From: Mary Szoka <mszoka@alachuacounty.us>
Subject: RE: Tara April- Do you know about this project?
To: Dycus, Douglas <Douglas.Dycus@dot.state.fl.us>; Smith, Brandon <Brandon.Smith@dot.state.fl.us>
Cc: Stacie Greco <sgreco@AlachuaCounty.US>; Adams, Bradley <Bradley.Adams@dot.state.fl.us>; Woodard, Randall <Randall.Woodard@dot.state.fl.us>; Senter, Chris <Chris.Senter@dot.state.fl.us>; Perez, Ernesto H <ErnestoH.Perez@dot.state.fl.us>
Sent: November 17, 2025 10:39 AM (UTC-05:00)
Attached: image001.png, image002.png, image003.png, image004.png, image005.png, image006.png, image007.png, image008.png, 250x80-solid_1d829c46-8f9d-4188-a3fb-6f9ac2866e6b.png, picture3_b83a3c75-d14f-41fe-8ec8-7a9caa87172b.png, facebooks_3ba592bc-4ba3-4ab5-9bc6-2aead9843ce5.png, x_eeea5be5-5ff9-4f7b-aad6-9f030441699a.png, insta_24e91629-23dc-4177-b3d5-cf9b61347ccb.png, you_c5569e06-2aac-451e-94b1-07707458460e.png, county_news_62f65719-1553-4875-a002-735233338cec.png, Tara_Drainage.pdf

Hi Doug,

Please see the Drainage report attached which sys, “The Tara April Infrastructure project proposes the construction of two dry retention basins for flood
plain compensation and water quality treatment volume for I-75, and two dry retention basins for the future Tara April development.”

The infrastructure plans previously shared shows splitting stormwater between two ponds from a structure that goes under April Boulevard.

Thanks for looking into this,
Mary

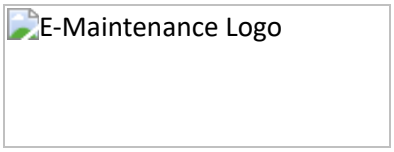
From: Dycus, Douglas <Douglas.Dycus@dot.state.fl.us>
Sent: Monday, November 17, 2025 10:27 AM
To: Mary Szoka <mszoka@alachuacounty.us>; Smith, Brandon <Brandon.Smith@dot.state.fl.us>
Cc: Stacie Greco <sgreco@AlachuaCounty.US>; Adams, Bradley <Bradley.Adams@dot.state.fl.us>; Woodard, Randall <Randall.Woodard@dot.state.fl.us>; Senter, Chris <Chris.Senter@dot.state.fl.us>; Perez, Ernesto H <ErnestoH.Perez@dot.state.fl.us>
Subject: Re: Tara April- Do you know about this project?

Mary,

We have had a Pre-Application meeting with EDA and there was no discussion of the Interstate runoff being directed into their ponds. They may be referring to the runoff from April Road which belongs to FDOT. I do not recall them discussing April Road either. I have copied the Permits team on this email and they may provide more insight since the meeting was so long ago.

Sincerely,

Douglas Dycus, P.E., CPM
District Environmental/Permit Engineer
FDOT District 2
1109 S. Marion Avenue
Lake City, FL 32025
386-961-7490
386-288-6882 Cell



From: Mary Szoka <mszoka@alachuacounty.us>
Sent: Monday, November 17, 2025 10:18 AM
To: Dycus, Douglas <Douglas.Dycus@dot.state.fl.us>; Smith, Brandon <Brandon.Smith@dot.state.fl.us>
Cc: Stacie Greco <sgreco@AlachuaCounty.US>
Subject: Tara April- Do you know about this project?

Good afternoon gentlemen,

We have been reviewing proposed developments in the City of Alachua surrounding Mill Creek Sink, near the intersection of i-75 and 441.

The Tara April project is proposing stormwater ponds to treat stormwater runoff from i-75 and claims coordination with FDOT for such. Can you speak to this? Are you all aware of and in agreement with this project? In my experience, FDOT typically does not allow other entities receiving and treating FDOT stormwater runoff.

Thanks for your insight,
Mary

Mary Szoka, P.E.
Environmental Professional Engineer
Environmental Protection Department
14 NE 1st Street • Gainesville • FL • 32601
352-264-6831 (office)


e
Maintenance

DISCOVER

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ALACHUA COUNTY

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COUNTY
NEWS

eda consultants inc.

DRAINAGE DESIGN NOTES

for

Tara April Infrastructure

Engineer of Record:
Claudia Vega, PE
Cert. No. 51532

Project Designer:
DJ McGrath, EI

Submitted to:
City of Alachua
Suwannee River Water Management District

Submitted:
January 31, 2022

DRAINAGE DESIGN NOTES

Prepared for

Tara April Infrastructure

Professional Engineer of Record:

Claudia Vega, PE
Cert. No. 51532

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Attachments

EXECUTIVE SUMMARY

The Tara April Infrastructure project proposes the consruction of two dry retention basins for flood plain compensation and water quality treatment volume for I-75, and two dry retention basins for the future Tara April development.

The proposed dry retention basins, Compensation Basin 1 and 2, are located adjacent to an existing flood plain. The compensation basins provide compensation volume for the propose Tara Pheonicia development which proposes to fill a portion of the flood plain. The basins are designed to provide signficantly more volume than the Tara Pheonicia development proposes to fill therefore avoiding any increase in the 100-year flood plain. The two compensation basins are also designed to provide water quality treatment volume for the existing drainage area which flows into the project parcel off of I-75 per coordination with the Florida Department of Transportation.

Two more dry retention basins are proposed to provide water quality treatment volume and attenuation of the future Tara April development. The basins are designed per SRWMD criteria for dry retention basins and have allocated a total of approximately 360,000 SF of impervious area for the future development.

DRAINAGE DESIGN NOTES

A. PROJECT NAME Tara April Infrastructure
Alachua, Florida

B. PROJECT LOCATION

County: Alachua
Sections: 9 Township: 8 South Range: 18 East
General Location: 17129 April Blvd
Alachua, FL 32608

C. GENERAL PROJECT INFORMATION

The proposed development consists of the construction of two dry retention basins for flood plain compensation and water quality treatment volume for I-75 and two dry retention basins for future development.

D. DRAINAGE AND DESIGN CRITERIA

1. Suwannee River Water Management District (SRWMD)
   Meet requirements of 40C-42.

2. City of Alachua, Florida
   Meet requirements of Engineering Design Manual

E. SITE SOILS INFORMATION

Universal Engineering Sciences, Inc. conducted a subsurface investigation on the site and summarized their findings in the report No. 1394818 dated November 28, 2016. A copy of the report is provided in Attachment A.

F. EXISTING SITE CONDITIONS

The existing site is undeveloped and lightly wooded.

G. DRAINAGE DESCRIPTION

1. Pre-development Conditions
   The site has a topography ranging from an elevation of 49 ft to 82 ft and has positive flow in the eastern direction towards an existing 100-year flood plain.

2. Post-development Conditions
   The proposed stormwater conveyance system consists of four above ground dry retention basins that will provide water quality treatment, recovery, and attenuation as required by the Suwannee River Water Management District and the City of Alachua, Florida.

   Pre and Post-development conditions have been met as required by the Suwannee River Water Management District and the City of Alachua, Florida.

H. DRAINAGE DESIGN

1) PRE DEVELOPMENT DRAINAGE AREA

Pre DA-1	Area (sf)	Area (ac)	Curve CN
Existing Impervious	54,436	1.25	98.0
Basin	0	0.00	100.0
Open	983,019	22.57	51.0
TOTALS	**1,037,455**	**23.82**	**53.5**

Offsite DA I-75	Area (sf)	Area (ac)	Curve CN
Existing Impervious	485,722	11.15	98.0
Basin	0	0.00	100.0
Open "B" - Good Condition	25,564	0.59	61.0
TOTALS	**511,286**	**11.74**	**96.2**

Open Space CN	% of Area	CN	Final CN
Type A (Good Condition)	33.33%	31	**51.0**
Type B (Good Condition)	66.67%	61	

2) POST DEVELOPMENT DRAINAGE AREAS

DA-1	Area (sf)	Area (ac)	Curve CN
Impervious	10,195	0.23	98.0
Open	32,996	0.76	51.0
Compensation Area	100,061	2.30	51.0
TOTALS	**143,252**	**3.29**	**54.3**

DA-2	Area (sf)	Area (ac)	Curve CN
Impervious	19,493	0.45	98.0
Open	69,353	1.59	51.0
Compensation Area	165,933	3.81	51.0
TOTALS	**254,779**	**5.85**	**54.6**

DA-3	Area (sf)	Area (ac)	Curve CN
Impervious	165,815	3.81	98.0
Open	75,189	1.73	51.0
Basin	48,959	1.12	100.0
TOTALS	**289,963**	**6.66**	**86.2**

DA-4	Area (sf)	Area (ac)	Curve CN
Impervious	218,929	5.03	98.0
Open	57,342	1.32	51.0
Basin	73,190	1.68	100.0
TOTALS	**349,461**	**8.02**	**90.7**

Offsite DA I-75	Area (sf)	Area (ac)	Curve CN
Existing Impervious	485,722	11.15	98.0
Basin	0	0.00	100.0
Open "B" - Good Condition	25,564	0.59	61.0
TOTALS	**511,286**	**11.74**	**96.2**

3) POST DEVELOPMENT BASIN STORAGE DATA

Compensation Basin-1

Stage (MSL)	Area (sf)	Area (ac)	Volume (cf)	Volume (ac-ft)
46.00	78,902	1.81	0	0.00
46.70	83,763	1.92	56,933	1.31
47.00	85,846	1.97	82,374	1.89
48.00	92,899	2.13	171,747	3.94
49.00	100,061	2.30	268,227	6.16
50.00	107,332	2.46	371,923	8.54

Compensation Basin-2

Stage (MSL)	Area (sf)	Area (ac)	Volume (cf)	Volume (ac-ft)
46.00	134,623	3.09	0	0.00
46.70	141,845	3.26	96,764	2.22
47.00	144,940	3.33	139,782	3.21
48.00	155,377	3.57	289,940	6.66
49.00	165,933	3.81	450,595	10.34
50.00	176,610	4.05	621,867	14.28

Basin-3

Stage (MSL)	Area (sf)	Area (ac)	Volume (cf)	Volume (ac-ft)
70.00	36,037	0.83	0	0.00
71.00	40,244	0.92	38,141	0.88
72.00	44,552	1.02	80,539	1.85
72.53	46,888	1.08	104,770	2.41
73.00	48,959	1.12	127,294	2.92
74.00	53,467	1.23	178,507	4.10
75.00	58,076	1.33	234,279	5.38

Basin-4

Stage (MSL)	Area (sf)	Area (ac)	Volume (cf)	Volume (ac-ft)
54.00	52,217	1.20	0	0.00
54.54	54,967	1.26	28,940	0.66
55.00	57,309	1.32	54,763	1.26
56.00	62,503	1.43	114,669	2.63
57.00	67,796	1.56	179,819	4.13
58.00	73,190	1.68	250,312	5.75
59.00	78,686	1.81	326,250	7.49
60.00	84,281	1.93	407,733	9.36

4) WATER QUALITY TREATMENT VOLUME

The proposed Compensation Basins 1 and 2 provide water quality treatment volume for the existing I-75 drainage area and the proposed drainage areas per SRWMD criteria. Proposed basins 3 and 4 provide water quality treatment volume for the future Tara April development per SRWMD criteria.

Volume = 2.00 inches over the total area

Drainage Area	Treat. Vol Required (cf)	Treat. Vol Provided (cf)
Compensation Area 1 (DA-1)	23,875	268,227
Compensation Area 2 (DA-2)	42,463	450,595
Offsite I-75 Area	85,214	0
TOTAL*	**151,553**	**718,822**

*Note: The qater quality treatment volume for Compensation Basins 1 and 2 and the Offsite I-75 area is split between the two basins due to the equalizer pipe connecting them.

Drainage Area	Treat. Vol Required (cf)	Treat. Vol Provided (cf)
DA-3	48,327	80,539
DA-4	85,214	114,669
TOTAL**	**133,542**	**195,208**

**The full WQTV for DA-4 does not infiltrate within 72 hours therefore additional WQTV is infiltrated in Basin 3 to compensate for the shortage.

5) SUBSURFACE INVESTIGATION INFORMATION

Based on the Soils Report No. 15138, dated July 12, 2021, prepared by GSE Engineering and Consulting, Inc. the recommendations of the soil characteristics are summarized below:

Soil Report No. 15138				
Soil Boring	B-1 thru B-6	B-7 thru B-16	B-17 thru B-20	B-21 thru B-24
Average Ground El.	51.00	51.50	57.00	75.00
Depth Confined layer (ft)	13.00	9.00	5.00	14.00
Depth of groundwater (ft)	12.50	8.50	4.50	12.00
Vertical Infiltration Rate (ft/d)	7.00	9.50	8.00	3.00
Safety Factor	2.00	2.00	2.00	2.00
Design Vertical Rate (ft/d)	3.50	4.75	4.00	1.50
Horizontal Infiltration Rate (ft/d)	10.50	14.50	12.00	4.50
Safety Factor	2.00	2.00	2.00	2.00
Design Horizontal Rate (ft/d)	5.25	7.25	6.00	2.25
Fillable porosity (%)	20.00	20.00	20.00	20.00

6) STRUCTURE INFORMATION

The stormwater management facilities will have a structures for discharge as follows:

CONTROL STRUCTURE	BASIN 1	BASIN 2	BASIN 3	BASIN 4
Weir Elevation (MSL)	50.00	50.00	72.75	57.00
Weir Width (ft)	1,174.00	614.00	0.50	0.50
Weir height (ft)	-	-	1.50	2.50

7) RECOVERY OF TREATMENT VOLUME FOR DRY RETENTION SYSTEM

The criteria for the recovery of the system is the recovery of the required water quality volume within 72 hours following the critical storm event.

WQTV Recovery				
	Compensation-1	**Compensation-2**	**Basin 3**	**Basin 4**
Total WQTV (ac-ft):	0.55	0.97	1.11	1.96
Recovery Time(hrs):	5.25	6.00	68.75	67.00

8) STORM ROUTING RESULTS

The computer program ICPR was used to route the critical design storms, the input data and results can be seen in Attachment C. The 'Runoff Area' is analyzed to compare the pre-existing runoff. Discharge rate and volume are reduced with the proposed design.

Storm Event	Compensation Basin 1 Stage	Compensation Basin 2 Stage
100YR-001HR	48.05	47.10
100YR-002HR	47.95	47.25
100YR-004HR	47.89	47.47
100YR-008HR	47.70	47.61
100YR-024HR	48.13	48.12
100YR-072HR	48.66	48.66
100YR-168HR	49.07	49.06
100YR-240HR	49.34	49.34

Storm	Basin 3		Basin 4	
Event	Stage	Freeboard (ft)	Stage	Freeboard (ft)
100YR-001HR	71.77	3.23	55.59	4.41
100YR-002HR	72.21	2.79	55.88	4.12
100YR-004HR	72.77	2.23	56.44	3.56
100YR-008HR	73.07	1.93	57.10	2.90
100YR-024HR	73.34	1.66	58.09	1.91
100YR-072HR	73.49	1.51	58.46	1.54
100YR-168HR	73.74	1.26	58.73	1.27
100YR-240HR	73.75	1.25	58.77	1.23

Storm	Rates		Volumes	
Event	Pre (cfs)	Post (cfs)	Pre (ac-ft)	Post (ac-ft)
100YR-001HR	119.12	0.00	5.12	0.00
100YR-002HR	98.89	0.00	7.01	0.00
100YR-004HR	65.33	0.00	9.75	0.00
100YR-008HR	77.55	0.05	12.61	0.04
100YR-024HR	27.14	1.82	19.93	2.41
100YR-072HR	20.81	2.81	27.01	4.78
100YR-168HR	15.14	3.63	32.84	6.91
100YR-240HR	19.79	3.77	38.26	9.02

Attachment A

Geotechnical Report by Universal Engineering Sciences

GSE Engineering & Consulting, Inc.

SUMMARY REPORT OF A
GEOTECHNICAL SITE EXPLORATION

PROPOSED TARA APRIL
ALACHUA, ALACHUA COUNTY, FLORIDA

GSE PROJECT NO. 15138

Prepared For:

TARA APRIL, LLC

JULY 2021

Certificate of Authorization No. 27430

GSE Engineering & Consulting, Inc.

July 12, 2021

Mr. Sayed Moukhtara
Tara April, LLC
7717 NW 20th Lane
Gainesville, FL 32605

Subject: Summary Report of a Geotechnical Site Exploration
Proposed Tara April
Alachua, Alachua County, Florida
GSE Project No. 15138

Dear Mr. Moukhtara:

GSE Engineering & Consulting, Inc. (GSE) is pleased to submit this geotechnical site exploration report for the above referenced project.

Presented herein are the findings and conclusions of our exploration, including the geotechnical parameters and recommendations to assist with stormwater management designs.

GSE appreciates this opportunity to have assisted you on this project. If you have any questions or comments concerning this report, please contact us.

Sincerely,

GSE Engineering & Consulting, Inc.

JASON E. GOWLAND LICENSE No.66467 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been digitally signed and sealed by Jason E Gowland
Digitally signed by Jason E Gowland
Date: 2021.07.12 09:33:17 -04'00'
on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Kevin P. Fisher, E.I.
Staff Engineer

Jason E. Gowland, P.E.
Senior Engineer
Florida Registration No. 66467

KPF / JEG:maj
Z:Projects\ 15138 Proposed Tara April / 15138.doc

Distribution: Addressee (1 - Electronic)
File (1)

GSE Engineering & Consulting, Inc.
5590 SW 64th Street, Suite B
Gainesville, Florida 32608
(352) 377-3233 Phone ◆ (352) 377-0335 Fax
www.gseengineering.com
Certificate of Authorization No. 27430

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LIST OF FIGURES

1.0 INTRODUCTION

1.1 General

GSE Engineering & Consulting, Inc. (GSE) has completed this geotechnical exploration for the proposed Tara April development located in Alachua, Alachua County, Florida. This exploration was performed in accordance with GSE Proposal No. 2021-321 dated June 2, 2021. Mr. Sayed Moukhtara, Owner, of Tara April, LLC authorized our services on June 3, 2021.

1.2 Project Description

This project will consist of a residential development. The site is located on the east side of I-75 and north of US Highway 441 in Alachua, Florida.

Mr. Sergio Reyes, P.E. with eda consultants provided information about the project and a site plan illustrating the locations of the proposed improvements.

The project will have five (5) SMF's. The SMF's will be located along the western property boundary. eda provided a site plan indicating the locations of the requested twenty-seven (27) soil borings for the site.

A recent aerial photograph of the site was obtained. The site plan and aerial photograph were used in preparation of this exploration and report.

1.3 Purpose

The purpose of this geotechnical exploration was to determine the general subsurface conditions, evaluate these conditions with respect to the proposed construction, and prepare geotechnical parameters and recommendations to assist with stormwater management designs.

2.0 FIELD AND LABORATORY TESTS

2.1 General Description

The procedures used for field sampling and testing are in general accordance with industry standards of care and established geotechnical engineering practices for this geographic region. This exploration consisted of performing twenty-seven (27) auger borings to depths of 15 feet bls within the proposed stormwater management facilities at the locations provided by eda.

The soil borings were performed at the approximate locations as shown on Figure 2. The borings were located at the site using the provided site plan and obvious site features as reference. The boring locations should be considered approximate. The soil borings were performed from June 23 through 24, 2021.

2.2 Auger Borings

The auger borings were performed in accordance with ASTM D1452. The borings were performed with flight auger equipment that was rotated into the ground in a manner that reduces soil disturbance. After penetrating to the required depth, the auger was retracted and the soils collected on the auger flights were field classified and placed in sealed containers. Representative samples of each stratum were retained from the auger boring. Results from the auger borings are provided in Section 5.1.

2.3 Soil Laboratory Tests

The soil samples recovered from the soil borings were returned to our laboratory, and examined to confirm the field descriptions. Representative samples were then selected for laboratory testing. The laboratory tests consisted of twenty (20) percent soil fines passing the No. 200 sieve determinations, twenty (20) natural moisture content determinations, and twelve (12) constant head hydraulic conductivity tests. These tests were performed in order to aid in classifying the soils and to further evaluate their engineering properties. The laboratory tests are provided in Section 5.2.

3.0 FINDINGS

3.1 Surface Conditions

Mr. Kevin P. Fisher, E.I. and Ms. Angelina X. Lie, E.I. with GSE visited the site on June 8, 2021 to observe the site conditions and mark the boring locations.

The site was previously planted in pine trees that were recently harvested. The site now consists of stumps, scrubs, and brush. The site is bordered by April Boulevard to the west.

The topography at the site is gently to moderately sloping down toward the east from the west. Regional topography is gently rolling hills. The Alachua County Growth Management website indicates the ground surface elevations at the site are near elevations 38 to 82 feet[1].

3.2 Subsurface Conditions

The locations of the auger borings are provided on Figure 2. Complete logs for the borings are provided in Section 5.1. Descriptions for the soils encountered are accompanied by the Unified Soil Classification System symbol (SM, SP-SM, etc.) and are based on visual examination of the recovered soil samples and the laboratory tests performed. Stratification boundaries between the soil types should be considered approximate, as the actual transition between soil types may be gradual.

The auger borings located in the proposed stormwater management facilities indicate the soils across these areas are relatively consistent. The auger borings initially penetrated 1 to 15 feet of a near-surface sandy stratum consisting of poorly graded sand, sand with silt, sand with clay, and silty sand (SP, SP-SM, SP-SC, SM). This was underlain by clayey to very clayey sand (SC, SC/CL) or clay-rich confining soils consisting of sandy clay, clay with sand, and clay (CL/CH) to the explored depths of 15 feet bls. Auger borings B-18, B-19, and B-24 encountered shallow limestone beginning at depths of 3.5 to 13 feet bls to the 15 feet boring termination depths. Auger borings B-5, B-12, B-16, B-17, B-18, B-20, and B-26 encountered shallow clay-rich soils beginning at depths of 0 to 2 feet bls.

The groundwater table was encountered in auger boring B-4 at a depth of 12.5 feet bls at the time of our investigation.

3.3 Review of Published Data

The majority of the site is mapped as six soil series by the Soil Conservation Service (SCS) Soil Survey for Alachua County[2]. The following soil descriptions are from the Soil Survey.

Surrency sand - This nearly level, very poorly drained soil is in ponds and depressional areas in the broad flatwoods and in areas of wet prairie on uplands. Slopes are less than 1 percent. The areas are relatively small and range from about 10 to 40 acres.

Typically, the surface layer is black sand about 15 inches thick. The subsurface layer is light gray sand to a depth of 28 inches. Between 28 and 80 inches, the subsoil is sandy clay loam. The upper 27 inches is gray, and the lower 25 inches is light gray.

[1] Alachua County Growth Management website, http://mapgenius.alachuacounty.us/.

[2] Soil Survey of Alachua County, Florida. Soil Conservation Service, U.S. Department of Agriculture.

Included with this soil in mapping are small areas of Monteocha, Pomona, Samsula, and Wauberg soils. Also included are small areas of soils that have a 10- to 24-inch, black or very dark gray sand or loamy sand surface layer over a gray sandy clay loam subsoil. In some delineations are small areas of soils which are similar to this Surrency soil but which have 3 to 10 inches of well-decomposed organic material covering the surface. In some small areas the subsoil decreases in clay content by 20 percent or more at a depth of about 55 to 60 inches. Total included areas are about 20 percent or less.

This Surrency soil has a water table that is within 10 inches of the surface for about 6 months or more during most years. Water is on the surface for 4 months or more. The available water capacity ranges from low to high in the surface and subsurface layers and from low to medium in the subsoil. Permeability is moderately rapid to rapid in the sandy surface and subsurface layers and slow to moderately slow in the loamy subsoil. Natural fertility is medium in the surface layer and is low in the subsurface layer and subsoil. Organic matter content is high to very high in the surface layer.

Kendrick sand, 2 to 5 percent slopes - This gently sloping, well drained soil is in both small and large areas on the gently rolling uplands. These areas are mostly irregularly shaped or elongated and range from about 20 to 200 acres.

Typically, the surface layer is dark grayish brown sand about 9 inches thick. The subsurface layer is yellowish brown loamy sand to a depth of 26 inches. The subsoil extends to a depth of 90 inches or more. The upper 5 inches is yellowish brown fine sandy loam; the next 20 inches is dark yellowish brown, mottled sandy clay loam; the next 22 inches is dark yellowish brown sandy clay loam; the next 10 inches is yellowish brown, mottled fine sandy loam; and the lower 7 inches is yellowish brown sandy clay loam.

Included with this soil in mapping are some small areas of soils that have similar characteristics to the Kendrick soils except that they have loamy sand surface and subsurface layers less than 20 inches thick over a sandy clay loam subsoil. Small areas of soils that are similar to the Kendrick soils but have fine sand surface and subsurface layers or have a subsoil that is sandy clay throughout are included. Also included are small areas of Arredondo, Blichton, Bonneau, Lochloosa, and Norfolk soils. A few areas of Kendrick soils have 0 to 2 percent slopes to 5 to 8 percent slopes. Small moderately eroded spots are in a few areas. Sinkholes and limestone boulders are in some areas and are shown by appropriate symbols. Total included areas are about 15 percent.

In this Kendrick soil, the available water capacity is low in the surface and subsurface layers, medium in the upper 5 inches of the subsoil, and medium to high below this depth. Permeability is rapid in the surface and subsurface layers. Permeability is moderate to moderately rapid in the upper 5 inches of the subsoil, moderately slow to moderate in the next 42 inches, slow in the lower 17 inches. Organic matter content is low to moderately low in the surface layer. The water table is more than 72 inches below the surface. Surface runoff is moderately slow.

Blichton sand, 0 to 2 percent slopes - This nearly level to gently sloping, poorly drained soil is on relatively broad flats and at the base of slopes of the gently rolling uplands. Areas are irregular in shape and range from about 10 to 50 acres.

Typically, the surface layer is very dark gray sand about 6 inches thick. The subsurface layer is light brownish gray sand to a depth of 24 inches and has about 2 percent nodules of ironstone and fragments of phosphatic limestone. The subsoil extends to a depth of 80 inches or more. The upper 6 inches is gray sandy loam; the next 33 inches is gray sandy clay loam that is 7 percent plinthite by volume; and the lower 14 inches is mixed gray and olive gray sandy clay loam that has mottles of brown, red, and yellow.

Included with this soil in mapping are small areas of Bivans, Lochloosa, and Lynne soils. Small areas of soils that are similar to this Blichton soil but that have a 10- to 18-inch, black or very dark gray loamy sand surface layer over a sandy clay subsoil are in some areas. Small areas of Blichton soils that have slopes of 2 to 5 percent are included. Total included areas are less than 20 percent.

This Blichton soil has a water table that is less than 10 inches below the surface for 1 to 4 months during most years. Surface runoff is slow. The available water capacity is low in the sandy surface and subsurface layers and low to medium in the loamy subsoil. Permeability is rapid in the sandy surface and subsurface layers and slow to moderately slow in the loamy Subsoil.

Arredondo fine sand, 5 to 8 percent slopes - This nearly level to sloping, well drained soil forms in thick beds of sandy and loamy marine materials. These soils are in broad rolling areas of the upland. Slopes range from 0 to 8 percent. The water table is more than 72 inches below the surface. These soils are loamy siliceous, hyperthermic Grossarenic Paleudults.

Arredondo soils are geographically associated with Apopka, Bonneau, Candler, Fort Meade, Gainesville, Jonesville, Kanapaha, Kendrick, Lake Millhopper, and Norfolk soils. Apopka soils have less than 5 percent silt and clay in the A2 horizon, and many of the sand grains are uncoated. Bonneau soils are moderately well drained and have an A horizon 20 to 40 inches thick. Candler soils are sandy to a depth of 80 inches or more and have less than 5 percent silt and clay in their 10- to 40-inch control section. Fort Meade and Gainesville soils are sandy to a depth of more than 80 inches. They have 10 to 15 percent silt and clay in their 10- to 40-inch control section. Fort Meade soils also have a thick, dark colored A1 horizon. Jonesville soils have underlying limestone at a depth of less than 60 inches. Lake soils are sandy to 80 inches or more. Kanapaha soils are poorly drained, and Millhopper soils are moderately well drained. Norfolk soils have an A horizon less than 20 inches thick.

Lochloosa fine sand, 5 to 8 percent slopes - This nearly level to sloping, somewhat poorly drained soil that forms in thick beds of loamy marine deposits. These soils are in broad areas of the gently rolling uplands and in slightly convex areas of the flatwoods. Slopes range from 0 to 8 percent. The water table is about 30 to 40 inches below the surface for about 1 to 4 months during most years. It rises to 15 to 30 inches for about 1 to 4 weeks during most years. During most of the remained of the year it is at a depth of more than 40 inches. These soils are loamy, siliceous, hyperthermic Aquic Arenic Paleudults.

Lochloosa soils are geographically associated with Blichton, Bivans, Boardman, Bonneau, Kanapaha, Kendrick, Micanopy, Millhopper, Sparr, and Wacahoota soils. Blichton, Bivans, Boardman, Kanapaha, and Wacahoota soils are all poorly drained. Blichton soils are more than 5 percent plinthite. In addition, the Bivans soils have a clayey Btg horizon within 20 inches of the surface and contain more than 5 percent nodules and fragments of limestone. Kanapaha soils have an A horizon 40 to 80 inches thick. Wacahoota soils are more than 5 percent nodules and fragments of limestone. Kendrick soils are well drained. Miacnopy soils have a clayey Bt horizon within 20 inches of the surface. Millhopper soils are moderately well drained and have an A horizon 40 to 80 inches thick. Sparr soils have a sandy A horizon 40 to 80 inches thick.

Bivans sand, 5 to 8 percent slopes - This is a sloping, poorly drained soil on short breaking slopes and along hillsides of the uplands. The areas are irregular and elongated in shape. They range from about 5 to 40 acres.

Typically, the surface layer is dark gray sand about 5 inches thick. The subsurface layer is light brownish gray sand about 5 inches thick. It has a few nodules of ironstone and fragments of phosphatic limestone. The subsoil extends to a depth of 59 inches. The upper 20 inches is gray sandy clay and a few nodules of ironstone and fragments of phosphatic limestone. The next 29 inches is gray, mottled sandy clay. Between depths of 59 and 80 inches, the underlying material is gray, mottled sandy clay.

Included with this soil in mapping are small areas of Blichton, Boardman, Lochloosa, and Wacahoota soils. Small areas of soils that are similar to Bivans soils but that have a very dark gray or black loamy sand surface layer 8 to 12 inches thick over a sandy clay loam subsoil are also included in some areas. Small areas of Bivans soil that have slopes of 2 to 5 percent are included. Total included areas are about 15 percent or less.

In this Bivans soil, the subsurface layer and upper part of the subsoil are saturated by a perched water table for 1 to 3 months during most years. Wetness is caused mainly by hillside seepage. Surface runoff is rapid. The available water capacity is low to medium. Permeability is moderate to moderately rapid in the surface and subsurface layers. It is very slow to slow in the subsoil. Natural fertility is low to medium, and the organic matter content is moderately low to moderate in the surface layer.

3.4 Laboratory Soil Analysis

Selected soil samples recovered from the soil borings were analyzed for the percent soil fines passing the No. 200 sieve, natural moisture content, and hydraulic conductivity. Samples selected for laboratory testing were collected at depths ranging from near ground surface to 14 feet bls. These tests were performed to confirm visual soil classification and evaluate their engineering properties. The complete laboratory report is provided in Section 5.2.

The laboratory tests indicate the tested soils consist of poorly graded sand, sand with silt, sand with clay, silty sand, clayey sand, and very clayey sand. The tested poorly graded sand (SP) contains approximately 4.8 to 4.9 percent soil fines passing the No. 200 sieve with natural moisture contents of about 7.1 to 11 percent. The tested sand with silt (SP-SM) contains approximately 5.4 to 6.4 percent soil fines passing the No. 200 sieve with natural moisture contents of about 6.0 to 11 percent. The tested sand with clay (SP-SC) contains approximately 9.2 to 10 percent soil fines passing the No. 200 sieve with natural moisture contents of about 8.7 to 19 percent. The tested silty sand (SM) contains approximately 11 to 12 percent soil fines passing the No. 200 sieve with natural moisture contents of about 16 to 19 percent. The tested clayey sand (SC) contains approximately 15 to 23 percent soil fines passing the No. 200 sieve with natural moisture contents of about 9 to 19 percent. The tested very clayey sand (SC/CL) contains approximately 31 to 45 percent soil fines passing the No. 200 sieve with natural moisture contents of about 17 to 26 percent.

The constant head hydraulic conductivity test results indicate the near-surface poorly graded sand (SP) has hydraulic conductivity values of 13 to 15 feet per day. The tested sand with silt (SP-SM) has hydraulic conductivity values of 18 to 24 feet per day. The tested sand with clay (SP-SC) has hydraulic conductivity values of 3.3 to 11 feet per day. The tested silty sand (SM) has a hydraulic conductivity value of 5.6 feet per day. The tested clayey sand (SC) has hydraulic conductivity values of 1.1 to 11 feet per day.

4.0 EVALUATION AND RECOMMENDATIONS

4.1 General

The following recommendations are made based upon our understanding of the proposed construction, a review of the attached soil borings and laboratory test data, and experience with similar projects and subsurface conditions. If plans or the location of proposed construction changes from those discussed previously, GSE requests the opportunity to review and possibly amend our recommendations with respect to those changes.

The performance of site improvements may be sensitive to their post-construction relationship to site groundwater levels, seepage zones, or soil/rock characteristics exposed at final site grades. GSE recommends that use of boring information for final design of all site improvements be predicated on proper horizontal and vertical control of borings.

In this section of the report, we present our geotechnical parameters and recommendations to assist with stormwater management designs.

4.2 Groundwater

The groundwater table was encountered in auger boring B-4 at a depth of 12.5 feet bls at the time of our exploration. However, you should expect water to perch on top of the clay-rich soils after periods of heavy and seasonal rainfall.

4.3 Stormwater Management

The soil conditions at the stormwater management facility are relatively consistent; initially penetrating 1 to 15 feet of a near-surface sandy stratum consisting of poorly graded sand, sand with silt, sand with clay, and silty sand (SP, SP-SM, SP-SC, SM). This was underlain by clayey to very clayey sand (SC, SC/CL) or clay-rich confining soils consisting of sandy clay, clay with sand, and clay (CL/CH) to the explored depths of 15 feet bls. Auger borings B-18, B-19, and B-24 encountered shallow limestone beginning at depths of 3.5 to 13 feet bls to the 15 feet boring termination depths. Auger borings B-5, B-12, B-16, B-17, B-18, B-20, and B-26 encountered shallow clay-rich soils beginning at depths of 0 to 2 feet bls.

The water table was encountered in auger boring B-4 at a depth of 12.5 feet bls at the time of our exploration. We anticipate the seasonal high groundwater table to be perched on the very clayey sands and clay-rich soils.

The laboratory permeability tests indicate the surficial layer of poorly graded sand, sand with silt, sand with clay, and silty sand has hydraulic conductivity values of 3.3 to 24 feet per day. The clayey sand has hydraulic conductivity values of 1.1 to 11 feet per day. The deeper clayey sand encountered below the surficial sandy material is friable and will have permeability values at least one order of magnitude lower than the sandy soils. The underlying very clayey sand, sandy clay, clay with sand, and clay are expected to be confining soils.

Based upon our findings and test results, our recommended soil parameters for the stormwater management design in the explored areas are presented below. The recommended parameters consider the results of the permeability tests, wash 200 determinations, and our experience with these types of soils. The parameters below do not consider a factor of safety.

Proposed Stormwater Management Facility (B-1 through B-6)

1. Base elevation of effective or mobilized aquifer (average depth of confining layer) equal to 13 feet bls.
2. Unsaturated vertical infiltration rate of 7 feet per day.
3. Horizontal hydraulic conductivity equal to 10.5 feet per day.
4. Specific yield (fillable porosity) of 20 percent.
5. Average seasonal high groundwater table depth equal to 12.5 feet bls.

Proposed Stormwater Management Facility (B-7 through B-16)

1. Base elevation of effective or mobilized aquifer (average depth of confining layer) equal to 9 feet bls.
2. Unsaturated vertical infiltration rate of 9.5 feet per day.
3. Horizontal hydraulic conductivity equal to 14.5 feet per day.
4. Specific yield (fillable porosity) of 20 percent.
5. Average seasonal high groundwater table depth equal to 8.5 feet bls.

Proposed Stormwater Management Facility (B-17 through B-20)

1. Base elevation of effective or mobilized aquifer (average depth of confining layer) equal to 5 feet bls.
2. Unsaturated vertical infiltration rate of 8 feet per day.
3. Horizontal hydraulic conductivity equal to 12 feet per day.
4. Specific yield (fillable porosity) of 20 percent.
5. Average seasonal high groundwater table depth equal to 4.5 feet bls.

Proposed Stormwater Management Facility (B-21 through B-24)

1. Base elevation of effective or mobilized aquifer (average depth of confining layer) equal to 14 feet bls.
2. Unsaturated vertical infiltration rate of 3 feet per day.
3. Horizontal hydraulic conductivity equal to 4.5 feet per day.
4. Specific yield (fillable porosity) of 20 percent.
5. Average seasonal high groundwater table depth equal to 12 feet bls.

Proposed Stormwater Management Facility (B-25 through B-27)

1. Base elevation of effective or mobilized aquifer (average depth of confining layer) equal to 10 feet bls.
2. Unsaturated vertical infiltration rate of 3.5 feet per day.
3. Horizontal hydraulic conductivity equal to 5 feet per day.
4. Specific yield (fillable porosity) of 20 percent.
5. Average seasonal high groundwater table depth equal to 9.5 feet bls.

In areas where plastic clay-rich soils having more than 20 percent soil fines passing the No. 200 sieve are present at the basin bottom, we recommend these soils be undercut a minimum of 2 feet and backfilled with the on-site sands and sands with silt (SP, SP-SM) having a maximum of 12 percent soil fines passing the No. 200 sieve. The intent of this undercutting and replacement is to provide a more uniform sand "blanket" at the basin bottom that allows the migration of water to the deeper deposits of sand. This sand blanket will also reduce the potential for clay-fines leaching out of the soils when water is present in the basin that can result in a thin layer of confining type material on the basin bottom that can reduce the effectiveness of the basin.

4.4 Fill Suitability

The soils encountered at this site within the explored depths range from sands (SP) to clays (CL/CH). A discussion of the suitability for reuse as structural fill for each soil classification according to the Unified Soil Classification System (USCS) designation is provided below.

SP, SP-SM – Sands (SP) and sand with silt (SP-SM) have less than 5 percent and 12 percent soil fines passing the No. 200 sieve, respectively, and are typically well draining soils that are suitable for reuse as structural fill. The sands with silt may require moisture conditioning (drying) to make the material more workable. These soils will require stockpiling and drying before they are reused if they are excavated from below the water table.

SM – Silty sands (SM) can have between 12 percent and 50 percent soil fines passing the No. 200 sieve. Silty sands are typically non-plastic or have low plasticity, and can be reused as structural fill with precautions. Silty sands can be moisture sensitive and difficult to work and compact and can rut if the moisture content is near or above the optimum moisture content. We recommend these soils be moisture conditioned (dried) so that the moisture content during use is at or below the optimum moisture content. Aerating and exposure to the sun is typically the most effective methods of drying these soils. It may not be practical to reuse these materials during the wet season, as frequent rain showers may not allow these soils to dry to a workable moisture content. Suitable silty sands are limited to soil having less than 30 percent soil fines passing the No. 200 sieve. Silty sands with more than 30 percent soil fines are especially moisture sensitive, and are not recommended for reuse as structural fill. These soils will behave more as sandy silt, and for this reason, very silty sands having more than 30 percent soil fines passing the No. 200 sieve have been assigned a dual classification of SM/ML. Silty sand soils that are excavated from below the water table are not recommended for reuse as structural fill due to the amount of time that will be required to dry these soils to a workable condition.

SC – Clayey sand (SC) soils can have between 12 percent and 50 percent soil fines passing the No. 200 sieve. Clayey sands can have a high range of plasticity, varying from a PI of 7 or greater and plotting above the A-line to highly plastic. Friable clayey sands are typically suitable for use as structural fill with precautions. Clayey sands will be moisture sensitive and difficult to work and compact and can rut during placement if the moisture content is near or above the natural moisture content. We recommend these soils be moisture conditioned (dried) so that the moisture content during use is at or below the optimum moisture content. Aerating and exposure to the sun is typically the most effective methods of drying these soils. It may not be practical to reuse these materials during the wet season, as frequent rain showers may not allow these soils to dry to a workable moisture content. Suitable clayey sands are limited to soil having less than 30 percent soil fines passing the No. 200 sieve. Clayey sands with more than 30 percent soil fines passing the No. 200 sieve are especially moisture sensitive and are typically highly plastic, and are not recommended for reuse as structural fill. These soils will behave more as sandy clay, and for this reason, very clayey sands having more than 30 percent soil fines passing the No. 200 sieve have been assigned a dual classification of SC/CH or SC/CL. Clayey sand soils that are excavated from below the water table are not recommended for reuse as structural fill due to the amount of time that will be required to dry these soils to a workable condition.

ML, MH, CL, CH – Silts and clays are not suitable materials for reuse as structural fill.

When using on-site soils as fill materials, we recommend the silty and clayey sand soils (SM, SC) be used in the lower depths of the fill. Sand and sand with silt (SP, SP-SM) should be used in the upper portions of the fill. We recommend a minimum of 2 feet of sand (SP, SP-SM) cover the silty and clayey sand fill materials to reduce the potential for soggy surface conditions due to the low permeability characteristics of the silty and clayey sand materials.

5.0 FIELD DATA

5.1 Auger Boring Logs

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CLIENT Tara April, LLC
PROJECT NAME Proposed Tara April
PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

BORING NUMBER B-1

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SC) Gray and brown SAND with clay
3.0	AU 2	(SC) Gray and brown clayey SAND
	AU 3	
	AU 4	
15.0		Bottom of borehole at 15.0 feet.

BORING NUMBER B-2

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1 PS	(SC) Brown, gray, and orange clayey SAND *%PASS - 200 = 15* *MC = 11* *k_h = 11 ft/day*
8.0	AU 2	(SC) Brown, gray, and orange clayey SAND
13.0	AU 3	(SP-SC) Gray and brown SAND with clay
15.0		Bottom of borehole at 15.0 feet.

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PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

BORING NUMBER B-3

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SC) Brown and gray SAND with clay
5.5	AU 2	(SC) Brown clayey SAND
12.0	AU 3	(SC) Brown and gray clayey SAND
15.0		Bottom of borehole at 15.0 feet.

BORING NUMBER B-4

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING 12.5 ft CHECKED BY KPF
ESTIMATED SEASONAL HIGH 12.5 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SC) Gray and brown SAND with clay
5.5	AU 2	(SC) Gray and brown clayey SAND
12.0	AU 3 PS	(SP-SC) Gray and brown SAND with clay *%PASS - 200 = 10* *MC = 19* *k_h = 9.8 ft/day*
15.0		Bottom of borehole at 15.0 feet.

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CLIENT Tara April, LLC
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PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

BORING NUMBER B-5

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH 1.0 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Gray and brown SAND with silt
1.5	AU 2	(SC/CL) Gray and brown very clayey SAND *%PASS -200 = 33* *MC = 21*
7.0	AU 3	(SC) Brown and orange clayey SAND
11.0	AU 4	(SC) Gray, brown, and orange clayey SAND
15.0		Bottom of borehole at 15.0 feet.

BORING NUMBER B-6

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Gray and brown SAND with silt
5.5	AU 2	(SP-SC) Brown SAND with clay
12.0	AU 3	(SP-SC) Brown, gray, and orange SAND with clay
15.0		Bottom of borehole at 15.0 feet.

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CLIENT Tara April, LLC
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PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

BORING NUMBER B-7

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Brown and gray SAND with silt *%PASS - 200 = 6.4* *MC = 6.0*
6.0	AU 2	(SC) Brown and orange clayey SAND
	AU 3 PS	*%PASS - 200 = 15* *MC = 9.0* *k_h = 11 ft/day*
15.0		Bottom of borehole at 15.0 feet.

BORING NUMBER B-8

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Brown and gray SAND with silt
5.5	AU 2	(SP-SC) Brown, gray, and orange SAND with clay
	AU 3	
15.0		Bottom of borehole at 15.0 feet.

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CLIENT Tara April, LLC
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PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

BORING NUMBER B-9

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Gray and brown SAND with silt
6.5	AU 2 PS	(SP-SC) Gray and brown SAND with clay *%PASS - 200 = 10* *MC = 13* $k_h = 3.3$ ft/day
11.5	AU 3	(SC) Gray and brown clayey SAND
15.0		Bottom of borehole at 15.0 feet.

BORING NUMBER B-10

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH 4.5 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0		(SP-SC) Gray and brown SAND with silt
5.0	AU 1 PS	*%PASS - 200 = 6.4* *MC = 8.9* $k_h = 18$ ft/day
5.0	AU 2	(CL/CH) Gray, brown, and orange CLAY with sand and traces of limestone
10.5	AU 3	(CL/CH) Gray, brown, and orange CLAY with traces of limestone
15.0		Bottom of borehole at 15.0 feet.

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CLIENT Tara April, LLC
PROJECT NAME Proposed Tara April
PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

BORING NUMBER B-11

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH 10.0 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1 PS	(SP) Brown and gray SAND *%PASS - 200 = 4.9* *MC = 7.1* $k_h = 15$ ft/day
5.5	AU 2	(SC) Brown, orange, and gray clayey SAND
10.5	AU 3	(CL/CH) Gray, brown, and orange CLAY
15.0		Bottom of borehole at 15.0 feet.

BORING NUMBER B-12

DATE PERFORMED 6/24/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH 0.5 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Gray and brown SAND with silt
1.0	AU 2	(SC/CL) Gray, brown, and orange very clayey SAND *%PASS - 200 = 41* *MC = 23*
6.5	AU 3	(SC) Gray, brown, and orange clayey SAND with traces of limestone
11.0	AU 4	(CL/CH) Gray, brown, and orage CLAY with traces of limestone
15.0		Bottom of borehole at 15.0 feet.

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CLIENT Tara April, LLC
PROJECT NAME Proposed Tara April
PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

BORING NUMBER B-13

DATE PERFORMED 6/23/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH 5.5 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1 PS	(SP-SM) Gray and brown SAND with silt *%PASS - 200 = 5.4* *MC = 6.3* $k_h = 24$ ft/day
6.0	AU 2	(CL/CH) Brown, gray, and orange sandy CLAY
11.5	AU 3	(CL/CH) Gray and brown CLAY
15.0		Bottom of borehole at 15.0 feet.

BORING NUMBER B-14

DATE PERFORMED 6/23/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH 7.5 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SM) Gray silty SAND
1.5	AU 2	(SC) Brown and gray clayey SAND
8.0	AU 3	(CL/CH) Gray, brown, and orange CLAY with sand and traces of limestone
11.5	AU 4	(CL/CH) Pale gray, brown, and orange CLAY with traces of limestone
15.0		Bottom of borehole at 15.0 feet.

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CLIENT Tara April, LLC
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PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

BORING NUMBER B-15

DATE PERFORMED 6/23/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1 PS	(SP-SM) Brown and gray SAND with silt *%PASS - 200 = 5.4* *MC = 11* $k_h = 20$ ft/day
3.0	AU 2	(SC) Brown and orange clayey SAND
6.0	AU 3	(SC) Gray, brown, and orange clayey SAND
12.0	AU 4	(CL/CH) Gray and brown CLAY with sand
15.0		Bottom of borehole at 15.0 feet.

BORING NUMBER B-16

DATE PERFORMED 6/23/2021
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
AT TIME OF DRILLING NE CHECKED BY KPF
ESTIMATED SEASONAL HIGH 1.0 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Brown and gray SAND with silt
1.5	AU 2	(SC/CL) Brown and orange very clayey SAND *%PASS - 200 = 45* *MC = 26*
6.0	AU 3	(CL/CH) Gray and brown CLAY with traces of limestone
11.0	AU 4	(CL/CH) Brown and orange CLAY with traces of limestone
15.0		Bottom of borehole at 15.0 feet.

(Continued Next Page)

GSE Engineering & Consulting, Inc.
5590 SW 64th St.
Gainesville, FL 32608
Telephone: (352) 377 -3233
Fax: (352) 377 -0335

CLIENT Tara April, LLC
PROJECT NAME Proposed Tara April
PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

DATE PERFORMED 6/23/2021 **BORING NUMBER B-17**
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
▼ AT TIME OF DRILLING NE CHECKED BY KPF
▽ ESTIMATED SEASONAL HIGH 1.5 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Brown and gray SAND with silt
2.0		
2.5	AU 2	(SC/CL) Brown, gray, and orange very clayey SAND *%PASS - 200 = 32* *MC = 17*
5.0	AU 3	
7.5		
10.0		
11.0		
12.5	AU 4	(CL/CH) Gray CLAY with traces of limestone
15.0		Bottom of borehole at 15.0 feet.

DATE PERFORMED 6/23/2021 **BORING NUMBER B-18**
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
▼ AT TIME OF DRILLING NE CHECKED BY KPF
▽ ESTIMATED SEASONAL HIGH 0.5 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(CL/CH) Brown, gray, and orange CLAY with sand and traces of limestone
2.5		
5.0		
7.5	AU 2	
10.0		
12.5		
13.0	AU 3	LIMESTONE
15.0		Bottom of borehole at 15.0 feet.

(Continued Next Page)

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CLIENT Tara April, LLC
PROJECT NAME Proposed Tara April
PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

DATE PERFORMED 6/23/2021 **BORING NUMBER B-19**
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
▼ AT TIME OF DRILLING NE CHECKED BY KPF
▽ ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Gray and brown SAND with silt and traces of limestone
3.5		
2.5	AU 2	LIMESTONE
5.0		
7.5	AU 3	
10.0		
12.5	AU 4	
15.0		Bottom of borehole at 15.0 feet.

DATE PERFORMED 6/23/2021 **BORING NUMBER B-20**
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
▼ AT TIME OF DRILLING NE CHECKED BY KPF
▽ ESTIMATED SEASONAL HIGH 1.5 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1 PS	(SP) Gray and brown SAND *%PASS - 200 = 4.8* *MC = 11* *k_h = 13 ft/day*
2.0		
2.5	AU 2	(CL/CH) Gray, brown, and orange CLAY with sand
5.0		
7.5	AU 3	
10.0		
12.0		
12.5	AU 4	(CL/CH) Gray and brown sandy CLAY
15.0		Bottom of borehole at 15.0 feet.

(Continued Next Page)

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Fax: (352) 377 -0335

CLIENT Tara April, LLC
PROJECT NAME Proposed Tara April
PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

DATE PERFORMED 6/23/2021 **BORING NUMBER B-21**
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
▼ AT TIME OF DRILLING NE CHECKED BY KPF
▽ ESTIMATED SEASONAL HIGH >15 ft
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Gray and brown SAND with silt
1.5		
2.5	AU 2	(SC) Gray and brown clayey SAND
5.0		
7.0		
7.5	AU 3	(SC) Gray, brown, and orange clayey SAND
10.0		
11.0		
12.5	AU 4	(SC) Gray and brown clayey SAND with traces of limestone
15.0		Bottom of borehole at 15.0 feet.

DATE PERFORMED 6/23/2021 **BORING NUMBER B-22**
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
▼ AT TIME OF DRILLING NE CHECKED BY KPF
▽ ESTIMATED SEASONAL HIGH 10.0 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Gray and brown SAND with silt
1.5		
2.5	AU 2	(SC) Gray and brown clayey SAND
5.0		
7.0		
7.5	AU 3 PS	(SC) Gray, brown, and orange clayey SAND *%PASS - 200 = 23* *MC = 19* *k_h = 1.1 ft/day*
10.0		
10.5		
12.5	AU 4	(SC/CL) Gray, brown, and orange very clayey SAND with traces of limestone
15.0		Bottom of borehole at 15.0 feet.

(Continued Next Page)

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CLIENT Tara April, LLC
PROJECT NAME Proposed Tara April
PROJECT NUMBER 15138
PROJECT LOCATION Alachua, Alachua County, Florida

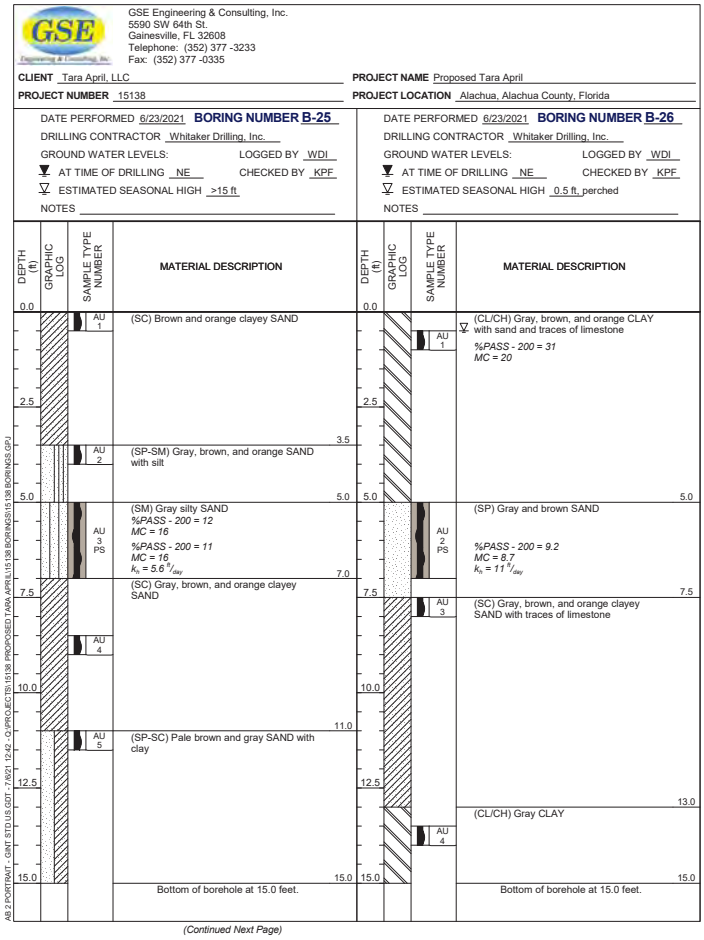
DATE PERFORMED 6/23/2021 **BORING NUMBER B-23**
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
▼ AT TIME OF DRILLING NE CHECKED BY KPF
▽ ESTIMATED SEASONAL HIGH >15 ft
NOTES

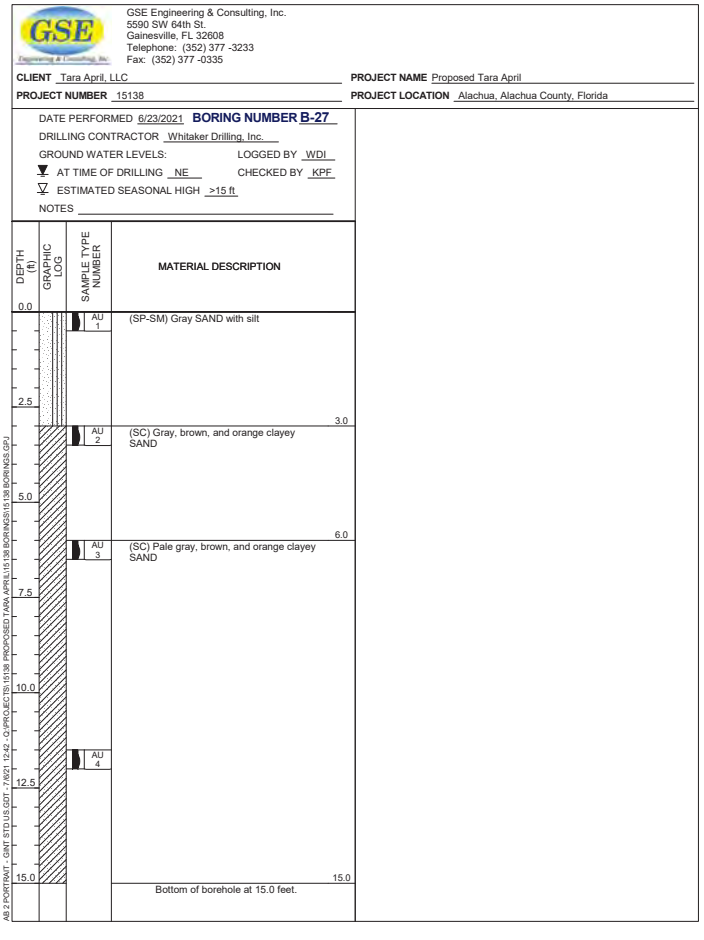
DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SP-SM) Gray and brown SAND with silt
1.0		
2.5	AU 2	(SC) Gray and brown clayey SAND
5.0		
7.5	AU 3	(SC) Gray, brown, and orange clayey SAND
10.0		
12.5	AU 4	
15.0		Bottom of borehole at 15.0 feet.

DATE PERFORMED 6/23/2021 **BORING NUMBER B-24**
DRILLING CONTRACTOR Whitaker Drilling, Inc.
GROUND WATER LEVELS: LOGGED BY WDI
▼ AT TIME OF DRILLING NE CHECKED BY KPF
▽ ESTIMATED SEASONAL HIGH 7.5 ft, perched
NOTES

DEPTH (ft)	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0	AU 1	(SM) Gray and brown silty SAND *%PASS - 200 = 12* *MC = 19*
1.0		
2.5	AU 2	(SC) Gray and brown clayey SAND
5.0		
7.5		
8.0	AU 3	(CL/CH) Gray, brown, and orange sandy CLAY
10.0		
10.0	AU 4	LIMESTONE
12.5		
15.0		Bottom of borehole at 15.0 feet.

(Continued Next Page)





(Continued Next Page)

5.2 Laboratory Results

SUMMARY REPORT OF LABORATORY TEST RESULTS

GSE Engineering & Consulting, Inc.

Project Number: 15138

Project Name: Proposed Tara April

Boring Number	Depth (ft)	Soil Description	Natural Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Percent Passing No. 200 Sieve	Organic Content (%)	Hydraulic Conductivity (ft/day)	Unified Soil Classification
B-2	2-4	Brown, gray, and orange clayey SAND	11				15		11	SC
B-4	12-14	Gray and brown SAND with clay	19				10		9.8	SP-SC
B-5	2-2.5	Gray and brown very clayey SAND	21				33			SC/CL
B-7	0.5-1	Brown and gray SAND with silt	6.0				6.4			SP-SM
B-7	12-14	Brown and orange clayey SAND	9.0				15		11	SC
B-9	7-9	Gray and brown SAND with clay	13				10		3.3	SP-SC
B-10	3-5	Gray and brown SAND with silt	8.9				6.4		18	SP-SM
B-11	3-5	Brown and gray SAND	7.1				4.9		15	SP
B-12	1.5-2	Gray, brown and orange very clayey SAND	23				41			SC/CL
B-13	3-5	Gray and brown SAND with silt	6.3				5.4		24	SP-SM
B-15	1-3	Brown and gray SAND with silt	11				5.4		20	SP-SM
B-16	2-2.5	Brown and orange very clayey SAND	26				45			SC/CL

5.3 Key to Soil Classification

SUMMARY REPORT OF LABORATORY TEST RESULTS

Project Number: 15138

Project Name: Proposed Tara April

Boring Number	Depth (ft)	Soil Description	Natural Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Percent Passing No. 200 Sieve	Organic Content (%)	Hydraulic Conductivity (ft/day)	Unified Soil Classification
B-17	2.5-3	Brown, gray, and orange very clayey SAND	17				32			SC/CL
B-20	0-2	Gray and brown SAND	11				4.8		13	SP
B-22	7-9	Gray, brown, and orange clayey SAND	19				23		1.1	SC
B-24	0.5-1	Gray and brown silty SAND	19				12			SM
B-25	5.5-6	Gray silty SAND	16				12			SM
B-25	5-7	Gray silty SAND	16				11		5.6	SM
B-26	0.5-1	Gray, brown and orange very clayey SAND with trace limestone	20				31			SC/CL
B-26	5-7	Gray and brown SAND with clay	8.7				9.2		11	SP-SC

KEY TO SOIL CLASSIFICATION CHART

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests				SYMBOLS GRAPHIC	SYMBOLS LETTER	GROUP NAME
COARSE-GRAINED SOILS	Gravels	Clean Gravels	Cu ≥ 4 and 1 ≤ Cc ≤ 3		GW	Well graded GRAVEL
More than 50% retained on No. 200 sieve	More than 50% of coarse fraction retained on No. 4 sieve	Less than 5% fines	Cu < 4 and/or 1 > Cc > 3		GP	Poorly graded GRAVEL
		Gravels with fines	Fines classify as ML or MH		GM	Silty GRAVEL
		More than 12% fines	Fines classify as CL or CH		GC	Clayey GRAVEL
	Sands	Clean Sands	Cu ≥ 6 and 1 ≤ Cc ≤ 3		SW	Well graded SAND
	50% or more of coarse fraction passes No. 4 sieve	Less than 5% fines	Cu < 6 and/or 1 > Cc > 3		SP	Poorly graded SAND
		Sand with fines	Fines classify as ML or MH		SP-SM	SAND with silt
		5% ≤ fines < 12%	Fines classify as CL or CH		SP-SC	SAND with clay
		Sand with fines	Fines classify as ML or MH		SM	Silty SAND
		12% ≤ fines < 30%	Fines classify as CL or CH		SC	Clayey SAND
		Sand with fines	Fines classify as ML or MH		SM	Very silty SAND
		30% fines or more	Fines classify as CL or CH		SC	Very clayey SAND
FINE-GRAINED SOILS	Clays	inorganic	50% ≤ fines < 70%		CL/CH	Sandy CLAY
50% or more passes the No. 200 sieve			70% ≤ fines < 85%		CL/CH	CLAY with sand
			fines ≥ 85%		CL/CH	CLAY
	Silts and Clays	inorganic	PI > 7 and plots on/above "A" line		CL	Lean CLAY
	Liquid Limit less than 50		PI < 4 or plots below "A" line		ML	SILT
		organic	Liquid Limit - oven dried / Liquid Limit - not dried < 0.75		OL	Organic clay / Organic silt
	Silts and Clays	inorganic	PI plots on or above "A" line		CH	Fat CLAY
	Liquid Limit 50 or more		PI plots below "A" line		MH	Elastic SILT
		organic	Liquid Limit - oven dried / Liquid Limit - not dried < 0.75		OH	Organic clay / Organic silt
HIGHLY ORGANIC SOILS	Primarily organic matter, dark in color, and organic odor				PT	PEAT

CORRELATION OF PENETRATION RESISTANCE WITH RELATIVE DENSITY AND CONSISTENCY

	No. OF BLOWS, N	RELATIVE DENSITY
SANDS:	0 - 4	Very Loose
	5 - 10	Loose
	11 - 30	Medium dense
	31 - 50	Dense
	OVER 50	Very Dense

	No. OF BLOWS, N	RELATIVE DENSITY
LIMESTONE:	0 - 8	Very Soft
	9 - 18	Soft
	19 - 32	Moderately Hard
	33 - 50	Hard
	OVER 50	Very Hard

	No. OF BLOWS, N	CONSISTENCY
SILTS & CLAYS:	0 - 2	Very Soft
	3 - 4	Soft
	5 - 8	Firm
	9 - 15	Stiff
	16 - 30	Very Stiff
	31 - 50	Hard
	OVER 50	Very Hard

SAMPLE GRAPHIC TYPE LEGEND

SPT 1 — Location of SPT Sample

AU 1 — Location of Auger Sample

PARTICLE SIZE IDENTIFICATION

BOULDERS:		Greater than 300 mm
COBBLES:		75 mm to 300 mm
GRAVEL:	Coarse -	19.0 mm to 75 mm
	Fine -	4.75 mm to 19.0 mm
SANDS:	Coarse -	2.00 mm to 4.75 mm
	Medium -	0.425 mm to 2.00 mm
	Fine -	0.075 mm to 0.425 mm
SILTS & CLAYS:		Less than 0.075 mm

LABORATORY TEST LEGEND

LL	=	Liquid Limit, %
PL	=	Plastic Limit, %
PI	=	Plasticity Index, %
% PASS - 200	=	Percent Passing the No. 200 Sieve
MC	=	Moisture Content, %
ORG	=	Organic Content, %
k_h	=	Horizontal Hydraulic Conductivity, ft/day

6.0 LIMITATIONS

6.1 Warranty

This report has been prepared for our client for his exclusive use, in accordance with generally accepted soil and foundation engineering practices, and makes no other warranty either expressed or implied as to the professional advice provided in the report.

6.2 Auger Borings

The determination of soil type and conditions was performed from the ground surface to the maximum depth of the borings, only. Any changes in subsurface conditions that occur between or below the borings would not have been detected or reflected in this report.

Soil classifications that were made in the field are based upon identifiable textural changes, color changes, changes in composition or changes in resistance to penetration in the intervals from which the samples were collected. Abrupt changes in soil type, as reflected in boring logs and/or cross sections may not actually occur, but instead, be transitional.

Depth to the water table is based upon observations made during the performance of the auger borings. This depth is an estimate and does not reflect the annual variations that would be expected in this area due to fluctuations in rainfall and rates of evapotranspiration.

6.3 Site Figures

The measurements used for the preparation of the figures in this report were made using the provided site plan and by estimating distances from existing structures and site features. Figures in this report were not prepared by a licensed land surveyor and should not be interpreted as such.

6.4 Unanticipated Soil Conditions

The analysis and recommendations submitted in this report are based upon the data obtained from soil borings performed at the locations indicated on Figure 2. This report does not reflect any variations that may occur between these borings.

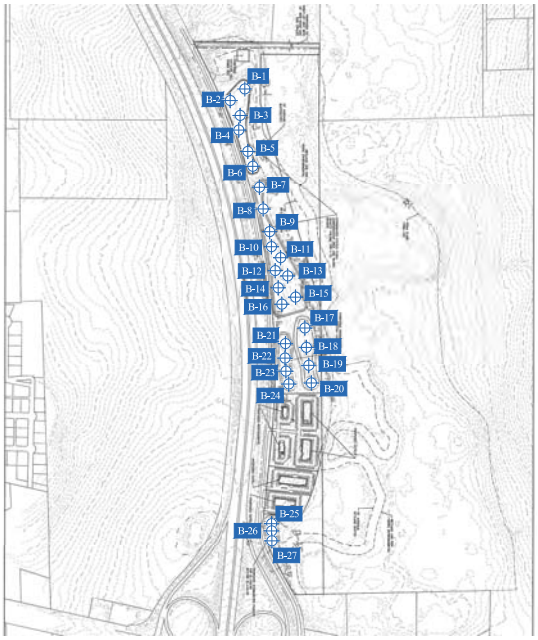
The nature and extent of variations between borings may not become known until excavation begins. If variations appear, we may have to re-evaluate our recommendations after performing on-site observations and noting the characteristics of any variations.

6.5 Misinterpretation of Soil Engineering Report

GSE Engineering & Consulting, Inc. is responsible for the conclusions and opinions contained within this report based upon the data relating only to the specific project and location discussed herein. If others make the conclusions or recommendations based upon the data presented, those conclusions or recommendations are not the responsibility of GSE.

FIGURES

PROPOSED TARA APRIL ALACHUA, ALACHUA COUNTY, FLORIDA GSE PROJECT NO. 15138	PROJECT SITE LOCATION MAP		
	DESIGNED BY: KPF CHECKED BY : KLH DRAWN BY : SHS	GSE Engineering & Consulting, Inc.	FIGURE 1



LEGEND:
AUGER BORING

NORTH
0 748 5/8'
SCALE: 1" = 748 5/8' APPROX.

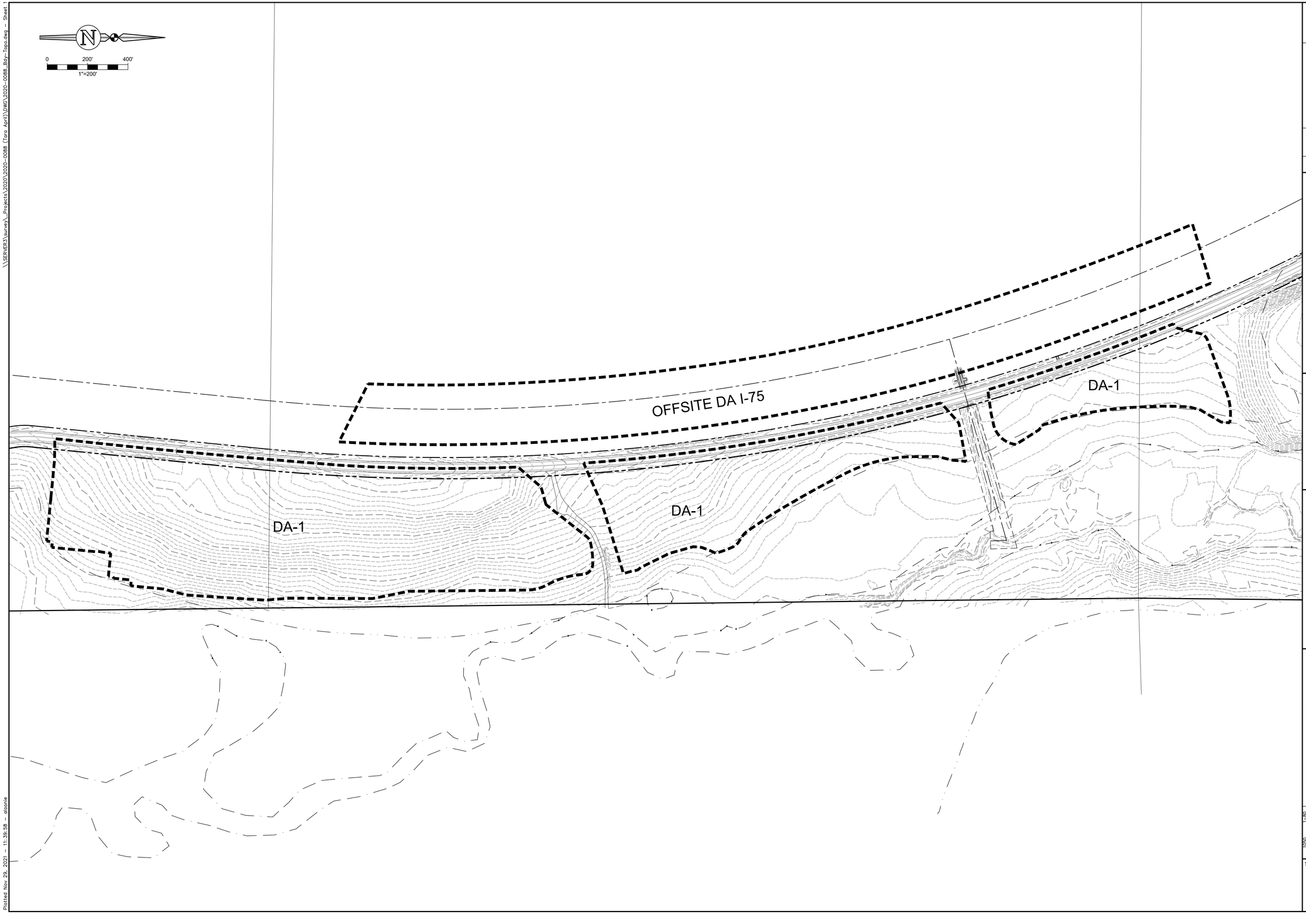
PROPOSED TARA APRIL ALACHUA, ALACHUA COUNTY, FLORIDA GSE PROJECT NO. 15138	SITE PLAN SHOWING APPROXIMATE LOCATIONS OF FIELD TESTS		
	DESIGNED BY: KPF CHECKED BY : KLH DRAWN BY : SHS	GSE Engineering & Consulting, Inc.	FIGURE 2

Attachment B

Aerial Photograph

Attachment C

Pre- and Post-Development Drainage Maps



SHEET 1 OF 11

No.	Date	Comment
1	11/29/2021	ADDED WETLANDS INFORMATION

FLOOD INSURANCE RATE MAP STATEMENT

THE REAL PROPERTY SHOWN HEREON LIES WITHIN ZONE(s) A & X (UNSHADED) AS DESIGNATED ON THE FLOOD INSURANCE RATE MAP NUMBER 12086C 0067 K COMMUNITY NUMBER: 120064 PANEL: 0067 SUFFIX: K EFFECTIVE DATE: 09/26/2008 SAID MAP DESCRIBES ZONE(s) AS BEING "SPECIAL FLOOD HAZARD AREAS (SHA) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD, NO BASE FLOOD ELEVATIONS DETERMINED" AND ZONE "X (UNSHADED) AS BEING "AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN."

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Project No. 2020-0088
Drawn A.L.
Check B.G.

eda consultants inc.
Corporate Authorization No. LB 2389
BY: ROBERT M. CRAVER P.S.M. 4239

NOT VALID WITHOUT THE ORIGINAL SIGNATURE AND THE RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

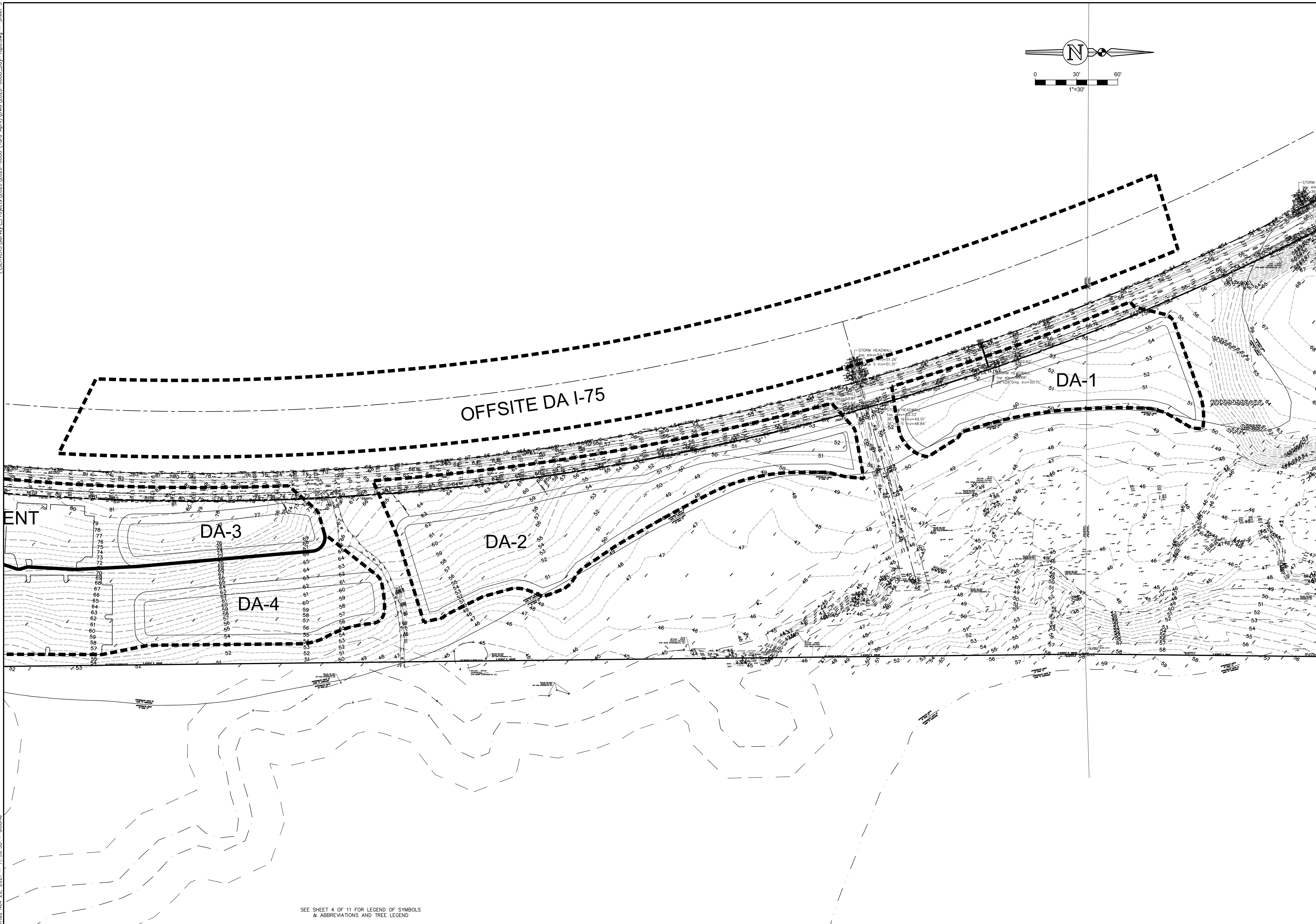
Fieldbook	Page	Survey Date	Drawing Completed	Revised
1088 1089	1-80	11/18/2021	11/18/2021	

PREPARED FOR: 1) TARA FOREST, LLC
2)
3)
4)

THIS SURVEY MEETS THE STANDARDS OF PRACTICE AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.
THIS SURVEY DEPICTS THE SITE CONDITIONS AS OF 11/18/2021

Sheet No.:

PRE



SEE SHEET 4 OF 11 FOR LEGEND OF SYMBOLS & ABBREVIATIONS AND TREE LEGEND

No.	Date	Comment
1	11/29/2021	ADDED WETLANDS INFORMATION

FLOOD INSURANCE RATE MAP STATEMENT

THE REAL PROPERTY SHOWN HEREON LIES WITHIN ZONE(s) A & X (UNSHADED) AS DESIGNATED ON THE FLOOD INSURANCE RATE MAP NUMBER 12001C0110D & 12001C0120D ; COMMUNITY NUMBER: 120664 ; PANEL: 0110D & 0120D ; EFFECTIVE DATE: 06/16/06 . SAID MAP DESCRIBES ZONE(s) 'A' AS BEING "SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD, NO BASE FLOOD ELEVATIONS DETERMINED" AND ZONE 'X' (UNSHADED) AS BEING "AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN."

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Project No. 2020-0088
Drawn A.L.
Check B.G.

eda consultants inc.
Corporate Authorization No. LB 2389
BY: ROBERT W. GRAVER P.S.M. 4239
SEE SHEET 1 OF 11 FOR ORIGINAL SIGNATURE AND RAISED SEAL

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Fieldbook	Page	Survey Date	Drawing Completed	Revised
1050 1059	1-80 1-20	11/18/2021	11/18/2021	

PREPARED FOR: 1) TARA FOREST, LLC
2)
3)
4)

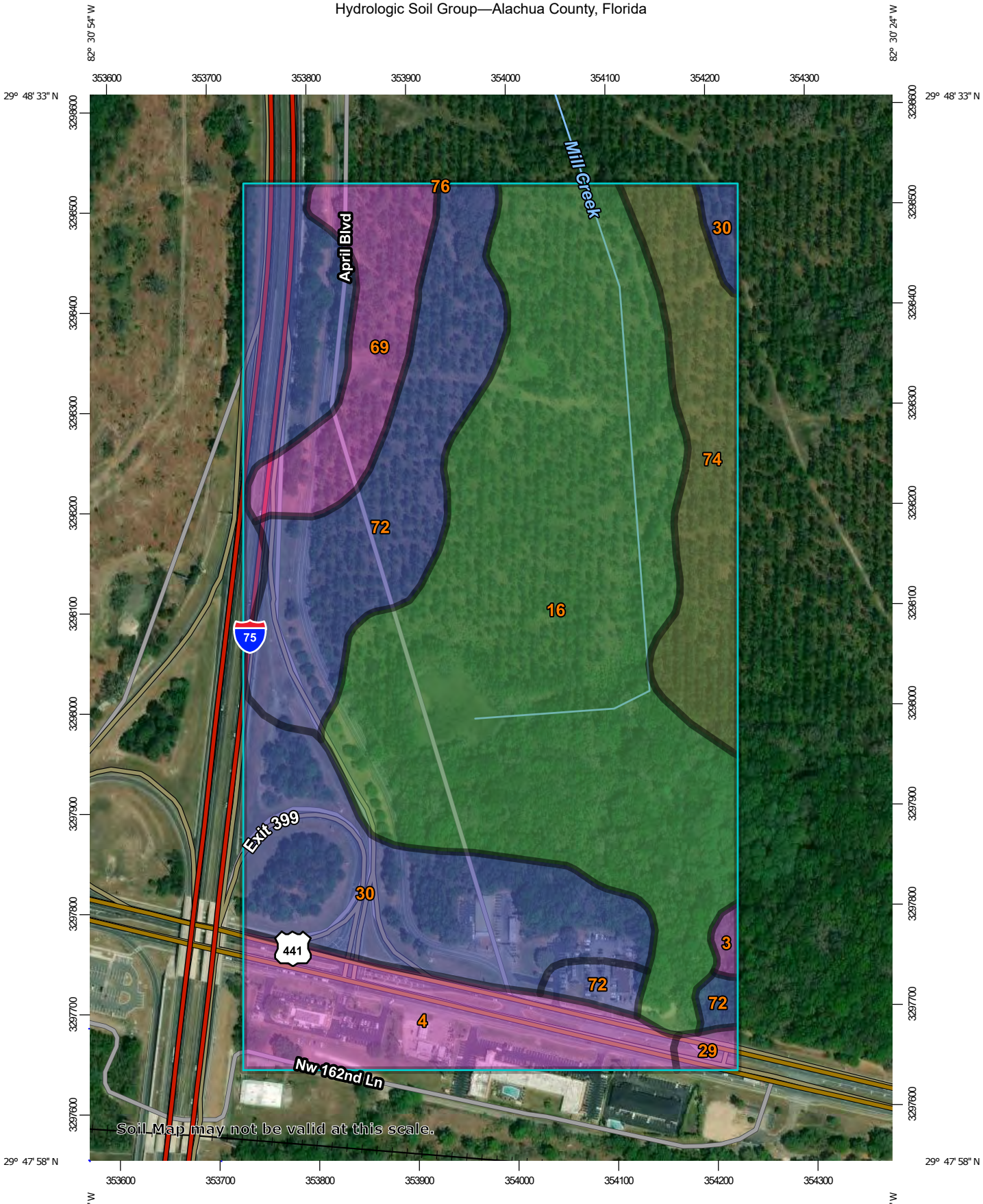
THIS SURVEY MEETS THE STANDARDS OF PRACTICE AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027 (2021), FLORIDA STATUTES. THIS SURVEY DEPICTS THE SITE CONDITIONS AS OF 11/18/2021

Sheet No.: POST-1

Attachment D

Soils Map

Hydrologic Soil Group—Alachua County, Florida



Map Scale: 1:5,190 if printed on A portrait (8.5" x 11") sheet.

N

Meters: 0 50 100 200 300

Feet: 0 250 500 1000 1500

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 17N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15,800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Alachua County, Florida
Survey Area Data: Version 22, Aug 31, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Nov 26, 2014—Dec 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
3	Arredondo fine sand, 0 to 5 percent slopes	A	0.4	0.4%
4	Arredondo-Urban land complex, 0 to 5 percent slopes	A	9.8	9.0%
16	Surrency sand	A/D	46.0	42.3%
29	Lochloosa fine sand, 2 to 5 percent slopes	A	0.6	0.6%
30	Kendrick sand, 2 to 5 percent slopes	B	21.6	19.8%
69	Arredondo fine sand, 5 to 8 percent slopes	A	6.8	6.2%
72	Lochloosa fine sand, 5 to 8 percent slopes	B	14.6	13.4%
74	Blichton sand, 2 to 5 percent slopes	C/D	9.1	8.3%
76	Bivans sand, 5 to 8 percent slopes	C/D	0.0	0.0%
Totals for Area of Interest			**108.9**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Attachment E

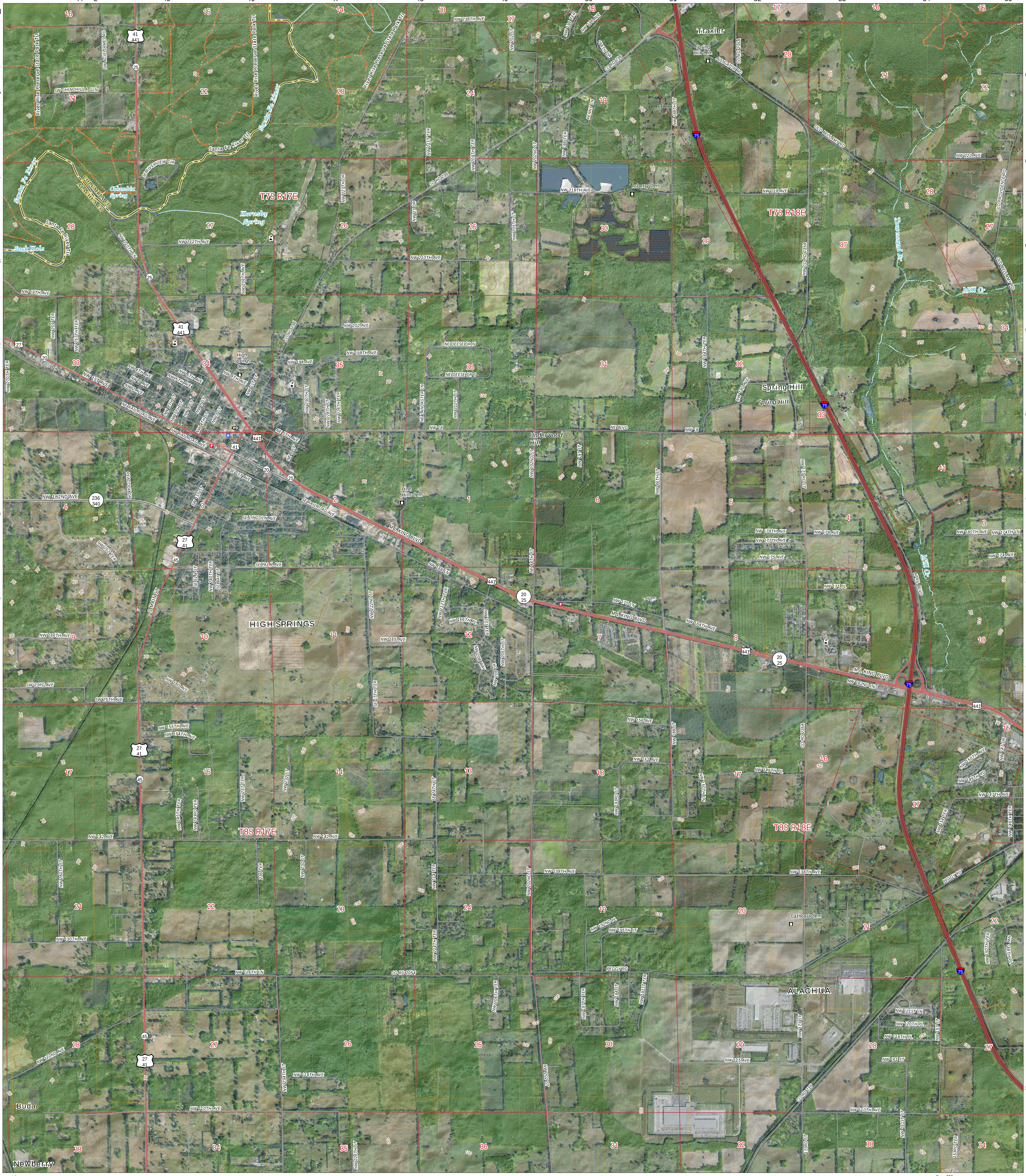
Quad Map

USGS
science for a changing world

U.S. DEPARTMENT OF THE INTERIOR
U.S. GEOLOGICAL SURVEY

The National Map
US Topo

HIGH SPRINGS QUADRANGLE
FLORIDA
7.5-MINUTE SERIES



Produced by the United States Geological Survey
North American Datum of 1983 (NAD83)
World Geodetic System of 1984 (WGS84). Projection and
1 000-meter grid:Universal Transverse Mercator, Zone 17R

This map is not a legal document. Boundaries may be generalized for this map scale. Private lands within government reservations may not be shown. Obtain permission before entering private lands.

Imagery....................NAIP, November 2019
Roads....................U.S. Census Bureau, 2016
Names....................GNIS, 1979 - 2020
Hydrography....................National Hydrography Dataset, 1899 - 2020
Contours....................National Elevation Dataset, 2018
Boundaries....................Multiple sources; see metadata file 2018 - 2019
Public Land Survey System....................BLM, 2020
Wetlands....................FWS National Wetlands Inventory 1983 - 1984

MN
GN
5°51´
104 MILS
0°47´
14 MILS

UTM GRID AND 2019 MAGNETIC NORTH DECLINATION AT CENTER OF SHEET

U.S. National Grid
100,000 - m Square ID
LP
LN
Grid Zone Designation
17R

SCALE 1:24 000

KILOMETERS
METERS
MILES
FEET

CONTOUR INTERVAL 5 FEET
NORTH AMERICAN VERTICAL DATUM OF 1988

This map was produced to conform with the National Geospatial Program US Topo Product Standard.

FLORIDA
QUADRANGLE LOCATION

ADJOINING QUADRANGLES
1 Fort White
2 Mikesville
3 Worthington Springs
4 High Springs SW
5 Alachua
6 Waters Lake
7 Newberry
8 Gainesville West

ROAD CLASSIFICATION
Expressway
Secondary Hwy
Ramp
Local Connector
Local Road
4WD
Interstate Route
US Route
State Route

HIGH SPRINGS, FL
2021

NSN. 7643016360641
NGA REF NO. USGSX24K20382

Attachment F

FEMA Map

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles, Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations tables in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations tables should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures.** Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures in this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 17. The **horizontal datum** was NAD 83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at www.ngs.noaa.gov or contact the National Geodetic Survey at the following address:

Spatial Reference System Division
National Geodetic Survey, NOAA
Silver Spring Metro Center
1315 East-West Highway
Silver Spring, Maryland 20910
(301) 713-3191

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at **(301) 713-3242**, or visit their website at www.ngs.noaa.gov.

Base map information shown on this FIRM was derived from U.S. Geological Survey Digital Orthophoto Quadrangles (DOQs) produced at a scale of 1:12,000 from photography dated 1994 or later.

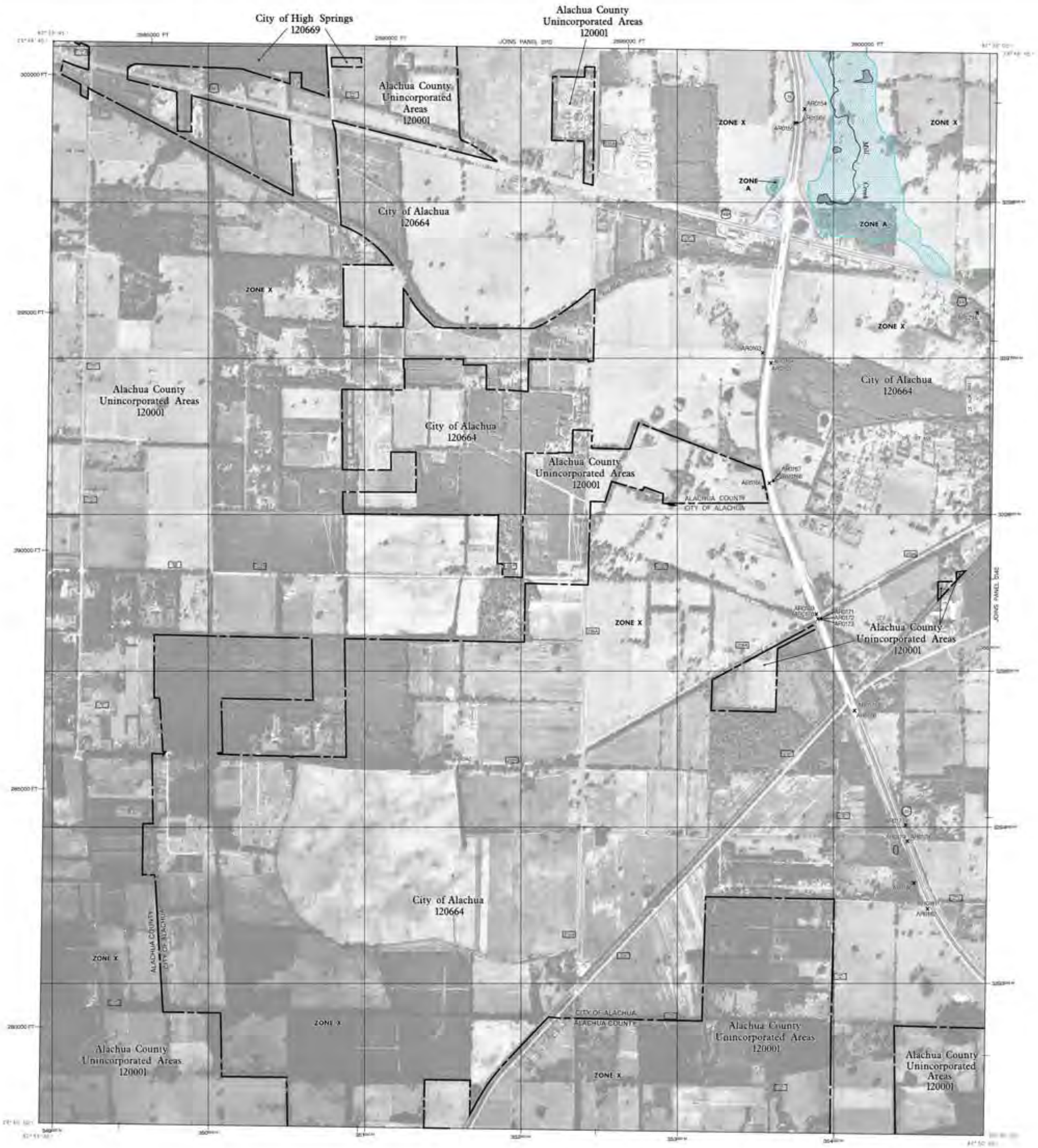
This map reflects more detailed and up-to-date **stream channel configurations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the **FEMA Map Service Center** at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and their website at www.fema.gov/msc.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at www.fema.gov.



LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D	Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

1% annual chance floodplain boundary
0.2% annual chance floodplain boundary
Floodway boundary
Zone D boundary
CBRS and OPA boundary
Boundary dividing Special Flood Hazard Areas Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
513 — Base Flood Elevation line and value; elevation in feet*
(EL 987) — Base Flood Elevation value where uniform within zone; elevation in feet*

*Referenced to the North American Vertical Datum of 1988

(A)—(A) Cross section line
(23)- - -(23) Transect line
97°07'30", 32°22'30" — Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
4275000mN — 1000-meter Universal Transverse Mercator grid values, zone 17
600000 FT — 5000-foot grid ticks: Florida State Plane coordinate system, north zone (FIPSZONE 903), Lambert Conformal Conic projection
DX5510 × — Bench mark (see explanation in Notes to Users section of this FIRM panel)
• M1.5 — River Mile

MAP REPOSITORY
Refer to listing of Map Repositories on Map Index

EFFECTIVE DATE OF COUNTYWIDE
FLOOD INSURANCE RATE MAP
JUNE 16, 2006

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 1000'

500 0 1000 2000 FEET

300 0 300 600 METERS

NFIP

NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0120D

FIRM

FLOOD INSURANCE RATE MAP

ALACHUA COUNTY, FLORIDA AND INCORPORATED AREAS

PANEL 120 OF 640

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
ALACHUA, CITY OF	120664	0120	D
ALACHUA COUNTY	120001	0120	D
HIGH SPRINGS, CITY OF	120669	0120	D

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
12001C0120D

EFFECTIVE DATE
JUNE 16, 2006

Federal Emergency Management Agency

NOTES TO USERS

LEGEND

FIRM

FLOOD INSURANCE RATE MAP

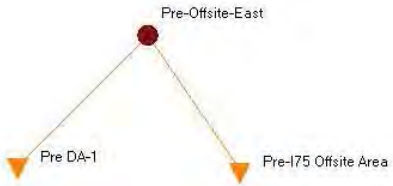
ALACHUA COUNTY, FLORIDA

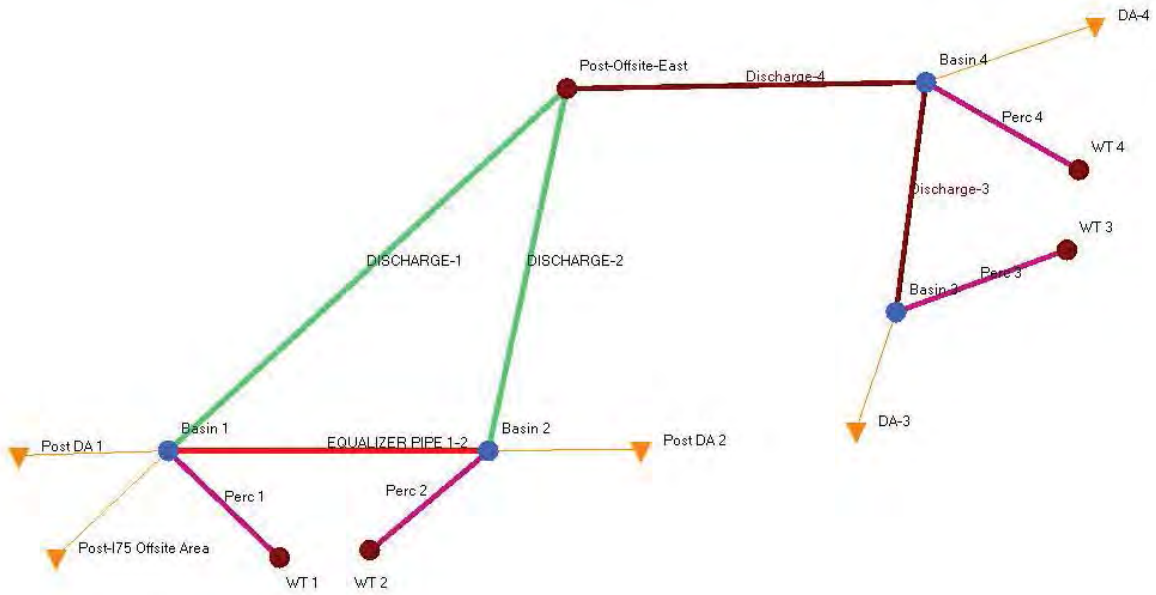
AND INCORPORATED AREAS

PANEL 95 OF 425

Attachment G

Pre- and Post-Development ICPR Model


Pre-Offsite-East
Pre DA-1
Pre-I75 Offsite Area


DA-4
Post-Offsite-East
Discharge-4
Basin 4
Perc 4
WT 4
Discharge-3
WT 3
DISCHARGE-1
DISCHARGE-2
Basin 3
Perc 3
DA-3
Basin 1
Post DA 1
EQUALIZER PIPE 1-2
Basin 2
Post DA 2
Perc 1
Perc 2
Post-I75 Offsite Area
WT 1
WT 2

Simple Basin: DA-3

Scenario:	Phase 1
Node:	Basin 3
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	10.0000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH484
Peaking Factor:	484.0
Area:	6.6600 ac
Curve Number:	86.9
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: DA-4

Scenario:	Phase 1
Node:	Basin 4
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	10.0000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH484
Peaking Factor:	484.0
Area:	8.0200 ac
Curve Number:	91.5
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: Post DA 1

Scenario:	Phase 1
Node:	Basin 1
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	10.0000 min
Max Allowable Q:	0.00 cfs

Time Shift:	0.0000 hr
Unit Hydrograph:	UH484
Peaking Factor:	484.0
Area:	3.2900 ac
Curve Number:	54.3
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: Post DA 2

Scenario:	Phase 1
Node:	Basin 2
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	10.0000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH484
Peaking Factor:	484.0
Area:	5.8500 ac
Curve Number:	54.6
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: Post-I75 Offsite Area

Scenario:	Phase 1
Node:	Basin 1
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	10.0000 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH484
Peaking Factor:	484.0
Area:	11.7400 ac
Curve Number:	96.2
% Impervious:	0.00
% DCIA:	0.00

% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: Pre DA-1

Scenario:	Phase 1
Node:	Pre-Offsite-East
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	13.8500 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH484
Peaking Factor:	484.0
Area:	23.8200 ac
Curve Number:	53.5
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Simple Basin: Pre-I75 Offsite Area

Scenario:	Phase 1
Node:	Pre-Offsite-East
Hydrograph Method:	NRCS Unit Hydrograph
Infiltration Method:	Curve Number
Time of Concentration:	23.8500 min
Max Allowable Q:	0.00 cfs
Time Shift:	0.0000 hr
Unit Hydrograph:	UH484
Peaking Factor:	484.0
Area:	11.7400 ac
Curve Number:	96.2
% Impervious:	0.00
% DCIA:	0.00
% Direct:	0.00
Rainfall Name:	

Comment:

Node: Basin 1

Scenario: Phase 1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 46.70 ft
Warning Stage: 50.00 ft

Stage [ft]	Area [ac]	Area [ft2]
46.00	1.8100	78844
47.00	1.9700	85813
48.00	2.1300	92783
49.00	2.3000	100188
50.00	2.4600	107158

Comment:

Node: Basin 2

Scenario: Phase 1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 46.70 ft
Warning Stage: 50.00 ft

Stage [ft]	Area [ac]	Area [ft2]
46.00	3.0900	134600
47.00	3.3300	145055
48.00	3.5700	155509
49.00	3.8100	165964
50.00	4.0500	176418

Comment:

Node: Basin 3

Scenario: Phase 1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 70.00 ft
Warning Stage: 74.00 ft

Stage [ft]	Area [ac]	Area [ft2]
70.00	0.8300	36155
71.00	0.9200	40075
72.00	1.0200	44431
73.00	1.1200	48787
74.00	1.2300	53579

Stage [ft]	Area [ac]	Area [ft2]
75.00	1.3300	57935

Comment:

Node: Basin 4

Scenario: Phase 1
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 54.00 ft
Warning Stage: 59.00 ft

Stage [ft]	Area [ac]	Area [ft2]
54.00	1.2000	52272
55.00	1.3700	59677
56.00	1.4300	62291
57.00	1.5600	67954
58.00	1.6800	73181
59.00	1.8100	78844
60.00	1.9300	84071

Comment:

Node: Post-Offsite-East

Scenario: Phase 1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: Pre-Offsite-East

Scenario: Phase 1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: WT 1

Scenario: Phase 1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: WT 2

Scenario: Phase 1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: WT 3

Scenario: Phase 1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: WT 4

Scenario: Phase 1
Type: Time/Stage
Base Flow: 0.00 cfs

Initial Stage:	0.00 ft
Warning Stage:	0.00 ft
Boundary Stage:	

Comment:

Drop Structure Link:		Upstream Pipe		Downstream Pipe	
Scenario:	Phase 1	Invert:	0.00 ft	Invert:	0.00 ft
From Node:		Manning's N:	0.0000	Manning's N:	0.0000
To Node:		Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	0.00 ft	Max Depth:	0.00 ft
Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	0.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.00	Op Table:		Op Table:	
Exit Loss Coef:	0.00	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Drop Structure Comment:

Weir Link: DISCHARGE-1			
Scenario:	Phase 1	Bottom Clip	
From Node:	Basin 1	Default:	0.00 ft
To Node:	Post-Offsite-East	Op Table:	
Link Count:	1	Ref Node:	
Flow Direction:	Both	Top Clip	
Damping:	0.0000 ft	Default:	0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:	
Geometry Type:	Rectangular	Ref Node:	
Invert:	50.00 ft	Discharge Coefficients	
Control Elevation:	50.00 ft	Weir Default:	2.800
Max Depth:	99999.00 ft	Weir Table:	
Max Width:	604.00 ft	Orifice Default:	0.600
Fillet:	0.00 ft	Orifice Table:	

Comment:

Weir Link: DISCHARGE-2			
Scenario:	Phase 1	Bottom Clip	
From Node:	Basin 2	Default:	0.00 ft
To Node:	Post-Offsite-East	Op Table:	
Link Count:	1	Ref Node:	
Flow Direction:	Both	Top Clip	
Damping:	0.0000 ft	Default:	0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:	
Geometry Type:	Rectangular	Ref Node:	
Invert:	50.00 ft	Discharge Coefficients	
Control Elevation:	50.00 ft	Weir Default:	2.800
Max Depth:	99999.00 ft	Weir Table:	
Max Width:	1172.00 ft	Orifice Default:	0.600
Fillet:	0.00 ft	Orifice Table:	
Comment:			

Drop Structure Link: Discharge-3		Upstream Pipe		Downstream Pipe	
Scenario:	Phase 1	Invert:	70.00 ft	Invert:	54.00 ft
From Node:	Basin 3	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	Basin 4	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	1.50 ft	Max Depth:	1.50 ft
Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	25.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.00	Op Table:		Op Table:	
Exit Loss Coef:	0.00	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				
Pipe Comment:					

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	72.50 ft	Op Table:	
Control Elevation:	72.50 ft	Ref Node:	
Max Depth:	18.00 ft	Discharge Coefficients	
Max Width:	0.50 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	

Orifice Default:	0.600
Orifice Table:	

Weir Comment:

Drop Structure Comment:

Drop Structure Link: Discharge-4		Upstream Pipe		Downstream Pipe	
Scenario:	Phase 1	Invert:	54.00 ft	Invert:	52.00 ft
From Node:	Basin 4	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	Post-Offsite-East	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	1.50 ft	Max Depth:	1.50 ft
Flow Direction:	Both	Bottom Clip			
Solution:	Combine	Default:	0.00 ft	Default:	0.00 ft
Increments:	0	Op Table:		Op Table:	
Pipe Count:	1	Ref Node:		Ref Node:	
Damping:	0.0000 ft	Manning's N:	0.0000	Manning's N:	0.0000
Length:	25.00 ft	Top Clip			
FHWA Code:	0	Default:	0.00 ft	Default:	0.00 ft
Entr Loss Coef:	0.00	Op Table:		Op Table:	
Exit Loss Coef:	0.00	Ref Node:		Ref Node:	
Bend Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Bend Location:	0.00 dec				
Energy Switch:	Energy				

Pipe Comment:

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	1	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Sharp Crested Vertical	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	57.00 ft	Op Table:	
Control Elevation:	57.00 ft	Ref Node:	
Max Depth:	2.50 ft	Discharge Coefficients	
Max Width:	0.50 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	

Weir Comment:

Drop Structure Comment:

Pipe Link: EQUALIZER PIPE 1-2	Upstream	Downstream

Scenario:	Phase 1	Invert:	45.00 ft	Invert:	45.00 ft
From Node:	Basin 1	Manning's N:	0.0120	Manning's N:	0.0120
To Node:	Basin 2	Geometry: Circular		Geometry: Circular	
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			
Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	147.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0.00	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	0.00	Top Clip			
Bend Loss Coef:	0.00	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Percolation Link: Perc 1

Scenario:	Phase 1	Surface Area Option:	Vary Based on Stage/Area Table
From Node:	Basin 1		
To Node:	WT 1	Vertical Flow Termination:	Horizontal Flow Algorithm
Link Count:	1	Perimeter 1:	1831.00 ft
Flow Direction:	Both	Perimeter 2:	1897.00 ft
Aquifer Base Elevation:	38.00 ft	Perimeter 3:	2205.00 ft
Water Table Elevation:	38.50 ft	Distance P1 to P2:	10.00 ft
Annual Recharge Rate:	0 ipy	Distance P2 to P3:	50.00 ft
Horizontal Conductivity:	5.250 fpd	# of Cells P1 to P2:	2
Vertical Conductivity:	3.500 fpd	# of Cells P2 to P3:	5
Fillable Porosity:	0.200		
Layer Thickness:	7.50 ft		

Comment:

Percolation Link: Perc 2

Scenario:	Phase 1	Surface Area Option:	Vary Based on Stage/Area Table
From Node:	Basin 2		
To Node:	WT 2	Vertical Flow Termination:	Horizontal Flow Algorithm
Link Count:	1	Perimeter 1:	2685.00 ft
Flow Direction:	Both	Perimeter 2:	2743.00 ft
Aquifer Base Elevation:	42.50 ft	Perimeter 3:	3060.00 ft
Water Table Elevation:	43.00 ft	Distance P1 to P2:	10.00 ft
Annual Recharge Rate:	0 ipy	Distance P2 to P3:	50.00 ft
Horizontal Conductivity:	7.250 fpd	# of Cells P1 to P2:	2
Vertical Conductivity:	4.750 fpd	# of Cells P2 to P3:	10
Fillable Porosity:	0.200		
Layer Thickness:	3.00 ft		

Comment:

Percolation Link: Perc 3

Scenario:	Phase 1	Surface Area Option:	Vary Based on Stage/Area Table
From Node:	Basin 3		
To Node:	WT 3	Vertical Flow Termination:	Horizontal Flow Algorithm
Link Count:	1	Perimeter 1:	1165.00 ft
Flow Direction:	Both	Perimeter 2:	1228.00 ft
Aquifer Base Elevation:	61.00 ft	Perimeter 3:	1542.00 ft
Water Table Elevation:	63.00 ft	Distance P1 to P2:	10.00 ft
Annual Recharge Rate:	0 ipy	Distance P2 to P3:	50.00 ft
Horizontal Conductivity:	2.250 fpd	# of Cells P1 to P2:	2
Vertical Conductivity:	1.500 fpd	# of Cells P2 to P3:	5
Fillable Porosity:	0.200		
Layer Thickness:	7.00 ft		

Comment:

Percolation Link: Perc 4

Scenario:	Phase 1	Surface Area Option:	Vary Based on Stage/Area Table
From Node:	Basin 4		
To Node:	WT 4	Vertical Flow Termination:	Horizontal Flow Algorithm
Link Count:	1	Perimeter 1:	1412.00 ft
Flow Direction:	Both	Perimeter 2:	1475.00 ft
Aquifer Base Elevation:	52.00 ft	Perimeter 3:	1789.00 ft
Water Table Elevation:	52.50 ft	Distance P1 to P2:	10.00 ft
Annual Recharge Rate:	0 ipy	Distance P2 to P3:	50.00 ft
Horizontal Conductivity:	6.000 fpd	# of Cells P1 to P2:	2
Vertical Conductivity:	4.000 fpd	# of Cells P2 to P3:	5
Fillable Porosity:	0.200		
Layer Thickness:	1.50 ft		

Comment:

Simulation: 100 yr - 001 hr

Scenario:	Phase 1
Run Date/Time:	1/31/2022 3:26:58 PM
Program Version:	ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	73.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000

Max Calculation Time: 30.0000

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft
Edge Length Option: Automatic

IA Recovery Time: 24.0000 hr
ET for Manual Basins: False
Smp/Man Basin Rain Opt: Global
OF Region Rain Opt: Global
Rainfall Name: ~FDOT-1
Rainfall Amount: 4.40 in
Storm Duration: 1.0000 hr

Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 100 yr - 002 hr

Scenario: Phase 1
Run Date/Time: 1/31/2022 3:27:14 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	74.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources		Lookup Tables	
Rainfall Folder:		Boundary Stage Set:	
Reference ET Folder:		Extern Hydrograph Set:	
Unit Hydrograph Folder:		Curve Number Set:	
		Green-Ampt Set:	
		Vertical Layers Set:	
		Impervious Set:	
		Roughness Set:	
		Crop Coef Set:	
		Fillable Porosity Set:	
		Conductivity Set:	
		Leakage Set:	

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~FDOT-2
		Rainfall Amount:	5.40 in
Edge Length Option:	Automatic	Storm Duration:	2.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 100 yr - 004 hr

Scenario:	Phase 1
Run Date/Time:	1/31/2022 3:27:26 PM
Program Version:	ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	76.0000

Hydrology [sec]	Surface Hydraulics	Groundwater [sec]

	[sec]		
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~FDOT-4

		Rainfall Amount:	6.72 in
Edge Length Option:	Automatic	Storm Duration:	4.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 100 yr - 008 hr

Scenario: Phase 1
Run Date/Time: 1/31/2022 3:27:40 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	80.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:	
Reference ET Folder:	
Unit Hydrograph Folder:	

Lookup Tables

Boundary Stage Set:	
Extern Hydrograph Set:	
Curve Number Set:	
Green-Ampt Set:	
Vertical Layers Set:	
Impervious Set:	
Roughness Set:	
Crop Coef Set:	
Fillable Porosity Set:	
Conductivity Set:	
Leakage Set:	

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~FDOT-8
		Rainfall Amount:	8.00 in
Edge Length Option:	Automatic	Storm Duration:	8.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 100 yr - 024 hr

Scenario:	Phase 1
Run Date/Time:	1/31/2022 3:27:55 PM
Program Version:	ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	500.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
dZ Tolerance: 0.0010 ft
IA Recovery Time: 24.0000 hr
ET for Manual Basins: False
Smp/Man Basin Rain Opt: Global

Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~FDOT-24
		Rainfall Amount:	11.04 in
Edge Length Option:	Automatic	Storm Duration:	24.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 100 yr - 072 hr

Scenario: Phase 1
Run Date/Time: 1/31/2022 3:29:29 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	144.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~FDOT-72
		Rainfall Amount:	13.80 in
Edge Length Option:	Automatic	Storm Duration:	72.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 100 yr - 168 hr

Scenario: Phase 1
Run Date/Time: 1/31/2022 3:30:14 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

Year	Month	Day	Hour [hr]

Start Time:	0	0	0	0.0000
End Time:	0	0	0	240.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight Fact: 0.5 dec
IA Recovery Time: 24.0000 hr
ET for Manual Basins: False

dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~FDOT-168
		Rainfall Amount:	16.00 in
Edge Length Option:	Automatic	Storm Duration:	168.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 100 yr - 240 hr

Scenario: Phase 1
Run Date/Time: 1/31/2022 3:31:08 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	312.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~FDOT-240
		Rainfall Amount:	18.00 in
Edge Length Option:	Automatic	Storm Duration:	240.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: WQTV

Scenario: Phase 1
Run Date/Time: 1/31/2022 2:32:30 PM
Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	72.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set:
Green-Ampt Set:
Vertical Layers Set:
Impervious Set:
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
IA Recovery Time: 24.0000 hr
ET for Manual Basins: False

Over-Relax Weight Fact:	0.5 dec		
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain Opt:	Global
Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	
		Rainfall Amount:	0.00 in
Edge Length Option:	Automatic	Storm Duration:	0.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area (2D):	100 ft2	Min Node Srf Area (1D):	100 ft2
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Sim	Node Name	Maximum Stage [ft]	Maximum Total Inflow Rate [cfs]
100 yr - 001 hr	Basin 1	48.05	105.05
100 yr - 001 hr	Basin 2	47.10	31.51
100 yr - 001 hr	Basin 3	71.77	47.05
100 yr - 001 hr	Basin 4	55.59	63.33
100 yr - 001 hr	Post-Offsite-East	0.00	0.00
100 yr - 001 hr	Pre-Offsite-East	0.00	119.12
100 yr - 002 hr	Basin 1	47.95	79.31
100 yr - 002 hr	Basin 2	47.25	29.49
100 yr - 002 hr	Basin 3	72.21	36.87
100 yr - 002 hr	Basin 4	55.88	48.10
100 yr - 002 hr	Post-Offsite-East	0.00	0.00
100 yr - 002 hr	Pre-Offsite-East	0.00	98.89
100 yr - 004 hr	Basin 1	47.89	44.66
100 yr - 004 hr	Basin 2	47.47	27.20
100 yr - 004 hr	Basin 3	72.77	21.04
100 yr - 004 hr	Basin 4	56.44	26.90
100 yr - 004 hr	Post-Offsite-East	0.00	0.00
100 yr - 004 hr	Pre-Offsite-East	0.00	65.33
100 yr - 008 hr	Basin 1	47.70	45.22
100 yr - 008 hr	Basin 2	47.61	27.90
100 yr - 008 hr	Basin 3	73.07	21.34
100 yr - 008 hr	Basin 4	57.10	26.56
100 yr - 008 hr	Post-Offsite-East	0.00	0.05
100 yr - 008 hr	Pre-Offsite-East	0.00	77.55
100 yr - 024 hr	Basin 1	48.13	15.05
100 yr - 024 hr	Basin 2	48.12	10.66
100 yr - 024 hr	Basin 3	73.34	7.08
100 yr - 024 hr	Basin 4	58.09	8.76
100 yr - 024 hr	Post-Offsite-East	0.00	1.82
100 yr - 024 hr	Pre-Offsite-East	0.00	27.14
100 yr - 072 hr	Basin 1	48.66	9.88
100 yr - 072 hr	Basin 2	48.66	8.06
100 yr - 072 hr	Basin 3	73.49	4.54
100 yr - 072 hr	Basin 4	58.46	6.34
100 yr - 072 hr	Post-Offsite-East	0.00	2.81
100 yr - 072 hr	Pre-Offsite-East	0.00	20.81
100 yr - 168 hr	Basin 1	49.07	6.91
100 yr - 168 hr	Basin 2	49.06	5.73
100 yr - 168 hr	Basin 3	73.74	3.14
100 yr - 168 hr	Basin 4	58.73	6.00
100 yr - 168 hr	Post-Offsite-East	0.00	3.63
100 yr - 168 hr	Pre-Offsite-East	0.00	15.14

Sim	Node Name	Maximum Stage [ft]	Maximum Total Inflow Rate [cfs]
100 yr - 240 hr	Basin 1	49.34	9.09
100 yr - 240 hr	Basin 2	49.34	7.53
100 yr - 240 hr	Basin 3	73.75	4.13
100 yr - 240 hr	Basin 4	58.77	7.22
100 yr - 240 hr	Post-Offsite-East	0.00	3.77
100 yr - 240 hr	Pre-Offsite-East	0.00	19.79

Sim	Link Name	Maximum Flow Rate [cfs]
100 yr - 001 hr	DISCHARGE-1	0.00
100 yr - 001 hr	DISCHARGE-2	0.00
100 yr - 001 hr	Discharge-3	0.00
100 yr - 001 hr	Discharge-4	0.00
100 yr - 002 hr	DISCHARGE-1	0.00
100 yr - 002 hr	DISCHARGE-2	0.00
100 yr - 002 hr	Discharge-3	0.00
100 yr - 002 hr	Discharge-4	0.00
100 yr - 004 hr	DISCHARGE-1	0.00
100 yr - 004 hr	DISCHARGE-2	0.00
100 yr - 004 hr	Discharge-3	0.22
100 yr - 004 hr	Discharge-4	0.00
100 yr - 008 hr	DISCHARGE-1	0.00
100 yr - 008 hr	DISCHARGE-2	0.00
100 yr - 008 hr	Discharge-3	0.69
100 yr - 008 hr	Discharge-4	0.05
100 yr - 024 hr	DISCHARGE-1	0.00
100 yr - 024 hr	DISCHARGE-2	0.00
100 yr - 024 hr	Discharge-3	1.23
100 yr - 024 hr	Discharge-4	1.82
100 yr - 072 hr	DISCHARGE-1	0.00
100 yr - 072 hr	DISCHARGE-2	0.00
100 yr - 072 hr	Discharge-3	1.57
100 yr - 072 hr	Discharge-4	2.81
100 yr - 168 hr	DISCHARGE-1	0.00
100 yr - 168 hr	DISCHARGE-2	0.00
100 yr - 168 hr	Discharge-3	2.21
100 yr - 168 hr	Discharge-4	3.63
100 yr - 240 hr	DISCHARGE-1	0.00
100 yr - 240 hr	DISCHARGE-2	0.00
100 yr - 240 hr	Discharge-3	2.24
100 yr - 240 hr	Discharge-4	3.77

Sim	Node Name	Total Inflow Volume [ac_ft]
003 yr - 001 hr	Post-Offsite-East	0.00
003 yr - 001 hr	Pre-Offsite-East	2.29
003 yr - 002 hr	Post-Offsite-East	0.00
003 yr - 002 hr	Pre-Offsite-East	3.13
003 yr - 004 hr	Post-Offsite-East	0.00
003 yr - 004 hr	Pre-Offsite-East	4.08
003 yr - 008 hr	Post-Offsite-East	0.00
003 yr - 008 hr	Pre-Offsite-East	5.27
003 yr - 024 hr	Post-Offsite-East	0.00
003 yr - 024 hr	Pre-Offsite-East	8.23
003 yr - 072 hr	Post-Offsite-East	0.00
003 yr - 072 hr	Pre-Offsite-East	11.70
003 yr - 168 hr	Post-Offsite-East	0.95
003 yr - 168 hr	Pre-Offsite-East	16.14
003 yr - 240 hr	Post-Offsite-East	2.29
003 yr - 240 hr	Pre-Offsite-East	19.33
005 yr - 001 hr	Post-Offsite-East	0.00
005 yr - 001 hr	Pre-Offsite-East	2.56
005 yr - 002 hr	Post-Offsite-East	0.00
005 yr - 002 hr	Pre-Offsite-East	3.44
005 yr - 004 hr	Post-Offsite-East	0.00
005 yr - 004 hr	Pre-Offsite-East	4.42
005 yr - 008 hr	Post-Offsite-East	0.00
005 yr - 008 hr	Pre-Offsite-East	5.86
005 yr - 024 hr	Post-Offsite-East	0.00
005 yr - 024 hr	Pre-Offsite-East	9.24
005 yr - 072 hr	Post-Offsite-East	0.03
005 yr - 072 hr	Pre-Offsite-East	12.61
005 yr - 168 hr	Post-Offsite-East	1.03
005 yr - 168 hr	Pre-Offsite-East	16.38
005 yr - 240 hr	Post-Offsite-East	2.49
005 yr - 240 hr	Pre-Offsite-East	19.83
010 yr - 001 hr	Post-Offsite-East	0.00
010 yr - 001 hr	Pre-Offsite-East	3.13
010 yr - 002 hr	Post-Offsite-East	0.00
010 yr - 002 hr	Pre-Offsite-East	4.42
010 yr - 004 hr	Post-Offsite-East	0.00
010 yr - 004 hr	Pre-Offsite-East	5.86
010 yr - 008 hr	Post-Offsite-East	0.00
010 yr - 008 hr	Pre-Offsite-East	7.90
010 yr - 024 hr	Post-Offsite-East	0.03
010 yr - 024 hr	Pre-Offsite-East	12.42

Sim	Node Name	Total Inflow Volume [ac_ft]
010 yr - 072 hr	Post-Offsite-East	0.54
010 yr - 072 hr	Pre-Offsite-East	14.71
010 yr - 168 hr	Post-Offsite-East	2.41
010 yr - 168 hr	Pre-Offsite-East	19.83
010 yr - 240 hr	Post-Offsite-East	3.95
010 yr - 240 hr	Pre-Offsite-East	23.64
025 yr - 001 hr	Post-Offsite-East	0.00
025 yr - 001 hr	Pre-Offsite-East	3.76
025 yr - 002 hr	Post-Offsite-East	0.00
025 yr - 002 hr	Pre-Offsite-East	5.12
025 yr - 004 hr	Post-Offsite-East	0.00
025 yr - 004 hr	Pre-Offsite-East	6.78
025 yr - 008 hr	Post-Offsite-East	0.00
025 yr - 008 hr	Pre-Offsite-East	9.41
025 yr - 024 hr	Post-Offsite-East	0.48
025 yr - 024 hr	Pre-Offsite-East	14.09
025 yr - 072 hr	Post-Offsite-East	2.77
025 yr - 072 hr	Pre-Offsite-East	19.83
025 yr - 168 hr	Post-Offsite-East	4.63
025 yr - 168 hr	Pre-Offsite-East	24.93
025 yr - 240 hr	Post-Offsite-East	6.60
025 yr - 240 hr	Pre-Offsite-East	30.17
050 yr - 001 hr	Post-Offsite-East	0.00
050 yr - 001 hr	Pre-Offsite-East	4.33
050 yr - 002 hr	Post-Offsite-East	0.00
050 yr - 002 hr	Pre-Offsite-East	5.86
050 yr - 004 hr	Post-Offsite-East	0.00
050 yr - 004 hr	Pre-Offsite-East	8.06
050 yr - 008 hr	Post-Offsite-East	0.00
050 yr - 008 hr	Pre-Offsite-East	10.27
050 yr - 024 hr	Post-Offsite-East	1.38
050 yr - 024 hr	Pre-Offsite-East	16.38
050 yr - 072 hr	Post-Offsite-East	3.93
050 yr - 072 hr	Pre-Offsite-East	22.36
050 yr - 168 hr	Post-Offsite-East	5.66
050 yr - 168 hr	Pre-Offsite-East	27.53
050 yr - 240 hr	Post-Offsite-East	7.74
050 yr - 240 hr	Pre-Offsite-East	32.84
100 yr - 001 hr	Post-Offsite-East	0.00
100 yr - 001 hr	Pre-Offsite-East	5.12
100 yr - 002 hr	Post-Offsite-East	0.00
100 yr - 002 hr	Pre-Offsite-East	7.01

Sim	Node Name	Total Inflow Volume [ac_ft]
100 yr - 004 hr	Post-Offsite-East	0.00
100 yr - 004 hr	Pre-Offsite-East	9.75
100 yr - 008 hr	Post-Offsite-East	0.46
100 yr - 008 hr	Pre-Offsite-East	12.61
100 yr - 024 hr	Post-Offsite-East	2.92
100 yr - 024 hr	Pre-Offsite-East	19.93
100 yr - 072 hr	Post-Offsite-East	6.05
100 yr - 072 hr	Pre-Offsite-East	27.01
100 yr - 168 hr	Post-Offsite-East	7.93
100 yr - 168 hr	Pre-Offsite-East	32.84
100 yr - 240 hr	Post-Offsite-East	10.04
100 yr - 240 hr	Pre-Offsite-East	38.26

Attachment H

WQTV Recovery Analysis

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 1	0.0000	46.70
WQTV	Basin 1	0.2511	46.66
WQTV	Basin 1	0.5037	46.61
WQTV	Basin 1	0.7509	46.57
WQTV	Basin 1	1.0027	46.52
WQTV	Basin 1	1.2501	46.48
WQTV	Basin 1	1.5038	46.43
WQTV	Basin 1	1.7510	46.39
WQTV	Basin 1	2.0036	46.34
WQTV	Basin 1	2.2506	46.30
WQTV	Basin 1	2.5027	46.26
WQTV	Basin 1	2.7510	46.21
WQTV	Basin 1	3.0038	46.17
WQTV	Basin 1	3.2561	46.14
WQTV	Basin 1	3.5061	46.12
WQTV	Basin 1	3.7561	46.10
WQTV	Basin 1	4.0061	46.09
WQTV	Basin 1	4.2561	46.07
WQTV	Basin 1	4.5061	46.05
WQTV	Basin 1	4.7561	46.04
WQTV	Basin 1	5.0061	46.02
WQTV	Basin 1	5.2561	46.00
WQTV	Basin 1	5.5061	46.00
WQTV	Basin 1	5.7561	46.00
WQTV	Basin 1	6.0061	46.00
WQTV	Basin 1	6.2511	46.00
WQTV	Basin 1	6.5012	46.00
WQTV	Basin 1	6.7528	46.00
WQTV	Basin 1	7.0038	46.00
WQTV	Basin 1	7.2538	46.00
WQTV	Basin 1	7.5038	46.00
WQTV	Basin 1	7.7538	46.00
WQTV	Basin 1	8.0038	46.00
WQTV	Basin 1	8.2538	46.00
WQTV	Basin 1	8.5038	46.00
WQTV	Basin 1	8.7538	46.00
WQTV	Basin 1	9.0038	46.00
WQTV	Basin 1	9.2538	46.00
WQTV	Basin 1	9.5038	46.00
WQTV	Basin 1	9.7538	46.00
WQTV	Basin 1	10.0038	46.00
WQTV	Basin 1	10.2538	46.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 1	63.0038	46.00
WQTV	Basin 1	63.2538	46.00
WQTV	Basin 1	63.5038	46.00
WQTV	Basin 1	63.7538	46.00
WQTV	Basin 1	64.0038	46.00
WQTV	Basin 1	64.2538	46.00
WQTV	Basin 1	64.5038	46.00
WQTV	Basin 1	64.7538	46.00
WQTV	Basin 1	65.0038	46.00
WQTV	Basin 1	65.2538	46.00
WQTV	Basin 1	65.5038	46.00
WQTV	Basin 1	65.7538	46.00
WQTV	Basin 1	66.0038	46.00
WQTV	Basin 1	66.2538	46.00
WQTV	Basin 1	66.5038	46.00
WQTV	Basin 1	66.7538	46.00
WQTV	Basin 1	67.0038	46.00
WQTV	Basin 1	67.2538	46.00
WQTV	Basin 1	67.5038	46.00
WQTV	Basin 1	67.7538	46.00
WQTV	Basin 1	68.0038	46.00
WQTV	Basin 1	68.2538	46.00
WQTV	Basin 1	68.5038	46.00
WQTV	Basin 1	68.7538	46.00
WQTV	Basin 1	69.0038	46.00
WQTV	Basin 1	69.2538	46.00
WQTV	Basin 1	69.5038	46.00
WQTV	Basin 1	69.7538	46.00
WQTV	Basin 1	70.0038	46.00
WQTV	Basin 1	70.2538	46.00
WQTV	Basin 1	70.5038	46.00
WQTV	Basin 1	70.7538	46.00
WQTV	Basin 1	71.0038	46.00
WQTV	Basin 1	71.2538	46.00
WQTV	Basin 1	71.5038	46.00
WQTV	Basin 1	71.7538	46.00
WQTV	Basin 2	0.0000	46.70
WQTV	Basin 2	0.2511	46.65
WQTV	Basin 2	0.5037	46.61
WQTV	Basin 2	0.7509	46.56
WQTV	Basin 2	1.0027	46.52
WQTV	Basin 2	1.2501	46.48

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 2	1.5038	46.43
WQTV	Basin 2	1.7510	46.39
WQTV	Basin 2	2.0036	46.34
WQTV	Basin 2	2.2506	46.30
WQTV	Basin 2	2.5027	46.25
WQTV	Basin 2	2.7510	46.21
WQTV	Basin 2	3.0038	46.17
WQTV	Basin 2	3.2561	46.15
WQTV	Basin 2	3.5061	46.14
WQTV	Basin 2	3.7561	46.12
WQTV	Basin 2	4.0061	46.11
WQTV	Basin 2	4.2561	46.09
WQTV	Basin 2	4.5061	46.08
WQTV	Basin 2	4.7561	46.06
WQTV	Basin 2	5.0061	46.05
WQTV	Basin 2	5.2561	46.03
WQTV	Basin 2	5.5061	46.02
WQTV	Basin 2	5.7561	46.01
WQTV	Basin 2	6.0061	46.00
WQTV	Basin 2	6.2511	46.00
WQTV	Basin 2	6.5012	46.00
WQTV	Basin 2	6.7528	46.00
WQTV	Basin 2	7.0038	46.00
WQTV	Basin 2	7.2538	46.00
WQTV	Basin 2	7.5038	46.00
WQTV	Basin 2	7.7538	46.00
WQTV	Basin 2	8.0038	46.00
WQTV	Basin 2	8.2538	46.00
WQTV	Basin 2	8.5038	46.00
WQTV	Basin 2	8.7538	46.00
WQTV	Basin 2	9.0038	46.00
WQTV	Basin 2	9.2538	46.00
WQTV	Basin 2	9.5038	46.00
WQTV	Basin 2	9.7538	46.00
WQTV	Basin 2	10.0038	46.00
WQTV	Basin 2	10.2538	46.00
WQTV	Basin 2	10.5038	46.00
WQTV	Basin 2	10.7538	46.00
WQTV	Basin 2	11.0038	46.00
WQTV	Basin 2	11.2538	46.00
WQTV	Basin 2	11.5038	46.00
WQTV	Basin 2	11.7538	46.00

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 2	64.5038	46.00
WQTV	Basin 2	64.7538	46.00
WQTV	Basin 2	65.0038	46.00
WQTV	Basin 2	65.2538	46.00
WQTV	Basin 2	65.5038	46.00
WQTV	Basin 2	65.7538	46.00
WQTV	Basin 2	66.0038	46.00
WQTV	Basin 2	66.2538	46.00
WQTV	Basin 2	66.5038	46.00
WQTV	Basin 2	66.7538	46.00
WQTV	Basin 2	67.0038	46.00
WQTV	Basin 2	67.2538	46.00
WQTV	Basin 2	67.5038	46.00
WQTV	Basin 2	67.7538	46.00
WQTV	Basin 2	68.0038	46.00
WQTV	Basin 2	68.2538	46.00
WQTV	Basin 2	68.5038	46.00
WQTV	Basin 2	68.7538	46.00
WQTV	Basin 2	69.0038	46.00
WQTV	Basin 2	69.2538	46.00
WQTV	Basin 2	69.5038	46.00
WQTV	Basin 2	69.7538	46.00
WQTV	Basin 2	70.0038	46.00
WQTV	Basin 2	70.2538	46.00
WQTV	Basin 2	70.5038	46.00
WQTV	Basin 2	70.7538	46.00
WQTV	Basin 2	71.0038	46.00
WQTV	Basin 2	71.2538	46.00
WQTV	Basin 2	71.5038	46.00
WQTV	Basin 2	71.7538	46.00
WQTV	Basin 3	0.0000	72.53
WQTV	Basin 3	0.2511	72.50
WQTV	Basin 3	0.5037	72.47
WQTV	Basin 3	0.7509	72.45
WQTV	Basin 3	1.0027	72.44
WQTV	Basin 3	1.2501	72.42
WQTV	Basin 3	1.5038	72.41
WQTV	Basin 3	1.7510	72.39
WQTV	Basin 3	2.0036	72.37
WQTV	Basin 3	2.2506	72.36
WQTV	Basin 3	2.5027	72.34
WQTV	Basin 3	2.7510	72.33

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 3	3.0038	72.31
WQTV	Basin 3	3.2561	72.30
WQTV	Basin 3	3.5061	72.28
WQTV	Basin 3	3.7561	72.27
WQTV	Basin 3	4.0061	72.25
WQTV	Basin 3	4.2561	72.23
WQTV	Basin 3	4.5061	72.22
WQTV	Basin 3	4.7561	72.20
WQTV	Basin 3	5.0061	72.19
WQTV	Basin 3	5.2561	72.17
WQTV	Basin 3	5.5061	72.16
WQTV	Basin 3	5.7561	72.14
WQTV	Basin 3	6.0061	72.12
WQTV	Basin 3	6.2511	72.11
WQTV	Basin 3	6.5012	72.09
WQTV	Basin 3	6.7528	72.08
WQTV	Basin 3	7.0038	72.06
WQTV	Basin 3	7.2538	72.05
WQTV	Basin 3	7.5038	72.03
WQTV	Basin 3	7.7538	72.02
WQTV	Basin 3	8.0038	72.00
WQTV	Basin 3	8.2538	71.98
WQTV	Basin 3	8.5038	71.97
WQTV	Basin 3	8.7538	71.95
WQTV	Basin 3	9.0038	71.94
WQTV	Basin 3	9.2538	71.92
WQTV	Basin 3	9.5038	71.91
WQTV	Basin 3	9.7538	71.89
WQTV	Basin 3	10.0038	71.87
WQTV	Basin 3	10.2538	71.86
WQTV	Basin 3	10.5038	71.84
WQTV	Basin 3	10.7538	71.83
WQTV	Basin 3	11.0038	71.81
WQTV	Basin 3	11.2538	71.80
WQTV	Basin 3	11.5038	71.78
WQTV	Basin 3	11.7538	71.77
WQTV	Basin 3	12.0038	71.75
WQTV	Basin 3	12.2538	71.73
WQTV	Basin 3	12.5038	71.72
WQTV	Basin 3	12.7538	71.70
WQTV	Basin 3	13.0038	71.69
WQTV	Basin 3	13.2538	71.67

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 3	13.5038	71.66
WQTV	Basin 3	13.7538	71.64
WQTV	Basin 3	14.0038	71.62
WQTV	Basin 3	14.2538	71.61
WQTV	Basin 3	14.5038	71.59
WQTV	Basin 3	14.7538	71.58
WQTV	Basin 3	15.0038	71.56
WQTV	Basin 3	15.2538	71.55
WQTV	Basin 3	15.5038	71.53
WQTV	Basin 3	15.7538	71.52
WQTV	Basin 3	16.0038	71.50
WQTV	Basin 3	16.2538	71.48
WQTV	Basin 3	16.5038	71.47
WQTV	Basin 3	16.7538	71.45
WQTV	Basin 3	17.0038	71.44
WQTV	Basin 3	17.2538	71.42
WQTV	Basin 3	17.5038	71.41
WQTV	Basin 3	17.7538	71.39
WQTV	Basin 3	18.0038	71.37
WQTV	Basin 3	18.2538	71.36
WQTV	Basin 3	18.5038	71.34
WQTV	Basin 3	18.7538	71.33
WQTV	Basin 3	19.0038	71.31
WQTV	Basin 3	19.2538	71.30
WQTV	Basin 3	19.5038	71.28
WQTV	Basin 3	19.7538	71.27
WQTV	Basin 3	20.0038	71.25
WQTV	Basin 3	20.2538	71.23
WQTV	Basin 3	20.5038	71.22
WQTV	Basin 3	20.7538	71.20
WQTV	Basin 3	21.0038	71.19
WQTV	Basin 3	21.2538	71.17
WQTV	Basin 3	21.5038	71.16
WQTV	Basin 3	21.7538	71.14
WQTV	Basin 3	22.0038	71.12
WQTV	Basin 3	22.2538	71.11
WQTV	Basin 3	22.5038	71.09
WQTV	Basin 3	22.7538	71.08
WQTV	Basin 3	23.0038	71.06
WQTV	Basin 3	23.2538	71.05
WQTV	Basin 3	23.5038	71.03
WQTV	Basin 3	23.7538	71.02

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 3	24.0038	71.00
WQTV	Basin 3	24.2538	70.98
WQTV	Basin 3	24.5038	70.97
WQTV	Basin 3	24.7538	70.95
WQTV	Basin 3	25.0038	70.94
WQTV	Basin 3	25.2538	70.92
WQTV	Basin 3	25.5038	70.91
WQTV	Basin 3	25.7538	70.89
WQTV	Basin 3	26.0038	70.87
WQTV	Basin 3	26.2538	70.86
WQTV	Basin 3	26.5038	70.84
WQTV	Basin 3	26.7538	70.83
WQTV	Basin 3	27.0038	70.81
WQTV	Basin 3	27.2538	70.80
WQTV	Basin 3	27.5038	70.78
WQTV	Basin 3	27.7538	70.77
WQTV	Basin 3	28.0038	70.75
WQTV	Basin 3	28.2538	70.73
WQTV	Basin 3	28.5038	70.72
WQTV	Basin 3	28.7538	70.70
WQTV	Basin 3	29.0038	70.69
WQTV	Basin 3	29.2538	70.67
WQTV	Basin 3	29.5038	70.66
WQTV	Basin 3	29.7538	70.64
WQTV	Basin 3	30.0038	70.62
WQTV	Basin 3	30.2538	70.61
WQTV	Basin 3	30.5038	70.60
WQTV	Basin 3	30.7538	70.58
WQTV	Basin 3	31.0038	70.57
WQTV	Basin 3	31.2538	70.56
WQTV	Basin 3	31.5038	70.55
WQTV	Basin 3	31.7538	70.53
WQTV	Basin 3	32.0038	70.52
WQTV	Basin 3	32.2538	70.51
WQTV	Basin 3	32.5038	70.51
WQTV	Basin 3	32.7538	70.50
WQTV	Basin 3	33.0038	70.49
WQTV	Basin 3	33.2538	70.48
WQTV	Basin 3	33.5038	70.47
WQTV	Basin 3	33.7538	70.46
WQTV	Basin 3	34.0038	70.46
WQTV	Basin 3	34.2538	70.45

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 3	34.5038	70.44
WQTV	Basin 3	34.7538	70.44
WQTV	Basin 3	35.0038	70.43
WQTV	Basin 3	35.2538	70.42
WQTV	Basin 3	35.5038	70.42
WQTV	Basin 3	35.7538	70.41
WQTV	Basin 3	36.0038	70.40
WQTV	Basin 3	36.2538	70.40
WQTV	Basin 3	36.5038	70.39
WQTV	Basin 3	36.7538	70.39
WQTV	Basin 3	37.0038	70.38
WQTV	Basin 3	37.2538	70.38
WQTV	Basin 3	37.5038	70.37
WQTV	Basin 3	37.7538	70.37
WQTV	Basin 3	38.0038	70.36
WQTV	Basin 3	38.2538	70.36
WQTV	Basin 3	38.5038	70.35
WQTV	Basin 3	38.7538	70.35
WQTV	Basin 3	39.0038	70.34
WQTV	Basin 3	39.2538	70.34
WQTV	Basin 3	39.5038	70.33
WQTV	Basin 3	39.7538	70.33
WQTV	Basin 3	40.0038	70.32
WQTV	Basin 3	40.2538	70.32
WQTV	Basin 3	40.5038	70.32
WQTV	Basin 3	40.7538	70.31
WQTV	Basin 3	41.0038	70.31
WQTV	Basin 3	41.2538	70.30
WQTV	Basin 3	41.5038	70.30
WQTV	Basin 3	41.7538	70.29
WQTV	Basin 3	42.0038	70.29
WQTV	Basin 3	42.2538	70.29
WQTV	Basin 3	42.5038	70.28
WQTV	Basin 3	42.7538	70.28
WQTV	Basin 3	43.0038	70.28
WQTV	Basin 3	43.2538	70.27
WQTV	Basin 3	43.5038	70.27
WQTV	Basin 3	43.7538	70.26
WQTV	Basin 3	44.0038	70.26
WQTV	Basin 3	44.2538	70.26
WQTV	Basin 3	44.5038	70.25
WQTV	Basin 3	44.7538	70.25

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 3	45.0038	70.25
WQTV	Basin 3	45.2538	70.24
WQTV	Basin 3	45.5038	70.24
WQTV	Basin 3	45.7538	70.24
WQTV	Basin 3	46.0038	70.23
WQTV	Basin 3	46.2538	70.23
WQTV	Basin 3	46.5038	70.23
WQTV	Basin 3	46.7538	70.22
WQTV	Basin 3	47.0038	70.22
WQTV	Basin 3	47.2538	70.22
WQTV	Basin 3	47.5038	70.21
WQTV	Basin 3	47.7538	70.21
WQTV	Basin 3	48.0038	70.21
WQTV	Basin 3	48.2538	70.20
WQTV	Basin 3	48.5038	70.20
WQTV	Basin 3	48.7538	70.20
WQTV	Basin 3	49.0038	70.20
WQTV	Basin 3	49.2538	70.19
WQTV	Basin 3	49.5038	70.19
WQTV	Basin 3	49.7538	70.19
WQTV	Basin 3	50.0038	70.18
WQTV	Basin 3	50.2538	70.18
WQTV	Basin 3	50.5038	70.18
WQTV	Basin 3	50.7538	70.18
WQTV	Basin 3	51.0038	70.17
WQTV	Basin 3	51.2538	70.17
WQTV	Basin 3	51.5038	70.17
WQTV	Basin 3	51.7538	70.16
WQTV	Basin 3	52.0038	70.16
WQTV	Basin 3	52.2538	70.16
WQTV	Basin 3	52.5038	70.16
WQTV	Basin 3	52.7538	70.15
WQTV	Basin 3	53.0038	70.15
WQTV	Basin 3	53.2538	70.15
WQTV	Basin 3	53.5038	70.15
WQTV	Basin 3	53.7538	70.14
WQTV	Basin 3	54.0038	70.14
WQTV	Basin 3	54.2538	70.14
WQTV	Basin 3	54.5038	70.13
WQTV	Basin 3	54.7538	70.13
WQTV	Basin 3	55.0038	70.13
WQTV	Basin 3	55.2538	70.13

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 3	55.5038	70.12
WQTV	Basin 3	55.7538	70.12
WQTV	Basin 3	56.0038	70.12
WQTV	Basin 3	56.2538	70.12
WQTV	Basin 3	56.5038	70.11
WQTV	Basin 3	56.7538	70.11
WQTV	Basin 3	57.0038	70.11
WQTV	Basin 3	57.2538	70.11
WQTV	Basin 3	57.5038	70.10
WQTV	Basin 3	57.7538	70.10
WQTV	Basin 3	58.0038	70.10
WQTV	Basin 3	58.2538	70.10
WQTV	Basin 3	58.5038	70.09
WQTV	Basin 3	58.7538	70.09
WQTV	Basin 3	59.0038	70.09
WQTV	Basin 3	59.2538	70.09
WQTV	Basin 3	59.5038	70.09
WQTV	Basin 3	59.7538	70.08
WQTV	Basin 3	60.0038	70.08
WQTV	Basin 3	60.2538	70.08
WQTV	Basin 3	60.5038	70.08
WQTV	Basin 3	60.7538	70.07
WQTV	Basin 3	61.0038	70.07
WQTV	Basin 3	61.2538	70.07
WQTV	Basin 3	61.5038	70.07
WQTV	Basin 3	61.7538	70.06
WQTV	Basin 3	62.0038	70.06
WQTV	Basin 3	62.2538	70.06
WQTV	Basin 3	62.5038	70.06
WQTV	Basin 3	62.7538	70.06
WQTV	Basin 3	63.0038	70.05
WQTV	Basin 3	63.2538	70.05
WQTV	Basin 3	63.5038	70.05
WQTV	Basin 3	63.7538	70.05
WQTV	Basin 3	64.0038	70.04
WQTV	Basin 3	64.2538	70.04
WQTV	Basin 3	64.5038	70.04
WQTV	Basin 3	64.7538	70.04
WQTV	Basin 3	65.0038	70.04
WQTV	Basin 3	65.2538	70.03
WQTV	Basin 3	65.5038	70.03
WQTV	Basin 3	65.7538	70.03

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 3	66.0038	70.03
WQTV	Basin 3	66.2538	70.02
WQTV	Basin 3	66.5038	70.02
WQTV	Basin 3	66.7538	70.02
WQTV	Basin 3	67.0038	70.02
WQTV	Basin 3	67.2538	70.02
WQTV	Basin 3	67.5038	70.01
WQTV	Basin 3	67.7538	70.01
WQTV	Basin 3	68.0038	70.01
WQTV	Basin 3	68.2538	70.01
WQTV	Basin 3	68.5038	70.01
WQTV	Basin 3	68.7538	70.00
WQTV	Basin 3	69.0038	70.00
WQTV	Basin 3	69.2538	70.00
WQTV	Basin 3	69.5038	70.00
WQTV	Basin 3	69.7538	70.00
WQTV	Basin 3	70.0038	70.00
WQTV	Basin 3	70.2538	70.00
WQTV	Basin 3	70.5038	70.00
WQTV	Basin 3	70.7538	70.00
WQTV	Basin 3	71.0038	70.00
WQTV	Basin 3	71.2538	70.00
WQTV	Basin 3	71.5038	70.00
WQTV	Basin 3	71.7538	70.00
WQTV	Basin 4	0.0000	54.54
WQTV	Basin 4	0.2511	54.49
WQTV	Basin 4	0.5037	54.44
WQTV	Basin 4	0.7509	54.39
WQTV	Basin 4	1.0027	54.34
WQTV	Basin 4	1.2501	54.29
WQTV	Basin 4	1.5038	54.25
WQTV	Basin 4	1.7510	54.21
WQTV	Basin 4	2.0036	54.17
WQTV	Basin 4	2.2506	54.13
WQTV	Basin 4	2.5027	54.12
WQTV	Basin 4	2.7510	54.12
WQTV	Basin 4	3.0038	54.12
WQTV	Basin 4	3.2561	54.12
WQTV	Basin 4	3.5061	54.12
WQTV	Basin 4	3.7561	54.11
WQTV	Basin 4	4.0061	54.11
WQTV	Basin 4	4.2561	54.11

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 4	4.5061	54.11
WQTV	Basin 4	4.7561	54.11
WQTV	Basin 4	5.0061	54.11
WQTV	Basin 4	5.2561	54.10
WQTV	Basin 4	5.5061	54.10
WQTV	Basin 4	5.7561	54.10
WQTV	Basin 4	6.0061	54.10
WQTV	Basin 4	6.2511	54.10
WQTV	Basin 4	6.5012	54.10
WQTV	Basin 4	6.7528	54.10
WQTV	Basin 4	7.0038	54.10
WQTV	Basin 4	7.2538	54.09
WQTV	Basin 4	7.5038	54.09
WQTV	Basin 4	7.7538	54.09
WQTV	Basin 4	8.0038	54.09
WQTV	Basin 4	8.2538	54.09
WQTV	Basin 4	8.5038	54.09
WQTV	Basin 4	8.7538	54.09
WQTV	Basin 4	9.0038	54.09
WQTV	Basin 4	9.2538	54.09
WQTV	Basin 4	9.5038	54.09
WQTV	Basin 4	9.7538	54.09
WQTV	Basin 4	10.0038	54.08
WQTV	Basin 4	10.2538	54.08
WQTV	Basin 4	10.5038	54.08
WQTV	Basin 4	10.7538	54.08
WQTV	Basin 4	11.0038	54.08
WQTV	Basin 4	11.2538	54.08
WQTV	Basin 4	11.5038	54.08
WQTV	Basin 4	11.7538	54.08
WQTV	Basin 4	12.0038	54.08
WQTV	Basin 4	12.2538	54.08
WQTV	Basin 4	12.5038	54.08
WQTV	Basin 4	12.7538	54.08
WQTV	Basin 4	13.0038	54.08
WQTV	Basin 4	13.2538	54.08
WQTV	Basin 4	13.5038	54.07
WQTV	Basin 4	13.7538	54.07
WQTV	Basin 4	14.0038	54.07
WQTV	Basin 4	14.2538	54.07
WQTV	Basin 4	14.5038	54.07
WQTV	Basin 4	14.7538	54.07

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 4	15.0038	54.07
WQTV	Basin 4	15.2538	54.07
WQTV	Basin 4	15.5038	54.07
WQTV	Basin 4	15.7538	54.07
WQTV	Basin 4	16.0038	54.07
WQTV	Basin 4	16.2538	54.07
WQTV	Basin 4	16.5038	54.07
WQTV	Basin 4	16.7538	54.07
WQTV	Basin 4	17.0038	54.07
WQTV	Basin 4	17.2538	54.07
WQTV	Basin 4	17.5038	54.07
WQTV	Basin 4	17.7538	54.06
WQTV	Basin 4	18.0038	54.06
WQTV	Basin 4	18.2538	54.06
WQTV	Basin 4	18.5038	54.06
WQTV	Basin 4	18.7538	54.06
WQTV	Basin 4	19.0038	54.06
WQTV	Basin 4	19.2538	54.06
WQTV	Basin 4	19.5038	54.06
WQTV	Basin 4	19.7538	54.06
WQTV	Basin 4	20.0038	54.06
WQTV	Basin 4	20.2538	54.06
WQTV	Basin 4	20.5038	54.06
WQTV	Basin 4	20.7538	54.06
WQTV	Basin 4	21.0038	54.06
WQTV	Basin 4	21.2538	54.06
WQTV	Basin 4	21.5038	54.06
WQTV	Basin 4	21.7538	54.06
WQTV	Basin 4	22.0038	54.06
WQTV	Basin 4	22.2538	54.06
WQTV	Basin 4	22.5038	54.06
WQTV	Basin 4	22.7538	54.06
WQTV	Basin 4	23.0038	54.06
WQTV	Basin 4	23.2538	54.05
WQTV	Basin 4	23.5038	54.05
WQTV	Basin 4	23.7538	54.05
WQTV	Basin 4	24.0038	54.05
WQTV	Basin 4	24.2538	54.05
WQTV	Basin 4	24.5038	54.05
WQTV	Basin 4	24.7538	54.05
WQTV	Basin 4	25.0038	54.05
WQTV	Basin 4	25.2538	54.05

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 4	25.5038	54.05
WQTV	Basin 4	25.7538	54.05
WQTV	Basin 4	26.0038	54.05
WQTV	Basin 4	26.2538	54.05
WQTV	Basin 4	26.5038	54.05
WQTV	Basin 4	26.7538	54.05
WQTV	Basin 4	27.0038	54.05
WQTV	Basin 4	27.2538	54.05
WQTV	Basin 4	27.5038	54.05
WQTV	Basin 4	27.7538	54.05
WQTV	Basin 4	28.0038	54.05
WQTV	Basin 4	28.2538	54.05
WQTV	Basin 4	28.5038	54.05
WQTV	Basin 4	28.7538	54.05
WQTV	Basin 4	29.0038	54.05
WQTV	Basin 4	29.2538	54.05
WQTV	Basin 4	29.5038	54.05
WQTV	Basin 4	29.7538	54.04
WQTV	Basin 4	30.0038	54.04
WQTV	Basin 4	30.2538	54.04
WQTV	Basin 4	30.5038	54.04
WQTV	Basin 4	30.7538	54.04
WQTV	Basin 4	31.0038	54.04
WQTV	Basin 4	31.2538	54.04
WQTV	Basin 4	31.5038	54.04
WQTV	Basin 4	31.7538	54.04
WQTV	Basin 4	32.0038	54.04
WQTV	Basin 4	32.2538	54.04
WQTV	Basin 4	32.5038	54.04
WQTV	Basin 4	32.7538	54.04
WQTV	Basin 4	33.0038	54.04
WQTV	Basin 4	33.2538	54.04
WQTV	Basin 4	33.5038	54.04
WQTV	Basin 4	33.7538	54.04
WQTV	Basin 4	34.0038	54.04
WQTV	Basin 4	34.2538	54.04
WQTV	Basin 4	34.5038	54.04
WQTV	Basin 4	34.7538	54.04
WQTV	Basin 4	35.0038	54.04
WQTV	Basin 4	35.2538	54.04
WQTV	Basin 4	35.5038	54.04
WQTV	Basin 4	35.7538	54.04

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 4	36.0038	54.04
WQTV	Basin 4	36.2538	54.04
WQTV	Basin 4	36.5038	54.04
WQTV	Basin 4	36.7538	54.04
WQTV	Basin 4	37.0038	54.04
WQTV	Basin 4	37.2538	54.04
WQTV	Basin 4	37.5038	54.03
WQTV	Basin 4	37.7538	54.03
WQTV	Basin 4	38.0038	54.03
WQTV	Basin 4	38.2538	54.03
WQTV	Basin 4	38.5038	54.03
WQTV	Basin 4	38.7538	54.03
WQTV	Basin 4	39.0038	54.03
WQTV	Basin 4	39.2538	54.03
WQTV	Basin 4	39.5038	54.03
WQTV	Basin 4	39.7538	54.03
WQTV	Basin 4	40.0038	54.03
WQTV	Basin 4	40.2538	54.03
WQTV	Basin 4	40.5038	54.03
WQTV	Basin 4	40.7538	54.03
WQTV	Basin 4	41.0038	54.03
WQTV	Basin 4	41.2538	54.03
WQTV	Basin 4	41.5038	54.03
WQTV	Basin 4	41.7538	54.03
WQTV	Basin 4	42.0038	54.03
WQTV	Basin 4	42.2538	54.03
WQTV	Basin 4	42.5038	54.03
WQTV	Basin 4	42.7538	54.03
WQTV	Basin 4	43.0038	54.03
WQTV	Basin 4	43.2538	54.03
WQTV	Basin 4	43.5038	54.03
WQTV	Basin 4	43.7538	54.03
WQTV	Basin 4	44.0038	54.03
WQTV	Basin 4	44.2538	54.03
WQTV	Basin 4	44.5038	54.03
WQTV	Basin 4	44.7538	54.03
WQTV	Basin 4	45.0038	54.03
WQTV	Basin 4	45.2538	54.03
WQTV	Basin 4	45.5038	54.03
WQTV	Basin 4	45.7538	54.03
WQTV	Basin 4	46.0038	54.03
WQTV	Basin 4	46.2538	54.02

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 4	46.5038	54.02
WQTV	Basin 4	46.7538	54.02
WQTV	Basin 4	47.0038	54.02
WQTV	Basin 4	47.2538	54.02
WQTV	Basin 4	47.5038	54.02
WQTV	Basin 4	47.7538	54.02
WQTV	Basin 4	48.0038	54.02
WQTV	Basin 4	48.2538	54.02
WQTV	Basin 4	48.5038	54.02
WQTV	Basin 4	48.7538	54.02
WQTV	Basin 4	49.0038	54.02
WQTV	Basin 4	49.2538	54.02
WQTV	Basin 4	49.5038	54.02
WQTV	Basin 4	49.7538	54.02
WQTV	Basin 4	50.0038	54.02
WQTV	Basin 4	50.2538	54.02
WQTV	Basin 4	50.5038	54.02
WQTV	Basin 4	50.7538	54.02
WQTV	Basin 4	51.0038	54.02
WQTV	Basin 4	51.2538	54.02
WQTV	Basin 4	51.5038	54.02
WQTV	Basin 4	51.7538	54.02
WQTV	Basin 4	52.0038	54.02
WQTV	Basin 4	52.2538	54.02
WQTV	Basin 4	52.5038	54.02
WQTV	Basin 4	52.7538	54.02
WQTV	Basin 4	53.0038	54.02
WQTV	Basin 4	53.2538	54.02
WQTV	Basin 4	53.5038	54.02
WQTV	Basin 4	53.7538	54.02
WQTV	Basin 4	54.0038	54.02
WQTV	Basin 4	54.2538	54.02
WQTV	Basin 4	54.5038	54.02
WQTV	Basin 4	54.7538	54.02
WQTV	Basin 4	55.0038	54.02
WQTV	Basin 4	55.2538	54.02
WQTV	Basin 4	55.5038	54.02
WQTV	Basin 4	55.7538	54.02
WQTV	Basin 4	56.0038	54.02
WQTV	Basin 4	56.2538	54.01
WQTV	Basin 4	56.5038	54.01
WQTV	Basin 4	56.7538	54.01

Sim	Node Name	Relative Time [hrs]	Stage [ft]
WQTV	Basin 4	57.0038	54.01
WQTV	Basin 4	57.2538	54.01
WQTV	Basin 4	57.5038	54.01
WQTV	Basin 4	57.7538	54.01
WQTV	Basin 4	58.0038	54.01
WQTV	Basin 4	58.2538	54.01
WQTV	Basin 4	58.5038	54.01
WQTV	Basin 4	58.7538	54.01
WQTV	Basin 4	59.0038	54.01
WQTV	Basin 4	59.2538	54.01
WQTV	Basin 4	59.5038	54.01
WQTV	Basin 4	59.7538	54.01
WQTV	Basin 4	60.0038	54.01
WQTV	Basin 4	60.2538	54.01
WQTV	Basin 4	60.5038	54.01
WQTV	Basin 4	60.7538	54.01
WQTV	Basin 4	61.0038	54.01
WQTV	Basin 4	61.2538	54.01
WQTV	Basin 4	61.5038	54.01
WQTV	Basin 4	61.7538	54.01
WQTV	Basin 4	62.0038	54.01
WQTV	Basin 4	62.2538	54.01
WQTV	Basin 4	62.5038	54.01
WQTV	Basin 4	62.7538	54.01
WQTV	Basin 4	63.0038	54.01
WQTV	Basin 4	63.2538	54.01
WQTV	Basin 4	63.5038	54.01
WQTV	Basin 4	63.7538	54.01
WQTV	Basin 4	64.0038	54.01
WQTV	Basin 4	64.2538	54.01
WQTV	Basin 4	64.5038	54.01
WQTV	Basin 4	64.7538	54.01
WQTV	Basin 4	65.0038	54.01
WQTV	Basin 4	65.2538	54.01
WQTV	Basin 4	65.5038	54.01
WQTV	Basin 4	65.7538	54.01
WQTV	Basin 4	66.0038	54.01
WQTV	Basin 4	66.2538	54.01
WQTV	Basin 4	66.5038	54.01
WQTV	Basin 4	66.7538	54.01
WQTV	Basin 4	67.0038	54.00
WQTV	Basin 4	67.2538	54.00