay properties cover a wide range of overconsolidation ratio (OCR) values (but mostly 1~10), a wide range of sensitivity (St) values (sites with St = 1 tens or hundreds are fairly typical), and a wide range of plasticity index (PI) values (but mostly 8~100). Figure 1 shows the plasticity chart and Robertson's CPTU soil classification chart (Robertson, 1990) of all data points in the database —most data points are classified as clays (some are sensitive or organic clays). Some data points are classified as clayey silts or silt mixtures, and a few are classified as sand mixtures or sands. The details of this database are shown in Appendix A.

Ten dimensionless parameters of clays are of primary interest. They are categorized into three groups:

1. Index properties, including liquid limit (LL), plasticity index

这项研究从文献中汇编了一个包含大量 数据点的粘土数据库(CLAY / 10/7490)。该 数据库包含来自 251 个研究的数据点。与每 个研究相关的数据点数量从1到419不等,每 个研究平均 30 个数据点。地理区域包括澳大 利亚, 奥地利, 巴西, 加拿大, 中国, 英国, 芬兰, 法国, 德国, 香港, 印度, 伊拉克, 意 大利,日本,韩国,马来西亚,墨西哥,新西 兰,北爱尔兰,挪威,波兰,新加坡,南非,西 班牙,瑞典,台湾,泰国,英国,美国和委内 瑞拉。黏土的特性涵盖了大范围的超固结比 (OCR) 值(但大多为 1~10), 大范围的敏 感度 (St) 值 ($S_t = 1 \sim$ 数十或数百的值是相 当典型的)和大范围的塑性指数(PI)值(但 多数为 8~100)。图1显示了数据库中所有数 据点的可塑性图和 Robertson 的 CPTU 土 壤分类图 (Robertson, 1990)——大多数数据 点被归类为黏土(一些是敏感黏土或有机黏 土)。一些数据点被分类为黏土粉砂或粉砂混 合物,而一些数据点被分类为砂混合物或砂。 该数据库的详细信息显示在附录A中。

十个无量纲的黏土参数是最重要的。它们分为三类:

1. 指数性质,包括液体极限(LL),可

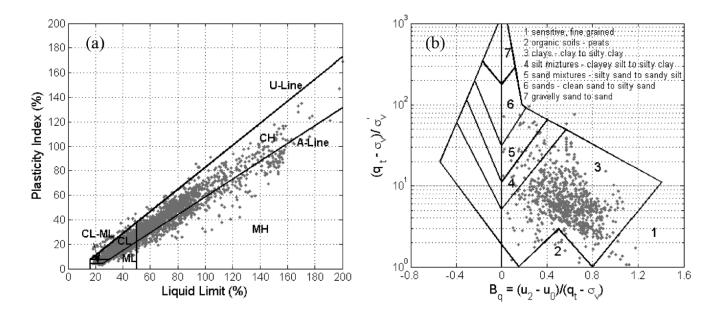


Figure 1: (a) Plasticity chart; (b) Robertson (1990) CPTU soil classification chart. B_a , pore pressure ratio; CH, highplasticity clay; CL, lowplasticity clay; MH, high-plasticity silt; ML, low-plasticity silt; q_t , corrected cone tip resistance; u_0 , hydrostatic pore pressure; u_2 , pore pressure behind the cone; σ_v , total effective stress; σ'_v , vertical effective stress. 图 1: (a) 可塑性图; (b) Robertson (1990) 的 CPTU 土壤分类图。 B_q ,孔隙压力比; CH,高塑性粘土; CL,低塑 性粘土; MH, 高塑性淤泥; ML, 低塑性淤泥; q_t , 校正的锥头电阻; u_0 , 静水孔隙压力; u_2 , 锥体后面的孔隙压力; σ_v , 总有效应力; σ'_v , 垂直有效应力。

(PI), and liquidity index (LI).

2. Stresses and strengths, including normalized vertical effective stress (σ'_v/P_a) , where P_a is one atmosphere pressure $(=101.3 \text{kN/m}^2)$), 力 (σ'_v/P_a) , 其中 P_a 是一个大气压,归一化预 normalized preconsolidation stress (σ'_p/P_a) , normalized undrained shear strength (s_u/σ'_v) , and sensitivity $(S_t = s_u/s_u^{re})$, where s_u^{re} is the remoulded undrained shear strength). The s_u values in the literature were obtained based on various types of tests, including isotropically consolidated undrained compression (CIUC), K_0 -consolidated undrained compression (C K_0 UC), K_0 -consolidated undrained extension (C K_0 UE), direct simple shear (DSS), unconsolidated undrained compression (UU), unconfined compression (UC), and field vane (FV). These values cannot be compared directly because s_u depends on stress state, strain rate, and sampling disturbance. By following the recommendations made by Bjerrum (1972), Kulhawy and Mayne (1990), and Mesri and Huvaj (2007), these s_u values are all converted to the "mobilized" s_u values, denoted by s_u (mob), which is defined as the in situ undrained shear strength mobilized in embankment and slope failures (Mesri and Huvaj, 2007). The transformation models used to convert undrained shear strengths derived from different test types to the reference $s_u(\text{mob})$ are given in Table 2.

3. Parameters from the piezocone test (CPTU), including pore pressure ratio $B_q = (u_2 - u_0)/(q_t - \sigma_v)$, where u_2 is the pore

塑性指数(PI)和流动性指数(LI)。

2. 应力和强度,包括归一化垂直有效应 固结应力 (σ'_p/P_a) , 归一化不排水剪切强度 (s_u/σ'_v) 和灵敏度 $(S_t = s_u/s_u^{re}, 其中 s_u^{re})$ 是 重新模制的不排水剪切强度)。文献中的 s_u 值是根据各种类型的测试获得的,包括各向 同性固结不排水压缩(CIUC), K_0 固结不 排水压缩(CK_0UC), K_0 固结不排水延伸 (CK_0UE) , 直接简单剪切 (DSS), 未固结 不排水压缩(UU), 无限制压缩(UC)和现 场叶片(FV)。这些值不能直接比较,因为 s_u 取决于应力状态,应变率和采样扰动。通 过遵循Bjerrum (1972), Kulhawy and Mayne (1990) 以及Mesri and Huvaj (2007) 的建议, 这些 s_u 值都被转换为"扰动" s_u 值,该值由 $s_u(\text{mob})$ 定义, 因为在路堤和边坡破坏中扰动 了原地不排水的抗剪强度 (Mesri and Huvaj, 2007)。表2给出了用于将源自不同测试类型的 不排水剪切强度转换为参考 $s_u(\text{mob})$ 的转换 模型。

3. 压电锥测试(CPTU)的参数,包括

pressure behind the cone, u_0 is the hydrostatic pore pressure, q_t is the corrected cone tip resistance, and σ_v is the total effective stress; normalized cone tip resistance $(q_t - \sigma_v)/\sigma'_v$; and normalized effective cone tip resistance $q_t - u_2/\sigma'_v$.

The CPTU data are nearly continuous with depth. However, only a few data points in a CPTU profile are adopted into the database at the depths where other clay parameters (such as s_u and PI) are also known. As a result, the vertical interval of the adopted CPTU data points at the same site is about 1 to 3 m. Note that the point data at the appropriate depths are adopted. A possible refinement involving averaging along the length of the undisturbed sample was not considered.

孔隙压力比 $B_q = (u_2 - u_0)/(q_t - \sigma_v)$,其中 u_2 是圆锥体后面的孔隙压力, u_0 是静水孔隙 压力, q_t 是校正的锥头阻力, σ_v 是总有效应力;归一化锥头电阻 $(q_t - \sigma_v)/\sigma_v'$;以及归一化的有效锥端电阻 $q_t - u_2/\sigma_v'$ 。

CPTU 数据在深度上几乎是连续的。但是,在已知其他黏土参数(例如 s_u 和 PI)的深度处,CPTU 剖面中只有几个数据点被采用到数据库中。结果,在同一站点上采用的CPTU 数据点的垂直间隔约为 1 至 3 m。注意,采用了适当深度的点数据。没有考虑可能的改进,包括沿未扰动样本的长度取平均值。

Table 2: Transformation models for $s_u(\text{mob})$.

表 2: $s_u(\text{mob})$	的转换模型
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Available s_u information	Transformation model	Reference					
FV	$s_u(\text{mob}) \approx s_u(\text{field}) \approx [s_u(\text{FV})]\mu$	Bjerrum (1972)					
UC	$s_u(\text{mob}) \approx s_u(\text{UC})$	Mesri and Huvaj (2007)					
UU	$s_{\rm u}({\rm mob})/\sigma_{\rm v}' \approx s_{\rm u}({\rm UC})/\sigma_{\rm v}' \approx -0.073 + 1.018 s_{\rm u}({\rm UU})/\sigma_{\rm v}'$	Chen and Kulhawy (1993); Mesri and Huvaj (2007)					
CIUC	$s_{\rm u}({\rm mob})/\sigma_{\rm v}' \approx s_{\rm u}({\rm UC})/\sigma_{\rm v}' \approx -0.278 + 1.172 s_{\rm u}({\rm CIUC})/\sigma_{\rm v}'$	Chen and Kulhawy (1993); Mesri and Huvaj (2007)					
CK_0UC , DSS, CK_0UE	$s_u(\text{mob}) \approx \{ [s_u(CK_0UC) + s_u(DSS) + s_u(CK_0UE)]/3 \} \mu_t $	Mesri and Huvaj (2007); Kulhawy and Mayne (1990)					
CK_0UC , CK_0UE	$s_u(\text{mob}) \approx \{ [s_u(CK_0UC) + s_u(CK_0UE)]/2 \} \mu_t^* $	Mesri and Huvaj (2007); Kulhawy and Mayne (1990)					
DSS	$s_u(\text{mob}) \approx [s_u(\text{DSS})] u_t^*$	Mesri and Huvaj (2007); Kulhawy and Mayne (1990)					
CK_0UC	$s_u(\text{mob}) \approx [s_u(\text{DSS})]u_t \approx [s_u(\text{CK}_0\text{UC})][0.67\mu_t]$	Mesri and Huvaj (2007); Kulhawy and Mayne (1990)					
$\mathrm{C}K_0\mathrm{UE}$	$s_u(\text{mob}) \approx [s_u(\text{DSS})]u_t \approx [s_u(\text{CK}_0\text{UE})][1.53^{\dagger}(\mu_t)]$	Mesri and Huvaj (2007); Kulhawy and Mayne (1990)					

Note: FV, field vane; UC, unconfined compression; UU, unconsolidated undrained compression; CIUC, isotropically consolidated undrained compression; CK_0UC , K_0 -consolidated undrained compression; DSS, direct simple shear; CK_0UEE , K_0 -consolidated undrained extension; μ , PI-dependent correction factor for $s_u(FV)$ proposed in Bjerrum (1972); μ_t , PI-dependent strain rate correction factor proposed in Terzaghi et al. (1996).

*These equations are based on the following two facts: $(i)s_u(\text{mob} \approx \{[s_u(CK_0UC) + s_u(DSS) + s_u(CK_0UC)]/3\}\mu_t$ and $(ii)s_u(DSS)$ is roughly the average of $s_u(CK_0UC)$ and $s_u(CK_0UE)$ (Kulhawy and Mayne, 1990).

†This constant of 1.53 is based on the following two facts: $(i)su(DSS) \approx 0.67s_u(CK_0UC)(Kulhawy and Mayne, 1990)$ and $(ii)s_u(DSS)$ is roughly the average of $s_u(CK_0UC)$ and $s_u(CK_0UE)$.

注意: FV, 现场试验; UC, 无限制压缩; UU, 不固结不排水压缩; CIUC, 各向同性固结不排水压缩; CK₀UC, K_0 固结不排水压缩; DSS, 直接剪切; CK₀UE, K_0 固结不排水拉伸; μ , Bjerrum (1972) 提出的 s_u (FV) 的 PI 依赖校正因子; μ_t , Terzaghi et al. (1996) 提出的依赖 PI 的应变率校正因子。
* 这些方程式基于以下两个事实: $(i)s_u$ (mob $\approx \{[s_u(CK_0UC) + s_u(DSS) + s_u(CK_0UC)]/3\}\mu_t$, $(ii)s_u(DSS)$ 大致为 $s_u(CK_0UC)$ 和 $s_u(CK_0UE)$ 的平均

值 (Kulhawy and Mayne, 1990)。 †1.53 的常数基于以下两个事实: $(i)su(DSS) \approx 0.67s_u(CK_0UC)$ (Kulhawy and Mayne, 1990), $(ii)s_u(DSS)$ 大约是 $s_u(CK_0UC)$ 和 $s_u(CK_0UE)$ 的平均

Some other dimensionless parameters of interest, such as s_u/σ_p' , OCR, and s_u^{re}/P_a , can be derived from the above 10 parameters. The basic statistics of all these parameters (10 basic parameters together with s_u/σ_p' , OCR, and s_u^{re}/P_a) are listed in Table 3. The numbers of available data points (n) are shown in the second column. The statistics are the mean value, coefficient of variation (COV), minimum value (min), and maximum value (max). Their percentiles are listed in Table 4, where the median values (50% percentiles) are shaded. It is worth mentioning that the statistical uncertainty is higher for the lower and higher percentiles.

可以从上述 10 个参数中得出其他一些 无量纲的感兴趣参数,例如 s_u/σ_p' ,OCR 和 s_u^{re}/P_a 。表3列出了所有这些参数的基本统计信息(连同 s_u/σ_p' ,OCR 和 s_u^{re}/P_a 一起的 10 个基本参数)。第二列显示了可用数据点的数量 (n)。统计信息是平均值,变异系数(COV),最小值(min)和最大值(max)。表4中列出了它们的百分位数,其中中值(50%百分数)用阴影表示。值得一提的是,上下百分位数的统计不确定性较高。

Table 3: Statistics of the data points in the database.

表 3: 数据库中数据点的统计信息。

Table 4: Statistics of the data points in the database.

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Variable	n	Mean	COV	Min	Max	Variable	2.5%	5%	25%	50%	75%	95%	97.5%
LL	3822	67.7	0.80	18.1	515	LL	23.6	26.2	39.0	54.3	76	149.1	200.0
PI	4265	39.7	1.08	1.9	363	PI	5.8	8.0	18.5	29.3	46	91.4	135.0
LI	3661	1.01	0.78	-0.75	6.45	$_{ m LI}$	-8.6E-2	4.7E-3	0.54	0.87	1.32	2.51	3.0
σ_y'/P_a	3370	1.8	1.47	4.13E-3	38.74	σ'_y/P_a	0.11	0.14	0.43	0.94	2.03	6.27	8.37
σ_p'/P_a	2028	4.37	2.31	0.094	193.3	σ_p'/P_a	0.26	0.33	0.8	1.71	3.69	19.57	29.19
s_u/σ_v'	3532	0.51	1.25	3.68E-3	7.78	s_u/σ_v'	0.08	0.11	0.21	0.31	0.56	1.46	2.25
S_t	1589	35.0	2.88	1.0	1467	S_t	1.7	2.3	5.0	8.0	23.0	140.8	217.6
B_q	1016	0.58	0.35	0.01	1.17	B_q	0.15	0.23	0.45	0.57	0.72	0.91	0.99
$(q_t - \sigma_v)/\sigma_v'$	862	8.9	1.17	0.48	95.98	$(q_t - \sigma_v)/\sigma_v'$	1.92	2.43	4.15	5.79	8.77	27.57	44.07
$(q_t - u_2)/\sigma'_v$	668	5.34	1.37	0.61	108.2	$(q_t - u_2)/\sigma'_v$	1.28	1.51	2.42	3.63	5.67	14.97	18.55
s_u/σ_p'	1467	0.23	0.55	3.68E-3	1.34	s_u/σ_p'	6.30E-2	8.38E-2	0.15	0.21	0.27	0.44	0.56
OCR	3531	3.85	1.56	1.0	60.23	OCR	1.0	1.0	1.04	1.73	3.57	15.79	24.00
s_u^{re}/P_a	1143	0.075	2.86	$9.67\mathrm{E}\text{-}5$	2.47	s_u^{re}/P_a	$5.14\mathrm{E}\text{-}4$	8.05E-4	6.48E-3	0.021	0.062	0.26	0.54

Calibration of existing transformation models 校准现有的转换模型

In the database, there are data points where two or more clay parameters are simultaneously known. For instance, a disturbed clay sample is extracted to determine PI, and an undisturbed clay sample is extracted at a nearby borehole at the same depth to determine s_u . In this case, PI and s_u are simultaneously known. These data points can be compared with transformation models proposed in literature as a rough check for consistency. Twentyfour transformation models shown in Table ?? are considered. Most of these models were developed based on certain clay databases, but these databases may not be global in the sense that the data are limited to certain clay types or certain geographic locations. In other words, they are typically site-specific models. It is recommended that the basic statistics of the database supporting the development of transformation models should be explicitly reported in the form of Table 3 and (or) Table 4. The characteristics of the databases underlying numerous existing transformation models are not known and this complicates comparisons with other databases such as the one presented in this study. These characteristics are of interest, because engineers can make a more informed decision on the applicability of a particular transformation model to his or her design scenario at hand.

In the section below, the global database is compared with existing transformation models to assess the quality of the compiled for this study. The global database is considered to be satisfactory if there is broad agreement with the transformation models published in the literature. Some differences are to be expected. In the absence of detailed information on the databases supporting published transformation models, it is assumed in this study that the differences arose because our global database covers a broader range of clays. As a result, correcting published transformation models will broaden the range of

在数据库中,存在同时知道两个或多个黏 土参数的数据点。例如,提取扰动的黏土样品 以确定 PI,并在附近井眼的相同深度提取未 扰动的黏土样品以确定 s_u 。在这种情况下, PI 和 s_u 同时为已知。这些数据点可以与文献中 提出的转换模型进行比较,作为对一致性的粗 略检查。考虑了表??中所示的二十四个转换 模型。这些模型中的大多数都是基于某些黏 土数据库开发的,但是从数据仅限于某些黏土 类型或某些地理位置的意义上讲,这些数据库 可能不是全球性的。换句话说,它们通常是特 定于站点的模型。建议以表3和(或)表4的形 式明确报告支持转换模型开发的数据库的基 本统计信息。尚不清楚许多现有转换模型所 基于的数据库的特征,这会使比较变得复杂与 其他数据库,例如本研究中介绍的数据库。这 些特性很有趣,因为工程师可以根据特定转换 模型对其手头设计方案的适用性做出更明智 的决定。

在下面的部分中,我们将全球数据库与现有的转换模型进行比较,以评估本研究的编译质量。如果与文献中发布的转换模型达成广泛共识,则认为全球数据库令人满意。可能会有一些差异。在缺乏支持已发布的转换模型的数据库的详细信息的情况下,本研究假设出现了差异,因为我们的全球数据库涵盖了更广泛的粘土。结果,更正已发布的转换模型将扩大其适用范围。通过根据本研究中编译的全

their applicability. The correction is undertaken by calibrating the bias factors and uncertainties for these models against the global database compiled in this study.

These 24 transformation models are labeled using the template: (primary input parameter) - (target parameter) - (secondary input parameter). They are categorized into four types (see Table ??):

- 1. Type A Models for S_t , including two LI (s_u^{re}/P_a) models and two LI S_t models.
- 2. Type B Models for effective stress, including one LI $(\sigma'_v/P_a) S_t$ model and three LI $(\sigma'_p/P_a) S_t$ models.
- 3. Type C Models for shear strength, including one LI (s_u/σ_p') model, one PI (s_u/σ_p') model, one OCR (s_u/σ_v') model, and one OCR (s_u/σ_v') S_t model.

In the LI – (s_u/σ_p') model proposed by Bjerrum and Simons (1960) for normally consolidated (NC) clays, the s_u/σ_p' values are obtained from CIUC tests. For NC clays, $s_u(\text{CIUC})/\sigma_p'$ is estimated to be 0.37, whereas $s_u(\text{mob})/\sigma_p'$ is about 0.22 (Mesri, 1975). As a result, the s_u/σ_p' value in the original LI – (s_u/σ_p') model by Bjerrum and Simons (1960) is multiplied by 0.22/0.37=0.59.

4. Type D - Models relevant to CPTU, including three CPTU - (s_u/σ_v') models, four CPTU - OCR models, and five CPTU - (σ_P'/P_a) models.

Note that for the CPTU $-(s_u/\sigma'_v)$ models by Ching and Phoon (2012a), the target parameters are actually the cone factors (namely $(q_t - \sigma_v)/s_u$, $(q_t - u_2)/s_u$, and $(u_2 - u_0)/s_u$). In these models, the s_u values are obtained from CIUC tests. Therefore, the cone factors in these models are divided by the same factor of 0.59.

Most of these 24 transformation models are derived empirically using regression analyses. The only exception is the LI – (s_u^{re}/P_a) model by Wroth and Wood (1978), which is derived theoretically from the modified Cam Clay model. Most models are not constructed for a specific type of clay. The exceptions are: (i) the LI – (σ_p'/P_a) – S_t and OCR – (s_u/σ_v') – S_t models by Ching and Phoon (2012b), constructed primarily from sensitive (structured) clay data (database CLAY/5/535 in Table 1); (ii) the LI – S_t model by Bjerrum (1954), constructed from Norwegian marine clay data only; and (iii) the LI – (s_u/σ_p') model by Bjerrum and Simons (1960), constructed from Norwegian NC clay data. Some transformation models are presented as graphical curves only: (i) the LI – (σ_v'/P_a) – S_t model by Mitchel (1993), (ii) the LI – (σ_p'/P_a) – S_t by NAVFAC (1982), and (iii) the LI – (s_u/σ_p') model by Bjerrum and Simons (1960). No equations were reported by the original authors.

球数据库对这些模型的偏倚因素和不确定性 进行校准,可以进行校正。

这 24 个转换模型使用以下模板标记:(主要输入参数)-(目标参数)-(辅助输入参数)。 它们分为四种类型(请参见表 ref table: 5):

- 1. 类 A 针对灵敏度 S_t 的模型,包括 $2 \uparrow LI (s_u^{re}/P_a)$ 模型和 $2 \uparrow LI S_t$ 模型
- **2.** 类 **B** 针对有效应力的模型,包括 1 个 LI (σ'_v/P_a) S_t 模型和 3 个 LI (σ'_p/P_a) S_t 模型。
- 3. 类 C 针对抗剪强度的模型,包括 $1 \uparrow \text{LI} (s_u/\sigma_p')$ 模型, $1 \uparrow \text{PI} (s_u/\sigma_p')$ 模型, $1 \uparrow \text{OCR} (s_u/\sigma_v')$ 模型,和 $1 \uparrow \text{OCR} (s_u/\sigma_v') S_t$ 模型。

在Bjerrum and Simons (1960) 针对正常 固结(NC)粘土提出的 LI $-(s_u/\sigma_p')$ 模型中, s_u/σ_p' 值是从 CIUC 试验获得的。对于 NC 粘土, $s_u(\text{CIUC})/\sigma_p'$ 估计为 0.37,而 $s_u(\text{mob})/\sigma_p'$ 约为 0.22(Mesri, 1975)。结果,Bjerrum and Simons (1960) 在原始 LI $-(s_u/\sigma_p')$ 模型中的 s_u/σ_p' 值乘以 0.22/0.37=0.59。

4. 类 D - 与 CPTU 相关的模型,包括 $3 \uparrow CPTU - (s_u/\sigma_v')$ 模型,4 $\uparrow CPTU - OCR$ 模型,和 $5 \uparrow CPTU - (\sigma_P'/P_a)$ 模型。

请注意,对于Ching and Phoon (2012a) 的 CPTU – (s_u/σ'_v) 模型,目标参数实际上是圆锥因子(即 $(q_t-\sigma_v)/s_u$, $(q_t-u_2)/s_u$ 和 $(u_2-u_0)/s_u$)。在这些模型中, s_u 值是从CIUC 测试获得的。因此,这些模型中的圆锥系数除以相同的系数 0.59。

这 24 个转换模型中的大多数都是根据经验使用回归分析得出的。唯一的例外是Wroth and Wood (1978) 的 $LI-(s_u^{re}/P_a)$ 模型,该模型理论上是从改进的 Cam Clay 模型推导出来的。大多数模型不是针对特定类型的粘土构造的。例外是:(i) Ching and Phoon (2012b) 的 $LI-(\sigma_p'/P_a)-S_t$ 和 $OCR-(s_u/\sigma_v')-S_t$ 模型,主要是根据敏感的(结构化)黏土数据构造的(表1中的数据库 CLAY/5/535);(ii) Bjerrum(1954)的 $LI-S_t$ 模型,仅根据挪威海洋黏土数据建立;(iii) Bjerrum and Simons(1960)的 $LI-(s_u/\sigma_p')$ 模型是根据挪威 NC 黏土数据构建的。一些转换模型仅以图形曲线形式显示:(i) Mitchel(1993)的

The comparison results between the transformation models and the database are shown in Figs. ?? $\overline{}$??. For the models with a secondary input parameter St, data points in our database are divided into four groups according to their St values, and four subplots are presented to compare with the transformation models. The four groups are obtained based on St < 4, 4 < St < 10, 10 < St < 20, and St > 20. Figures ?? $\overline{}$?? show that the global data follow similar trends to most transformation models reported in the literature. The exceptions are the following six models:

- 1. The LI (s_u^{re}/P_a) model proposed by Wroth and Wood (1978). This model was developed based on the modified Cam Clay model. It provides a reasonable average fit to the data for LI < 1 as shown in Fig. ??. However, it deviates significantly from the data points in our global database for LI > 1.5.
- 2. The LI $-(\sigma'_v/P_a)$ S_t model proposed by Mitchel (1993). Despite the wide scatter as shown in Fig. 4, there is general agreement between this model and the global data for data points with 4 < St < 20. However, for data with small St values (St < 4) or with large St values (St > 20), the agreement is poor. It is possible that this model was developed with most data points falling between 4 < St < 20. In other words, the empirical support for small and large St values may be weak.
- 3. The $\rm LI-(\sigma'_p/P_a)-S_t$ model proposed by NAVFAC (1982). The observations here are similar to those for the Mitchell's model: there is a reasonably good agreement between the model and the global data for data points with $4 < \rm St < 20$ (see Fig. ??). The agreement is poor outside this range of St. One may venture to guess that the empirical support for small and large St values is also weak for this model.
- 4. The LI $-(\sigma_p'/P_a)$ S_t model proposed by Ching and Phoon (2012b). The agreement between this model and the global data are reasonable for data points with large St values (St > 20) as shown in Fig. ??. However, this model does not fit data with small St values (St < 4). This is because the database CLAY/5/345 used to develop this LI (σ_p'/P_a) S_t model contains only structured clays.
- 5. LI $-(s_u/\sigma_p')$ model proposed by Bjerrum and Simons (1960). In Fig. ??, only data points with OCR < 1.2 (nearly NC clays) in the global database are plotted. Nonetheless, the discrepancy between the model and the data is clear. Note that this model was developed based on Norwegian NC clays only. It is most likely that this site-specific model fits to the Norwegian data, but not to the global data from diverse geographic origins.

 $LI - (\sigma'_v/P_a) - S_t$ 模型,(ii)NAVFAC(1982)的 $LI - (\sigma'_p/P_a) - S_t$ 模型,以及(iii)Bjerrum and Simons(1960)的 $LI - (s_u/\sigma'_p)$ 模型。原始作者未提出任何方程式。

转换模型与数据库之间的比较结果如图??-??所示。对于具有辅助输入参数 St 的模型,我们数据库中的数据点根据其 St 值分为四组,并提出了四个子图与转换模型进行比较。根据 St < 4,4 < St < 10,10 < St < 20和 St > 20获得这四个组。图??-??显示,全局数据遵循与文献中报道的大多数转换模型相似的趋势。以下六个模型除外:

- 1. Wroth and Wood (1978) 提出的 LI $-(s_u^{re}/P_a)$ 模型。该模型是基于修改后的 Cam Clay 模型开发的。如图??所示,它为 LI < 1 的数据提供了合理的平均拟合。但是,对于我们的 LI > 1.5,它与我们的全局数据库中的数据点有很大的出入。
- 2. Mitchel (1993) 提出的 $LI-(\sigma'_v/P_a)-S_t$ 模型。尽管如图4所示分散范围很广,但是对于 4 < St < 20 的数据点,该模型与全局数据之间存在普遍共识。但是,对于 St 值小的 (St < 4) 或 St 值大的数据 (St > 20),匹配性很差。该模型可能是在大多数数据点落在 St < 20 之间的情况下开发的。换句话说,对小和大的 St 值的经验支持可能很弱。
- 3. NAVFAC (1982) 提出的 $LI-(\sigma'_{p}/P_{a})-S_{t}$ 模型。这里的观察结果与 Mitchell 模型的 观察结果相似: 对于 4 < St < 20 的数据点,模型与全局数据之间存在相当好的一致性(见图??)。在 S_{t} 的这一范围之外,该匹配的效果很差。有人可能会猜测,对于该模型,对于小和大的 S_{t} 值的经验支持也很弱。
- 4. Ching and Phoon (2012b) 提出的 LI— (σ_p'/P_a) S_t 模型。如图??所示,此模型与全局数据之间的一致性对于具有较大 St 值(St > 20)的数据点是合理的。但是,此模型不适用于具有较小 St 值(St < 4)的数据。这是因为用于开发此 LI (σ_p'/P_a) S_t 模型的数据库 CLAY/5/345 仅包含结构化粘土。
- 5. Bjerrum and Simons (1960) 提出的 $LI (s_u/\sigma_p')$ 模型。在图??中,仅绘制了全局数据库中 OCR < 1.2 (接近 NC 黏土)的数据点。尽管如此,模型和数据之间的差异仍然很明显。请注意,此模型仅基于挪威 NC 黏

- 6. One of the CPTU $-(\sigma_p'/P_a)$ models proposed by Chen and Mayne (1996) (the third model that relates σ_p' to $(u_2 u_0)/P_a$ in Table ??). The discrepancy between this model and our global data is apparent. However, the first two models developed by Chen and Mayne (1996) (the two models that relate σ_p'/P_a to $(q_t \sigma_v)/P_a$ and $(q_t u_2)/P_a$) provide reasonable fits to our global data. We are unable to explain this anomaly.
- 土开发。此特定于站点的模型很可能适合挪 威的数据,但不适合来自不同地理来源的全球 数据。
- 6. Chen and Mayne (1996) 提出的 CPTU $-(\sigma'_p/P_a)$ 模型之一(表??中 σ'_p 与 $(u_2-u_0)/P_a$ 相关的第三种模型)。该模型 与我们的全局数据之间的差异显而易见。然 而,由Chen and Mayne (1996) 开发的前两个模型 $(\sigma'_p/P_a$ 与 $(q_t-\sigma_v)/P_a$ 相关的模型和 $(q_t-u_2)/P_a$ 模型)合理地拟合了我们的全球数据。我们无法解释这种异常。

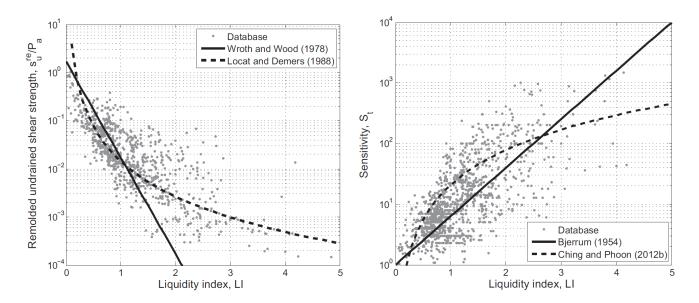


Figure 2: $LI - (s_u^{re}/P_a)$ models proposed by Wroth and Figure 3: $LI - S_t$ models proposed by Bjerrum (1954) Wood (1978) and Locat and Demers (1988). and Ching and Phoon (2012b).

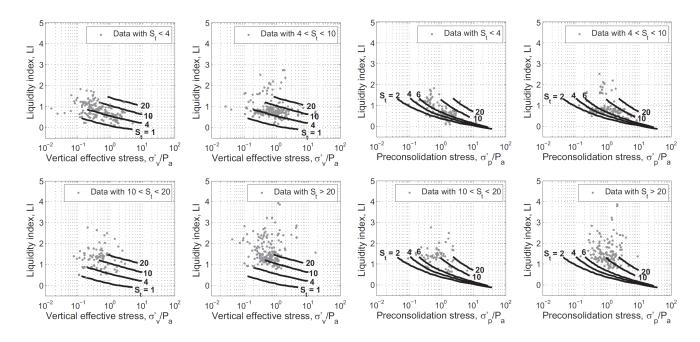


Figure 4: $LI - (\sigma'_v/P_a) - S_t$ model proposed by Mitchel Figure 5: $LI - (\sigma'_p/P_a) - S_t$ models proposed by NAVFAC (1993).

图 4: Mitchel (1993) 提出的 $LI - (\sigma_v'/P_a) - S_t$ 模型。 图 5: NAVFAC (1982) 提出的 $LI - (\sigma_p'/P_a) - S_t$ 模型。

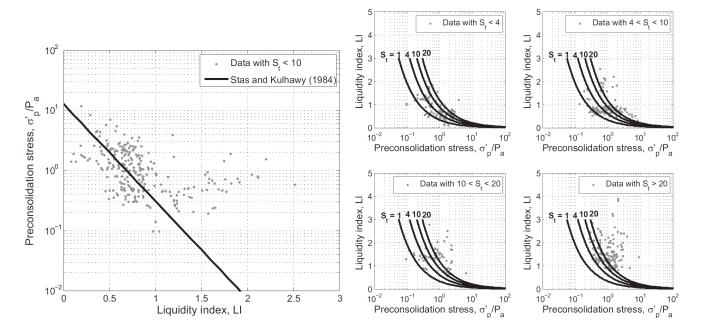


Figure 6: $LI - (\sigma'_p/P_a) - S_t$ model proposed by Stas and Figure 7: $LI - (\sigma'_p/P_a) - S_t$ models proposed by Ching Kulhawy (1984). and Phoon (2012b).

图 6: Stas and Kulhawy (1984) 提出的 $LI - (\sigma'_p/P_a) - S_t$ 图 7: Ching and Phoon (2012b) 提出的 $LI - (\sigma'_p/P_a) - S_t$ 模型。

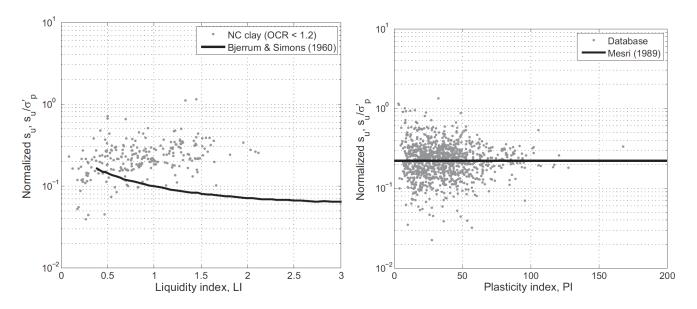


Figure 8: LI $-(s_u/\sigma_p')$ model proposed by Bjerrum and Figure 9: LI $-(s_u/\sigma_p')$ models proposed by Mesri (1975, Simons (1960).

图 8: Bjerrum and Simons (1960) 提出的 LI $-(s_u/\sigma_p')$ 模 图 9: Mesri (1975, 1989) 提出的 LI $-(s_u/\sigma_p')$ 模型。型。

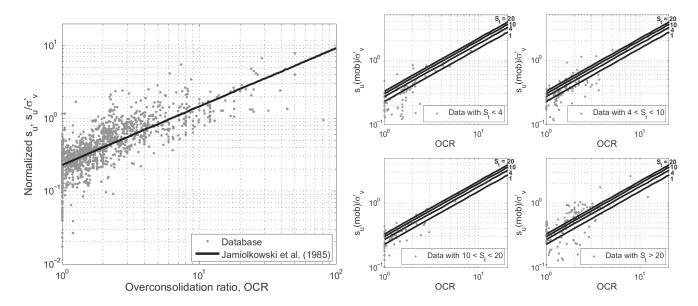


Figure 10: OCR $-(s_u/\sigma_p')$ model proposed by Figure 11: OCR $-(s_u/\sigma_p') - S_t$ models proposed by Ching Jamiolkowski et al. (1985). and Phoon (2012b).

图 10: Jamiolkowski et al. (1985) 提出的 $OCR - (s_u/\sigma_p')$ 图 11: Ching and Phoon (2012b) 提出的 $OCR - (s_u/\sigma_p') -$ 模型。

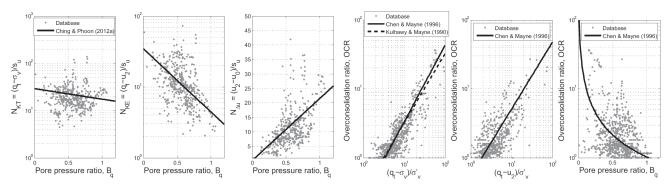


Figure 12: $CPTU - s_u/\sigma'_v$ model proposed by Ching and Figure 13: CPTU-OCR model proposed by Chen and Phoon (2012b). Mayne (1996); Kulhawy and Mayne (1990).

图 12: Ching and Phoon (2012b) 提出的 CPTU – s_u/σ_v' 图 13: Chen and Mayne (1996); Kulhawy and Mayne 模型。 (1990) 提出的 CPTU-OCR 模型。

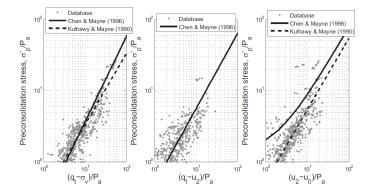


Figure 14: $CPTU - \sigma'_p/P_a$ model proposed by Chen and Mayne (1996); Kulhawy and Mayne (1990).

图 14: Chen and Mayne (1996); Kulhawy and Mayne (1990) 提出的 CPTU – $\sigma_{\rm p}'/P_{\rm a}$ 模型。

REFERENCE 参考文献 15

Reference 参考文献

- L. Bjerrum. Geotechnical properties of norwegian marine clays. *Geotechnique*, 4(2):49–69, 1954. doi: 10.1680/geot. 1954.4.2.49.
- L. Bjerrum. Embankments on soft ground. Proceedings of the American Society of Civil Engineers Specialty Conference on Performance of Earth and Earth-Supported Structures, 2:1–54, 1972.
- L. Bjerrum and N.E. Simons. Comparison of shear strength characteristics of normally consolidated clays. *Proceedings* of the ASCE Research Conference on the Shear Strength of Cohesive Soils, pages 711–726, 1960.
- B.S.Y. Chen and P.W. Mayne. Statistical relationships between piezocone measurements and stress history of clays. *Canadian Geotechnical Journal*, 33(3):488–498, 1996. doi: 10.1139/t96-070.
- Y.-J. Chen and F.H. Kulhawy. Undrained strength interrelationships among ciuc, uu, and uc tests. *Journal of Geotechnical Engineering*, 119(11):1732–1750, 1993. doi: 10.1061/(ASCE)0733-9410(1993)119:11(1732).
- J. Ching and K.-K. Phoon. Establishment of generic transformations for geotechnical design parameters. *Structural Safety*, 35:52–62, 2012a. doi: 10.1016/j.strusafe.2011.12.003.
- J. Ching and K.-K. Phoon. Modeling parameters of structured clays as a multivariate normal distribution. *Canadian Geotechnical Journal*, 49(5):522–545, 2012b. doi: 10.1139/T2012-015.
- J. Ching and K.-K. Phoon. Multivariate distribution for undrained shear strengths under various test procedures. Canadian Geotechnical Journal, 50(9):907–923, 2013. doi: 10.1139/cgj-2013-0002.
- J. Ching and K.-K. Phoon. Correlations among some clay parameters the multivariate distribution. *Canadian Geotechnical Journal*, 51(6):686-704, 2014. doi: 10.1139/cgj-2013-0353.
- J. Ching, K.-K. Phoon, and C.-H. Chen. Modeling piezocone cone penetration (cptu) parameters of clays as a multivariate normal distribution. *Canadian Geotechnical Journal*, 51(1):77–91, 2014. doi: 10.1139/cgj-2012-0259.
- M. Jamiolkowski, C.C. Ladd, J.T. Germaine, and R. Lancellotta. New developments in field and laboratory testing of soils. 1985. cited By 433.
- Armen Der Kiureghian and Ove Ditlevsen. Epistemic or aleatory? does it matter? Proceedings of the Special Workshop on Risk Acceptance and Risk Communication, 2007.
- F.H. Kulhawy and P.W. Mayne. Manual on estimating soil properties for foundation design. *Manual on Estimating Soil Properties for Foundation Design*, 1990.
- J. Locat and D. Demers. Viscosity, yield stress, remolded strength, and liquidity index relationships for sensitive clays. Canadian Geotechnical Journal, 25(4):799–806, 1988. doi: 10.1139/t88-088.
- P. W. Mayne, B. R. Christopher, and J. Dejong. Manual on subsurface investigations. *National Highway Institute Publication No. FHWA NHI-01-031*, 2001.
- G Mesri. Discussion (new design procedure for stability of soft clays). *Journal of Geotechnical Engineering ASCE*, 101(4):409–412, 01 1975.
- G. Mesri. A reevaluation of su(mob) = 0.22 p'using laboratory shear tests. Canadian Geotechnical Journal, 26(1): 162-164, 1989.

- G. Mesri and N. Huvaj. Shear strength mobilized in undrained failure of soft clay and silt deposits. Number 173, page 1, 2007. doi: 10.1061/40917(236)1.
- J. K. Mitchel. Fundamentals of soil behaviour. 2nd ed. John Wiley and Sons, 1993.
- NAVFAC. Soil mechanics: Design manual 7-1. Naval Facilities Engineering Command, Alexandria, 1982.
- K.-K. Phoon and F.H. Kulhawy. Characterization of geotechnical variability. *Canadian Geotechnical Journal*, 36(4): 612–624, 1999a. doi: 10.1139/t99-038.
- K.-K. Phoon and F.H. Kulhawy. Evaluation of geotechnical property variability. *Canadian Geotechnical Journal*, 36 (4):625–639, 1999b. doi: 10.1139/t99-039.
- K.-K. Phoon and F.H. Kulhawy. Serviceability limit state reliability-based design. 2008. doi: 10.1201/9781482265811-14.
- P.K. Robertson. Soil classification using the cone penetration test. Canadian Geotechnical Journal, 27(1):151–158, 1990. doi: 10.1139/t90-014.
- C. Stas and F. Kulhawy. Critical evaluation of design methods for foundations under axial uplift and compression loading, final report. *Electric Power Re-search Institute*, *Palo Alto*, *Calif*, 1984.
- K. Terzaghi, R. Peck, and G. Mesri. Soil mechanics in engineering practice. 1996.
- C.P. Wroth and D.M. Wood. Correlation of index properties with some basic engineering properties of soils. *Canadian Geotechnical Journal*, 15(2):137–145, 1978. doi: 10.1139/t78-014.

A Summary of database CLAY/10/7490 数据库 CLAY/10/7490 概览