

EX_5

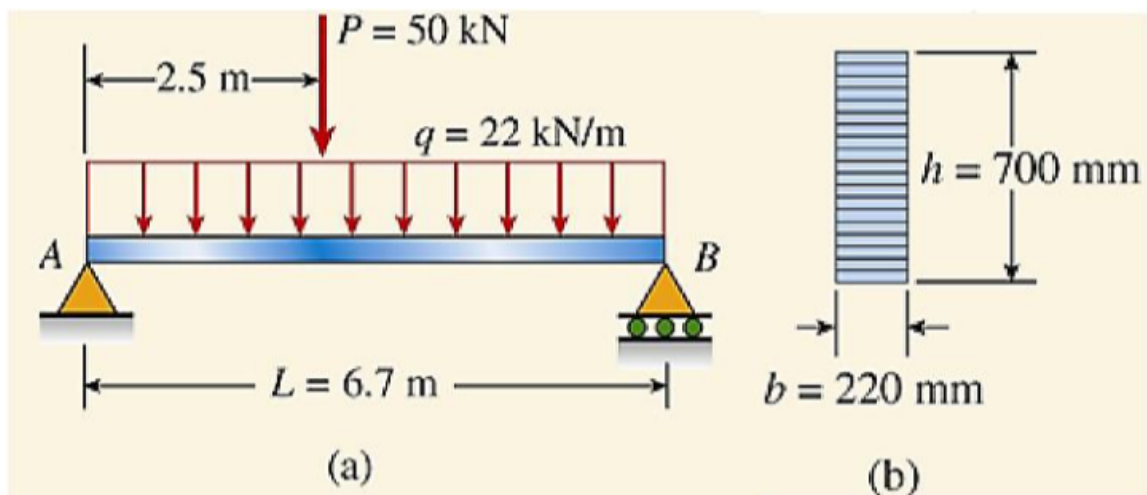
EX_5

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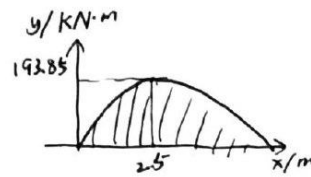
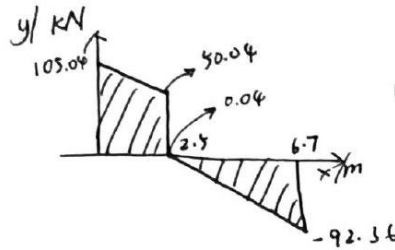
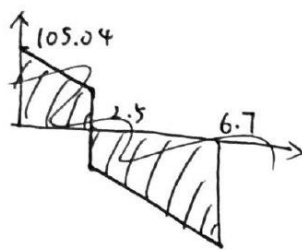
EX 5.1

A simple beam AB of span length $L=6.7\text{m}$ supports a uniform load of intensity $q=22\text{kN/m}$ and a concentrated load $P=50\text{kN}$. The concentrated load acts at a point 2.5 m from the left-hand end of the beam. The beam is constructed of glued laminated wood and has a cross section of width $b=220\text{mm}$ and height $h=700\text{mm}$.

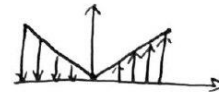
Determine the maximum tensile and compressive stresses in the beam due to loading.



$$\left. \begin{aligned} R_A + R_B &= 50 + 22 \times 6.7 \\ R_B L &= 50 \times 2.5 + 22 \times 6.7 \times 6.7 \times \frac{1}{2} \end{aligned} \right\} \Rightarrow \begin{cases} R_A = 105.04 \text{ kN} \\ R_B = 92.36 \text{ kN} \end{cases}$$



$$I = \frac{1}{12} b h^3 = 6.280 \times 10^{-3} \text{ m}^4$$



$$\sigma_1 = \sigma_2 = \frac{M \cdot \frac{h}{2}}{I}$$

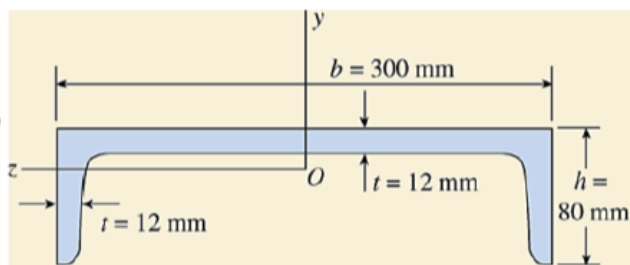
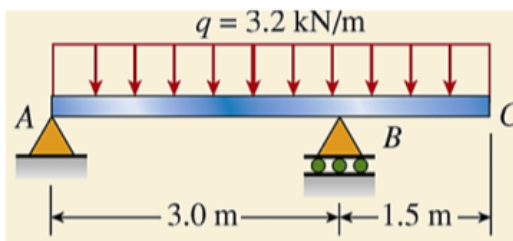
$$\sigma_1 = - \frac{M \left(\frac{h}{2} \right)}{I} = -10.80 \text{ kPa}$$

$$\sigma_2 = - \frac{M \left(-\frac{h}{2} \right)}{I} = 10.80 \text{ kPa}$$

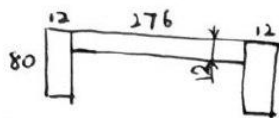
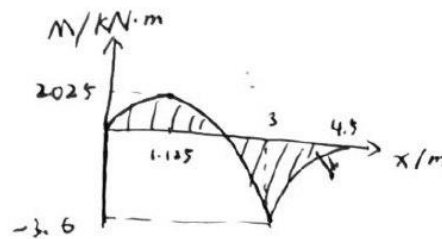
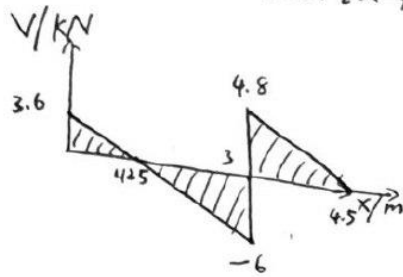
EX 5.2

The beam ABC has simple supports at A and B and an overhang from B to C. The length of the span is 3 m and the length of the overhang is 1.5 m. A uniform load of intensity $q = 3.2 \text{ kN/m}$ acts throughout the entire length of the beam. The beam has a cross section of channel shape.

Determine the maximum tensile and compressive stresses in the beam due to loading.



$$\left. \begin{aligned} R_A + R_B &= 3.2 \times 4.5 \\ 3R_B &= 3.2 \times 4.5 \times \frac{1}{2} \times 4.5 \end{aligned} \right\} \Rightarrow \begin{cases} R_A = 3.6 \text{ kN} \\ R_B = 10.8 \text{ kN} \end{cases}$$



$$\bar{y} = \frac{\sum \bar{y}_i A_i}{\sum A_i} = \frac{2 \times 12 \times 80 \times 40 + 276 \times 12 \times 6}{2 \times 12 \times 80 + 276 \times 12} = 18.48 \text{ mm}$$

$$I = \sum (I_i + A_i d_i^2) = \frac{1}{12} \times 276 \times 12^3 + 276 \times 12 \times (\bar{y} - 6)^2 + 2 \times \left(\frac{1}{12} \times 12 \times 80^3 + 12 \times 80 \times (\bar{y} - 40)^2 \right)$$

$$= 2.469 \times 10^{-6} \text{ m}^4$$

$$\text{for positive moment, } \sigma_c = -\frac{M \bar{y}}{I} \quad \sigma_t = \frac{M(\bar{y} - h)}{I}$$

$$\sigma_c = -\frac{M \bar{y}}{I} = -15.16 \text{ MPa}$$

$$\sigma_t = -\frac{M(\bar{y} - 80)}{I} = 50.46 \text{ MPa}$$

for negative moment

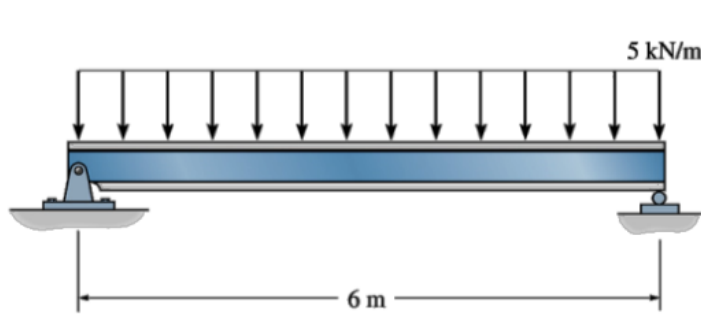
$$\sigma_c = -\frac{M(\bar{y} - h)}{I} = -89.70 \text{ MPa}$$

$$\sigma_t = -\frac{M \bar{y}}{I} = 26.95 \text{ MPa}$$

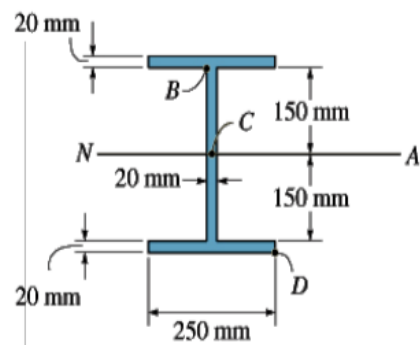
EX 5.3

The simply supported beam in the following figure (a) has the cross-sectional area shown in figure (b).

Determine the absolute maximum bending stress in the beam and draw the stress distribution over the cross section at this location.



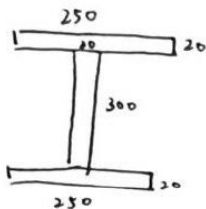
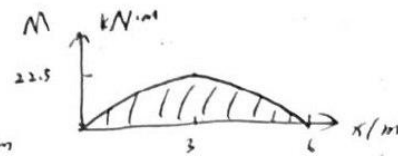
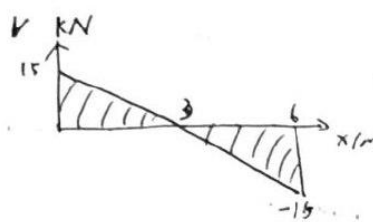
(a)



(b)

$$2R = 5 \times 6$$

$$R = 15 \text{ N}$$



due to the symmetry, $\bar{y} = \frac{300 + 20 + 20}{2} = 170 \text{ mm}$

$$I = \sum (I_i + A_i d_i^2)$$

$$= \frac{1}{12} \times 250 \times 20^3 + 250 \times 20 \times (170 - 10)^2$$

$$+ \frac{1}{12} \times 20 \times 300^3 + 20 \times 300 \times (170 - 170)^2$$

$$+ \frac{1}{12} \times 250 \times 20^3 + 20 \times 250 \times (330 - 170)^2$$

$$= \underline{\underline{3.013 \times 10^{-4} \text{ m}^4}}$$

for positive bending moment $\sigma_c = -\frac{M \bar{y}}{I}$, $\sigma_t = -\frac{M(\bar{y} - h)}{I}$

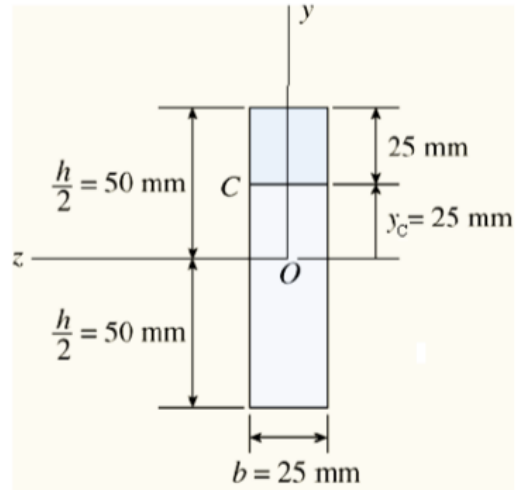
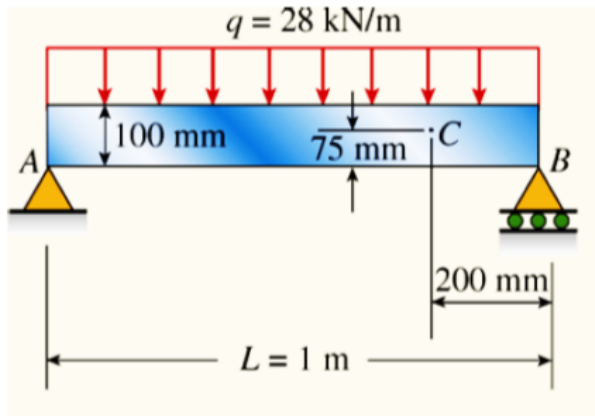
$$\sigma_c = -\frac{22.5 \times 170}{3.013 \times 10^{-4}} = \underline{\underline{-12.69 \text{ MPa}}}$$

$$\sigma_t = -\frac{M(\bar{y} - h)}{I} = \underline{\underline{12.69 \text{ MPa}}}$$

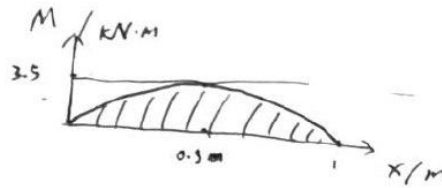
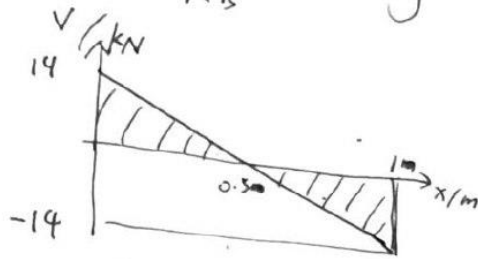
EX 5.3 (torsion)

A metal beam with span $L = 1 \text{ m}$ is simply supported at points A and B. The uniform load on the beam is $q = 28 \text{ kN/m}$. The cross section of the beam is rectangular with width $b = 25 \text{ mm}$ and height $h = 100 \text{ mm}$.

Determine the normal stress σ_C and shear stress τ_C at point C, which is located 25 mm below the top of the beam and 200 mm from the righthand support. Show these stresses on a sketch of a stress element at point C.



$$\left. \begin{array}{l} R_A + R_B = 28 \times 1 \\ R_A = R_B \end{array} \right\} \Rightarrow R_A = R_B = 14 \text{ kN}$$



$$I = \frac{1}{12} b h^3 = 2.08 \times 10^{-6} \text{ m}^4$$

$$V(x) = 14 - 28x \text{ (kN)}$$

$$V(0.8) = -8.4 \text{ kN}$$

$$M(x) = 14x(1-x) \text{ (kN·m)}, \quad M(0.8) = 2.24 \text{ kN·m}$$

$$y_c = \frac{h}{2} = 50 \text{ mm}$$

$$\sigma_c = - \frac{M(0.8)(y_c - 25)}{I} = -26.92 \text{ MPa}$$

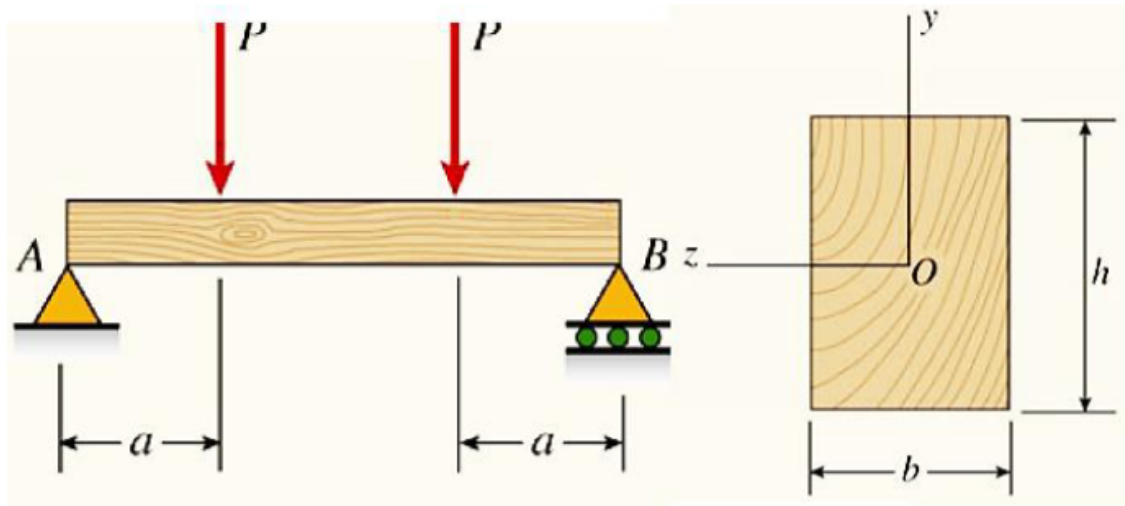
$$Q = \frac{1}{2} b \left(\frac{h^2}{4} - (y_c - 25)^2 \right) = 2.34 \times 10^{-5} \text{ m}^3$$

$$\tau = - \frac{VQ}{Ib} = 3.78 \text{ MPa}$$

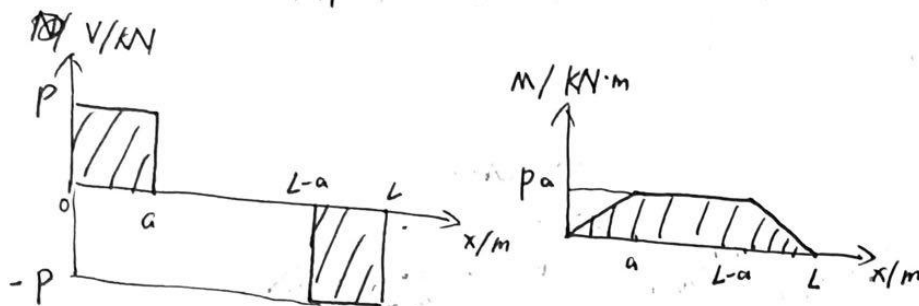
EX 5.4

A wood beam AB supporting two concentrated loads P has a rectangular cross section of width $b=100 \text{ mm}$ and height $h=150 \text{ mm}$. The distance from each end of the beam to the nearest load is $a=0.5 \text{ m}$.

Determine the maximum permissible value P_{max} of the loads if the allowable stress in bending is $\sigma_{allow} = 11 \text{ MPa}$ and the allowable stress in horizontal shear is $\tau_{allow} = 1.2 \text{ MPa}$.



$$R_A = R_B = P$$



$$M_{\max} = Pa, \quad V_{\max} = P, \quad I = \frac{1}{12} bh^3 = 0.28125 \times 10^{-5} \text{ m}^4$$

$$Q = \bar{y} = \frac{h}{2} \quad Q = \frac{1}{2} b \left(\frac{h^2}{4} - \frac{h^2}{4} \right) = 0$$

$$Q_{\max} = \frac{1}{2} b \left(\frac{h^2}{4} - 0 \right) = \frac{1}{8} bh^2$$

$$\sigma_{\max} = \frac{M_{\max} \cdot \frac{h}{2}}{I} = \frac{Pa \cdot h}{\frac{1}{6} bh^3} = \frac{6Pa}{bh^2}$$

$$\tau_{\max} = \frac{VQ}{Ib} = \frac{P \cdot \frac{1}{8} bh^2}{\frac{1}{12} bh^3 \cdot b} = \frac{3P}{2bh}$$

$$\Rightarrow \begin{cases} P_{\max,1} = \frac{\sigma_{\max} \cdot bh^2}{6a} = 8.25 \text{ kN} \\ P_{\max,2} = \frac{\tau_{\max} \cdot 2bh}{3} = 12 \text{ kN} \end{cases} \Rightarrow P_{\max} = \min \{P_{m,1}, P_{m,2}\} = 8.25 \text{ kN}$$