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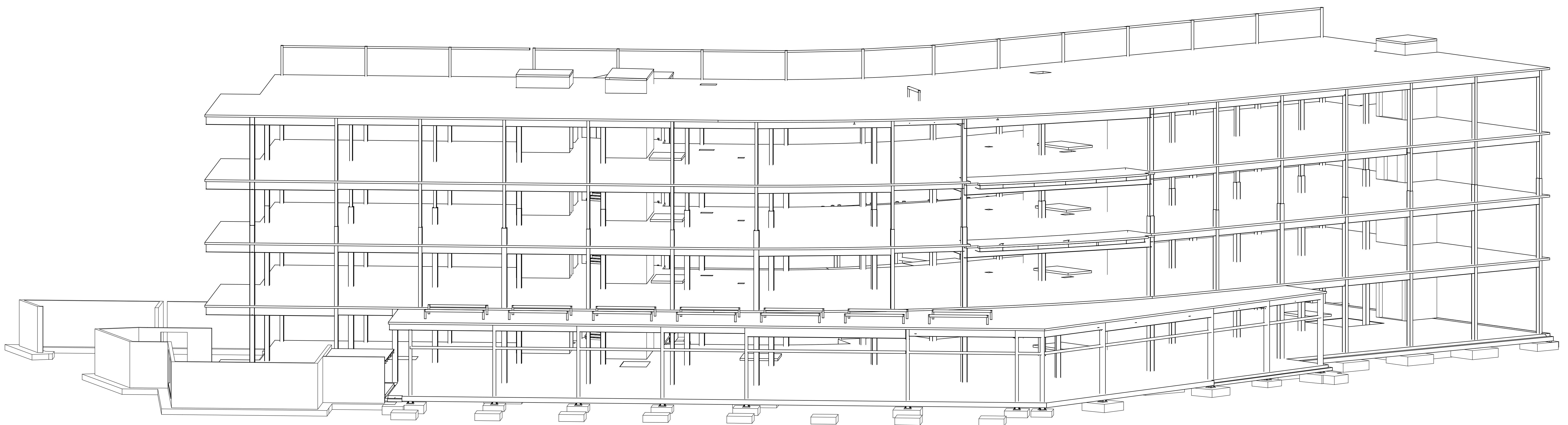
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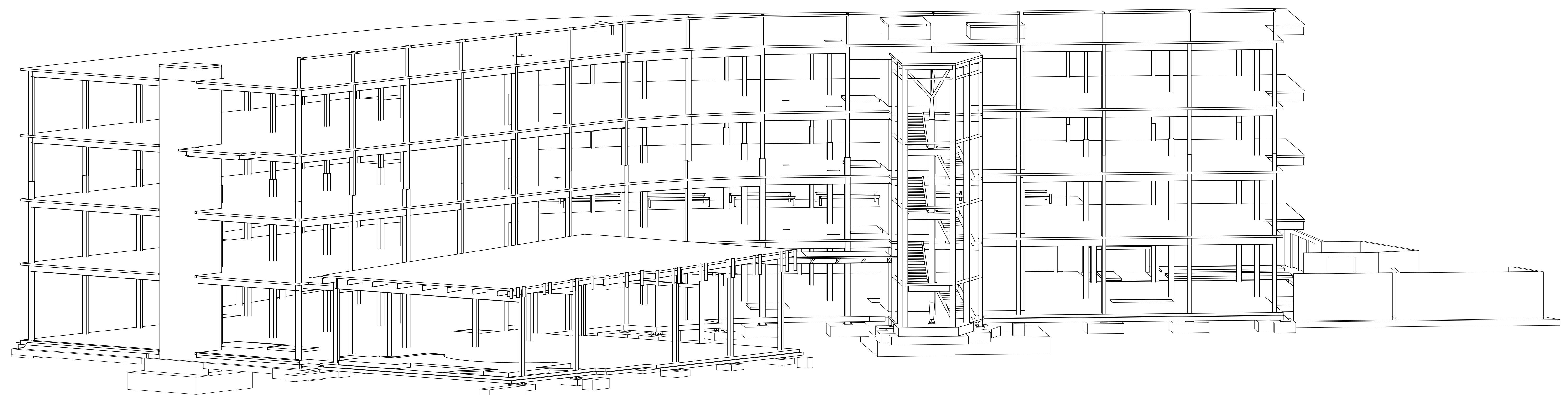
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1 SOUTH FACE OF BUILDING



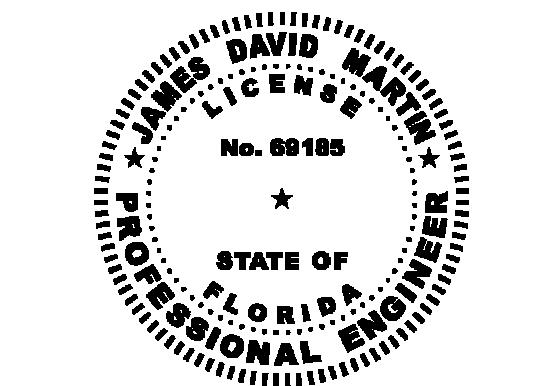
2 NORTH FACE OF BUILDING

**Sarasota County  
Administration Center**

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:



James David Martin, P.E., FL P.E. No. 66185  
WPM Project No. S05-22040-00  
Certificate of Authorization No. 3818

To the best of the Engineer's knowledge, the plans and specifications comply with all applicable laws, including codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 663 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S000**  
COVER SHEET

THESE PERSPECTIVE VIEWS ARE TO HELP VISUALIZATION AND  
UNDERSTANDING OF THE STRUCTURE, AND SHOULD NOT BE  
USED TO DEFINE SCOPE OR DETERMINE QUANTITIES.



## GENERAL STRUCTURAL NOTES

### PART X - MISCELLANEOUS (CONTINUED)

#### F. RESPONSIBILITY OF THE CONTRACTOR FOR CONSTRUCTION LOADS

1. THE STRUCTURE HAS BEEN DESIGNED FOR THE LOADS IDENTIFIED WITHIN THE DRAWINGS. LOADS THAT ARE NOT APPLIED TO THE FINAL STRUCTURE ON CONSTRUCTION AND OCCUPANCY, CONTRACTOR SHALL NOT OVERLOAD THE STRUCTURE DURING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING THE ADEQUACY OF THE STRUCTURE TO SUPPORT ANY APPLIED CONSTRUCTION LOADS, INCLUDING THOSE DUE TO CONSTRUCTION ACTIVITIES, SUCH AS SCAFFOLDING, FORMING, BRACED SHORING OR RESHORING, OR ANY OTHER CONSTRUCTION ACTIVITY. THE CONTRACTOR SHALL SUBMIT CALCULATIONS SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED VERIFYING THE ADEQUACY OF THE STRUCTURE TO SUPPORT CONSTRUCTION LOADS THAT ARE IN EXCESS OF THE STATED DESIGN LOADS. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE TO DESIGN OR CHECK THE STRUCTURE FOR LOADS APPLIED TO THE STRUCTURE FOR ANY CONSTRUCTION ACTIVITY.

#### G. CONTRACTOR SUBSTITUTIONS

1. ANY MATERIALS OR PRODUCTS SUBMITTED FOR APPROVAL THAT ARE DIFFERENT FROM THE MATERIALS OR PRODUCTS SPECIFIED IN THE STRUCTURAL CONTRACT DOCUMENTS WILL BE APPROVED ONLY IF THE FOLLOWING CRITERIA ARE SATISFIED:
  - a. A COST SAVINGS TO THE OWNER DOCUMENTED AND SUBMITTED WITH THE REQUEST.
  - b. THE MATERIAL OR PRODUCT HAS BEEN APPROVED BY THE INTERNATIONAL CODE COUNCIL (ICC) AND THE ICC REPORT IS SUBMITTED WITH THE REQUEST.
  - 1) THE FABRICATOR THAT IS SUBMITTED MUST REFERENCE THE BUILDING CODE UNDER WHICH THE PROJECT IS PERMITTED.
  - 2) ICC REPORTS THAT HAVE BEEN DISCONTINUED AT THE TIME OF PRODUCT INSTALLATION WILL NOT BE ACCEPTED.
2. SUBMITTALS NOT SATISFYING THE ABOVE CRITERIA WILL NOT BE CONSIDERED.

#### H. ELEVATOR GUIDE RAIL SUPPORTS

1. THE GENERAL CONTRACTOR SHALL PROVIDE INTERMEDIATE GUIDE RAIL SUPPORTS FOR ELEVATOR CAB RAILS AND COUNTERWEIGHT RAILS WHEREVER THE FLOOR TO FLOOR HEIGHT EXCEEDS 14'-0".

#### I. MECHANICAL EQUIPMENT WEIGHTS

1. THE GENERAL CONTRACTOR SHALL SUBMIT ACTUAL WEIGHTS OF EQUIPMENT TO BE USED IN THE PROJECT TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOADS USED IN THE DESIGN AT LEAST THREE WEEKS PRIOR TO FABRICATION AND CONSTRUCTION OF THE SUPPORTING STRUCTURE.

#### J. COLUMN SHORTENING

1. THE STRUCTURAL STEEL FABRICATOR SHALL MAKE THE ADJUSTMENTS (OVERLENGTH) IN THE FABRICATED COLUMN LENGTHS BETWEEN THE FLOORS INDICATED AS REQUIRED FOR DIFFERENTIAL COLUMN SHORTENING AS SPECIFIED ON THE DRAWINGS.

#### K. THE STRUCTURE'S ROLE DURING CONSTRUCTION

1. THE ENGINEER SHALL NOT HAVE CONTROL NOR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONSTRUCTION. THESE ARE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
2. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF WALTER P. MOORE AND ASSOCIATES IS SOLELY FOR THE PURPOSE OF BECOMING GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE WORK COMPLETED, AND DETERMINING WHETHER THE WORK OBSERVED IS BEING PERFORMED IN A MANNER INDICATING THAT THE WORK, WHEN FULLY COMPLETED, WILL BE IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE WORK ON A DAILY BASIS. THE OWNER MAY MAKE FURTHER PERIODIC AND EFFORT TO KEEP THE OWNER REASONABLY INFORMED ABOUT THE PROGRESS AND QUALITY OF THE PORTION OF THE STRUCTURE COMPLETED.

#### L. MAINTENANCE STATEMENT

1. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXTEND LIFESPAN AND TO ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ELEMENT. A PERIODIC PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE BUILDING OWNER. THIS PROGRAM SHALL INCLUDE SUCH ITEMS SUCH AS BUT NOT LIMITED TO PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATING FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO A SALT ENVIRONMENT OR OTHER HARSH CHEMICALS.

#### M. SPECIALTY ENGINEER REQUIREMENTS

1. THE FLORIDA STATE BOARD OF PROFESSIONAL ENGINEERS HAS ISSUED RULES ON RESPONSIBILITIES OF PROFESSIONAL ENGINEERS CONCERNING THE DESIGN OF STRUCTURES, PURSUANT TO CHAPTER 61G15-31 OF THE FLORIDA ADMINISTRATIVE CODE. CERTAIN COMPONENTS OF THE STRUCTURE REQUIRE THE WORK OF A SPECIALTY ENGINEER FOR THE DESIGN. ALL PROCEDURES STATED IN THE RULES SHALL APPLY TO THESE SPECIALTY COMPONENTS.

### PART XI - TERMITE CONTROL

- A. TERMITE CONTROL SHALL BE PROVIDED IN ACCORDANCE WITH FLORIDA BUILDING CODE SECTION 1816 AND PROJECT SPECIFICATIONS SECTION 33116.16 "TERMITE CONTROL".
- B. UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED BY THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

### PART XII - DRAWING INTERPRETATION

#### A. DRAWING VIEWS LABELED AS "TYPICAL"

1. PARTIAL PLANS, ELEVATIONS, SECTIONS, DETAILS, OR SCHEDULES LABELED WITH "TYPICAL" AT THE BEGINNING OF THEIR TITLE SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY SHOWN. THE APPLICABILITY OF THE CONTENT OF THESE VIEWS TO LOCATIONS WHERE THEY CAN BE DETERMINED FROM THE TITLE OF THE VIEWS, SUCH AS ETC., SHALL ALSO BE NOTED. WHETHER OR NOT THEY ARE KEYED IN AT EACH LOCATION, DECISIONS REGARDING APPLICABILITY OF THESE "TYPICAL" VIEWS SHALL BE DETERMINED BY THE STRUCTURAL ENGINEER.

#### B. STRUCTURAL ABBREVIATIONS, SYMBOLS, AND NOTATIONS

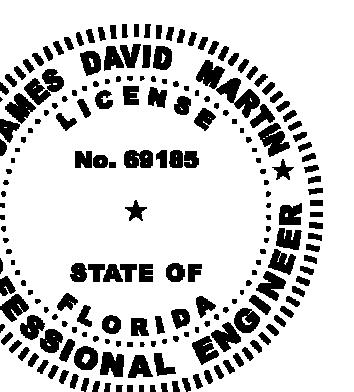
1. REFER TO SHEET S004 FOR ABBREVIATIONS, SYMBOLS, AND NOTATIONS USED ON THE STRUCTURAL DRAWINGS.

**Sarasota County  
Administration Center**

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Revisions:



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**CONFORMED SET**  
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**S002**  
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NOTES

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## THRESHOLD INSPECTION PLAN

PART I - GENERAL		PART III - FOUNDATIONS		PART IV - REINFORCED CONCRETE (CONTINUED)		PART VI - STRUCTURAL STEEL (CONTINUED)	
A.	PURPOSE 1. THE PURPOSE OF THIS THRESHOLD INSPECTION PLAN, AS REQUIRED BY FLORIDA STATUTE CHAPTER 553 AND THE FLORIDA BUILDING CODE, IS TO PROVIDE SPECIAL INSPECTION PROCEDURES AND SCHEDULES SO THAT THE BUILDING CAN BE ADEQUATELY INSPECTED FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS.	A.	SHALLOW FOUNDATIONS 1. THE SPECIAL INSPECTOR MUST BE PRESENT FULL TIME DURING THE ENTIRE PLACEMENT OF THE FIRST TWO MAJOR FOUNDATION CONCRETE POURS AND THEN MUST BE PRESENT AT THE START OF ALL OTHER CONCRETE POURS. 2. OBSERVE THAT THE OWNER'S GEOTECHNICAL ENGINEER IS MONITORING THE SUB-SURFACE DENSIFICATION. 3. OBSERVE THAT FORMS ARE PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT, AND LUBRICATED FOR REMOVAL. 4. VERIFY FOUNDATION DIMENSIONS. 5. VERIFY ANCHOR RODS AND/OR DOWELS ARE INSTALLED WITH THE PROPER LAP SPICE LENGTHS AND IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. 6. VERIFY PLACEMENT OF REINFORCEMENT AND OBSERVE CONCRETING OPERATIONS AS OUTLINED IN THE REINFORCED CONCRETE SECTION OF THIS INSPECTION PLAN.	3. VERIFY THAT DEBRIS AND FOREIGN MATERIALS HAVE BEEN REMOVED BEFORE CONCRETE IS PLACED. 4. PERIODICALLY INSPECT CONCRETE UPON ARRIVAL TO VERIFY THE FOLLOWING: a. PROPER CONCRETE MIX NUMBER, TYPE OF CONCRETE, AND CONCRETE STRENGTH FOR THE PLACEMENT LOCATION. b. REPORT CONCRETE NOT MEETING THE SPECIAL REQUIREMENTS FOR THE PLACEMENT LOCATION TO THE BATCH PLANT INSPECTOR, ARCHITECT, ENGINEER OF RECORD, AND OWNER. 5. VERIFY THAT THE CONTRACTOR IS FOLLOWING APPROPRIATE CONCRETING PRACTICES AT THE POINT OF PLACEMENT AS DEFINED BELOW: a. VERIFY THAT THE CONCRETE IS NOT OVER 90 MINUTES OLD AT THE TIME OF PLACEMENT. b. VERIFY THAT HOT-ATHER OR COLD-WEATHER TECHNIQUES ARE BEING USED AS REQUIRED. c. VERIFY THAT CONCRETE IS BEING DEPOSITED IN UNIFORM, THAT THE VERTICAL DROP DOES NOT EXCEED SIX FEET, AND THAT IT IS NOT PERMITTED TO DROP FRESH OVER REINFORCEMENT CAUSING SEPARATION. d. VERIFY THAT THE CONCRETE IS PROPERLY VIBRATED. e. VERIFY THAT EMBEDDED ITEMS AND REINFORCING STEEL ARE NOT ADVERSELY ALTERED DURING PLACEMENT. NOTE IF ANYTHING WAS DISPLACED OR OTHERWISE ALTERED DURING PLACEMENT. f. VERIFY THAT THERE ARE NO COLD JOINTS WITHIN THE AREA OF THE POUR.	7. PERIODICALLY INSPECT THE STEEL FRAME FOR SUCH ITEMS AS BRACING, STIFFENERS, MEMBER LOCATIONS, AND JOINT DETAILS AT EACH CONNECTION FOR COMPLIANCE WITH CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS. 8. ENDEAVOR TO GUARD THE OWNER AGAINST THE CONTRACTOR CUTTING, GRINDING, REAMING, OR MAKING ANY OTHER FIELD MODIFICATION TO STRUCTURAL STEEL WITHOUT THE PRIOR APPROVAL OF THE ENGINEER OF RECORD. REPORT ANY NOTED UNAUTHORIZED MODIFICATIONS TO THE ENGINEER OF RECORD.	B.	CONNECTIONS 1. VISUALLY CHECK THAT ALL CONNECTIONS ARE COMPLETED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND THE APPROVED SHOP DRAWINGS. 2. WELD CONNECTIONS a. OBSERVE THAT PROPER WELDING PROCEDURES ARE BEING IMPLEMENTED. b. VERIFY THAT WELDS ARE INSPECTED AND TESTED BY THE TESTING LABORATORY. 3. BOLTED CONNECTIONS a. VERIFY BOLT AND WASHER SIZES AND TYPES THAT ARE BEING INSTALLED. b. VERIFY THE TIGHTENING METHOD FOR HIGH STRENGTH BOLTS AND THAT IMPACT WRENCHES ARE BEING PROPERLY CALIBRATED. c. VISUALLY CHECK THAT BOLTS ARE BEING TIGHTENED PROPERLY.
B.	QUALIFICATIONS 1. ONLY ENGINEERS CERTIFIED BY THE STATE OF FLORIDA AS SPECIAL INSPECTORS OF THRESHOLD BUILDINGS, IN ACCORDANCE WITH FLORIDA STATUTE CHAPTER 471, CAN ASSUME THE RESPONSIBILITIES OF A SPECIAL INSPECTOR. 2. IN ACCORDANCE WITH FLORIDA STATUTE CHAPTER 553, THE ARCHITECT OR ENGINEER OF RECORD MAY ACT AS THE SPECIAL INSPECTOR PROVIDED SHE OR HE IS ON THE BOARD OF PROFESSIONAL ENGINEERS' OR THE BOARD OF ARCHITECTURE AND INTERIOR DESIGN'S LIST OF PERSONS QUALIFIED TO BE SPECIAL INSPECTORS. 3. THE SPECIAL INSPECTOR SHALL BE PERMITTED TO SEND AN AUTHORIZED REPRESENTATIVE TO THE JOB SITE TO PERFORM THE NECESSARY INSPECTIONS PROVIDED ALL REQUIRED WRITTEN REPORTS ARE PREPARED BY BEAR THE SEAL OF THE SPECIAL INSPECTOR. 4. THE SPECIAL INSPECTOR'S AUTHORIZED REPRESENTATIVE SHALL BE QUALIFIED BY EDUCATION OR LICENSURE TO PERFORM THE DUTIES ASSIGNED BY THE SPECIAL INSPECTOR. THE QUALIFICATIONS SHALL INCLUDE ONE OF THE FOLLOWING: LICENSURE AS A PROFESSIONAL ENGINEER OR ARCHITECT; GRADUATE FROM AN APPROVED COLLEGE OF PROFESSIONAL ARCHITECTURE OR STRUCTURAL ENGINEERING; GRADUATION FROM AN ARCHITECTURAL EDUCATION PROGRAM; SUCCESSFUL COMPLETION OF THE NCEES FUNDAMENTALS EXAMINATION; LICENSURE AS A BUILDING INSPECTOR PER CHAPTER 468, F.S.; OR LICENSURE AS A GENERAL CONTRACTOR PER CHAPTER 489, F.S. 5. THE TERM "SPECIAL INSPECTOR" IN THE BALANCE OF THIS INSPECTION PLAN REFERS TO THE SPECIAL INSPECTOR AND/OR THE SPECIAL INSPECTOR'S AUTHORIZED REPRESENTATIVE.	B.	FOUNDATION WALLS AND PITS 1. THE SPECIAL INSPECTOR MUST BE PRESENT FULL TIME DURING THE ENTIRE PLACEMENT OF THE FIRST TWO MAJOR WALL OR PIT CONCRETE POURS AND THEN MUST BE PRESENT AT THE START OF ALL OTHER CONCRETE POURS. 2. OBSERVE THAT FORMS ARE PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT, AND LUBRICATED FOR REMOVAL. 3. VERIFY FOUNDATION WALL AND PIT DIMENSIONS. 4. VERIFY PLACEMENT OF REINFORCEMENT AND OBSERVE CONCRETING OPERATIONS AS OUTLINED IN THE REINFORCED CONCRETE SECTION OF THIS INSPECTION PLAN. 5. VERIFY THAT WALLS ARE NOT BACKFILLED UNTIL FLOOR CONSTRUCTION AT TOP OF WALL IS COMPLETE OR TEMPORARY BRACING IS PROVIDED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.	D.	COLUMNS, WALLS, AND SHEAR WALLS 1. THE SPECIAL INSPECTOR SHALL PERIODICALLY INSPECT 25% OF THE CONCRETE PLACEMENT AS IT OCCURS. 2. CHECK ALL REINFORCEMENT BEFORE VERTICAL FORMS ARE PLACED AS OUTLINED IN THE REINFORCING STEEL SECTION. 3. OBSERVE THAT FORMS ARE PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT, LUBRICATED FOR REMOVAL, AND CONFORM TO APPROVED SHOP DRAWINGS. 4. VERIFY PROPER DIMENSIONS AND ORIENTATION. 5. FOR FLAT SLAB CONSTRUCTION, VERIFY THAT TOP OF COLUMN OR WALL ELEVATION IS SET 1/2 INCH BELOW THE FUTURE SLAB SOFFIT, OR AS SHOWN ON THE CONTRACT DOCUMENTS. 6. VERIFY WALL OPENINGS AND SLEEVES AS FOLLOWS: a. CORRECT SIZE AND LOCATION AS SHOWN ON THE CONTRACT DOCUMENTS. b. CHECK PLACEMENT OF ADDITIONAL REINFORCEMENT AROUND OPENINGS. c. REPORT ALL WALL OPENINGS LARGER THAN 12" THAT ARE NOT SHOWN ON THE CONTRACT DOCUMENTS. 7. VERIFY THAT ALL DEBRIS IS REMOVED FROM WITHIN THE FORMS PRIOR TO CONCRETE PLACEMENT.	E.	ERCTION 1. OBTAIN A COPY OF THE APPROVED ERECTION PLAN THAT HAS BEEN SIGNED AND SEALED BY A PROFESSIONAL ENGINEER IN THE STATE OF FLORIDA. 2. VERIFY THAT STEEL IS ERECTED IN ACCORDANCE WITH THE APPROVED ERECTION PLAN. 3. VERIFY THAT TEMPORARY BRACING IS INSTALLED AND IS IN CONFORMANCE WITH THE APPROVED ERECTION DRAWINGS. 4. VISUALLY CHECK PLUMBNESS AND FRAME ALIGNMENT AS ERECTION PROCEEDS. 5. VERIFY THAT SURVEYS ARE OCCURRING AS REQUIRED BY THE CONTRACT DOCUMENTS TO CHECK PLUMBNESS AND FRAME ALIGNMENT AS ERECTION PROGRESSES. REVIEW THE SUBMITTED SURVEY REPORT.
C.	SPECIAL INSPECTOR'S RESPONSIBILITIES 1. THE SPECIAL INSPECTOR MUST DEDICATE THEIR TIME ON-SITE TO THE INSPECTION REQUIREMENTS OF THE INSPECTION PLAN. THEY SHALL COORDINATE WITH THE CONTRACTOR TO ALLOW ENOUGH TIME TO PERFORM THE INSPECTIONS AS PROVIDED IN THIS PLAN. 2. THE SPECIAL INSPECTOR SHALL CONTACT THE ENGINEER AT LEAST ONCE A WEEK TO DISCUSS DETAILS ON THE PROJECT AND TO CONFIRM THEY HAVE REVIEWED ALL APPROVED RFIS, SUPPLEMENTAL SKETCHES, ETC. 3. THE SPECIAL INSPECTOR SHALL COOPERATE WITH THE CONTRACTOR AND SHALL REFRAIN FROM DIRECTING WORK, AS THIS IS EXPRESSLY NOT PART OF THE THRESHOLD INSPECTION FUNCTION. 4. THE SPECIAL INSPECTOR MAY NOT SERVE AS A SURROGATE IN CARRYING OUT THE RESPONSIBILITIES OF THE BUILDING OFFICIAL, ARCHITECT, OR ENGINEER OF RECORD. 5. THE SPECIAL INSPECTOR SHALL NOT PERFORM OTHER CONSTRUCTION RELATED WORK ON THE PROJECT, INCLUDING MATERIALS TESTING SERVICES.	C.	SLABS-ON-GRADE 1. THE SPECIAL INSPECTOR MUST BE PRESENT FULL TIME DURING THE ENTIRE PLACEMENT OF THE FIRST TWO MAJOR SLAB-ON-GRADE CONCRETE POURS AND THEN MUST BE PRESENT AT THE START OF ALL OTHER CONCRETE POURS. 2. OBSERVE SUBGRADE PREPARATION INCLUDING BACKFILLING, COMPACTION, AND PERFORMANCE OF COMPACTION TESTS BY THE TESTING LABORATORY PRIOR TO CONCRETE PLACEMENT. 3. NOTE ANY ALTERATION AND SUBSEQUENT REPLACEMENT OF SUBGRADE MATERIALS REQUIRED BY OTHER TRADES. 4. VERIFY THAT THE MOISTURE RETARDER OR VAPOR BARRIER IS PROVIDED, IS LAPPED PROPERLY, AND IS NOT TORN OR PUNCTURED. 5. OBSERVE THAT FORMWORK AT TURNDOWS AND SLAB EDGES IS PLUMB AND STRAIGHT, BRACED AGAINST MOVEMENT AND LUBRICATED FOR REMOVAL. 6. OBSERVE PLACEMENT OF SCREWS TO OBTAIN PROPER LEVEL AND THICKNESS OF SLABS. OBSERVE LOCATION OF SLAB DEPRESSIONS AND STEPS IN SLAB WHILE MAINTAINING REQUIRED SLAB THICKNESS. 7. VERIFY THE POUR AREA IS FREE OF STANDING WATER OR OTHER DEBRIS. 8. VERIFY PLACEMENT OF REINFORCEMENT AND OBSERVE CONCRETING OPERATIONS AS OUTLINED IN THE REINFORCED CONCRETE SECTION OF THIS INSPECTION PLAN. 9. CHECK THAT THE LOCATION AND TYPE OF SLAB CONTROL JOINTS AND CONSTRUCTION JOINTS CONFORM TO THE CONTRACT DOCUMENTS. 10. VERIFY THAT SAWCUT CONTROL JOINTS ON SLABS-ON-GRADE ARE CUT WITHIN 12 HOURS OF PLACEMENT.	F.	HORIZONTAL FRAMING 1. THE SPECIAL INSPECTOR SHALL BE PRESENT DURING THE PLACEMENT OF ALL CONCRETE ON HORIZONTAL FORMWORK. CHECK ALL REINFORCEMENT AS OUTLINED IN THE REINFORCING STEEL SECTION. 2. VERIFY THE PROPER DIMENSIONS OF GIRDERS, BEAMS AND JOISTS. 3. VERIFY THAT THE SLAB THICKNESS AND TOP OF SLAB ELEVATION IS CORRECT. 4. VERIFY OPENINGS AND SLEEVES AS FOLLOWS: a. CORRECT SIZE AND LOCATION AS SHOWN ON THE CONTRACT DOCUMENTS. b. CHECK PLACEMENT OF ADDITIONAL REINFORCEMENT AROUND OPENINGS. c. REPORT ALL SLAB OPENINGS LARGER THAN 12" THAT ARE NOT SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS. d. NO SLEEVES OR OPENINGS WILL BE PERMITTED IN BEAMS WITHOUT THE PRIOR APPROVAL OF THE ENGINEER OF RECORD. 5. CHECK THAT ALL DEBRIS AND FOREIGN MATERIAL HAS BEEN REMOVED BEFORE CONCRETE IS PLACED.	G.	COMPOSITE DECK 1. VISUALLY INSPECT THE DECK FOR DAMAGE DURING SHIPPING. 2. VERIFY THAT THE DECK DEPTH, GAGE, TYPE OR PROPERTIES, AND FINISH COMPLY WITH THE CONTRACT DOCUMENTS. 3. VERIFY THAT THE DECK ATTACHMENT TO THE SUPPORTING STEEL IS AS SPECIFIED IN THE CONTRACT DOCUMENTS.
D.	CONTRACTOR'S RESPONSIBILITIES 1. THE CONTRACTOR SHALL BE FAMILIAR WITH THE SCOPE OF THE INSPECTIONS THAT WILL BE PERFORMED AND ALSO THE REQUIREMENTS OF THE CONTRACTOR AS DESCRIBED HEREIN, PARTICULARLY REGARDING FORMWORK DESIGN AND INSPECTIONS. 2. THE CONTRACTOR SHALL COOPERATE WITH AND ASSIST THE SPECIAL INSPECTOR IN PERFORMING THEIR INSPECTION DUTIES AND SHALL PROVIDE FREE ACCESS TO THE PROJECT AT ALL TIMES. 3. THE CONTRACTOR SHALL COORDINATE THE SCHEDULE OF WORK TO ACCOMMODATE THE REQUIRED INSPECTIONS. THEY SHALL ADVISE THE SPECIAL INSPECTOR OF ADJUSTMENTS TO THE SCHEDULE SO AS TO PLANNED OPERATIONS IN ORDER TO ASSURE TIMELY AND APPROPRIATE INSPECTION. A MINIMUM OF TWENTY-FOUR (24) HOURS NOTICE IS REQUIRED. 4. THESE INSPECTIONS SHALL NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITIES TO CARRY OUT THEIR OWN QUALITY CONTROL INSPECTIONS AND TESTING. THE CONTRACTOR'S CONTRACTUAL OR STATUTORY OBLIGATIONS ARE NOT RELIEVED BY ANY ACTION OF THE SPECIAL INSPECTOR.	G.	VERTICAL FRAMING 1. THE SPECIAL INSPECTOR SHALL BE PRESENT DURING THE PLACEMENT OF ALL CONCRETE ON VERTICAL FORMWORK. CHECK ALL REINFORCEMENT AS OUTLINED IN THE REINFORCING STEEL SECTION.	H.	ROOF DECK 1. VISUALLY INSPECT THE DECK FOR DAMAGE DURING SHIPPING. 2. VERIFY THAT THE DECK DEPTH, GAGE, TYPE OR PROPERTIES, AND FINISH COMPLY WITH THE CONTRACT DOCUMENTS. 3. VERIFY THAT THE DECK ATTACHMENT TO THE SUPPORTING STEEL IS AS SPECIFIED IN THE CONTRACT DOCUMENTS.		
E.	OWNER'S RESPONSIBILITIES 1. THE OWNER SHALL RETAIN A SPECIAL INSPECTOR TO PERFORM THE INSPECTION SERVICES AS DESCRIBED HEREIN. 2. THE OWNER SHALL PAY ALL COSTS OF EMPLOYING A SPECIAL INSPECTOR, BUT THE SPECIAL INSPECTOR SHALL BE RESPONSIBLE TO THE ENFORCEMENT AGENCY. 3. THE OWNER SHALL RETAIN A GEOTECHNICAL ENGINEER TO PERFORM ALL SOIL RELATED TESTS AND INSPECTIONS, INCLUDING BUT NOT LIMITED TO THE FOLLOWING: a. SOIL EXCAVATION, FILL, AND COMPACTION. b. SOIL IMPROVEMENT (STONE COLUMN) INSTALLATION: 1.) PROBE POWER CONSUMPTION. 2.) HOLE PENETRATION DEPTH AND AGGREGATE CONSUMED. 3.) PLATE LOAD TEST TO VERIFY LOAD SETTLEMENT CHARACTERISTICS. 4.) STANDARD PENETRATION TESTS (SPT) BORINGS AS REQUIRED TO EVALUATE THE EFFECTIVENESS OF SOIL IMPROVEMENT.	H.	CONSTRUCTION JOINTS 1. VERIFY THE LOCATION OF VERTICAL AND HORIZONTAL CONSTRUCTION JOINTS FOR COMPLIANCE WITH THE CONSTRUCTION JOINT LOCATION PLAN SUBMITTED BY THE CONTRACTOR TO THE ARCHITECT AND ENGINEER OF RECORD.	I.	POST-INSTALLED ANCHORS AND DOWELS 1. INSPECT POST-INSTALLED ANCHOR INSTALLATION AT THE FREQUENCY NOTED IN THE CONTRACT DOCUMENTS AND IN ACCORDANCE WITH THE PUBLISHED, CURRENTLY VALID, EVALUATION SERVICE REPORT (ESR) FOR EACH ANCHOR PRODUCT. FREQUENCIES ARE DEFINED AS FOLLOWS: a. PERIODIC: VERIFY INITIAL INSTALLATION OF POST-INSTALLED AND OVERLAPPED CONCRETE FOR EACH INDIVIDUAL INSTALLER WITH EACH INDIVIDUAL ANCHOR PRODUCT IN ACCORDANCE WITH THE REQUIREMENTS STATED BELOW FOR EACH TYPE OF ANCHOR. PERIODICALLY INSPECT ANCHOR INSTALLATION AND IN-PLACE VERIFICATION. b. CONTINUOUS: VERIFY EACH INSTALLATION OF POST-INSTALLED ANCHORS IN CONCRETE IN ACCORDANCE WITH THE REQUIREMENTS STATED BELOW FOR EACH TYPE OF ANCHOR.		
F.	FREQUENCY OF INSPECTION 1. THE SPECIAL INSPECTOR SHALL PERFORM SITE VISITS AT A FREQUENCY DETERMINED BY HIM TO SATISFY HIMSELF THAT THE INSPECTIONS ARE BEING PERFORMED IN ACCORDANCE WITH THIS PLAN.	I.	2. VERIFY THAT THE ANCHOR IS INSTALLED IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AS WELL AS THE FOLLOWING DESIGN REQUIREMENTS: a. VERIFY THE FOLLOWING ARE IN COMPLIANCE WITH THE CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS: 1.) ANCHOR PRODUCT TYPE 2.) MANUFACTURER 3.) MATERIAL GRADE 4.) ANCHOR LENGTH 5.) ANCHOR LENGTH AND INSTALLED EMBEDMENT DEPTH 6.) HOLE DIAMETER AND DEPTH 7.) HOLE PREPARATION (CLEANING PROCEDURE AND CLEANLINESS) 8.) EDGE DISTANCES AND SPACING	J.	STRUCTURAL STEEL 1. FRAMING a. VISUALLY INSPECT STEEL AS IT IS RECEIVED FOR POSSIBLE DAMAGE IN SHIPPING, WORKMANSHIP, AND PIECE MARKING. b. VERIFY THAT STEEL MEMBER SIZES AND STEEL GRADE CONFORM TO THE CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS.		
G.	DISCREPANCIES AND DEVIATIONS 1. THE SPECIAL INSPECTOR SHALL IMMEDIATELY ALERT THE CONTRACTOR OF ALL DISCREPANCIES AND DEVIATIONS FROM THE CONTRACT DOCUMENTS. 2. THE CONTRACTOR SHALL, UPON BEING INFORMED BY THE SPECIAL INSPECTOR, IMMEDIATELY CAUSE TO ELIMINATE SUCH DISCREPANCIES AND DEVIATIONS.	J.	2. CHECK THE INSTALLATION OF ANCHOR RODS FOR CONFORMITY WITH THE CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS FOR THE FOLLOWING: a. LOCATION b. SIZE, QUANTITY, AND MATERIAL GRADE c. EMBEDMENT LENGTH INTO THE FOUNDATION OR SUPPORTING MEMBER.	K.	CONCRETE PLACEMENT 1. THE SPECIAL INSPECTOR SHALL BE PRESENT DURING THE PLACEMENT OF CONCRETE AT A FREQUENCY AS INDICATED IN OTHER SECTIONS OF THIS INSPECTION PLAN.		
H.	NON-COMPLIANCE AND CORRECTIONS LOG 1. THE SPECIAL INSPECTOR SHALL KEEP AN EXCEPTIONS AND CORRECTIONS LOG FOR FOLLOW-UP. 2. THE LOG SHALL INCLUDE, FOR EACH NON-COMPLIANCE ITEM, THE DATE IT WAS NOTED, THE CORRECTIVE ACTION TAKEN, AND THE DATE IT WAS RECTIFIED. 3. THIS LOG SHALL BE REVIEWED ON A DAILY BASIS AND UPDATED AS EXCEPTIONS ARE RECTIFIED.	K.	2. VERIFY THE FOLLOWING REGARD TO THE TESTING LABORATORY: a. THE TESTING LABORATORY SUPPORT STAFF HAS BEEN INFORMED AND WILL BE AVAILABLE TO MAKE TESTS AS REQUIRED. b. SLUMP TEST IS MADE AT THE POINT OF DISCHARGE FOR EACH DAY AND AS OF CONCRETE PLACEMENT.	L.	2. REVIEW THAT THE CAMBER OF STEEL BEAMS COMPLIES WITH THE CONTRACT DOCUMENTS AND APPROVED SHOP DRAWINGS.		
C.	LETTER OF COMPLETION 1. IN ACCORDANCE WITH SECTION 553.79 (7A) OF THE FLORIDA STATUTES, THE SPECIAL INSPECTOR SHALL SUBMIT A LETTER OF COMPLETION AND PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY, FILE A SIGNED AND SEALED STATEMENT WITH THE ENFORCEMENT AGENCY IN SUBSTANTIALLY THE FOLLOWING FORM: TO THE BEST OF MY KNOWLEDGE AND BELIEF, THE CONSTRUCTION OF ALL STRUCTURAL LOAD-BEARING COMPONENTS DESCRIBED IN THE THRESHOLD INSPECTION PLAN COMPLIES WITH THE CONTRACT DOCUMENTS.	L.	3. INSPECT 100% OF THE COLUMN COMPRESSION AND BASE JOINTS FOR VERTICALITY AND PLUMBNESS. GAPS LESS THAN 1/16 INCH BUT GREATER THAN 1/4 INCH. a. GAPS GREATER THAN 1/16 INCH BUT LESS THAN 1/4 INCH SHALL BE REPORTED TO THE ENGINEER OF RECORD FOR ASSESSMENT. b. ALL GAPS GREATER THAN 1/4 INCH SHALL BE SHIMMED WITH NON-TAPERED MILD STEEL SHIMS.	M.	STRUCTURAL STEEL (CONTINUED) 1. CONNECTIONS a. VISUALLY CHECK THAT ALL CONNECTIONS ARE COMPLETED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND THE APPROVED SHOP DRAWINGS.		

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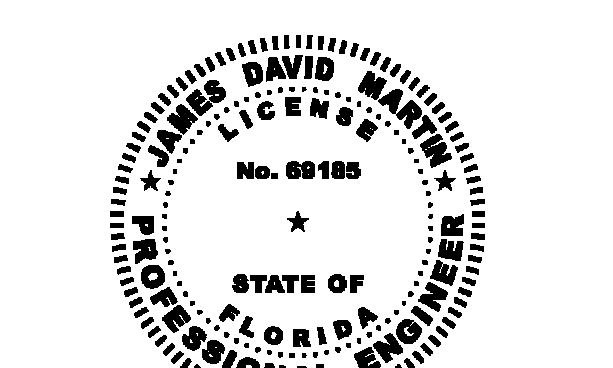
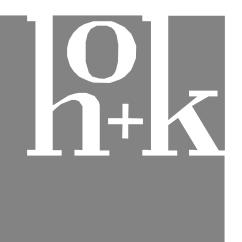
**SIEBEIN**  
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**Sarasota County Administration Center**  
1 Apex Road  
Sarasota, Florida 34240  
Project No. 22.2300.050  
Drawn By BD  
Checked By JDM  
Date 09/08/2023  
Revisions:

James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22045-00  
Certificate of Authorization No. 3818  
To the best of my knowledge, the plans and specifications comply with all applicable laws, codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

**CONFORMED SET**  
02/14/2024  
**S003**  
THRESHOLD  
INSPECTION PLAN

Originals printed at 30" x 42"  
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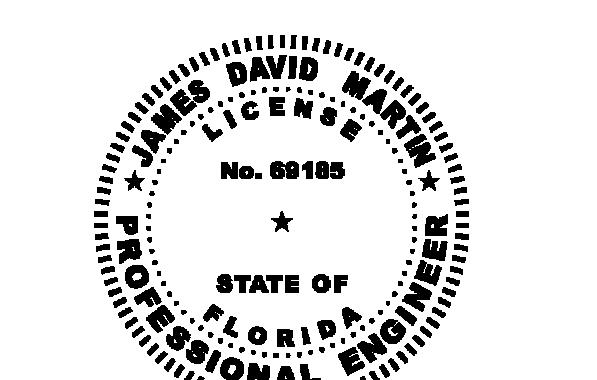


MATERIAL IDENTIFICATION SYMBOLS	PLAN SYMBOLS	STRUCTURAL ELEMENTS TAGS	FOUNDATION ANNOTATION STYLES	STEEL ANNOTATION STYLES
<p>THE FOLLOWING MATERIAL IDENTIFICATION SYMBOLS MAY BE USED IN THE SECTIONS AND DETAILS:</p>	<p>THE FOLLOWING SYMBOLS MAY BE USED THROUGHOUT THE PLANS.</p>	<p>THE FOLLOWING STRUCTURAL ELEMENT TAGS, USED IN CONJUNCTION WITH SCHEDULES, MAY APPEAR ON THE DRAWINGS:</p> <p>Bx DENOTES SLAB OR GROUT REINFORCEMENT TYPE "xx" BMx DENOTES CONCRETE COLUMN TYPE "xx" BPx DENOTES BASE PLATE TYPE "xx" Cxx DENOTES COLUMN TYPE "xx" CFxx DENOTES CONTINUOUS FOOTING TYPE "xx" DPxx DENOTES DRILLED PIER TYPE "xx" EPxx DENOTES EXPANDED PIER TYPE "xx" Fx DENOTES SPREAD FOOTING TYPE "xx" FPxx DENOTES FRAMING PENETRATION TYPE "xx" GBxx DENOTES GRADE BEAM TYPE "xx" Mlx DENOTES CMU WALL TYPE "xx" Px DENOTES PIER TYPE "xx" PCx DENOTES PIPE CAP TYPE "xx" Sxx DENOTES ONE-WAY SLAB TYPE "xx" SRxx DENOTES HEADED SHEAR STUD REINFORCEMENT TYPE "xx" SFxx DENOTES STRIP FOOTING TYPE "xx" Tx DENOTES SLAB TOP REINFORCEMENT TYPE "xx" WFxx DENOTES WALL FOOTING TYPE "xx"</p>	<p>THE FOLLOWING STRUCTURAL ANNOTATIONS MAY APPEAR ON THE FOUNDATION PLANS:</p> <img alt="Foundation annotations: F7.0, C12, +640'-0", "PIER MARK", "COLUMN MARK (IF PROVIDED)", "TOP OF FOOTING ELEVATION (IF PROVIDED)", "R25, W16x26 C=1.25, R30, LEFT END VERTICAL SHEAR REACTION (IN KIPS), BEAM SIZE, BEAM CAMBER, NON-COMPOSITE BEAM", "WALL FOOTING MARK", "WALL MARK (IF PROVIDED)", "STEP IN FOOTING, ALL STEPS WILL BE NOTED", "C4, SR8, COLUMN MARK (IF PROVIDED), HEADED SHEAR STUD REINFORCEMENT MARK (IF PROVIDED), BEAM MARK, BM4 <1'-0", TOP OF CONCRETE BEAM ELEVATION RELATIVE TO TYPICAL TOP OF CONCRETE ELEVATION, TYPICAL PAN SIZE PROVIDED IN PLAN NOTES IS NOT PROVIDED ON PLAN, ATYPICAL PAN SIZE NOTED IN PLAN AT EACH PAN, 4-#4x5'-0" TOP, REINFORCING STEEL NOTED IN PLAN SHALL BE LOCATED ACCORDING TO PLAN NOTES, VERTICAL AXIS, A SINGLE ARROWED LINE INDICATES THE EXTENT OVER WHICH THE REINFORCING STEEL IS TO BE DISTRIBUTED WITH AN EQUAL SPACING BETWEEN BARS, #4x20'-0" BOTTOM BARS @12", A DOUBLE ARROWED LINE INDICATES THAT THE REINFORCING STEEL IS TO BE PROVIDED IN EACH DIRECTION UNTIL ONE OF THE FOLLOWING OCCURS: 1. AN EDGE OF SLAB, 2. A SIGNIFICANT CHANGE IN THE SLAB GEOMETRY, 3. OTHER REINFORCING STEEL OF A SIMILAR NATURE IS SHOWN, SHEAR WALL DESIGNATION, OPENING IN WALL, SUPPORT CENTERLINE, DRAPES AT MIDSPAN ARE PROVIDED BELOW OR TO THE RIGHT OF TENDONS, EFFECTIVE TENDON FORCES (AFTER ALL LOSSES) ARE PROVIDED IN KIPS FOR CONCENTRATED TENDON GROUPS AND IN KIPS PER FOOT FOR UNIFORM TENDONS, DIFFERENT VALUES MAY APPEAR IN DIFFERENT SPANS OF THE SAME TENDON, IF ONLY ONE VALUE IS GIVEN, IT APPLIES FOR THE ENTIRE LENGTH OF THE TENDONS, DRAPES AT TENDON ENDS ARE PROVIDED ABOVE OR TO THE LEFT OF TENDONS, DRAPES AT SUPPORTS ARE PROVIDED ABOVE OR TO THE LEFT OF TENDONS, SUPPORT CENTERLINE, JOIST, LINE OF BRIDGING, LOCATION OF CROSS-BRIDGING (IF PROVIDED), BOTTOM FLANGE BRACE, STEEL BEAM, INDICATOR SYMBOL POINTS AWAY FROM BOTTOM FLANGE OF BEAM BEING BRACED, BRACING ATTACHED DIRECTLY TO FLOOR SYSTEM, TOP OF STEEL ELEVATION RELATIVE TO TYPICAL TOP OF STEEL ELEVATION, A POSITIVE VALUE INDICATES THE STEEL BEAM IS ABOVE THE TYPICAL ELEVATION, BRACED FRAME DESIGNATION, BRACES CONTINUING ABOVE ARE INDICATED BY HIDDEN LINES ABOVE BEAM, BRACES CONTINUING BELOW ARE INDICATED BY HIDDEN LINES BELOW BEAM, JOIST, LINE OF BRIDGING, LOCATION OF CROSS-BRIDGING (IF PROVIDED), BOTTOM FLANGE BRACE, STEEL BEAM, INDICATOR SYMBOL POINTS AWAY FROM BOTTOM FLANGE OF BEAM BEING BRACED, BRACING ATTACHED DIRECTLY TO FLOOR SYSTEM, DRAPES AT MIDSPAN ARE PROVIDED BELOW OR TO THE RIGHT OF TENDONS, EFFECTIVE TENDON FORCES (AFTER ALL LOSSES) ARE PROVIDED IN KIPS FOR CONCENTRATED TENDON GROUPS AND IN KIPS PER FOOT FOR UNIFORM TENDONS, DIFFERENT VALUES MAY APPEAR IN DIFFERENT SPANS OF THE SAME TENDON, IF ONLY ONE VALUE IS GIVEN, IT APPLIES FOR THE ENTIRE LENGTH OF THE TENDONS, DRAPES AT TENDON ENDS ARE PROVIDED ABOVE OR TO THE LEFT OF TENDONS, DRAPES AT SUPPORTS ARE PROVIDED ABOVE OR TO THE LEFT OF TENDONS, SUPPORT CENTERLINE, JOIST, LINE OF BRIDGING, LOCATION OF CROSS-BRIDGING (IF PROVIDED), BOTTOM FLANGE BRACE, STEEL BEAM, INDICATOR SYMBOL POINTS AWAY FROM BOTTOM FLANGE OF BEAM BEING 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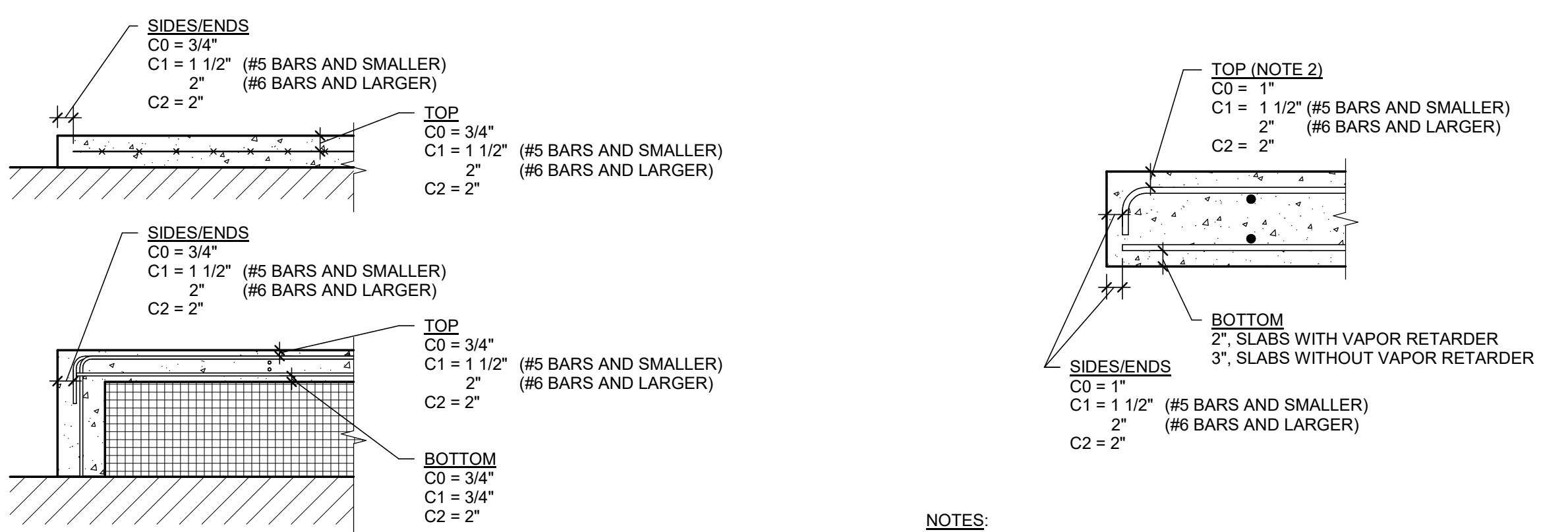
In Association with



James David Martin, P.E., FL PE No. 69185  
WPM Project No. S05-2204-00  
Certificate of Authorization No. 3818

To the best of the Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 503 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024



**NOTES:**  
1. CONCRETE PROFILES AND REINFORCING STEEL CONFIGURATIONS ARE SCHEMATIC AND ARE PROVIDED FOR ESTABLISHING TYPICAL CLEAR CONCRETE COVERS ONLY. REFER TO STRUCTURAL PLANS AND DETAILS FOR ALL OTHER INFORMATION.  
2. ALL COVERS SHOWN ARE CLEAR FROM THE OUTERMOST SURFACE OF REINFORCING STEEL TO THE EXTERIOR SURFACE OF CONCRETE, INCLUDING REVEALS, DRIP GROOVES, OR RUSTICATIONS.  
3. WHERE COVERS ARE DIFFERENT AS A FUNCTION OF BAR SIZE, DETAILEER SHALL ADJUST LOCATION OF TRANSVERSE REINFORCING STEEL AS REQUIRED SUCH THAT CLEAR CONCRETE COVERS ARE AS SHOWN.  
4. C0, C1, AND C2 REFER TO THE CORROSION EXPOSURE CLASS OF THE CONCRETE ELEMENT. REFER TO THE CLASSES OF CONCRETE MATRIX AND GENERAL NOTES FOR ADDITIONAL INFORMATION.

**NOTES:**  
1. REFER TO "NOTES FOR TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL".  
2. REFER TO "NOTES FOR TYPICAL CONSTRUCTION AND CONTROL JOINTS IN SLAB-ON-GRADE" FOR MORE STRINGENT TOP COVER REQUIREMENTS FOR SLABS-ON-GRADE WITH CONSTRUCTION OR CONTROL JOINTS.

### 5 TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL IN TOPPING SLABS

NO SCALE

### 4 TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL IN SLABS-ON-GRADE

NO SCALE

### 3 NOTES FOR TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL

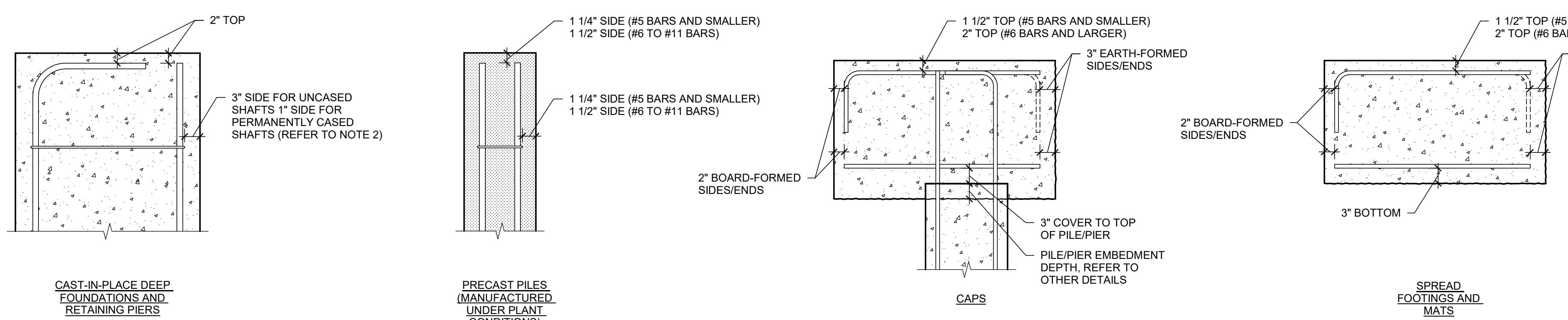
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CLASSES OF CONCRETE MATRIX								
CONCRETE USAGE	MINIMUM COMPRESSIVE STRENGTH (f'c)	CONCRETE TYPE	EXPOSURE CLASSES	MAXIMUM W/C M RATIO	PERMISSIBLE AIR CONTENT	REQUIRED CEMENT REPLACEMENT	MAXIMUM AGGREGATE SIZE	ADDITIONAL REMARKS
SPREAD FOOTINGS	4,000 PSI AT 56 DAYS	NWC	C1	N/A	N/A	40-70%	1-1/2"	
FOUNDATIONS/WALLS	4,000 PSI AT 28 DAYS	NWC	C1	N/A	N/A	25-50%	1"	
SLABS-ON-GRADE, INTERIOR	4,000 PSI AT 28 DAYS	NWC	-	0.45	N/A	0-50%	1-1/2"	
SLABS-ON-GRADE, EXTERIOR	4,000 PSI AT 28 DAYS	NWC	C1	0.45	N/A	0-50%	1-1/2"	
TOPPING SLAB, ALL OTHERS	4,000 PSI AT 28 DAYS	NWC	-	0.45	N/A	0-50%	3/4"	
CONCRETE SHEAR WALLS	5,000 PSI AT 28 DAYS	NWC	C1	N/A	N/A	1"		
MAT FOUNDATIONS BELOW SHEAR WALLS	5,000 PSI AT 28 DAYS	NWC	C1	N/A	N/A	40-70%	1-1/2"	
FOUNDATION PLINTHS	4,000 PSI AT 56 DAYS	NWC	C1	N/A	N/A	40-70%	1-1/2"	
SLABS ON METAL DECKS	3,500 PSI AT 28 DAYS	LWC	-	0.45	4-7%	0-50%	1"	

**NOTES:**  
1. ALL CONCRETE SHALL BE CONSIDERED TO BE IN EXPOSURE CLASS F0, P0, AND C0 ACCORDING TO ACI 318-08 UNLESS NOTED OTHERWISE. THE EXPOSURE CLASS NOTED ON THE DRAWING IS THE MAXIMUM EXPOSURE CLASS FOR THE CONCRETE.  
2. CONCRETE NOTED ABOVE OR ON PLAN TO BE IN EXPOSURE CLASSES F1, F2, F3, S1, S2, S3, P1, C1, OR C2 SHALL BE PROPORTIONED TO COMPLY WITH ACI 318-08 TABLES 4.3.1, 4.4.1, AND 4.4.2 IN ADDITION TO THE NOTATIONS IN THE TABLE ABOVE AND THE STRICHER REQUIREMENTS SHALL GOVERN. REFER TO THE SPECIFICATIONS FOR OTHER REQUIREMENTS FOR VARIOUS EXPOSURE CLASSES RELATIVE TO CEMENT TYPE, AIR ENTRAINMENT REQUIREMENTS, CHLORIDE ION LIMITS, AND POZZOLAN LIMITS.  
3. CONCRETE BACKING FOR SHEAR WALLS SHALL BE PROPORTIONED AS EXPLAINED IN THE CLOSING COMMENTS.  
4. WHERE INDICATED IN THE "ADDITIONAL REMARKS" COLUMN OF THE TABLE ABOVE, CONCRETE SHALL BE PROPORTIONED FOR A MAXIMUM ALLOWABLE UNIT SHRINKAGE OF 0.035% MEASURED AT 28 DAYS AFTER CURING IN LIME WATER AS DETERMINED BY ASTM C157, USING AIR STORAGE.

### 2 CLASSES OF CONCRETE MATRIX

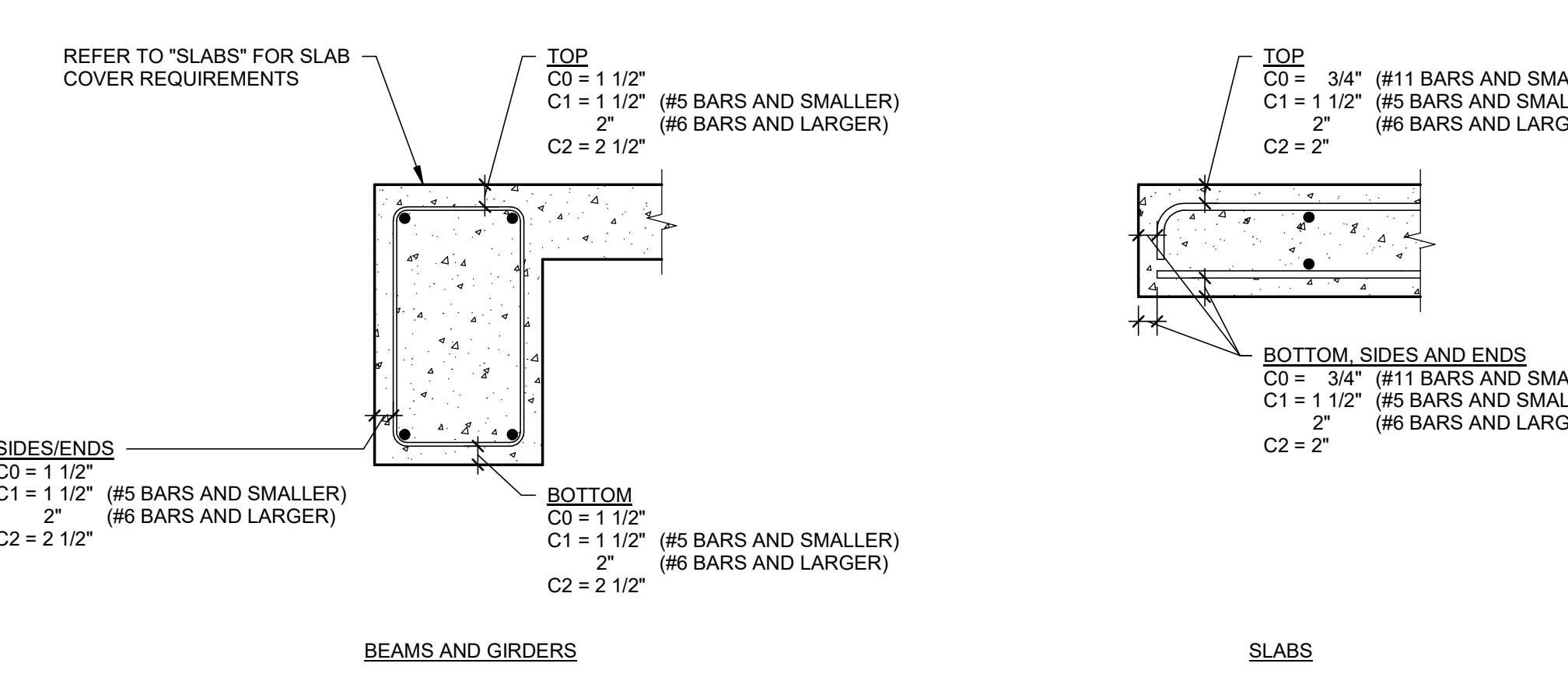
NO SCALE



**NOTES:**  
1. REFER TO "NOTES FOR TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL" FOR ADDITIONAL INFORMATION.  
2. SIDE COVER FOR DEEP FOUNDATIONS CAST INSIDE OF PERMANENT STEEL CASING SHALL BE MEASURED FROM INSIDE FACE OF CASING.  
3. FOR PRECAST ELEMENTS NOT MANUFACTURED UNDER PLANT CONDITIONS, COVERS NOTED FOR CAST-IN-PLACE ELEMENTS SHALL BE USED.

### 10 TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL IN FOUNDATION ELEMENTS

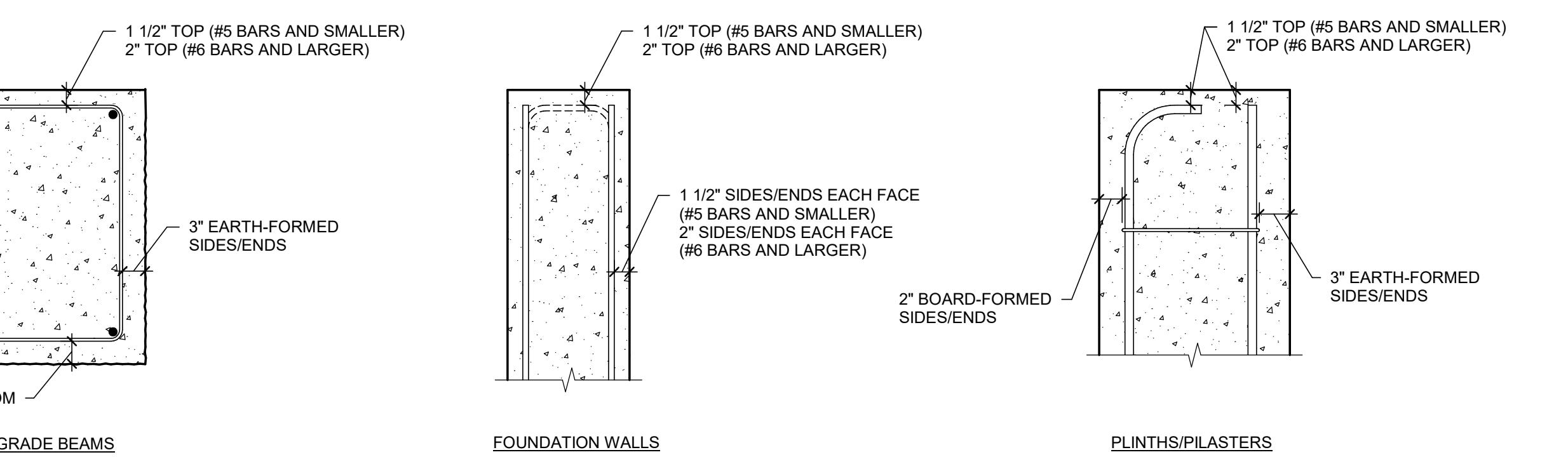
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**NOTES:**  
1. REFER TO "NOTES FOR TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL" FOR ADDITIONAL INFORMATION.

### 14 TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL IN CAST-IN-PLACE ELEVATED FRAMING

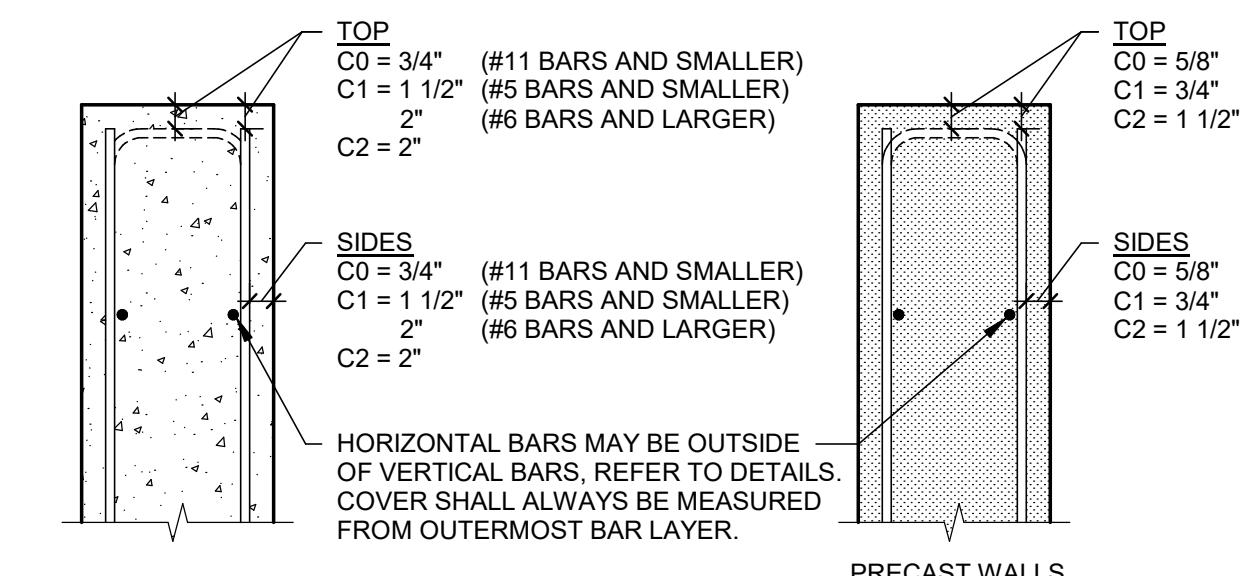
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**NOTES:**  
1. REFER TO "NOTES FOR TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL" FOR ADDITIONAL INFORMATION.

### 12 TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL IN CAST-IN-PLACE SLABS ON STEEL DECK

NO SCALE



**NOTES:**  
1. REFER TO "NOTES FOR TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL" FOR ADDITIONAL INFORMATION.  
2. FOR TILT-UP WALLS AND PRECAST WALLS NOT MANUFACTURED UNDER PLANT CONDITIONS, COVERS NOTED FOR CAST-IN-PLACE WALLS SHALL BE USED.

### 11 TYPICAL CLEAR CONCRETE COVER FOR REINFORCING STEEL IN WALLS

NO SCALE



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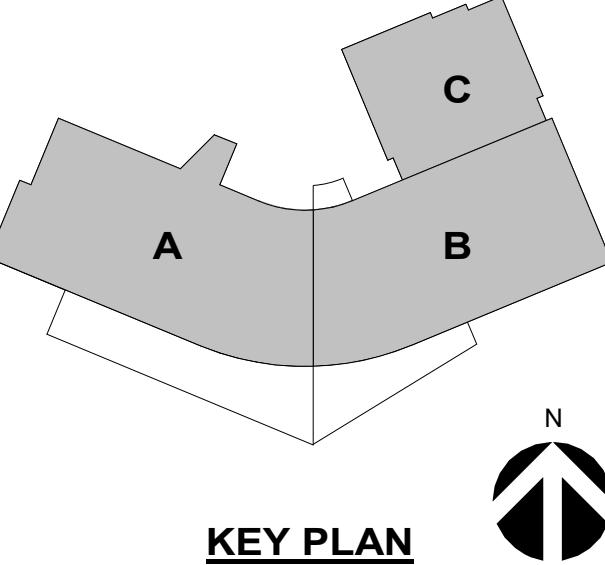
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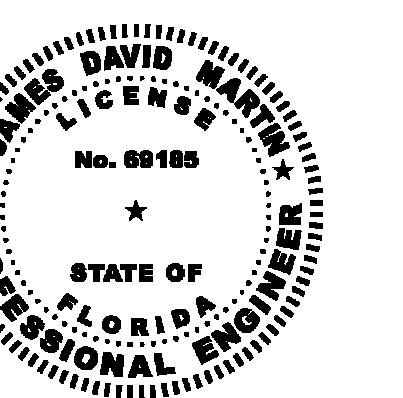


### Sarasota County Administration Center

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:



James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22045-09  
Certificate of Authorization No. 3818

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CONFORMED SET  
02/14/2024

**S006**  
LOAD KEY PLANS





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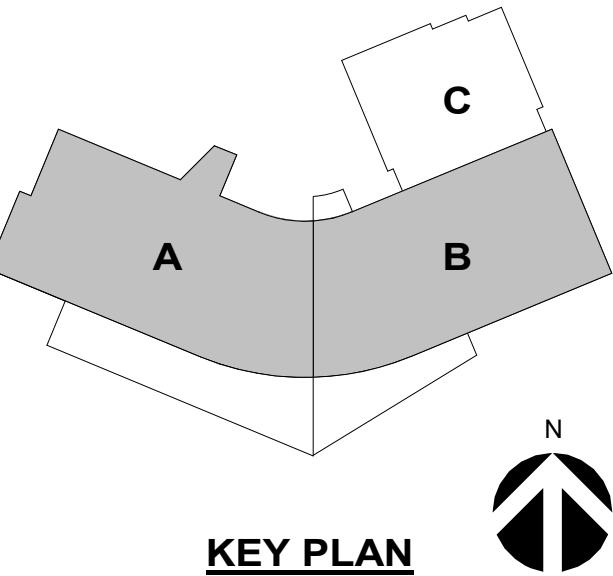
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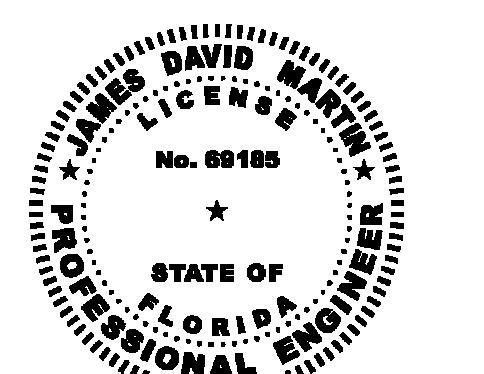


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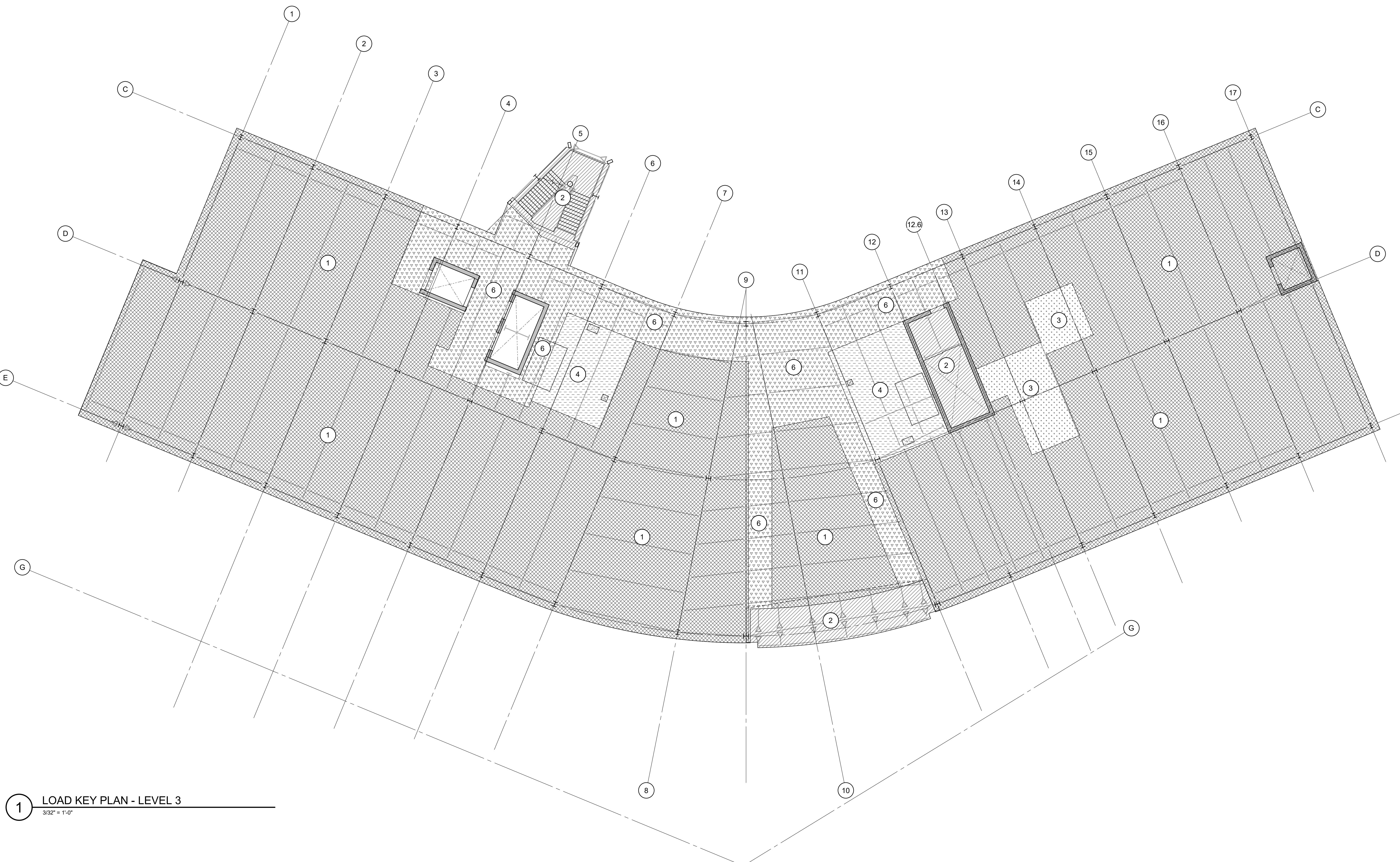
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CONFORMED SET  
02/14/2024

**S007**  
LOAD KEY PLANS



1 LOAD KEY PLAN - LEVEL 3

3/32" = 1'-0"



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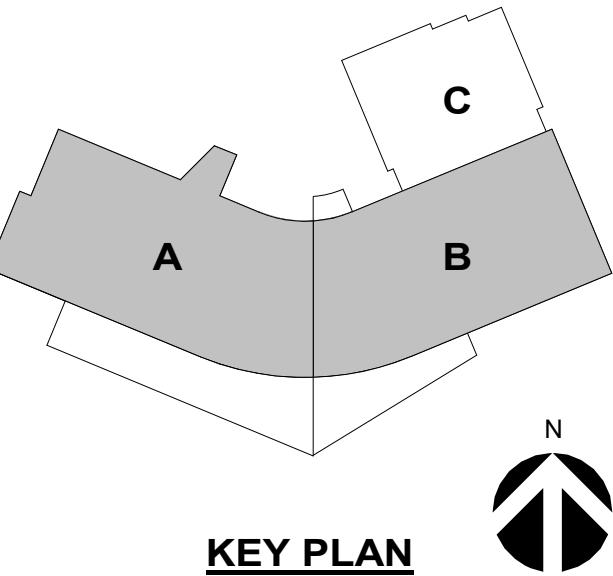
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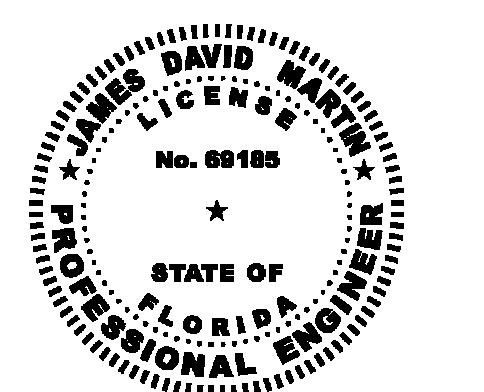


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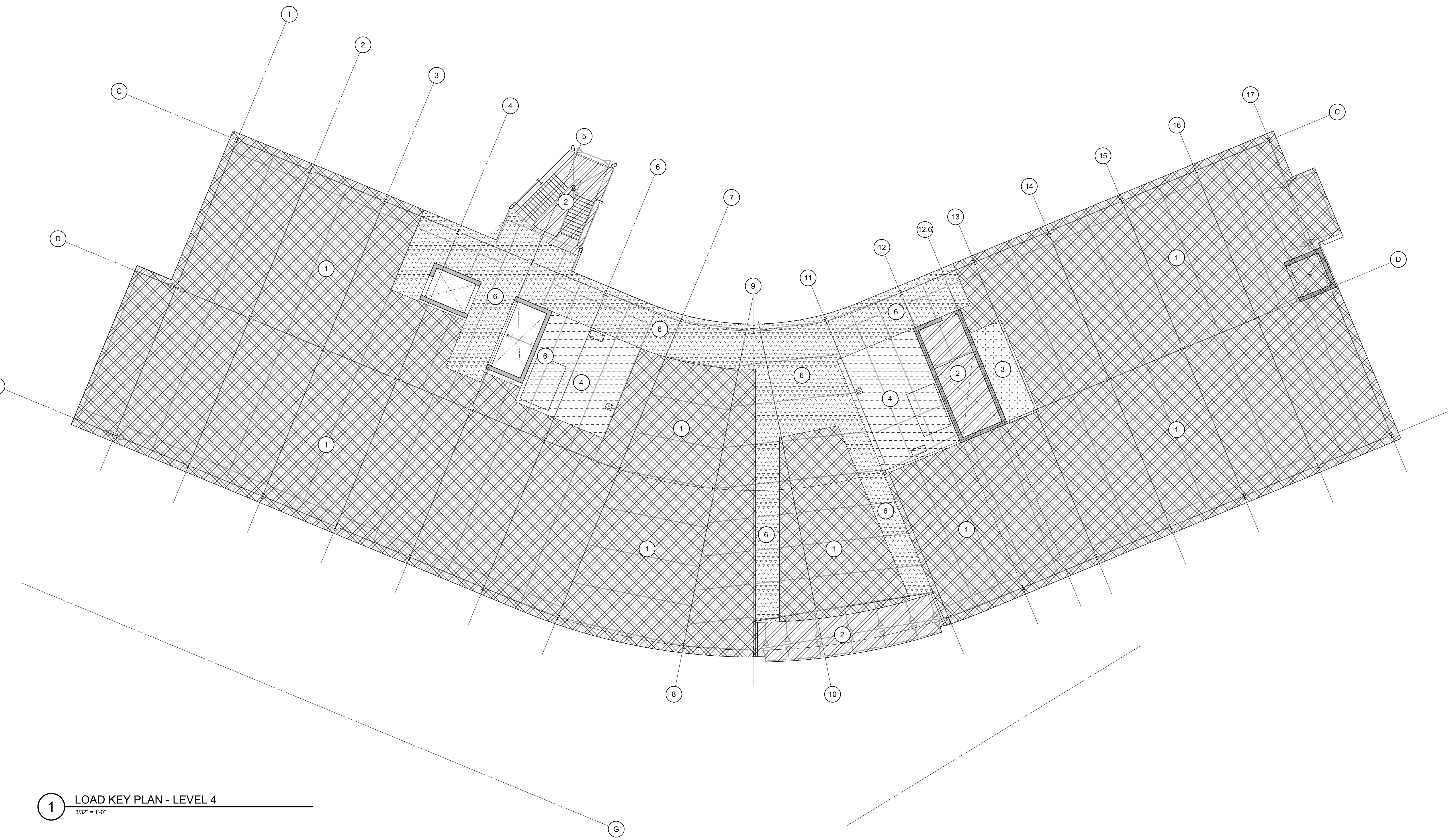


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CONFORMED SET  
02/14/2024

**S008**  
LOAD KEY PLANS

LOAD KEY LEGEND			
PATTERN	TYPE OR LOCATION	SUPERIMPOSED DEAD LOAD	LIVE LOAD
1	OFFICE	20 PSF	50 PSF (RED) + 20 PSF (PARTITIONS)
2	STAIRS + 1ST FLR CORRIDOR + ASSEMBLY	20 PSF	100 PSF
3	LIGHT STORAGE	20 PSF	125 PSF (UNREDUCIBLE)
4	MECHANICAL	20 PSF	150 PSF (UNREDUCIBLE)
5	ROOF/PENTHOUSE ROOF	35 PSF	20 PSF (UNREDUCIBLE)
6	CORRIDOR ABOVE LEVEL 1	20 PSF	80 PSF
7	ROOF/LANDSCAPED	60 PSF	100 PSF
8	ROOF/LANDSCAPED	60 PSF	20 PSF



1 LOAD KEY PLAN - LEVEL 4

3/32" = 1'-0"



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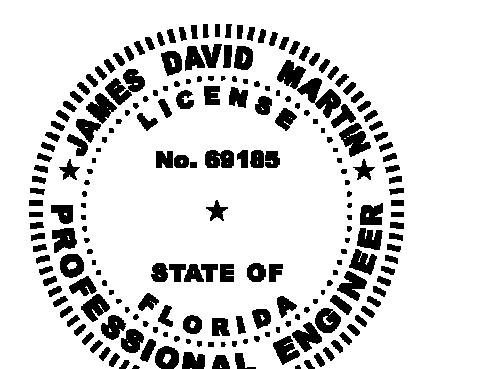
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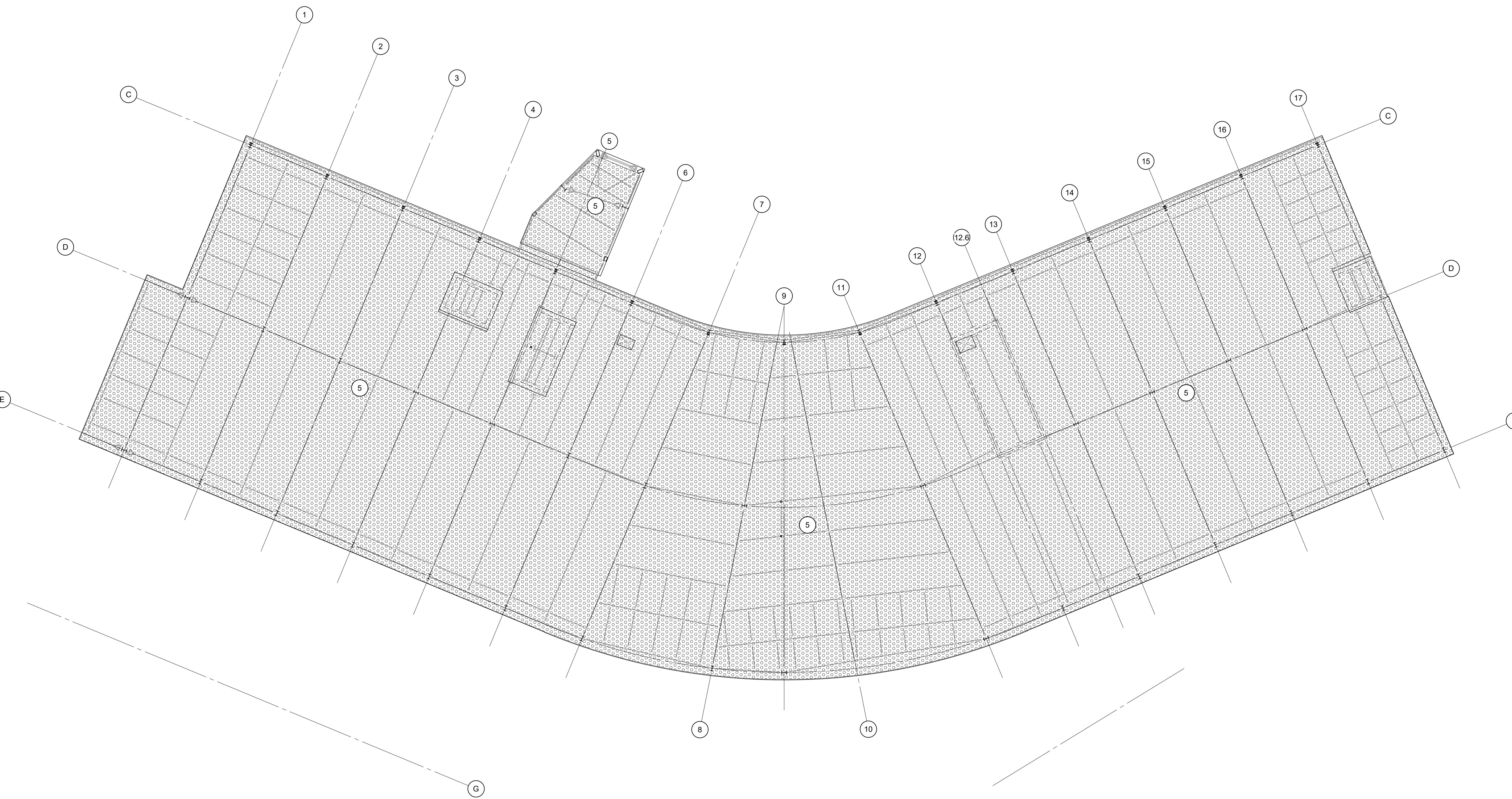
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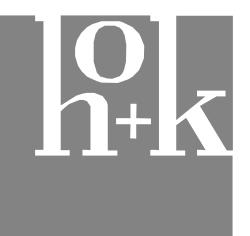
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CONFORMED SET  
02/14/2024

**S009**  
LOAD KEY PLANS

LOAD KEY LEGEND			
PATTERN	TYPE OR LOCATION	SUPERIMPOSED DEAD LOAD	LIVE LOAD
1	OFFICE	20 PSF	50 PSF (RED) + 20 PSF (PARTITIONS)
2	STAIRS + 1ST FLR CORRIDOR + ASSEMBLY	20 PSF	100 PSF
3	LIGHT STORAGE	20 PSF	125 PSF (UNREDUCIBLE)
4	MECHANICAL	20 PSF	150 PSF (UNREDUCIBLE)
5	ROOF/PENTHOUSE ROOF	35 PSF	20 PSF (UNREDUCIBLE)
6	CORRIDOR ABOVE LEVEL 1	20 PSF	80 PSF
7	ROOF/LANDSCAPED	60 PSF	100 PSF
8	ROOF/LANDSCAPED	60 PSF	20 PSF





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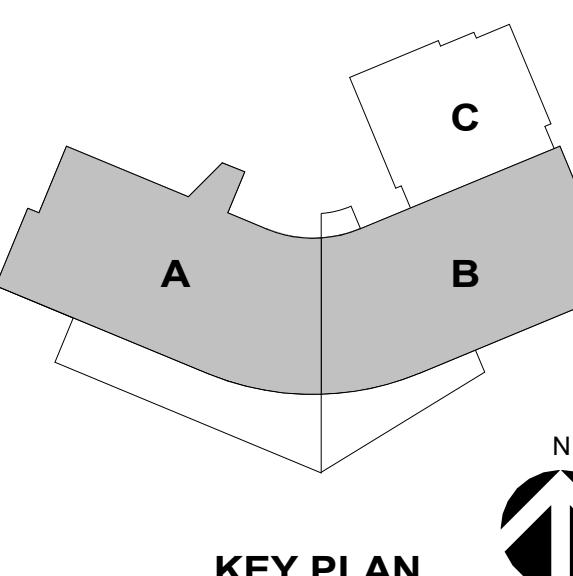
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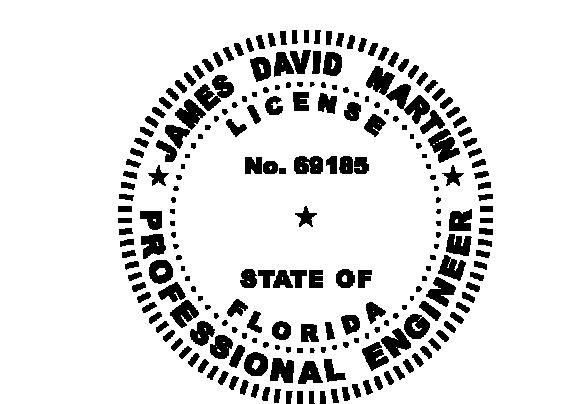
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### Sarasota County Administration Center

Project No.: 22.29005.00  
Drawn By: BD  
Checked By: JDM  
Date: 09/08/2023

Revisions:

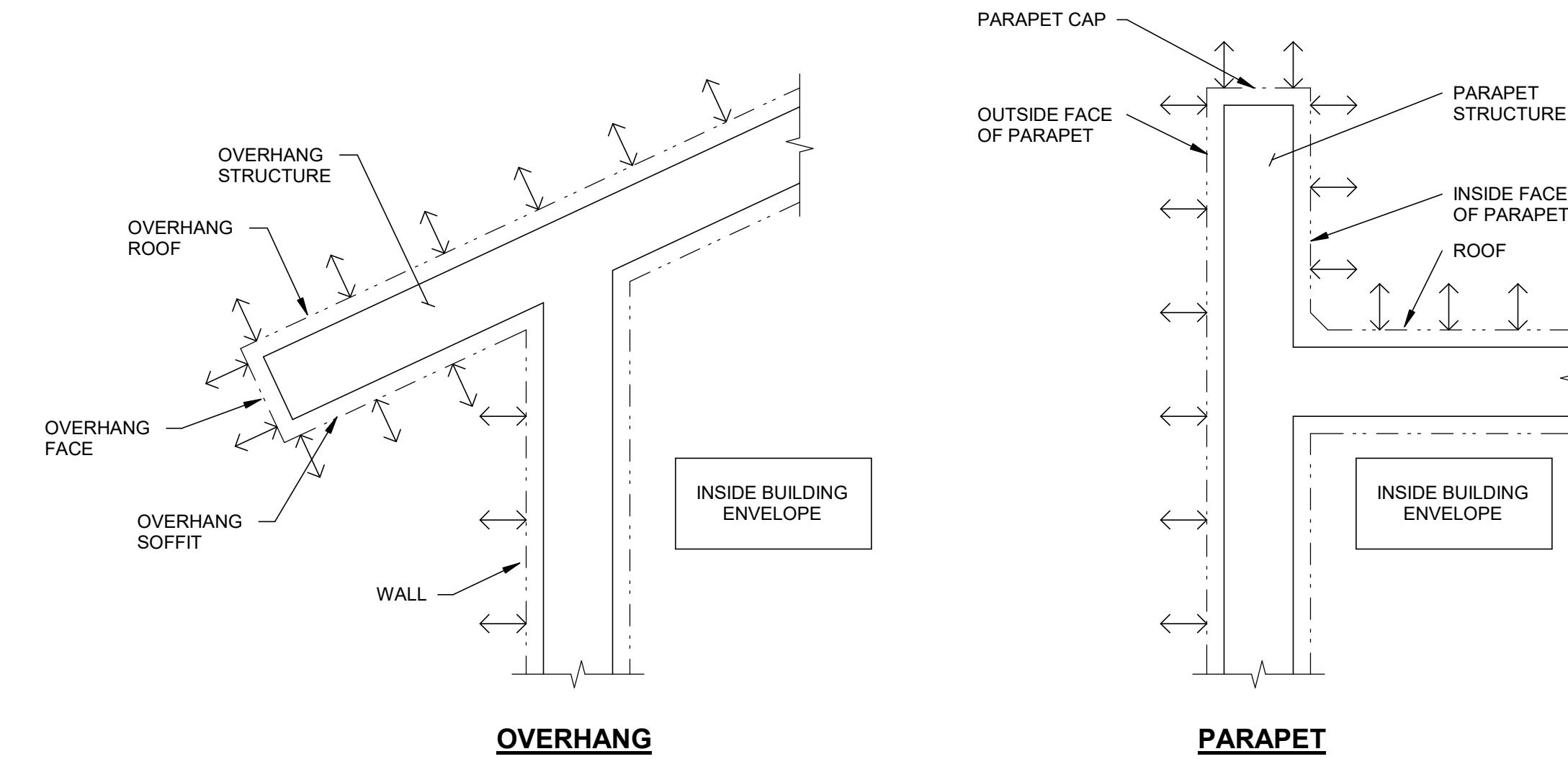


James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22045-00  
Certificate of Authorization No. 3818

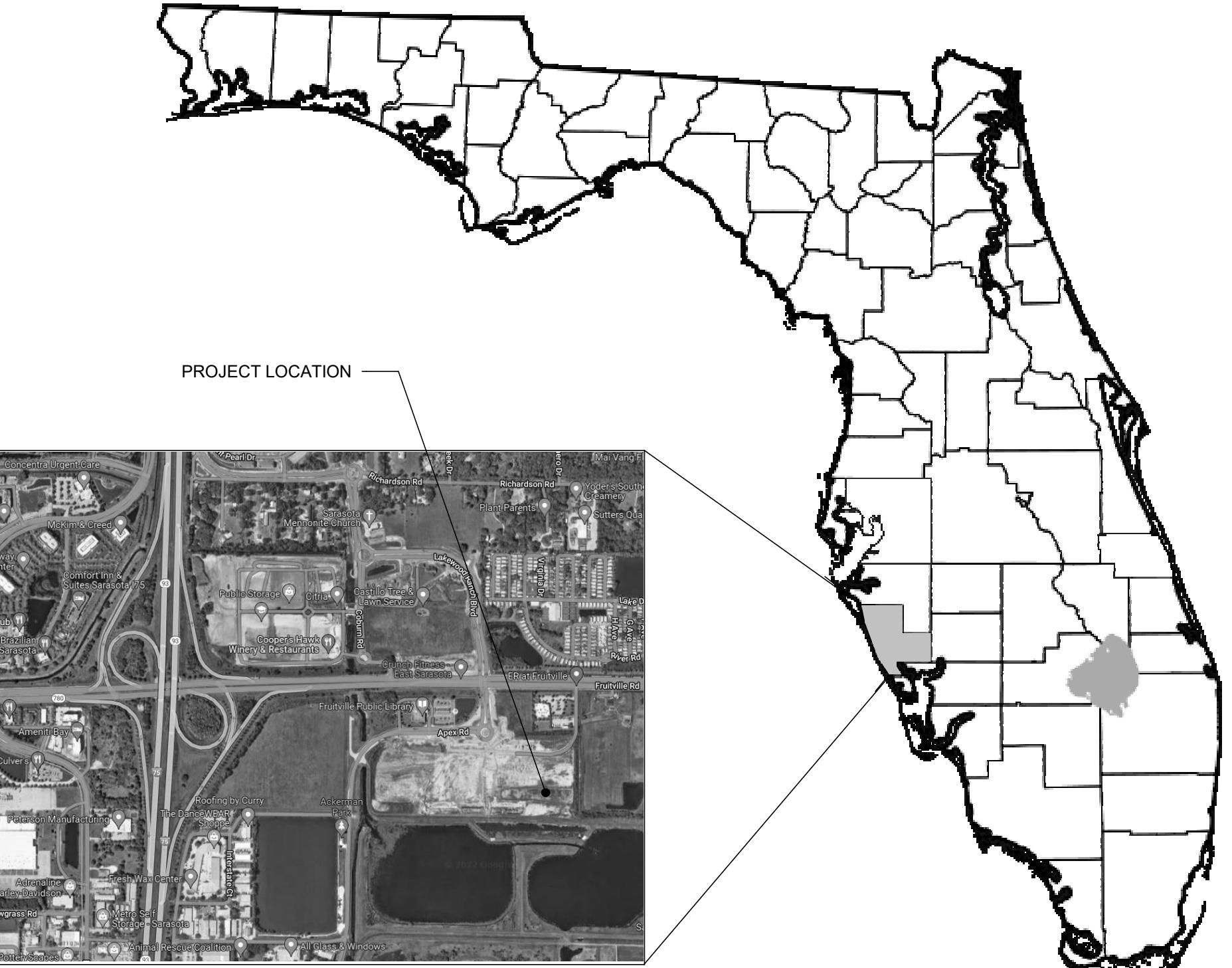
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CONFORMED SET  
02/14/2024

**S010**  
WIND CLADDING PRESSURES

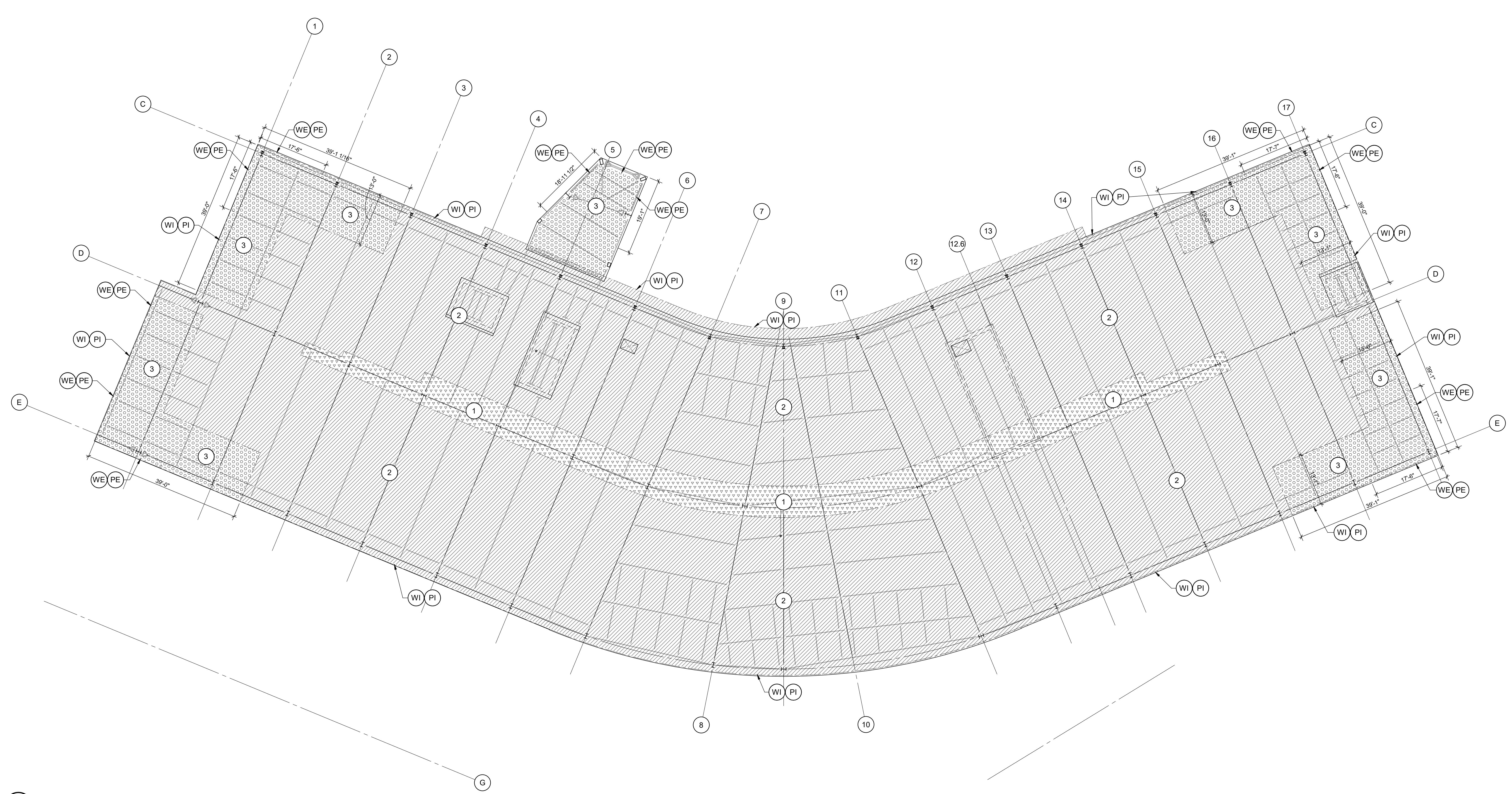


WALL CLADDING PRESSURES				ROOF CLADDING PRESSURES				ROOFTOP EQUIPMENT AND STRUCTURES			
CLADDING TYPE	LOCATION	EFFECTIVE AREA (SQ-FT)	WIND PRESSURES (PSF)	CLADDING TYPE	LOCATION	EFFECTIVE AREA (SQ-FT)	WIND PRESSURES (PSF)	CLADDING TYPE	LOCATION	EFFECTIVE AREA (SQ-FT)	WIND PRESSURES (PSF)
WALL	INTERIOR ZONE (WI)	10	+60 / -64	ROOF	INTERIOR ZONE (I <sup>1</sup> )	10	+26 / -60	ROOFTOP EQUIPMENT AND STRUCTURES	LOAD TYPE	WIND PRESSURES (PSF)	
	EDGE ZONE (WE)	20	+57 / -62		20	+25 / -60			LATERAL	+105 / -105	
	17'-6" WIDE	50	+53 / -58		50	+23 / -60			UPLIFT	+83 / -83	
		100	+51 / -56		100	+21 / -60					
		500	+45 / -51		500	+21 / -40					
		10	+60 / -79		10	+26 / -104					
		20	+57 / -74		20	+25 / -97					
		50	+53 / -67		50	+23 / -88					
		100	+51 / -62		100	+21 / -81					
		500	+45 / -51		500	+21 / -65					
PARAPET	INTERIOR ZONE (PI)	10	+178 / -105		10	+60 / -137					
	EDGE ZONE (PE)	20	+166 / -100		20	+57 / -128					
	17'-6" LONG	50	+151 / -93		50	+53 / -116					
		100	+140 / -87		100	+51 / -107					
		500	+45 / -87		500	+45 / -71					
		10	+178 / -120		10	+60 / -137					
		20	+166 / -112		20	+57 / -128					
		50	+151 / -102		50	+53 / -116					
		100	+140 / -94		100	+51 / -107					
		500	+45 / -87		500	+45 / -71					



- NOTES:**
1. REFER TO LOADING DIAGRAMS FOR LOCATION OF EACH CLADDING TYPE.
  2. FOR EACH OF THE SPECIFIED OVERHANG CLADDING ELEMENTS, USE THE FOLLOWING VALUES FROM THE TABLES IN 3/S005:  
a. OVERHANG SOFFIT CLADDING - USE INTERIOR OR EDGE WALL PRESSURES.  
b. OVERHANG ROOF CLADDING - USE INTERIOR OR CORNER ROOF PRESSURES.  
c. OVERHANG ROOF CLADDING - USE INTERIOR, EDGE, OR CORNER ROOF PRESSURES.  
d. OVERHANG STRUCTURE (SUPPORTING BOTH SOFFIT AND ROOF COMPONENTS)  
1) POSITIVE (DOWNWARD) PRESSURE = [POSITIVE ROOF PRESSURE] + [NEGATIVE WALL PRESSURE]  
2) NEGATIVE (UPWARD) PRESSURE = [NEGATIVE ROOF PRESSURE] + [POSITIVE WALL PRESSURE]
  3. FOR EACH OF THE SPECIFIED PARAPET CLADDING ELEMENTS, USE THE FOLLOWING VALUES FROM THE TABLES IN 1/S0.33:  
a. PARAPET OUTSIDE FACE CLADDING - USE INTERIOR OR EDGE WALL PRESSURES.  
b. PARAPET INSIDE FACE CLADDING - USE THE EDGE OR CORNER ROOF PRESSURES.  
c. PARAPET ROOF CLADDING - USE INTERIOR, EDGE, OR CORNER ROOF PRESSURES.  
d. PARAPET STRUCTURE (SUPPORTING BOTH FACES OF THE PARAPET) - USE THE TABULATED PARAPET PRESSURES.
  4. THE LOADING DIAGRAMS INDICATED ARE SCHEMATIC IN NATURE ONLY AND ARE NOT INTENDED TO PROVIDE CONSTRUCTION-RELATED INFORMATION. REFER TO DETAILS FOR INFORMATION REGARDING STRUCTURAL FRAMING AND CONNECTIONS.

5 APPLICATION OF WIND PRESSURE FOR PARAPETS AND OVERHANGS  
NO SCALE

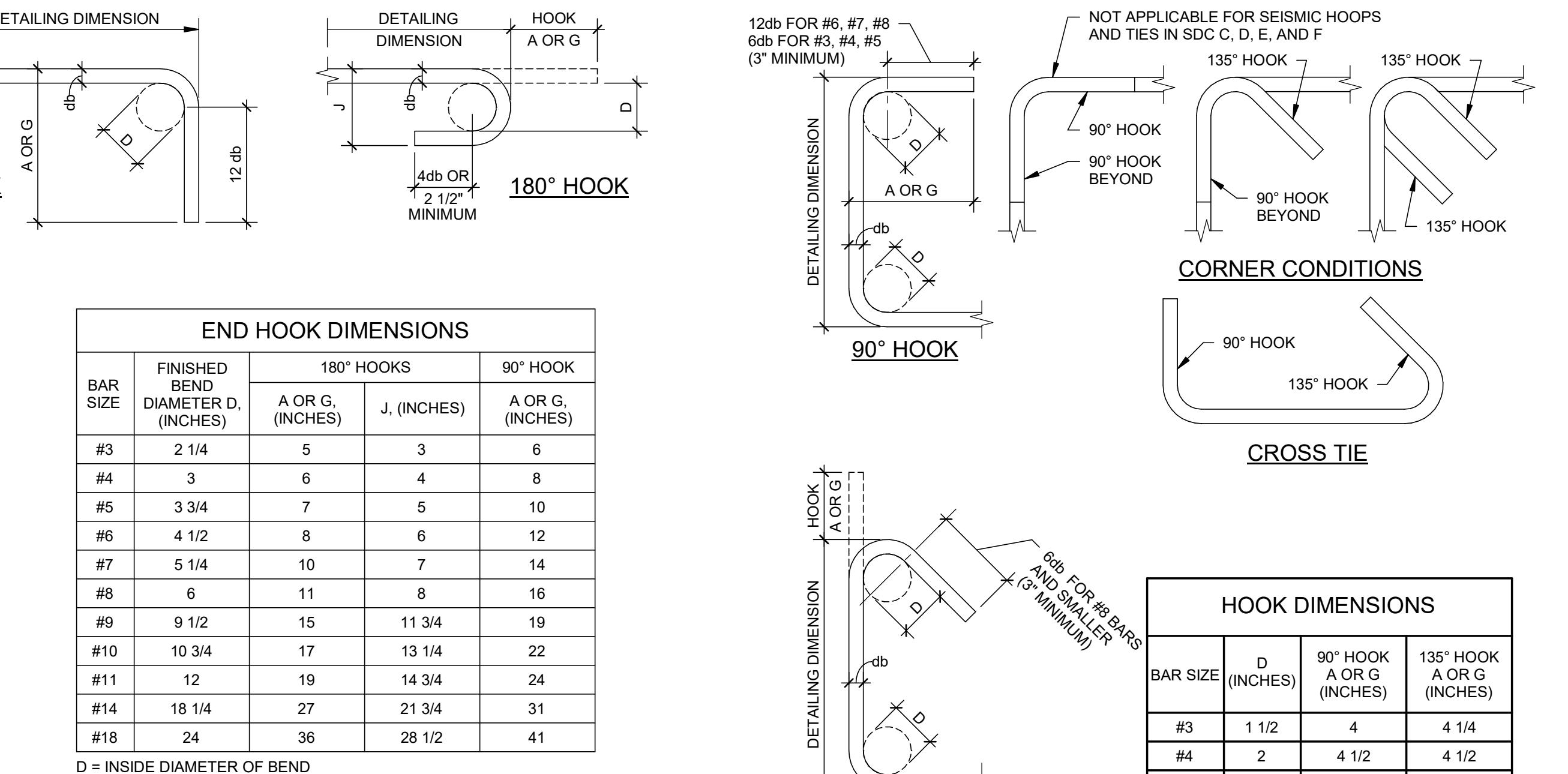


15 ROOF WIND PRESSURE PLAN  
3/32" = 1'-0"



WALL VERTICAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE											
BAR SIZE	LAP CLASS	$f_c = 3000 \text{ PSI}$		$f_c = 4000 \text{ PSI}$		$f_c = 5000 \text{ PSI}$		$f_c = 6000 \text{ PSI}$		$f_c = 7000 \text{ PSI}$	
		CATEGORY 1	CATEGORY 2								
#4	A	17	17	15	15	15	15	15	15	15	15
	B	22	22	19	19	17	17	16	15	15	15
#5	A	21	25	19	21	19	19	19	19	19	19
	B	27	32	24	28	21	25	19	23	19	20
#6	A	25	33	23	29	23	26	23	24	23	23
	B	33	43	28	37	25	34	23	31	23	27
#7	A	41	48	35	42	32	38	29	34	27	32
	B	53	63	46	54	41	49	38	45	35	41
#8	A	51	55	44	48	40	43	36	39	34	36
	B	66	72	57	62	51	56	47	51	44	47
#9	A	62	62	54	54	48	48	44	44	41	41
	B	81	81	70	70	63	63	57	57	53	50
#10	A	71	71	61	61	55	55	50	50	47	47
	B	92	92	80	80	71	71	65	65	60	60
#11	A	84	84	73	73	65	65	60	60	55	52
	B	109	109	95	95	85	85	77	77	72	72
		67	67								

NOTES:  
 1. ALL SPLICE LENGTHS ARE IN INCHES.  
 2. THIS TABLE SHALL BE USED FOR WALL VERTICAL BARS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR WALL HORIZONTAL BARS AND OTHER MEMBERS.  
 3. THE TENSION DEVELOPMENT LENGTH ( $L_d$ ) IS EQUAL TO THE SCHEDULED "CLASS A" LAP SPLICE LENGTH. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS BY 1.5.  
 4. WHEN LAP SPlicing BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE "CLASS A" SPLICE LENGTH OF THE LARGER BAR.  
 5. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.



### 5 TYPICAL WALL VERTICAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS

NO SCALE

WALL HORIZONTAL BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, NORMALWEIGHT CONCRETE											
BAR SIZE	LAP CLASS	$f_c = 3000 \text{ PSI}$		$f_c = 4000 \text{ PSI}$		$f_c = 5000 \text{ PSI}$		$f_c = 6000 \text{ PSI}$		$f_c = 7000 \text{ PSI}$	
		BOTTOM BARS	OTHER BARS								
#3	A	13	12	12	12	12	12	12	12	12	12
	B	17	16	16	16	16	16	16	16	16	16
#4	A	22	19	17	16	16	14	14	14	14	14
	B	29	25	23	21	19	19	19	19	19	19
#5	A	32	28	25	23	23	21	20	20	20	20
	B	42	37	33	30	28	26	26	26	26	26
#6	A	43	37	34	31	31	28	27	27	27	27
	B	56	49	45	41	41	37	36	36	36	36
#7	A	69	60	54	49	46	43	43	43	43	43
	B	90	78	71	64	60	56	56	56	56	56
#8	A	86	74	67	61	56	53	53	53	53	53
	B	112	97	88	80	73	69	69	69	69	69
#9	A	104	90	81	74	68	64	64	64	64	64
	B	136	117	106	97	89	84	84	84	84	84
#10	A	125	108	97	88	82	77	77	77	77	77
	B	163	141	127	115	107	101	101	101	101	101
#11	A	146	127	114	104	96	90	90	90	90	90
	B	190	166	149	136	125	117	117	117	117	117

NOTES:  
 1. ALL SPLICE LENGTHS ARE IN INCHES.  
 2. THIS TABLE SHALL BE USED FOR WALL HORIZONTAL BARS ONLY. REFER TO OTHER DEVELOPMENT LENGTH TABLES FOR WALL VERTICAL BARS AND OTHER MEMBERS.  
 3. THE TENSION DEVELOPMENT LENGTH ( $L_d$ ) IS EQUAL TO THE SCHEDULED "CLASS A" LAP SPLICE LENGTH. FOR EPOXY-COATED BARS, MULTIPLY THE TABULATED SPLICE LENGTHS BY 1.5.  
 4. WHEN LAP SPlicing BARS OF DIFFERENT SIZES, THE LAP LENGTH IS DETERMINED BY THE SMALLER BAR BUT MAY NOT BE LESS THAN THE "CLASS A" SPLICE LENGTH OF THE LARGER BAR.  
 5. FOR CONCRETE STRENGTHS IN BETWEEN THOSE TABULATED HERE, USE DEVELOPMENT AND LAP SPLICE LENGTHS OF LOWER CONCRETE STRENGTH.

SLAB TENSION DEVELOPMENT AND LAP SPLICE LENGTHS GRADE 60 REINFORCEMENT, LIGHTWEIGHT CONCRETE											
BAR SIZE	LAP CLASS	$f_c = 3000 \text{ PSI}$		$f_c = 4000 \text{ PSI}$		$f_c = 5000 \text{ PSI}$		$f_c = 6000 \text{ PSI}$		$f_c = 7000 \text{ PSI}$	
		BOTTOM BARS	OTHER BARS								
#3	A	14	18	12	15	12	14	12	13	12	12
	B	19	24	16	20	16	19	16	17	16	16
#4	A	22	29	19	25	17	23	16	21	15	19
	B	29	38	25	33	23	30	21	28	20	24
#5	A	33	42	28	37	25	33	23	30	22	26
	B	43									



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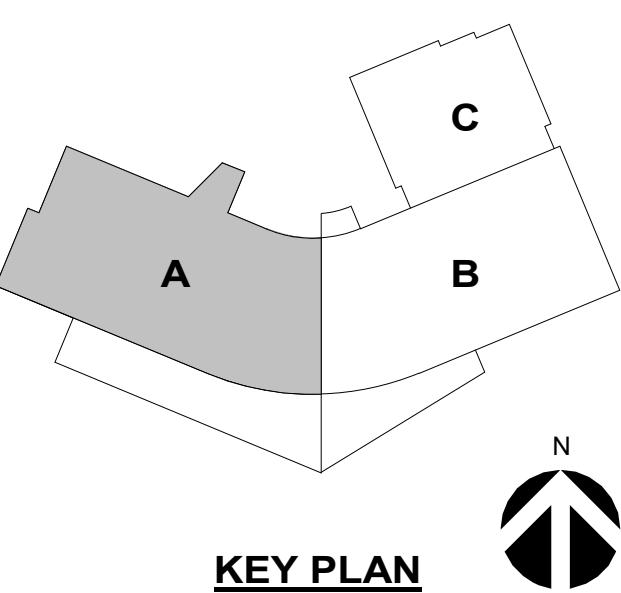
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**Walter P. Moore** Structural Engineers  
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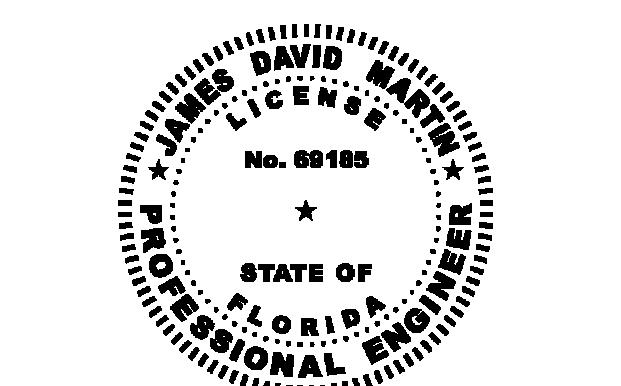
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Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
2 ASI-02 05/17/2024  
5 ASI-06 08/16/2024  
6 ASI-07 09/06/2024  
7 ASI-09 10/25/2024



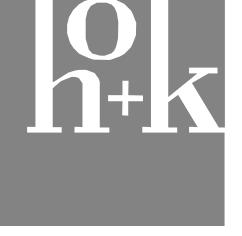
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WPM Project No. S05-2204-09  
Certificate of Authorization No. 3818

To the best of Engineer's knowledge, the plans and specifications comply with applicable codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

**CONFORMED SET**  
02/14/2024

**S100.A**  
FOUNDATION PLAN - AREA A





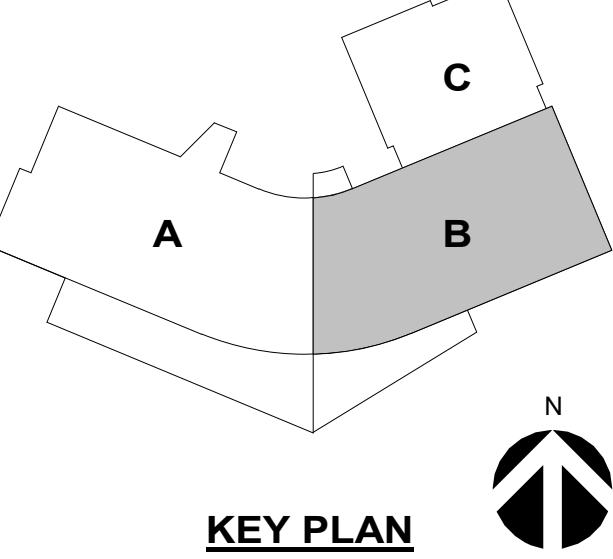
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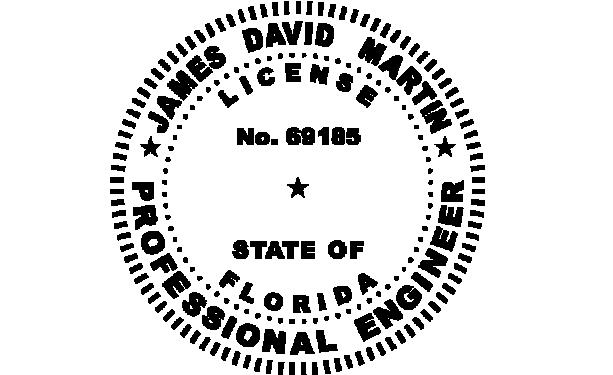
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Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions: 1 ASI-02 05/17/2024

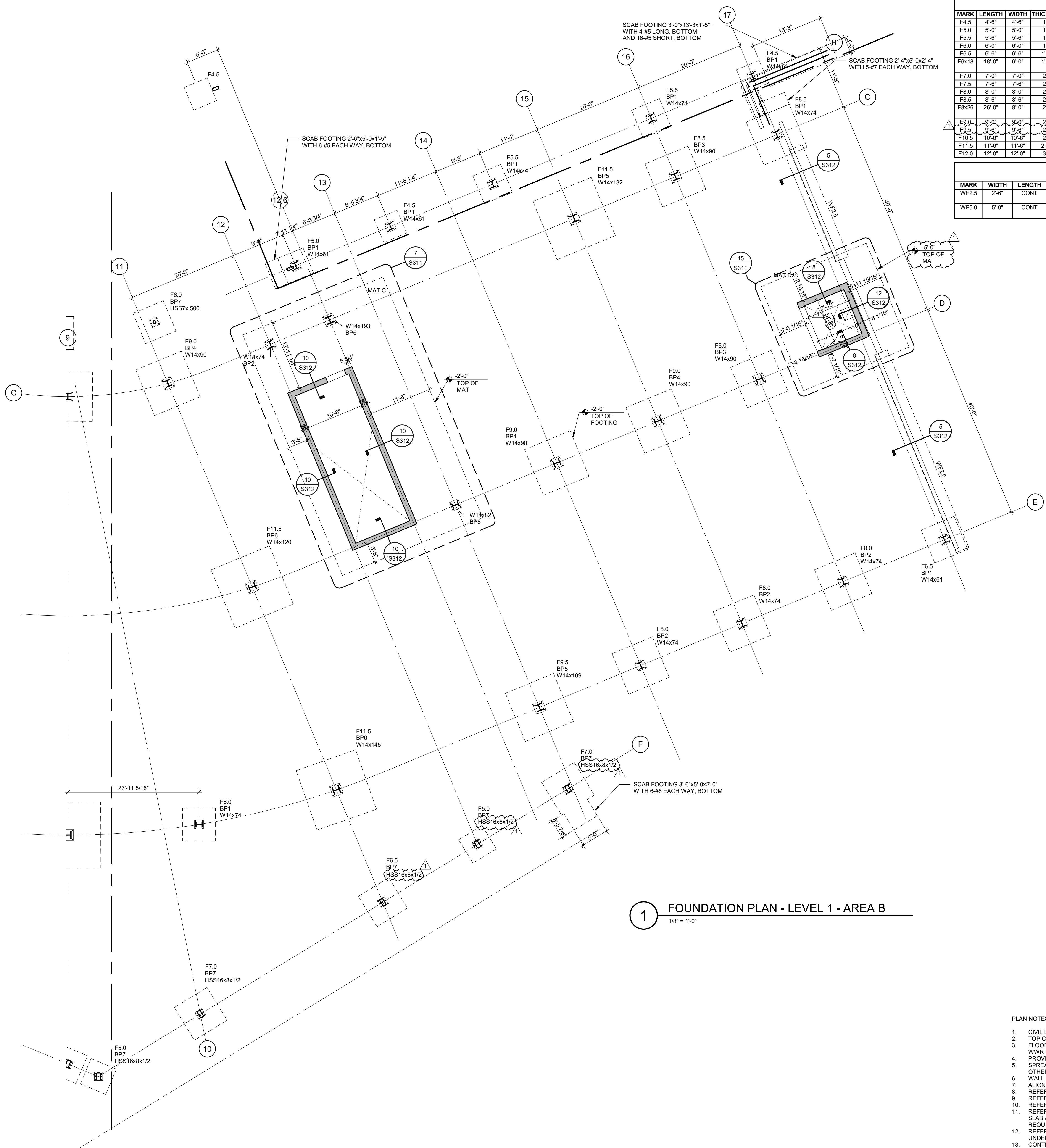


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WPM Project No. S05-2204-00  
Certificate of Authorization No. 69185

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CONFORMED SET  
02/14/2024

**S100.B**  
FOUNDATION PLAN -  
AREA B





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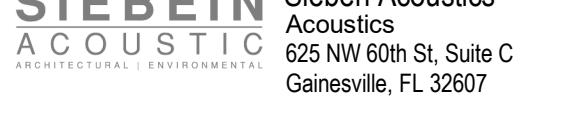
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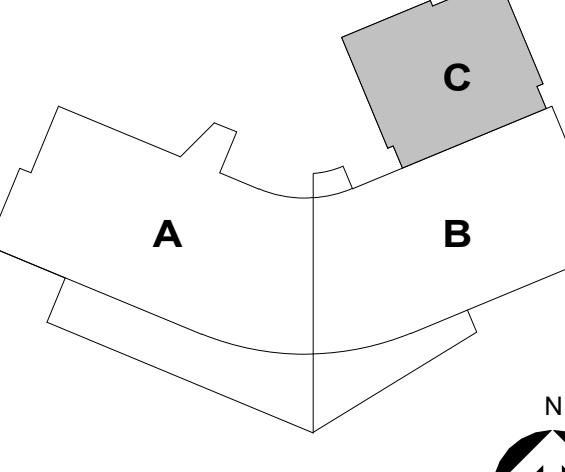
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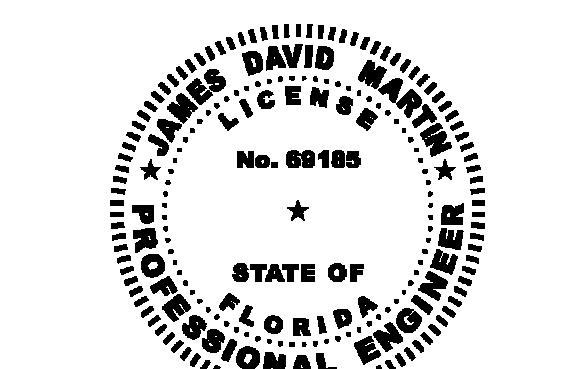
KEY PLAN

## Sarasota County Administration Center

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
A Pre-GMP ASI #1 09/27/2023

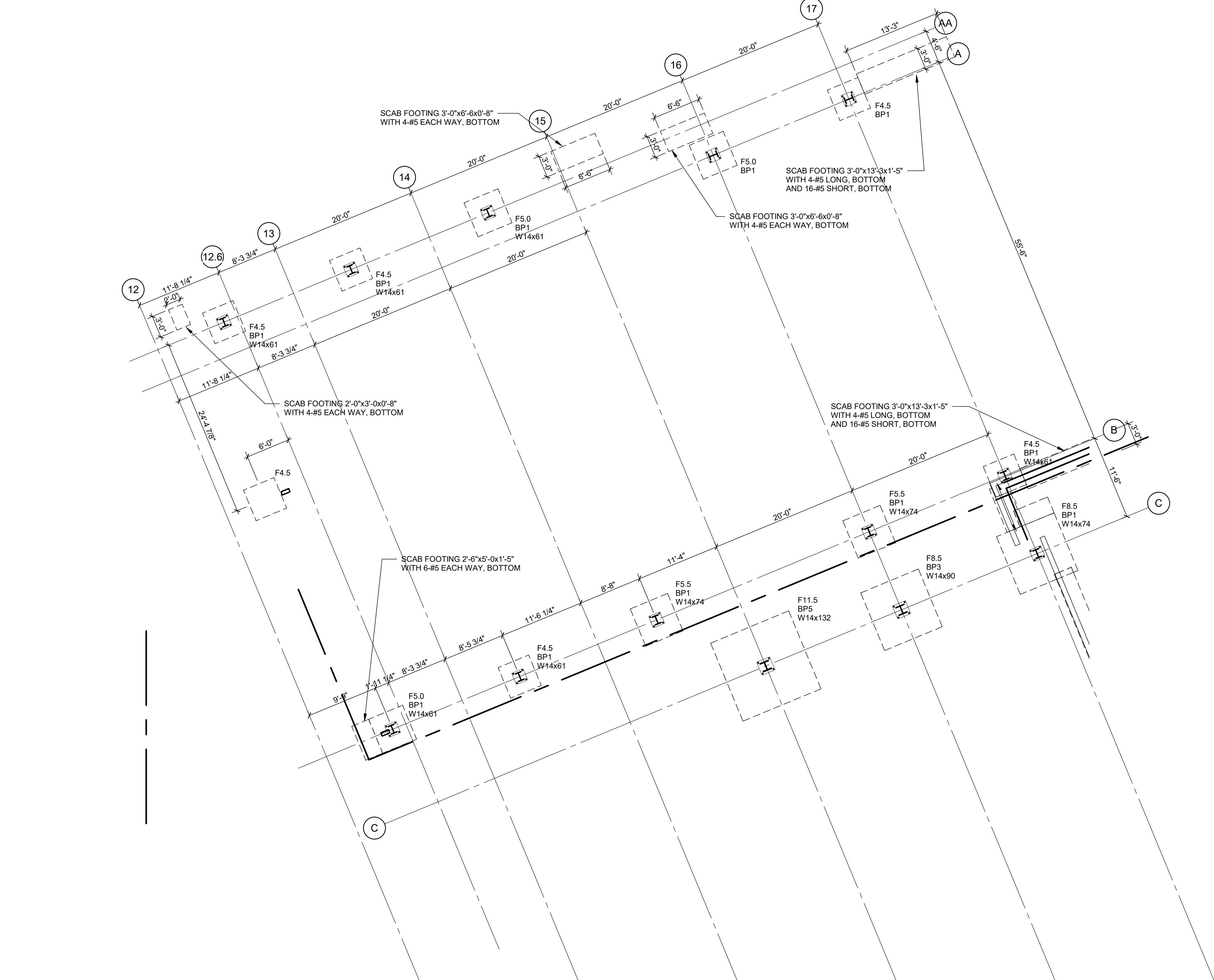


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WPM Project No. S03-2204-09  
Certificate of Authorization No. 3818

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CONFORMED SET  
02/14/2024

**S100.C**  
FOUNDATION PLAN -  
AREA C



1 FOUNDATION PLAN - LEVEL 1 - AREA C

1/8" = 1'-0"

SPREAD FOOTING SCHEDULE					
MARK	LENGTH	WIDTH	DEPTH	REINFORCEMENT	REMARKS
F4.5	6'-0"	6'-0"	1'-0"	6# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F5.0	5'-0"	6'-0"	1'-5"	6# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F5.5	5'-6"	5'-6"	1'-7"	8# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F6.0	6'-0"	6'-0"	1'-8"	9# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F6.5	6'-6"	6'-6"	1'-10"	7# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F6x18	18'-0"	6'-0"	1'-10"	#4@8 LONG, TOP AND BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F7.0	7'-0"	7'-0"	2'-0"	9# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F7.5	7'-6"	7'-6"	2'-1"	10# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F8.0	8'-0"	8'-0"	2'-2"	9# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F8.5	8'-6"	8'-6"	2'-4"	10# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F8x26	26'-0"	8'-0"	2'-2"	#8@12" LONG, TOP AND BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F9.0	9'-0"	9'-0"	2'-6"	10# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F9.5	9'-6"	9'-6"	2'-6"	12# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F10.0	10'-6"	10'-6"	2'-9"	10# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F11.5	11'-6"	11'-6"	2'-11"	9# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
F12.0	12'-0"	12'-0"	3'-1"	14# EACH WAY BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT

WALL FOOTING SCHEDULE					
MARK	WIDTH	LENGTH	DEPTH	REINFORCEMENT	REMARKS
WF2.5	2'-6"	CONT	1'-4"	3#5 LONG, TOP AND BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT
WF5.0	5'-0"	CONT	1'-4"	5#5 LONG, TOP AND BOTTOM	6000 PSF ALLOWABLE BEARING ON VIBROREPLACEMENT

- PLAN NOTES
- CIVIL DRAWINGS ELEVATION 20.00' = DATUM ELEVATION 0'-0"
  - TOP OF FOUNDATION ELEVATION IS 2'-0" UNLESS NOTED OTHERWISE ON PLAN.
  - FLOOR STRUCTURE IS A 5" THICK NORMALWEIGHT CONCRETE SLAB-ON-GRADE REINFORCED WITH WWR 6x6-2.9xW2.9.
  - PROVIDE 2-4x4"-0" ADDITIONAL SLAB REINFORCEMENT AT ALL RE-ENTRANT CORNERS.
  - REFER TO SHEET SERIES S60X FOR COLUMN SCHEDULES.
  - WALL FOOTINGS SHALL BE CENTERED ON THE CENTROID OF SUPPORTED COLUMN, UNLESS NOTED OTHERWISE.
  - WALL FOOTINGS SHALL BE CENTERED ON THE SUPPORTED WALL, UNLESS NOTED OTHERWISE.
  - ALIGN TOP OF GRADE BEAM WITH TOP OF ADJACENT FOOTINGS, UNLESS NOTED OTHERWISE.
  - GRADE BEAMS ARE TO BE TIED TO EXISTING GRADE SECTIONS AND DETAILS.
  - REFER TO SHEET SERIES S60X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
  - REFER TO ARCHITECTURAL DRAWINGS FOR EXTENTS AND DIMENSIONS OF RAISED OR DEPRESSED SLABS, TERRACES, SLOPES, CURBS, AND DRAINS. REFER TO TYPICAL DETAILS FOR REINFORCEMENT REQUIREMENTS.
  - REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLAB PENETRATIONS AND UNDERGROUND UTILITIES.
  - CONFIRM ALL EXISTING CONDITIONS AND DIMENSIONS. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES.
  - METAL PAN TREADS, RISERS, LANDINGS, STEEL STRINGERS, EMBEDED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.



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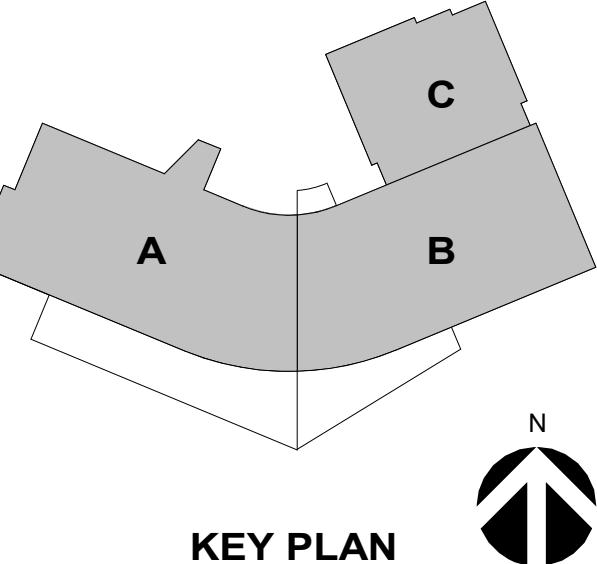
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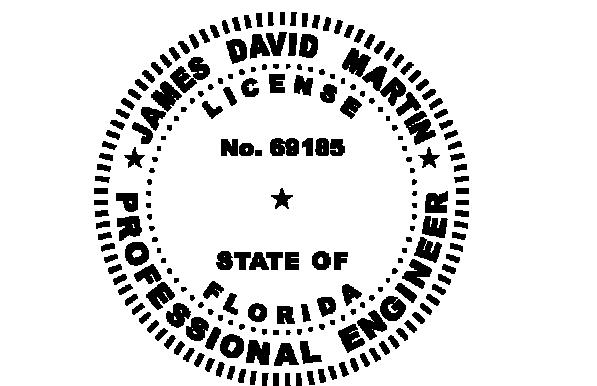


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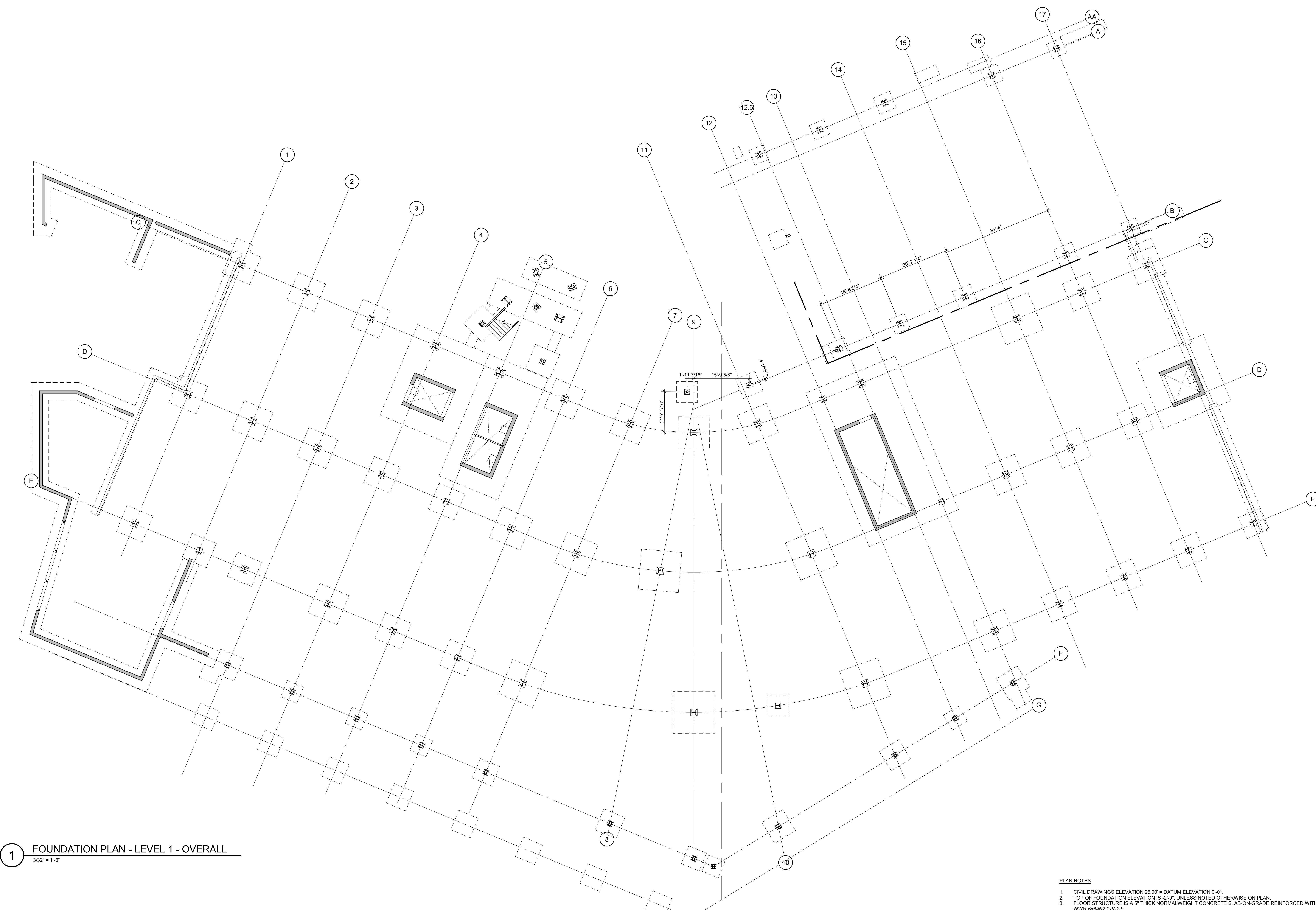


James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22045-00  
Certificate of Authorization No. 3818

To the best of the Engineer's knowledge, the plans and specifications comply with all applicable laws, including codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S100**  
FOUNDATION PLAN -  
OVERALL



PLAN NOTES

- CIVIL DRAWINGS ELEVATION 20.00' = DATUM ELEVATION 0'-0".
- TOP OF FOUNDATION ELEVATION IS 2'-0" UNLESS NOTED OTHERWISE ON PLAN.
- FLOOR STRUCTURE IS A 5" THICK NORMALWEIGHT CONCRETE SLAB-ON-GRADE REINFORCED WITH WWR 6x6-2.9xW2.9.
- PROVIDE 2-4x4'-0" ADDITIONAL SLAB REINFORCEMENT AT ALL RE-ENTRANT CORNERS.
- REFER TO SHEET SERIES S60X FOR COLUMN SCHEDULES.
- WALL FOOTINGS SHALL BE CENTERED ON THE CENTROID OF SUPPORTED COLUMN, UNLESS NOTED OTHERWISE.
- WALL FOOTINGS SHALL BE CENTERED ON THE SUPPORTED WALL, UNLESS NOTED OTHERWISE.
- ALIGN TOP OF GRADE BEAM WITH TOP OF ADJACENT FOOTINGS, UNLESS NOTED OTHERWISE.
- DO NOT USE SPACERS OR SPACERS FOR SPANNING GRADE SECTIONS AND DETAILS.
- REFER TO SHEET SERIES S60X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO ARCHITECTURAL DRAWINGS FOR EXTENTS AND DIMENSIONS OF RAISED OR DEPRESSED SLABS, ROOF SLOPES, CURBS, AND DRAINS. REFER TO TYPICAL DETAILS FOR REINFORCEMENT REQUIREMENTS.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLAB PENETRATIONS AND UNDERGROUND UTILITIES.
- COORDINATE AND VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES.
- METAL PAN TREADS, RISERS, LANDINGS, STEEL STRINGERS, EMBEDED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.





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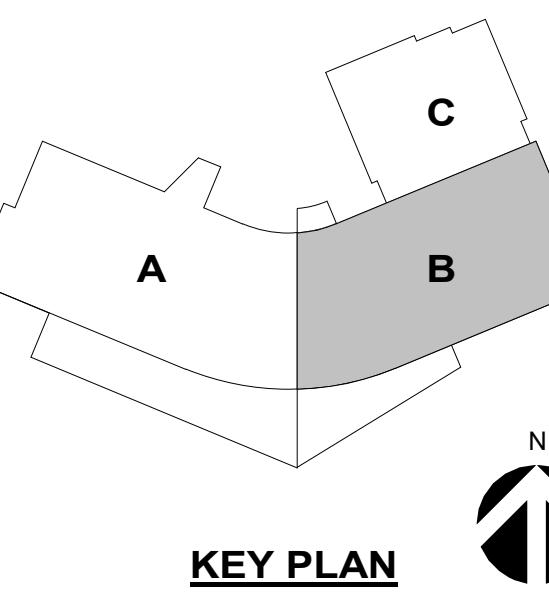
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Walter P. Moore  
Structural Engineers  
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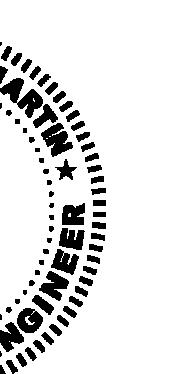
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**Sarasota County  
Administration Center**

1 Apex Road  
Sarasota, Florida 34240

Autodesk Docs/22/2009/00\_Sarasota City Admin Bldg/S02/2204/00\_Sarasota CAC\_Structure R22.m

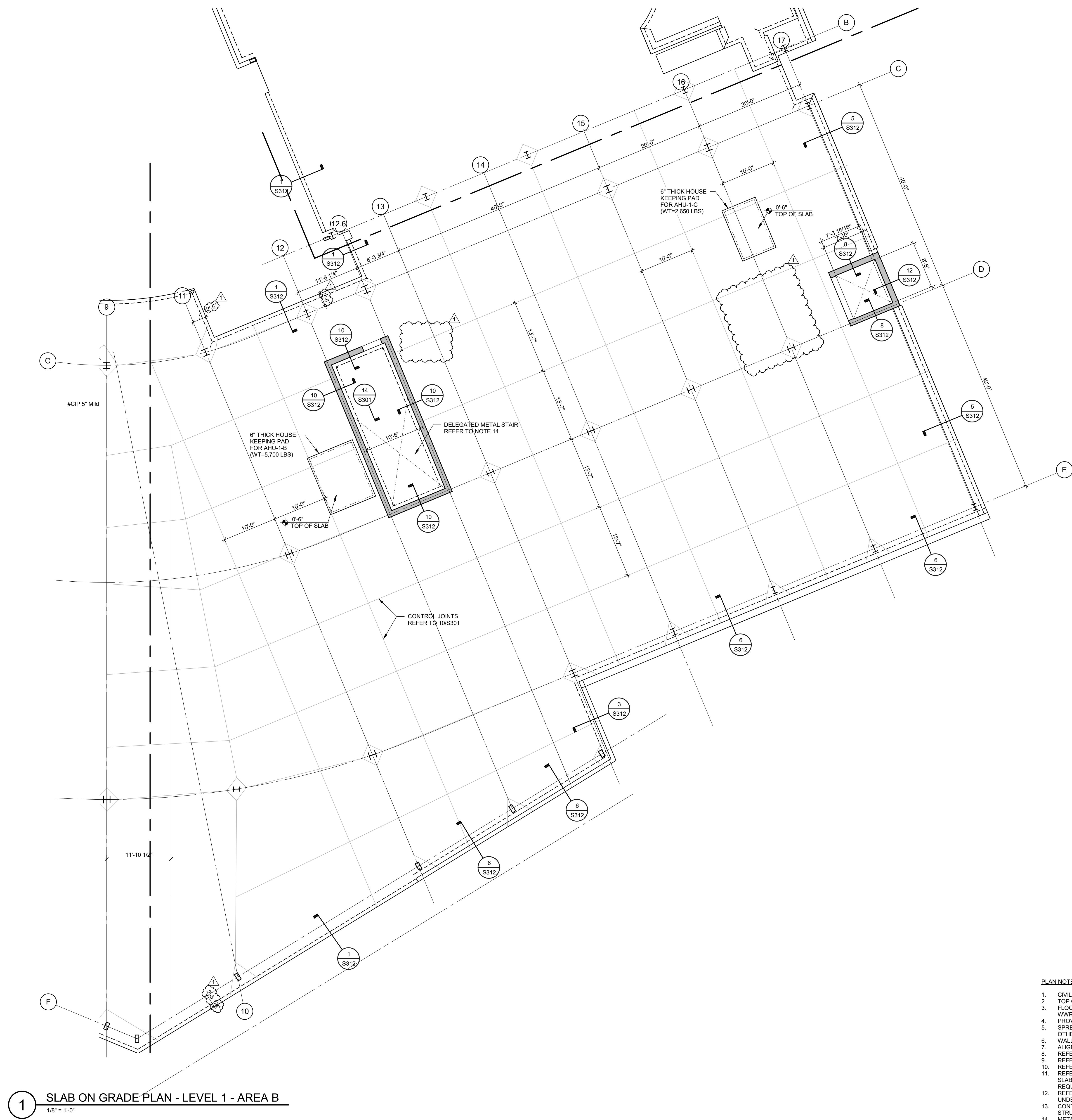


James David Martin, P.E. FL PE No. 69185  
WPM Project No. S02/2204/00  
Certificate of Authorization No. 3818

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**CONFORMED SET  
02/14/2024**

**S101.B**  
SLAB ON GRADE  
PLAN - LEVEL 1 -  
AREA B





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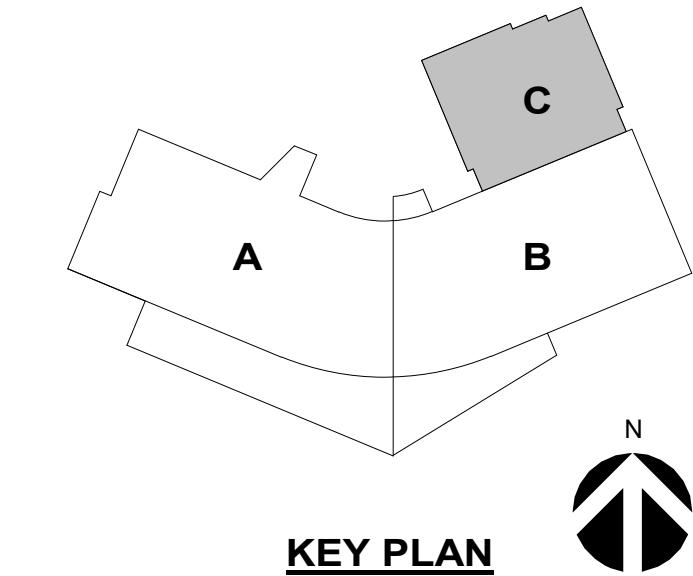
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Tampa, FL 33602

**SIEBEIN ACOUSTIC**  
Acoustics  
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Gainesville, FL 32607

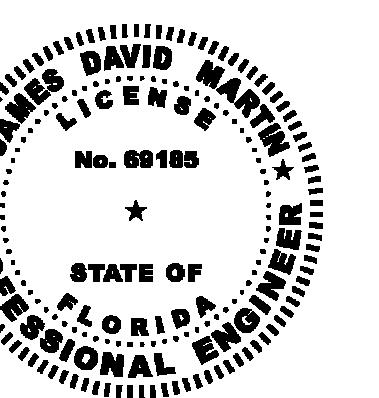


### Sarasota County Administration Center

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
2 ASI-02 05/17/2024  
6 ASI-07 09/26/2024



James David Martin, P.E. FL PE No. 69185  
WPM Project No. S03-2204-09  
Certificate of Authorization No. 3818

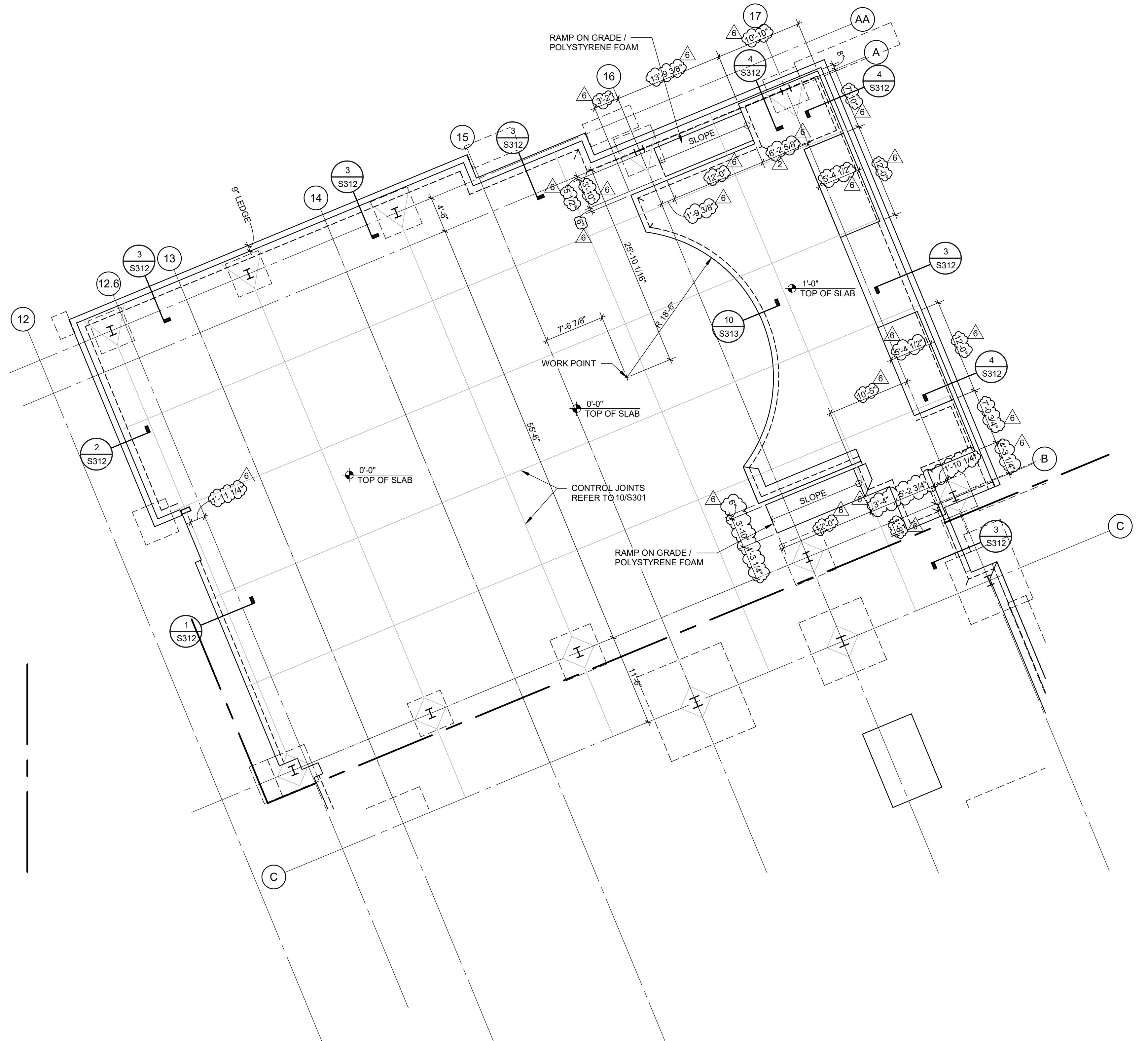
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CONFORMED SET  
02/14/2024

**S101.C**  
SLAB ON GRADE  
PLAN - LEVEL 1 -  
AREA C

### 1 SLAB ON GRADE PLAN - LEVEL 1 - AREA C

1/8" = 1'-0"



#### PLAN NOTES

- CIVIL DRAWINGS ELEVATION 20.00' = DATUM ELEVATION 0'-0".
- TOP OF FOUNDATION ELEVATION IS 2'-0" UNLESS NOTED OTHERWISE ON PLAN.
- FLOOR STRUCTURE IS A 5" THICK NORMALWEIGHT CONCRETE SLAB-ON-GRADE REINFORCED WITH WWR 6x6-W2.9xW2.9.
- PROVIDE 2-4x4"-0" ADDITIONAL SLAB REINFORCEMENT AT ALL RE-ENTRANT CORNERS.
- WALL FOOTINGS SHALL BE CENTERED ON THE CENTROID OF SUPPORTED COLUMN, UNLESS NOTED OTHERWISE.
- WALL FOOTINGS SHALL BE CENTERED ON THE SUPPORTED WALL, UNLESS NOTED OTHERWISE.
- ALIGN TOP OF GRADE BEAM (WITH TOP OF ADJACENT FOOTINGS), UNLESS NOTED OTHERWISE.
- REFER TO SHEET SERIES S03X FOR FOUNDATION AND SLAB-ON-GRADE SECTIONS AND DETAILS.
- REFER TO SHEET SERIES S00X FOR COLUMN SCHEDULES.
- REFER TO SHEET SERIES S01X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO ARCHITECTURAL DRAWINGS FOR EXTENTS AND DIMENSIONS OF RAISED OR DEPRESSED SLABS, TERRACES, SLOPES, CURBS, AND DRAINS. REFER TO TYPICAL DETAILS FOR REINFORCEMENT REQUIREMENTS.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLAB PENETRATIONS AND UNDERGROUND UTILITIES.
- COORDINATE THIS PLAN WITH ALL EXISTING CONDITIONS AND DIMENSIONS. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES.
- METAL PAN TREADS, RISERS, LANDINGS, STEEL STRINGERS, EMBEDED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.



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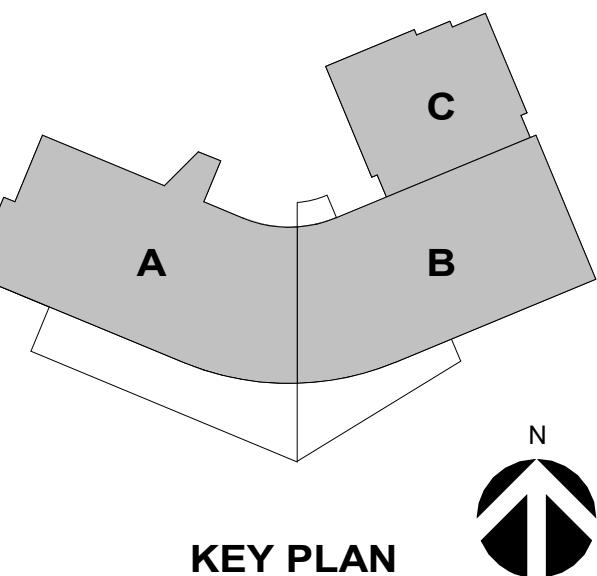
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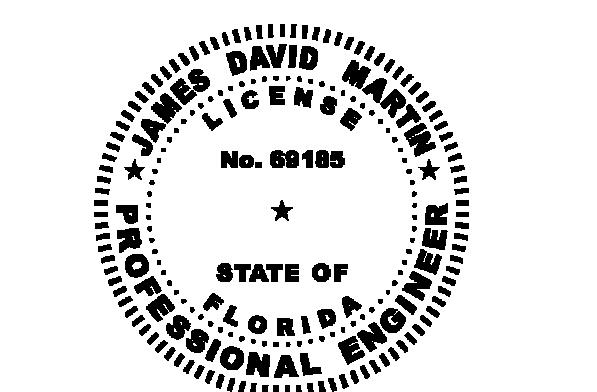


**Sarasota County  
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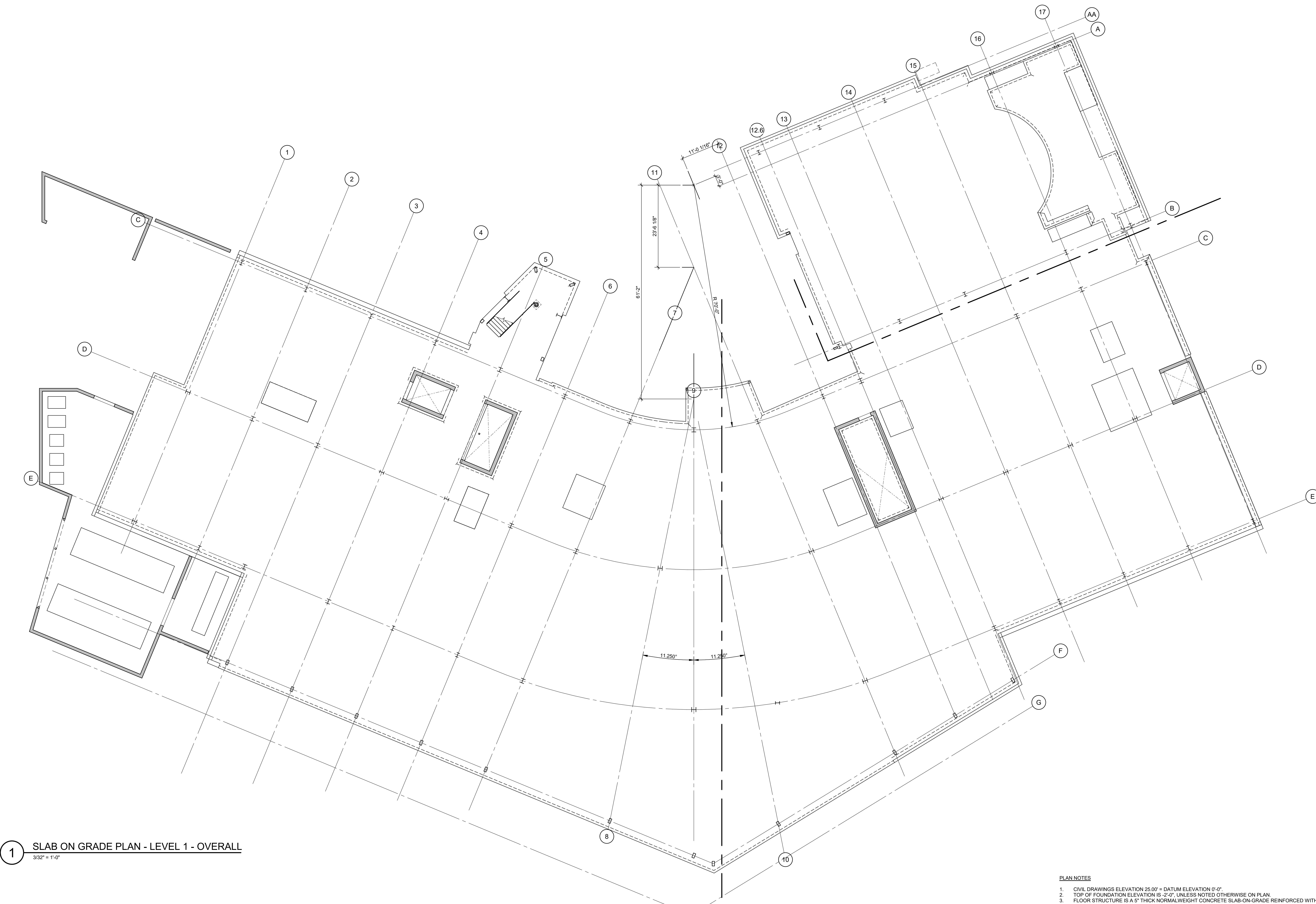
Revisions:



James David Martin, P.E. FL PE No. 69185  
WPM Project No. S03-22045-00  
Certificate of Authorization No. 3818

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**CONFORMED SET  
02/14/2024**



- PLAN NOTES**
- CIVIL DRAWINGS ELEVATION 20.00' = DATUM ELEVATION 0'-0".
  - TOP OF FOUNDATION ELEVATION IS 2'-0" UNLESS NOTED OTHERWISE ON PLAN.
  - FLOOR STRUCTURE IS A 5" THICK NORMALWEIGHT CONCRETE SLAB-ON-GRADE REINFORCED WITH WWR 6x6-2.9xW2.9.
  - PROVIDE 2-4x4'-0" ADDITIONAL SLAB REINFORCEMENT AT ALL RE-ENTRANT CORNERS.
  - WALL FOOTINGS SHALL BE CENTERED ON THE CENTROID OF SUPPORTED COLUMN, UNLESS NOTED OTHERWISE.
  - WALL FOOTINGS SHALL BE CENTERED ON THE SUPPORTED WALL, UNLESS NOTED OTHERWISE.
  - ALIGN TOP OF GRADE BEAM WITH TOP OF ADJACENT FOOTINGS, UNLESS NOTED OTHERWISE.
  - DO NOT CENTER WALL FOOTINGS ON SLAB-ON-GRADE SECTIONS AND DETAILS.
  - REFER TO SHEET SERIES S60X FOR COLUMN SCHEDULES.
  - REFER TO SHEET SERIES S61X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
  - REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR SLAB PENETRATIONS AND UNDERGROUND UTILITIES.
  - COORDINATE THIS DRAWING WITH EXISTING CONDITIONS AND DIMENSIONS. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES.
  - METAL PAN TREADS, RISERS, LANDINGS, STEEL STRINGERS, EMBEDED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.

**S101**  
SLAB ON GRADE  
PLAN - LEVEL 1 -  
OVERALL

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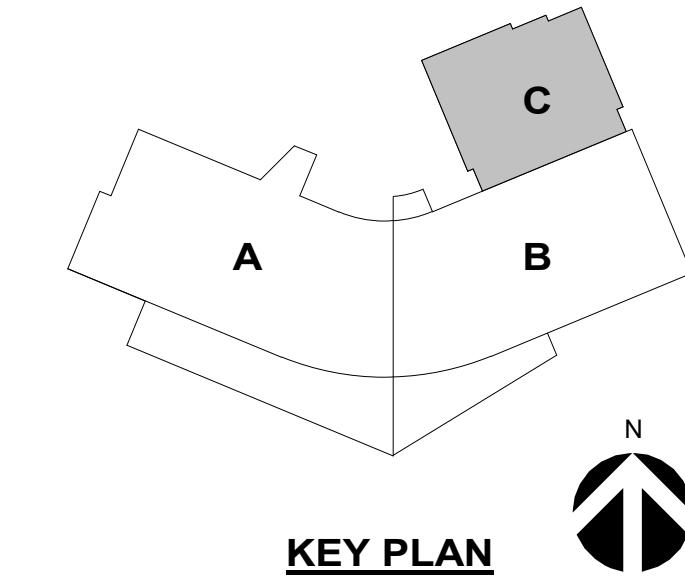
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## Sarasota County Administration Center

1 Apex Road  
Sarasota, Florida 34240

No. 68185

STATE OF

FLORIDA

PROFESSIONAL ENGINEER

No. 68185

DAVID MARTIN

LICEN

CE

22.23005.00

BD

JDM

09/08/2023

Revisions:

A Pre-GMP ASI #1

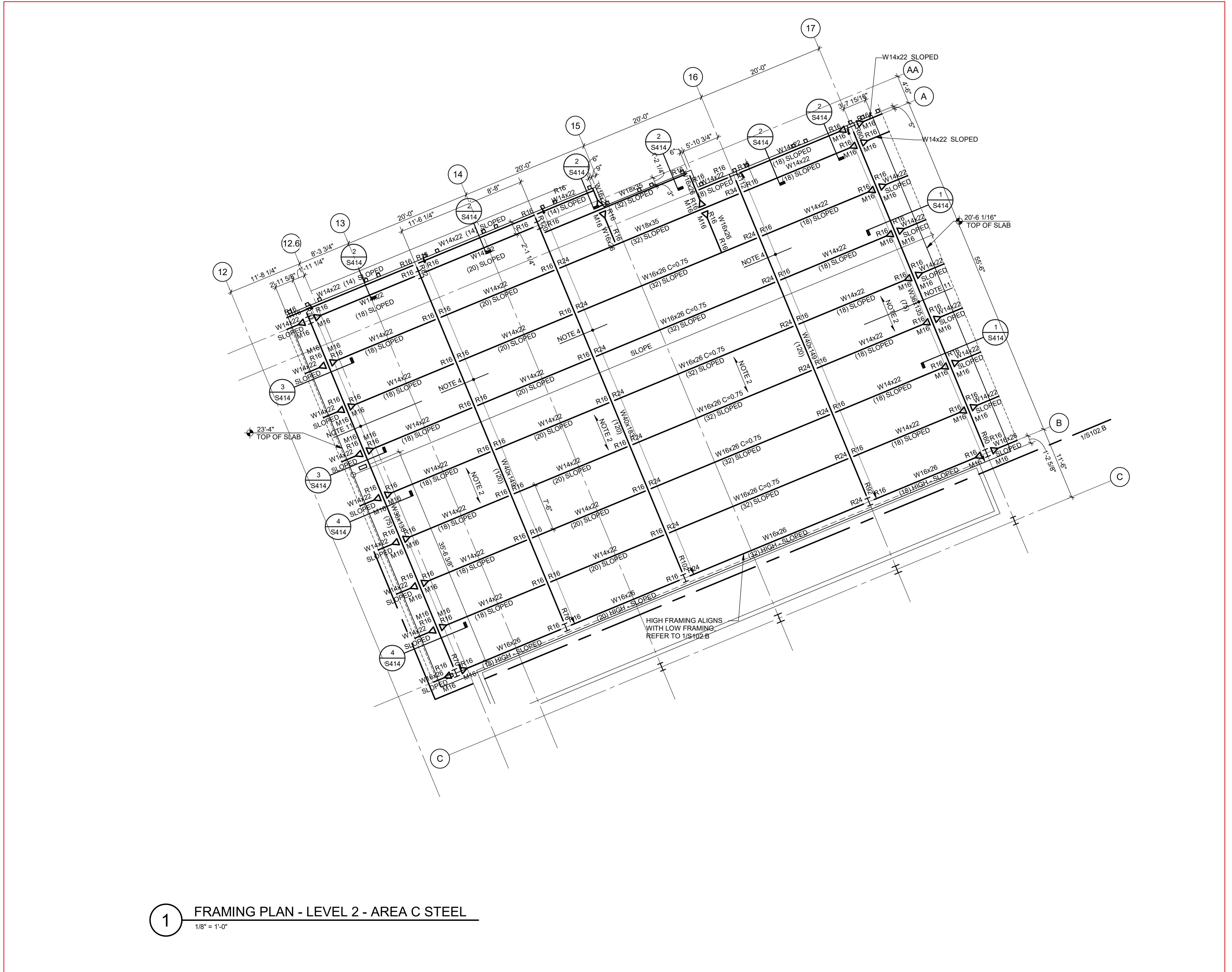
09/27/2023

To the best of Engineer's knowledge, the plans and specifications comply with all applicable codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S102.C**  
FRAMING PLAN -  
LEVEL 2 - AREA C

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### 1 FRAMING PLAN - LEVEL 2 - AREA C STEEL

1/8" = 1'-0"

#### PLAN NOTES

- TOP OF CONCRETE ELEVATION IS NOTED ON PLAN.
- FLOOR STRUCTURE CONSISTS OF 3 1/2" LIGHT WEIGHT CONCRETE ON 2" DEEP, 18 GAGE GALVANIZED (G90) COMPOSITE STEEL DECK (5 1/2" TOTAL THICKNESS). STEEL DECK SHALL BE PLACED WITH A TWO-SPAN CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD. THE CONCRETE SLAB SHALL BE REINFORCED WITH WELDED SMOOTH WIRE REINFORCEMENT AT 4#4@12" AND 4#4@12".
- ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
- REFER TO SHEET SERIES S40X AND S41X FOR FLOOR FRAMING SECTIONS AND DETAILS.
- PLACE #4#4@12" ACROSS STEEL GIRDERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL I/S402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- REFER TO SHEET SERIES S41X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS.
- GRAVITY CONNECTIONS ARE ONLY ALLOWED WHERE SHOWN. REFER TO S5.5X SERIES FOR PROPOSED LAYOUT.
- PLACE #4#4@12" ACROSS STEEL GIRDERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL I/S402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PLACE #4#4@12" ACROSS STEEL BEAMS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL I/S402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PROVIDE ANGLE OR LOOKER/KICKER AT EACH COLUMN ALONG GRID C AND E FOR DECK SUPPORT. REFER TO TYPICAL DETAILS.
- METAL PAN TREADS: LANDINGS, STEEL STRINGERS, EMBED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.
- EPX = EMBED PLATE TYPE "X". REFER TO I/S41.11 FOR EMBED PLATE ELEVATIONS.
- REFER TO SHEET SERIES S41X FOR WINDOW WASHING EQUIPMENT LOCATIONS. LOCATIONS SHOWN FOR CONCEPT ONLY AND ARE NOT FINAL. ALL FINAL QUANTITIES AND LOCATIONS ARE TO BE DETERMINED BY THE WINDOW WASHING EQUIPMENT MANUFACTURER IN ACCORDANCE WITH APPLICABLE CODES AND REGULATIONS.

James David Martin, P.E. FL PE No. 68185  
WPM Project No. S05-2204900  
Certificate of Authorization No. 3818



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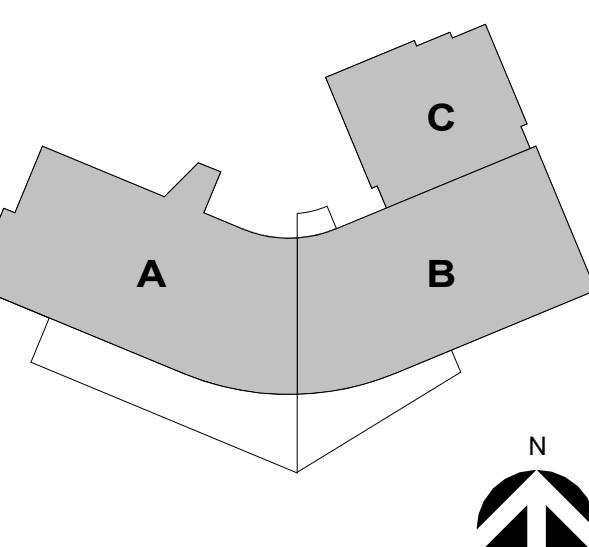
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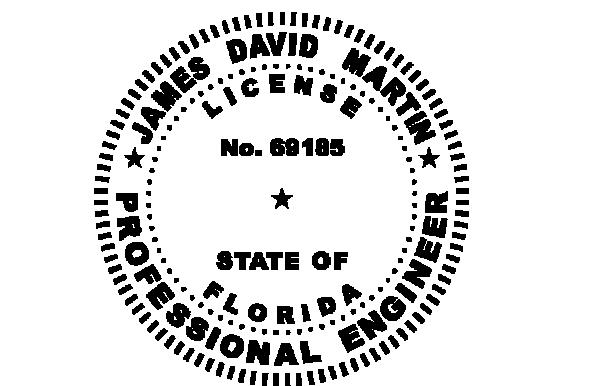


KEY PLAN

**Sarasota County  
Administration Center**  
1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

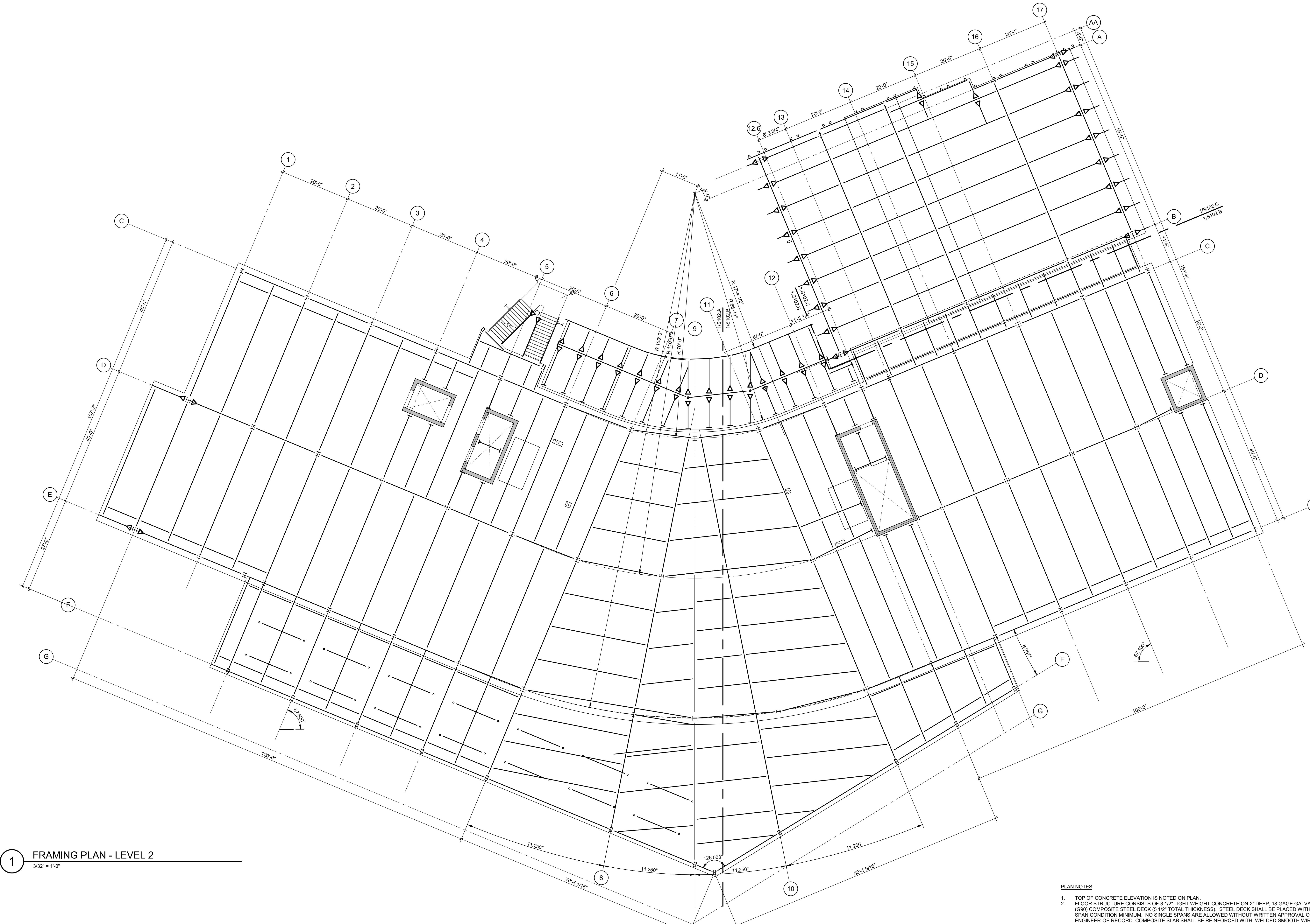
Revisions:



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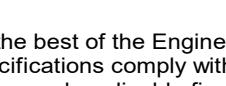
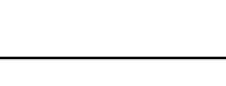
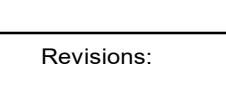
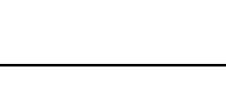
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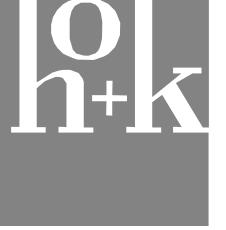
CONFORMED SET  
02/14/2024



**S102**  
FRAMING PLAN -  
LEVEL 2 - OVERALL

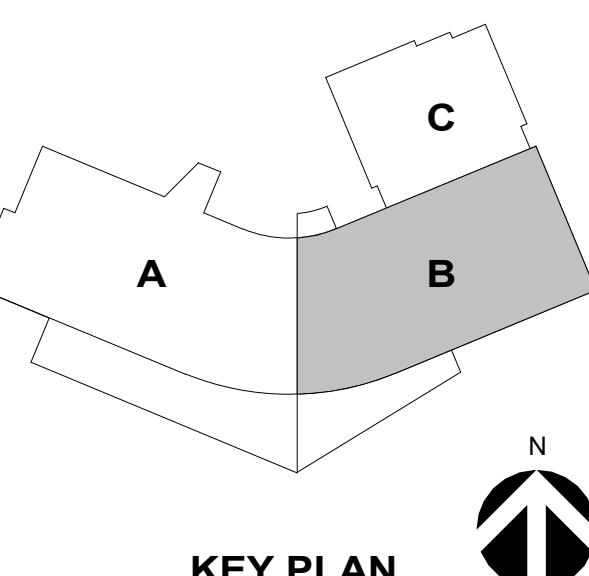
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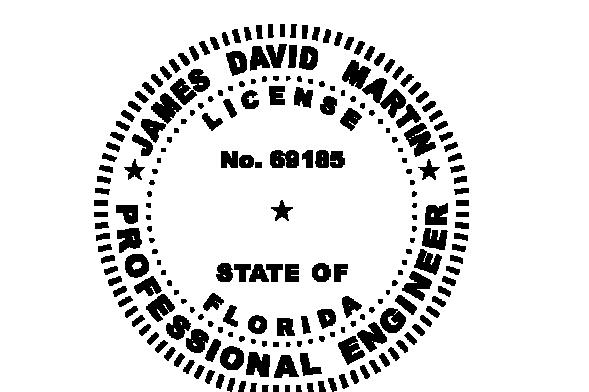


## Sarasota County Administration Center

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.23005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
6 ASI-07 09/26/2024  
7 ASI-09 10/25/2024



To the best of the Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024



### PLAN NOTES

- TOP OF CONCRETE ELEVATION IS NOTED ON PLAN.
- FLOOR STRUCTURE CONSISTS OF 3 1/2" LIGHT WEIGHT CONCRETE ON 2" DEEP, 18 GAGE GALVANIZED (G90) COMPOSITE STEEL DECK (5 1/2" TOTAL THICKNESS). STEEL DECK SHALL BE PLACED WITH A TWO-Span CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD. THE CONCRETE SLAB SHALL BE REINFORCED WITH WELDED SMOOTH WIRE REINFORCEMENT AT #4@14" AND #4@14".
- ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
- PROVIDE A CONTINUOUS 1/4" BENT PLATE WITH 3/4" x 10" HEADED STUDS SPACED AT 1'-0" AT ALL SLAB EDGES AND OPENINGS UNLESS NOTED OTHERWISE.
- PLACE #4#8-0" @12" ACROSS STEEL GIRDERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 14-0202. USE #4#8-0" ACROSS GIRDERS AT END OF SPANNERS.
- ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
- REFER TO SHEET SERIES S40X AND S41X FOR FLOOR FRAMING SECTIONS AND DETAILS.
- REFER TO SHEET SERIES S41X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO SHEET SERIES S41X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS.
- GRAVITY CONNECTIONS ARE ONLY ALLOWED WHERE SHOWN. REFER TO S5.SX SERIES FOR PROPOSED LAYOUT.
- PLACE #4#16-0" @12" ACROSS STEEL GIRDERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 14-0202. USE #4#16-0" ACROSS GIRDERS AT END OF SPANNERS.
- PLACE #4#25-0" @12" ACROSS STEEL BEAMS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 11-0402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PROVIDE METAL PAN TREADS, LANDINGS, STEEL STRINGERS, EMBED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.
- EPK = EMBED PLATE TYPE "X". REFER TO S2/S4.11 FOR EMBED PLATE ELEVATIONS.

**S103.B**  
FRAMING PLAN -  
LEVEL 3 - AREA B



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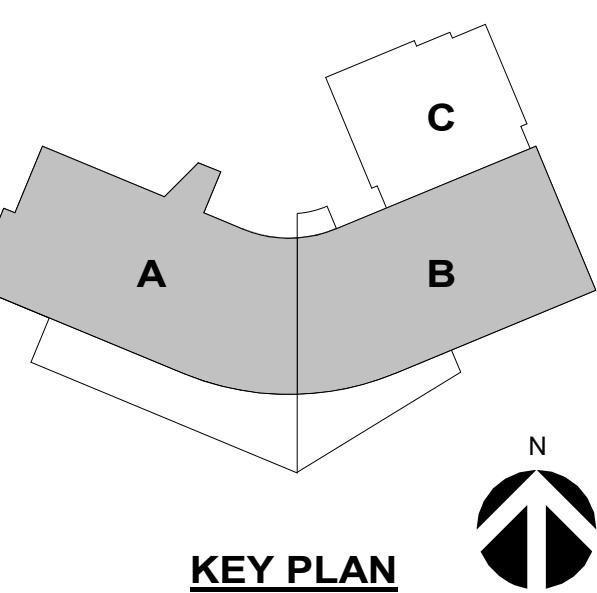
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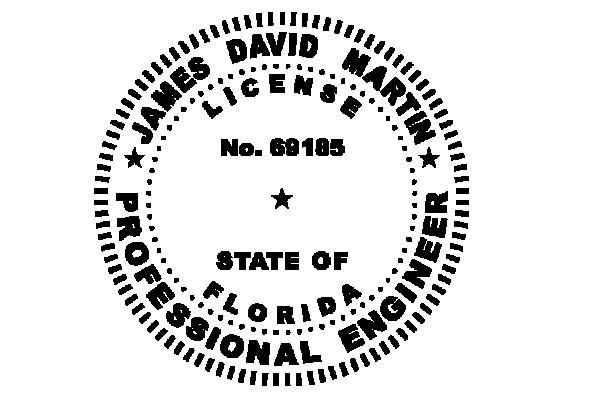


**Sarasota County  
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Project No. 22.29005.00  
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Date 09/08/2023

Revisions:



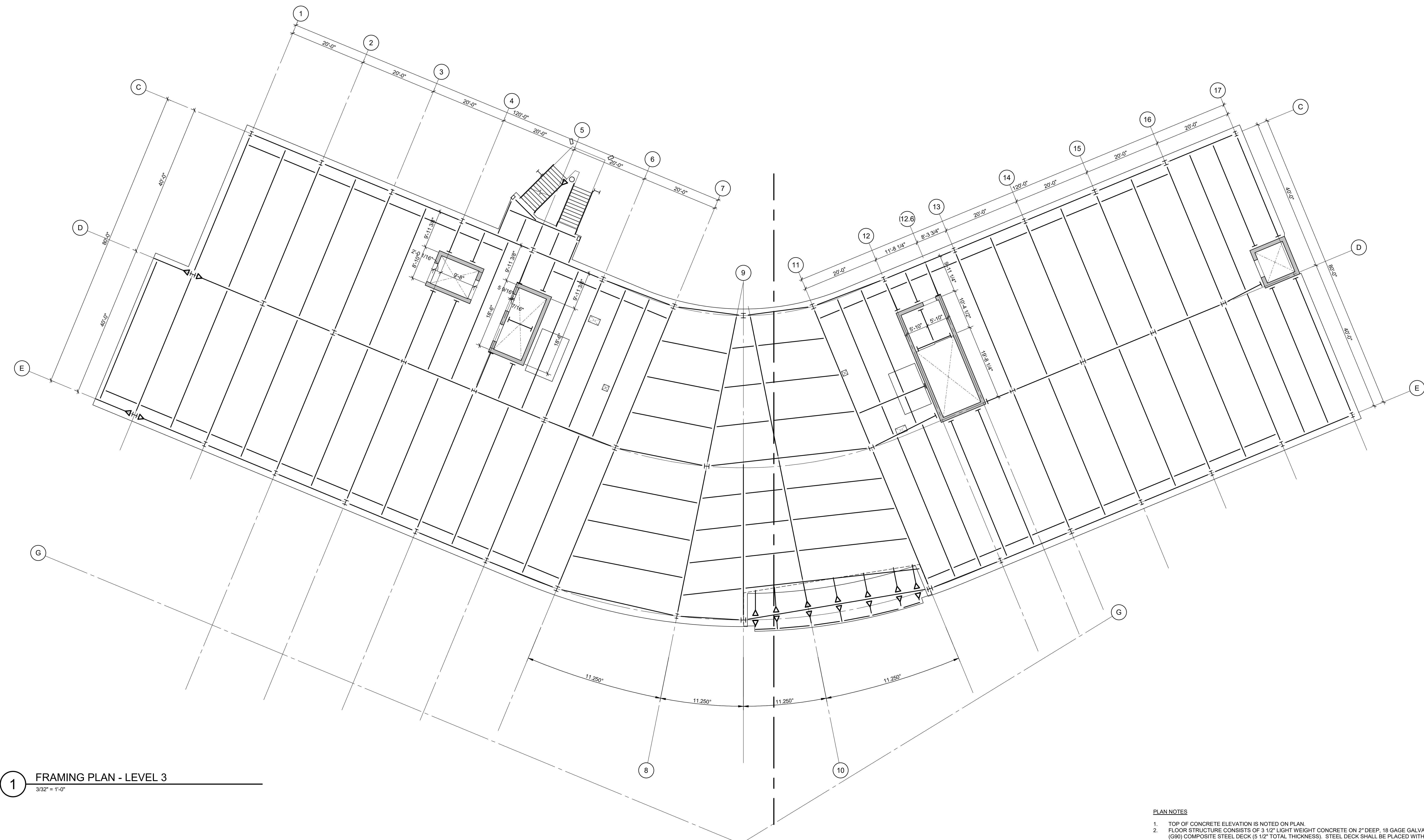
James David Martin, P.E. FL PE No. 69185  
WPM Project No. S03-2204-09  
Certificate of Authorization No. 3818

To the best of the Engineer's knowledge, the plans and specifications comply with all applicable laws, including codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

**CONFORMED SET  
02/14/2024**

**S103**  
FRAMING PLAN -  
LEVEL 3 - OVERALL

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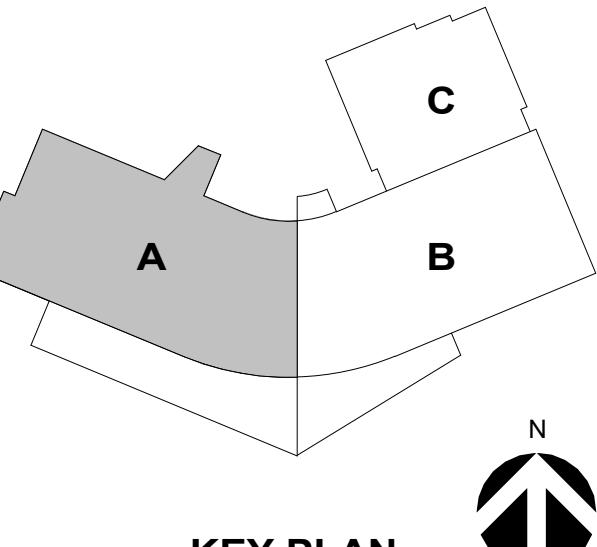
PLAN NOTES

- TOP OF CONCRETE ELEVATION IS NOTED ON PLAN.
- FLOOR STRUCTURE CONSISTS OF 3 1/2" LIGHT WEIGHT CONCRETE ON 2" DEEP, 18 GAGE GALVANIZED (G90) COMPOSITE STEEL DECK (5 1/2" TOTAL THICKNESS). STEEL DECK SHALL BE PLACED WITH A TWO-Span Condition Minimum. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD. COMPOSITE SLAB SHALL BE REINFORCED WITH WELDED SMOOTH WIRE REINFORCEMENT AS SHOWN IN THE DRAWINGS.
- ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
- REFER TO SHEET SERIES S40X AND S41X FOR FLOOR FRAMING SECTIONS AND DETAILS.
- REFER TO SHEET SERIES S40X AND S41X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS.
- LOCATIONS OF PRE-CAST GRAVITY CONNECTION TO STRUCTURE, GRAVITY CONNECTIONS ARE ONLY ALLOWED WHERE SHOWN. REFER TO S5.5X SERIES FOR PROPOSED LAYOUT.
- PLACE #4#16-0" @ 12" ACROSS STEEL GIRDERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL H-104. PLACE #4#16-0" ACROSS STEEL GIRDERS AND STEEL MEMBERS.
- PLACE #4#25-0" @ 12" ACROSS STEEL BEAMS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 11/5402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PROVIDE ANGLE OUTLOOKER/KICKER AT EACH COLUMN ALONG GRID C AND E FOR DECK SUPPORT. REFER TO DETAIL H-104.
- METAL PAN TRENDERS, LANDINGS, STEEL STRINGERS, EMBED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.
- EPX = EMBED PLATE TYPE "X". REFER TO S284.11 FOR EMBED PLATE ELEVATIONS.
- REFERS TO HOW WASHERS, PEGS, AND EQUIPMENT DATA LOCATIONS ARE SHOWN FOR CONCEPT ONLY AND ARE NOT FINAL. ALL FINAL QUANTITIES AND LOCATIONS ARE TO BE DETERMINED BY THE WINDOW WASHING EQUIPMENT MANUFACTURER IN ACCORDANCE WITH APPLICABLE CODES AND REGULATIONS.



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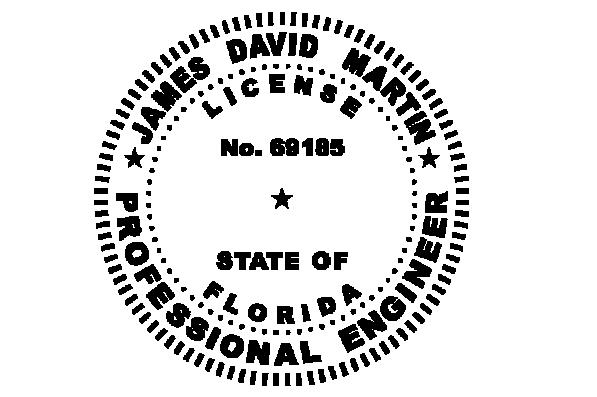


KEY PLAN

**Sarasota County Administration Center**  
1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
2 ASI-02 05/17/2024  
7 ASI-09 10/25/2024



James David Martin, P.E. FL PE No. 69185  
WPM Project No. 505-22045-00  
Certificate of Authorization No. 3818

To the best of Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

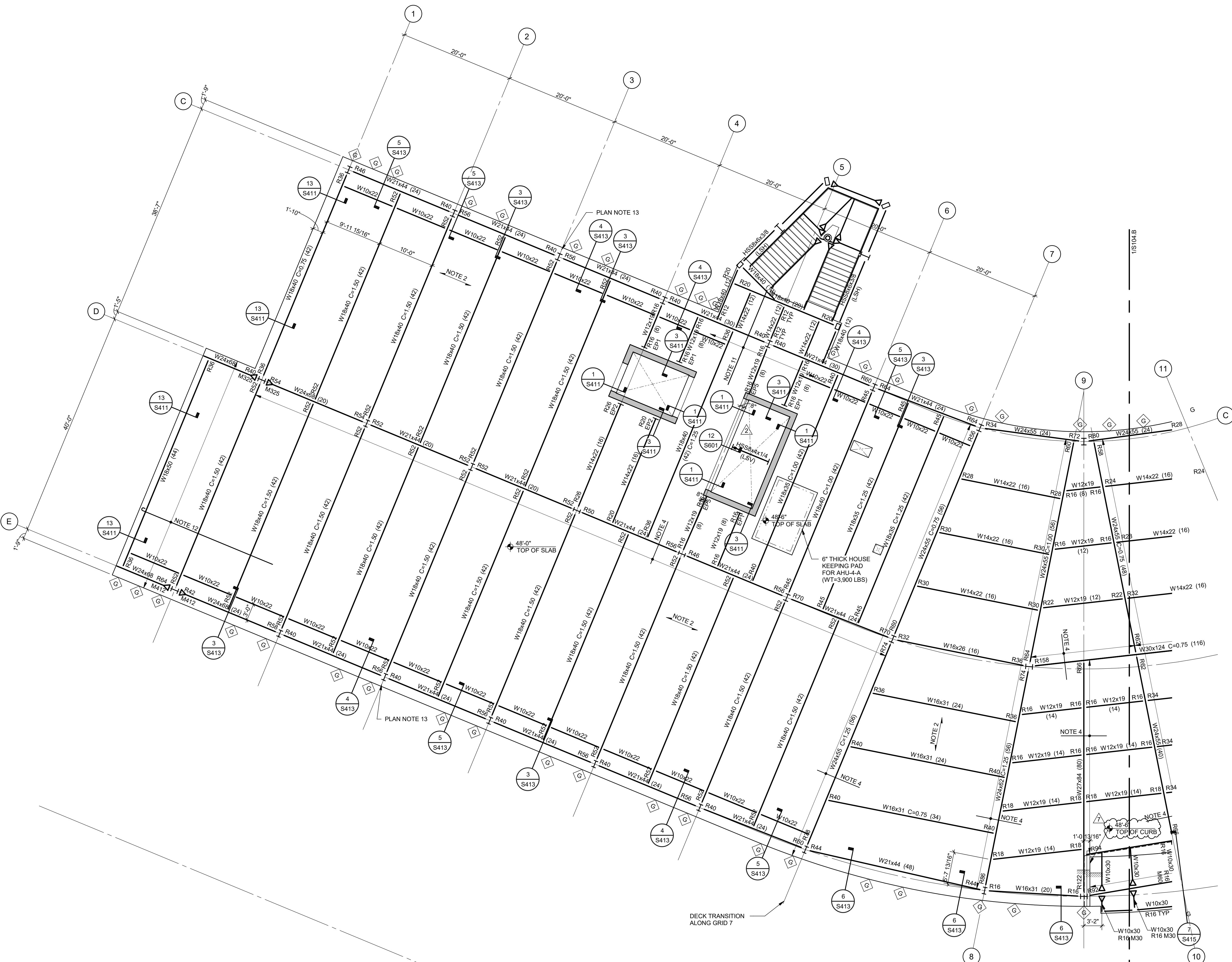
**CONFORMED SET**  
02/14/2024

**1 FRAMING PLAN - LEVEL 4 - AREA A - STEEL**

1/8" = 1'-0"

**S104.A**  
FRAMING PLAN -  
LEVEL 4 - AREA A

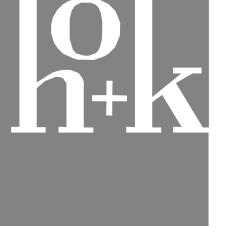
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PLAN NOTES

- TOP OF CONCRETE ELEVATION IS NOTED ON PLAN.
- FLOOR STRUCTURE CONSISTS OF 3 1/2" LIGHT WEIGHT CONCRETE ON 2" DEEP, 18 GAGE GALVANIZED (G90) COMPOSITE STEEL DECK (5 1/2" TOTAL THICKNESS). STEEL DECK SHALL BE PLACED WITH A TWO-Span Condition Minimum. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD. THE CONCRETE SLAB SHALL BE REINFORCED WITH WELDED SMOOTH WIRE REINFORCEMENT AT #4@14" AND #4@14" IN SPANS.
- ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
- REFER TO SHEET SERIES S40X AND S41X FOR FLOOR FRAMING SECTIONS AND DETAILS.
- REFER TO SHEET SERIES S40X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN.
- REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS.
- ALL CONNECTIONS ARE ONLY ALLOWED WHERE SHOWN. REFER TO S5.5X SERIES FOR PROPOSED LAYOUT.
- PLACE #4#16-0" @ 17" ACROSS STEEL GIRDERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 11/4402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PLACE #4#25-0" @ 17" ACROSS STEEL BEAMS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 11/4402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PROVIDE ANGLE OR LOOKER/KICKER AT EACH COLUMN ALONG GRID C AND E FOR DECK SUPPORT. REFER TO SHEET SERIES S40X AND S41X FOR FLOOR FRAMING SECTIONS AND DETAILS.
- METAL PAN TREADS, RISERS, LANDINGS, STEEL STRINGERS, EMBED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS ARE PROVIDED BY DELEGATED STAIR ENGINEER.
- EPK = EMBED PLATE TYPE "X". REFER TO S4.11 FOR EMBED PLATE ELEVATIONS.





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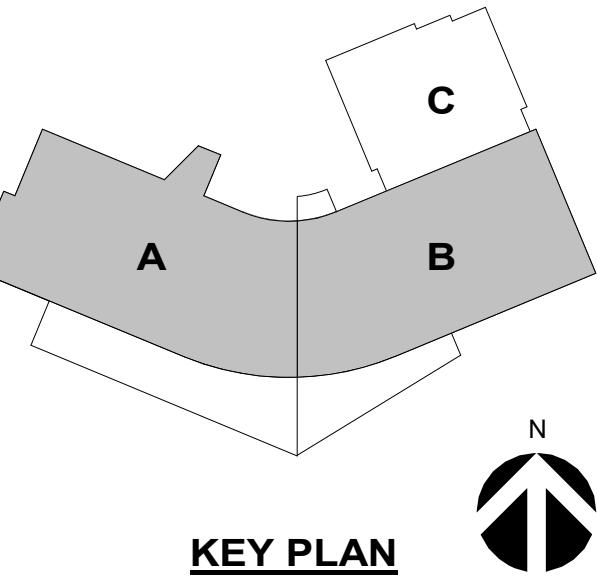
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Walter P. Moore  
Structural Engineers  
201 East Kennedy Blvd., Suite 700  
Tampa, FL 33602

**SIEBEIN** Sieben Acoustics  
ACOUSTICS  
625 NW 60th St, Suite C  
Gainesville, FL 32607

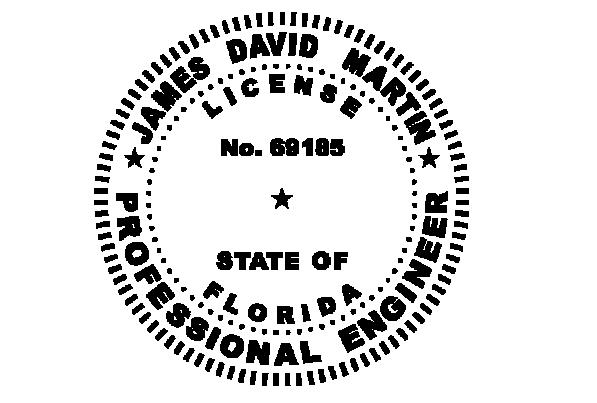


**Sarasota County  
Administration Center**

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

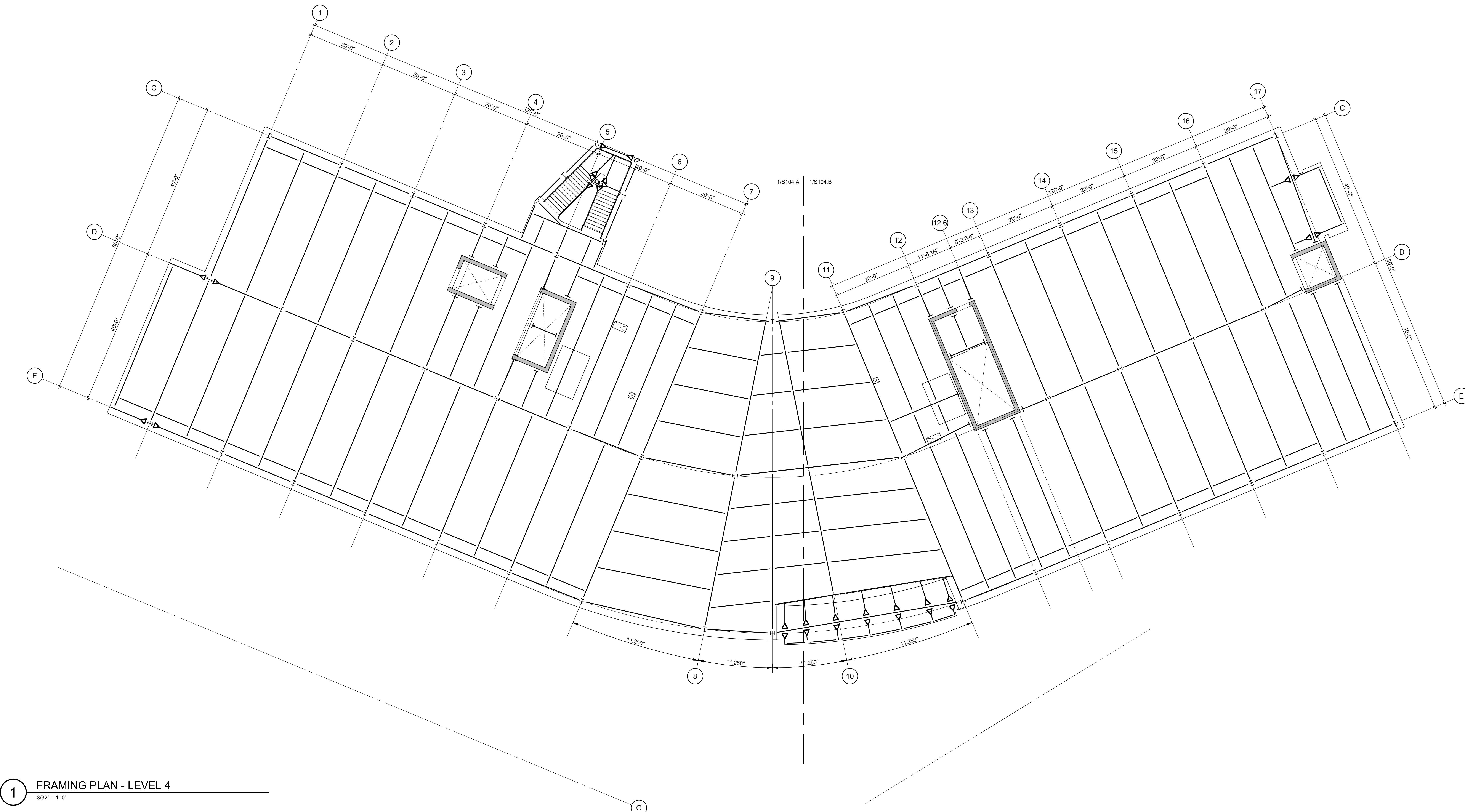
Revisions:



James David Martin, P.E., FL PE No. 69185  
WPM Project No. S05-2204-00  
Certificate of Authorization No. 69185

To the best of the Engineer's knowledge, the plans and specifications comply with all applicable laws, including codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

**CONFORMED SET  
02/14/2024**



1 FRAMING PLAN - LEVEL 4

3/32" = 1'-0"

PLAN NOTES

- TOP OF CONCRETE ELEVATION IS NOTED ON PLAN.
- FLOOR STRUCTURE CONSISTS OF 3 1/2" LIGHT WEIGHT CONCRETE ON 2" DEEP, 18 GAGE GALVANIZED (G90) COMPOSITE STEEL DECK (5 1/2" TOTAL THICKNESS). STEEL DECK SHALL BE PLACED WITH A TWO-Span SECTION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD. COMPOSITE SLAB SHALL BE REINFORCED WITH WELDED SMOOTH WIRE REINFORCEMENT AS SHOWN ON DRAWINGS.
- ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
- REFER TO SHEET SERIES S40X AND S41X FOR FLOOR FRAMING SECTIONS AND DETAILS.
- REFER TO SHEET SERIES S40X AND S41X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO SHEET SERIES S41X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS.
- THESES LOCATIONS OF PRE-CAST GRAVITY CONNECTION TO STRUCTURE. GRAVITY CONNECTIONS ARE ONLY ALLOWED WHERE SHOWN. REFER TO S5.X SERIES FOR PROPOSED LAYOUT.
- PLACE #4#16-0" @ 12" ACROSS STEEL GRID BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL H-1. PROVIDE 180 DEGREE HOOK ON END OF CANTILEVER.
- PLACE #4#25-0" @ 12" ACROSS STEEL BEAMS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL H-1/402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PROVIDE ANGLE OUTLOOKER/KICKER AT EACH COLUMN ALONG GRID C AND E FOR DECK SUPPORT. REFER TO TYPICAL DETAILS.
- METAL PAN TREADS: RISERS, LANDINGS, STEEL STRINGERS, EMBED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.
- EPX = EMBED PLATE TYPE "XX". REFER TO S2/S4.11 FOR EMBED PLATE ELEVATIONS.
- REFERS TO SHEET S40X HOW WASHERS/FITTINGS/JOINTS/LOCATIONS ARE SHOWN FOR CONCEPT ONLY AND ARE NOT FINAL. ALL FINAL QUANTITIES AND LOCATIONS ARE TO BE DETERMINED BY THE WINDOW WASHING EQUIPMENT MANUFACTURER IN ACCORDANCE WITH APPLICABLE CODES AND REGULATIONS.

**S104**  
FRAMING PLAN -  
LEVEL 4 - OVERALL

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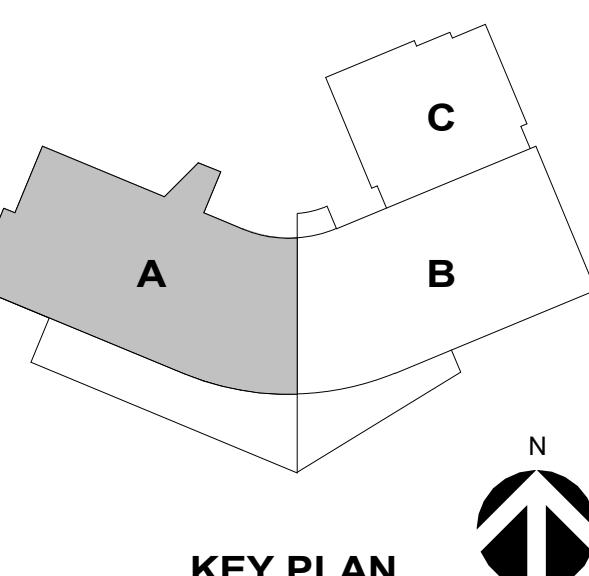
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Structural Engineers  
201 East Kennedy Blvd., Suite 700  
Tampa, FL 33602

**SIEBEIN ACOUSTIC**  
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Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607



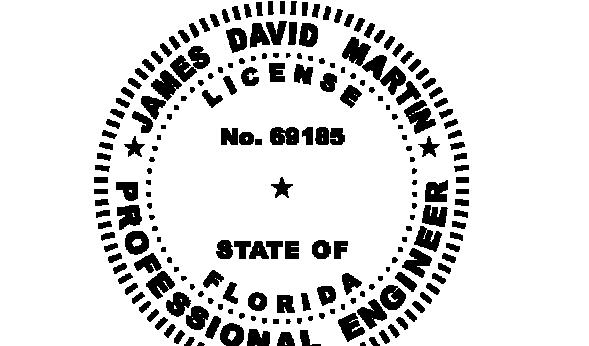
KEY PLAN

## Sarasota County Administration Center

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
2 ASI-02 05/17/2024  
3 ASI-03 06/07/2024  
6 ASI-07 09/26/2024



To the best of the Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S105.A**  
FRAMING PLAN -  
ROOF - AREA A

FRAMING PLAN -  
ROOF - AREA A

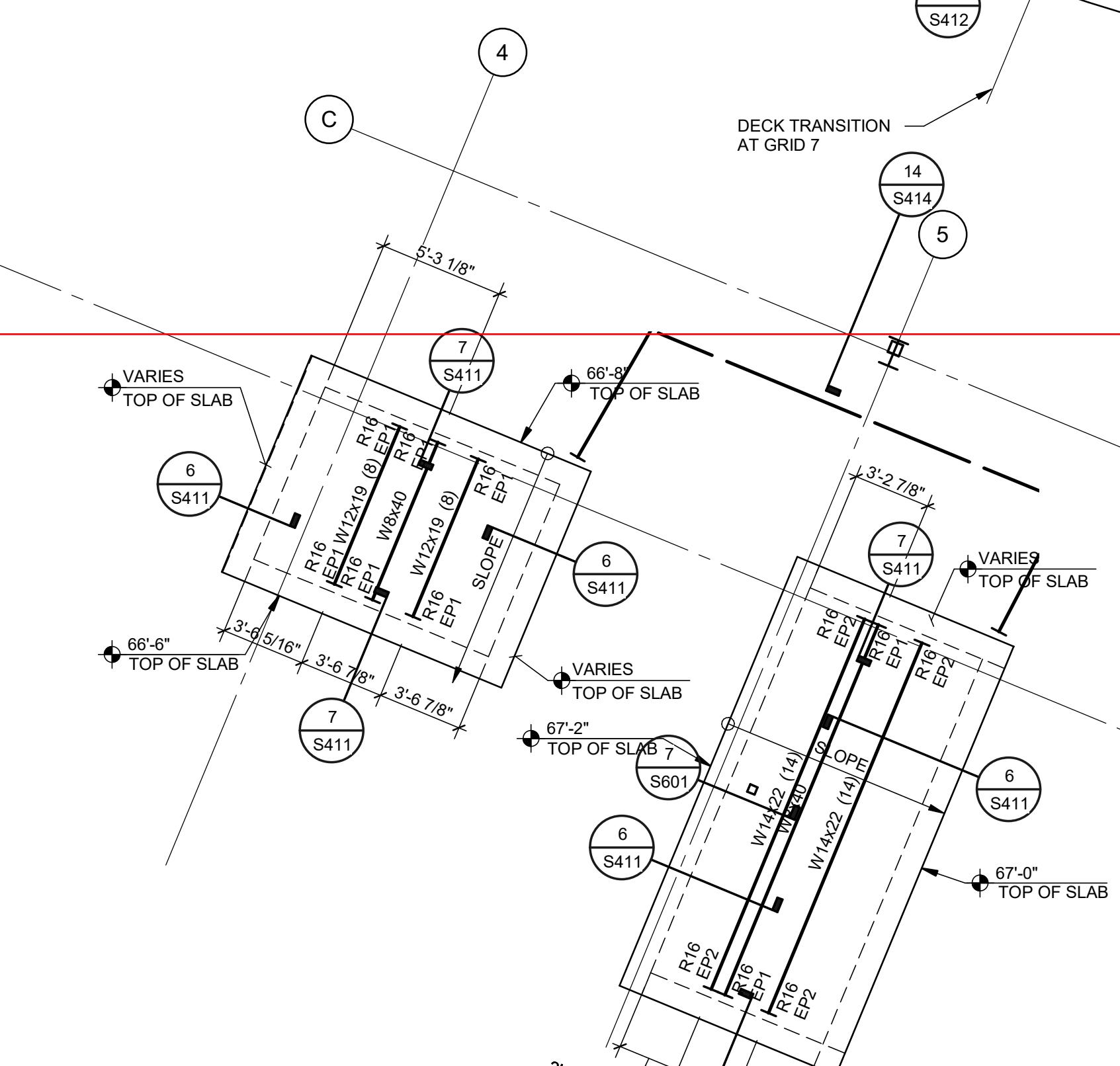
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### 1 FRAMING PLAN - ROOF - AREA A - STEEL

1/8" = 1'-0"

ROOF DAVITS AND ASSOCIATED STEEL  
FRAMING, STIFFENERS, AND  
CONNECTIONS HAVE BEEN DELETED  
FROM THE PROJECT SCOPE.



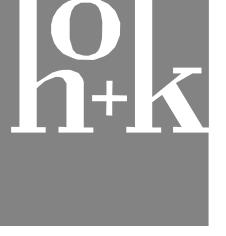
### 2 PARTIAL PLAN - ELEVATOR OVERRUN

3/16" = 1'-0"

#### PLAN NOTES

- TOP OF CONCRETE ELEVATION IS NOTED ON PLAN.
- FLOOR STRUCTURE CONSISTS OF 3 1/2" LIGHT WEIGHT CONCRETE ON 2" DEEP, 18 GAGE GALVANIZED (G90) COMPOSITE STEEL DECK (5 1/2" TOTAL THICKNESS). STEEL DECK SHALL BE PLACED WITH A TWO-Span CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD. THE CONCRETE SLAB SHALL BE REINFORCED WITH WELDED SMOOTH WIRE REINFORCEMENT AT #4@14" AND #4@14" IN SPANS.
- ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
- PROVIDE A CONTINUOUS 1/4" BENT PLATE WITH 3/4" x 10" HEADED STUDS SPACED AT 1'-0" AT ALL SLAB EDGES AND OPENINGS UNLESS NOTED OTHERWISE.
- PLACE #4#8-0" @12" ACROSS SLAB SPANNERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL #4-16-0". USE #4#8-0" SPANNERS FOR SHEAR WALL SUPPORTS.
- REFER TO SHEET SERIES S40X AND S41X FOR FLOOR FRAMING SECTIONS AND DETAILS.
- REFER TO SHEET SERIES S41X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN.
- REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS.
- GRAVITY CONNECTIONS ARE ONLY ALLOWED WHERE SHOWN. REFER TO S5.5X SERIES FOR PROPOSED LAYOUT.
- PLACE #4#16-0" @12" ACROSS STEEL GIRDERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL #4-16-0". USE #4#16-0" SPANNERS FOR SHEAR WALL SUPPORTS.
- PLACE #4#25-0" @12" ACROSS STEEL BEAMS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 11/40Z. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PROVIDE ANGLE LOOKER/KICKER AT EACH COLUMN ALONG GRID C AND E FOR DECK SUPPORT. REFER TO TYPICAL DETAILS.
- METAL PAN TREADS: RISERS, LANDINGS, STEEL STRINGERS, EMBED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.
- EPX = EMBED PLATE TYPE "X". REFER TO 12/S4.11 FOR EMBED PLATE ELEVATIONS.





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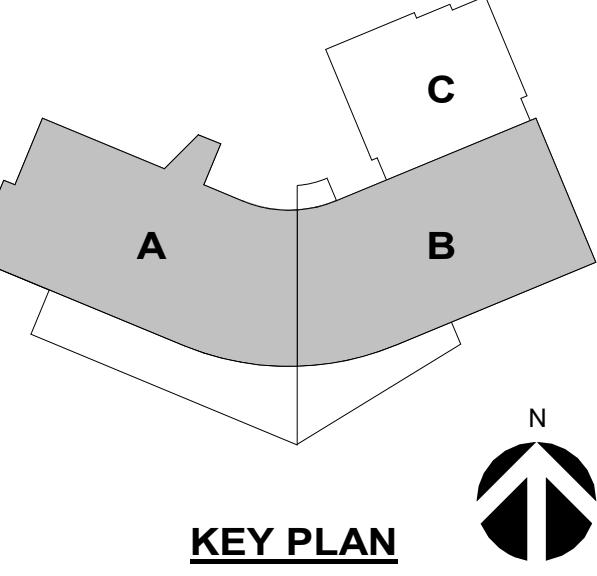
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625 NW 60th St, Suite C  
Gainesville, FL 32607

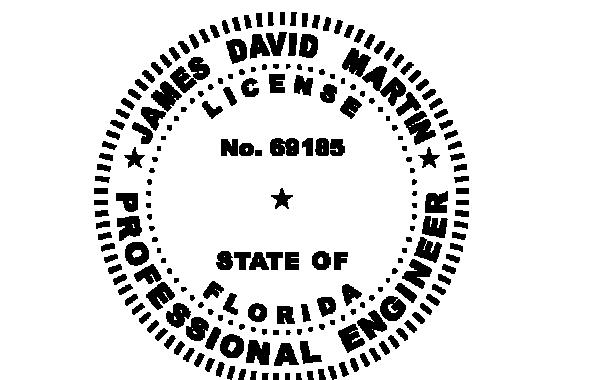


KEY PLAN

**Sarasota County  
Administration Center**  
1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:



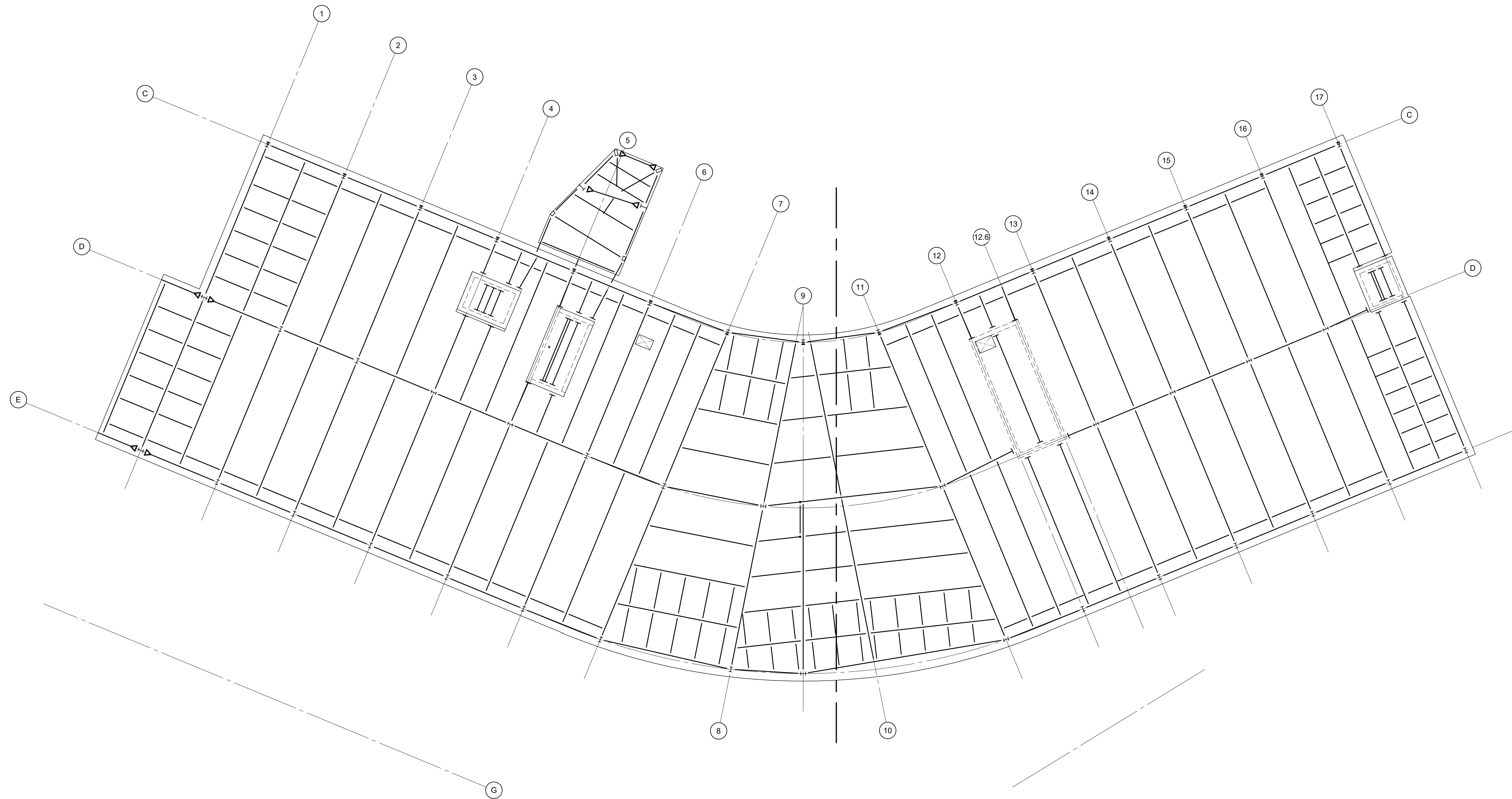
James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-2204-00  
Certificate of Authorization No. 3818

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**CONFORMED SET  
02/14/2024**

**S105**  
FRAMING PLAN -  
ROOF - OVERALL

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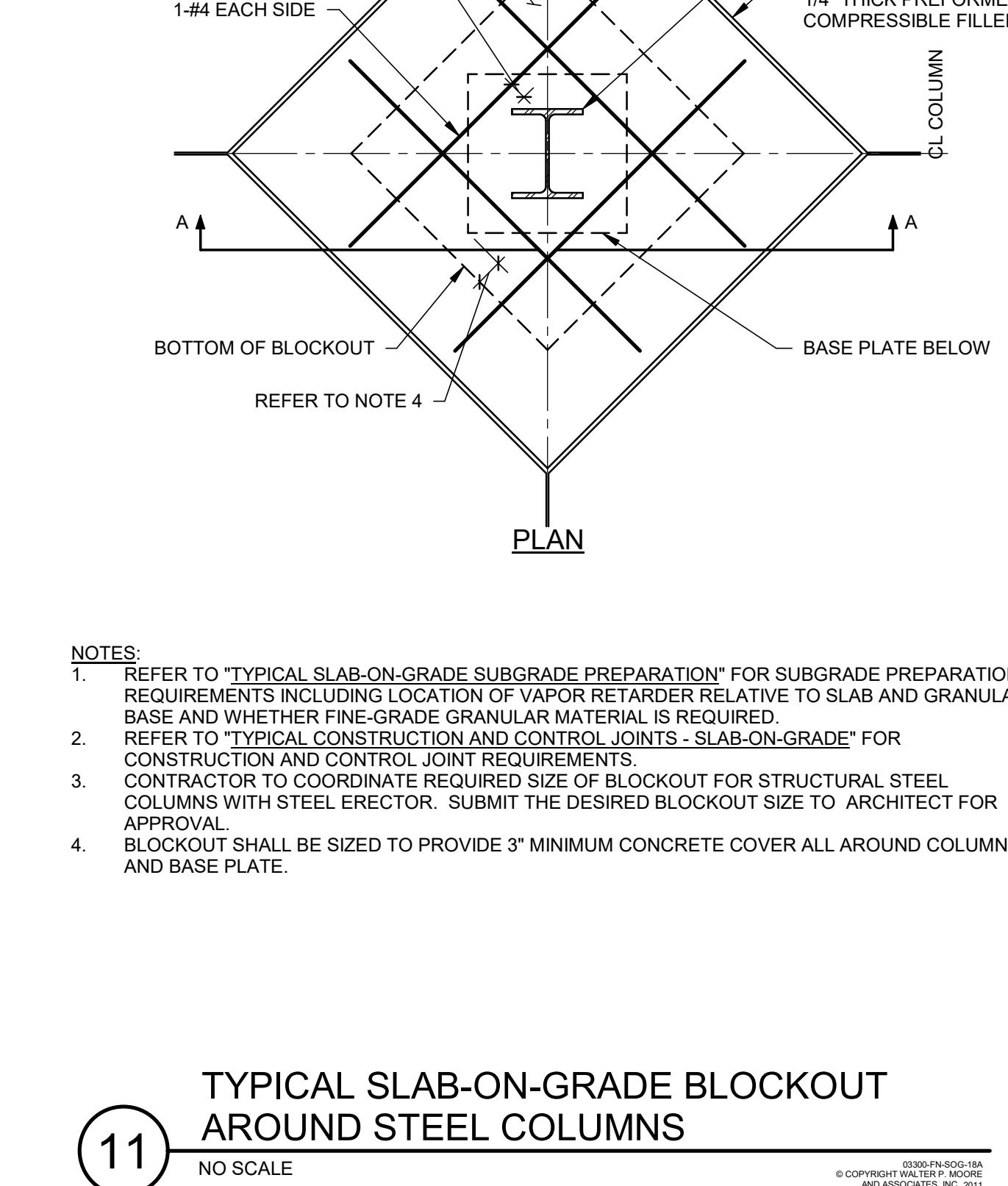
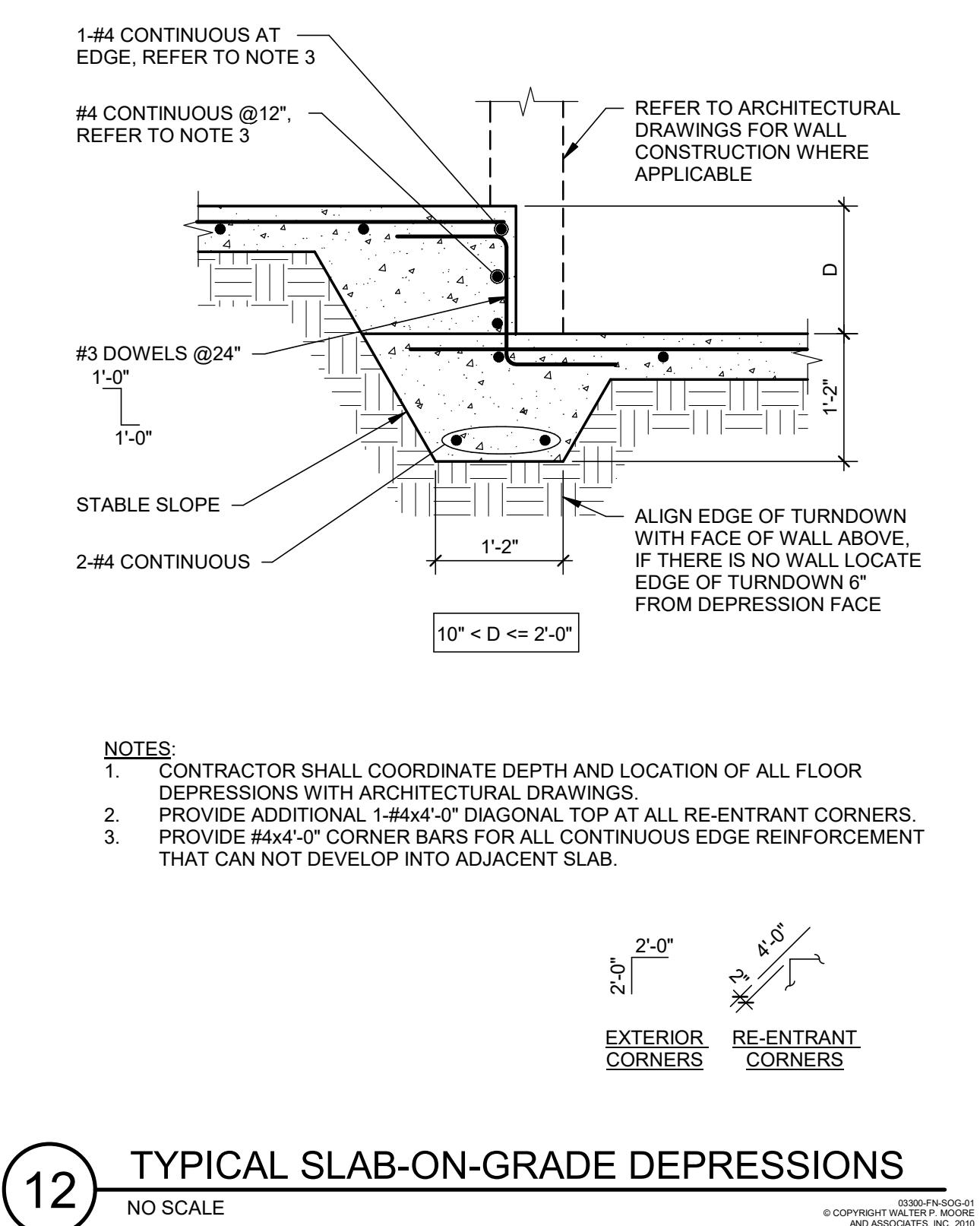
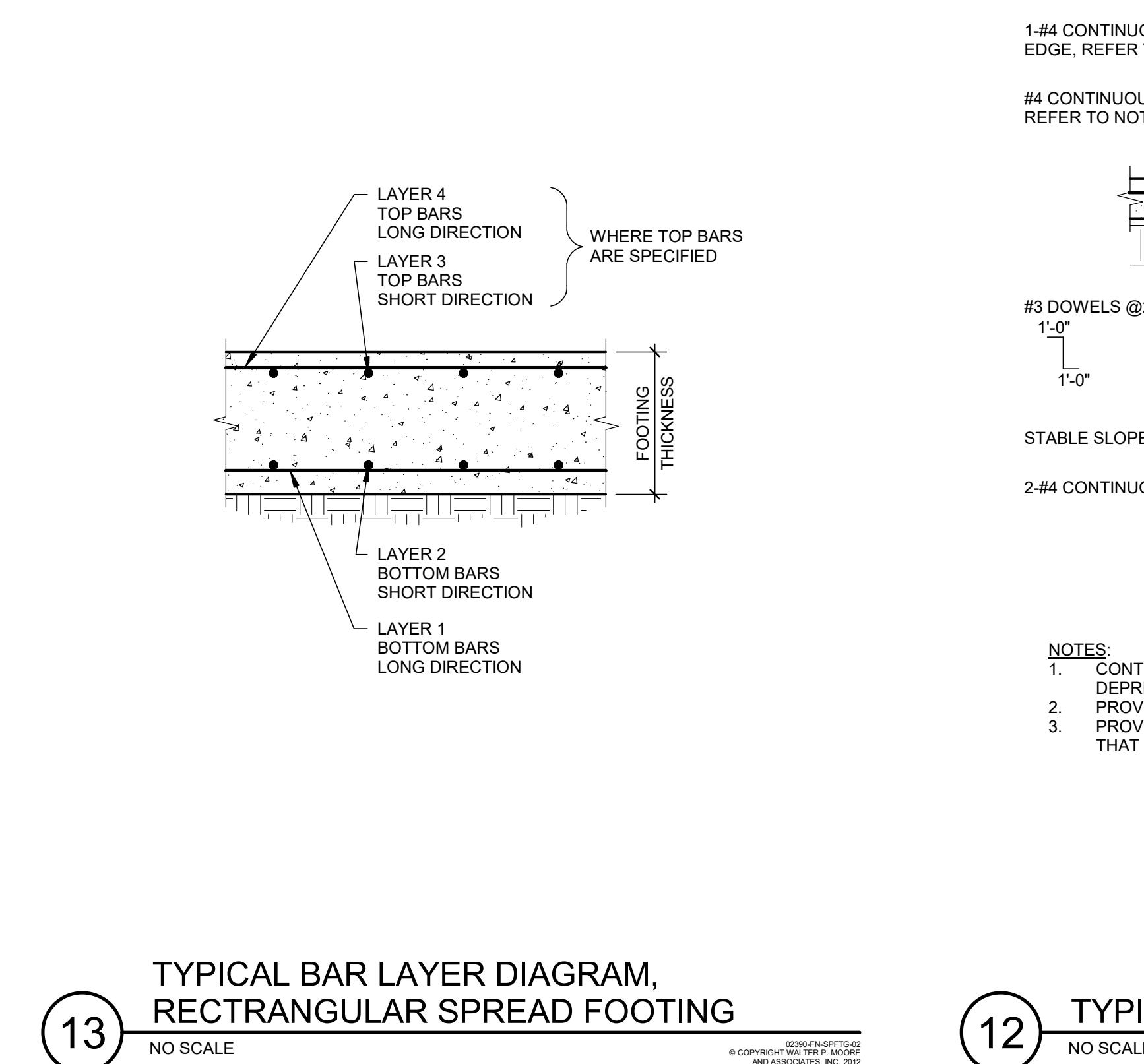
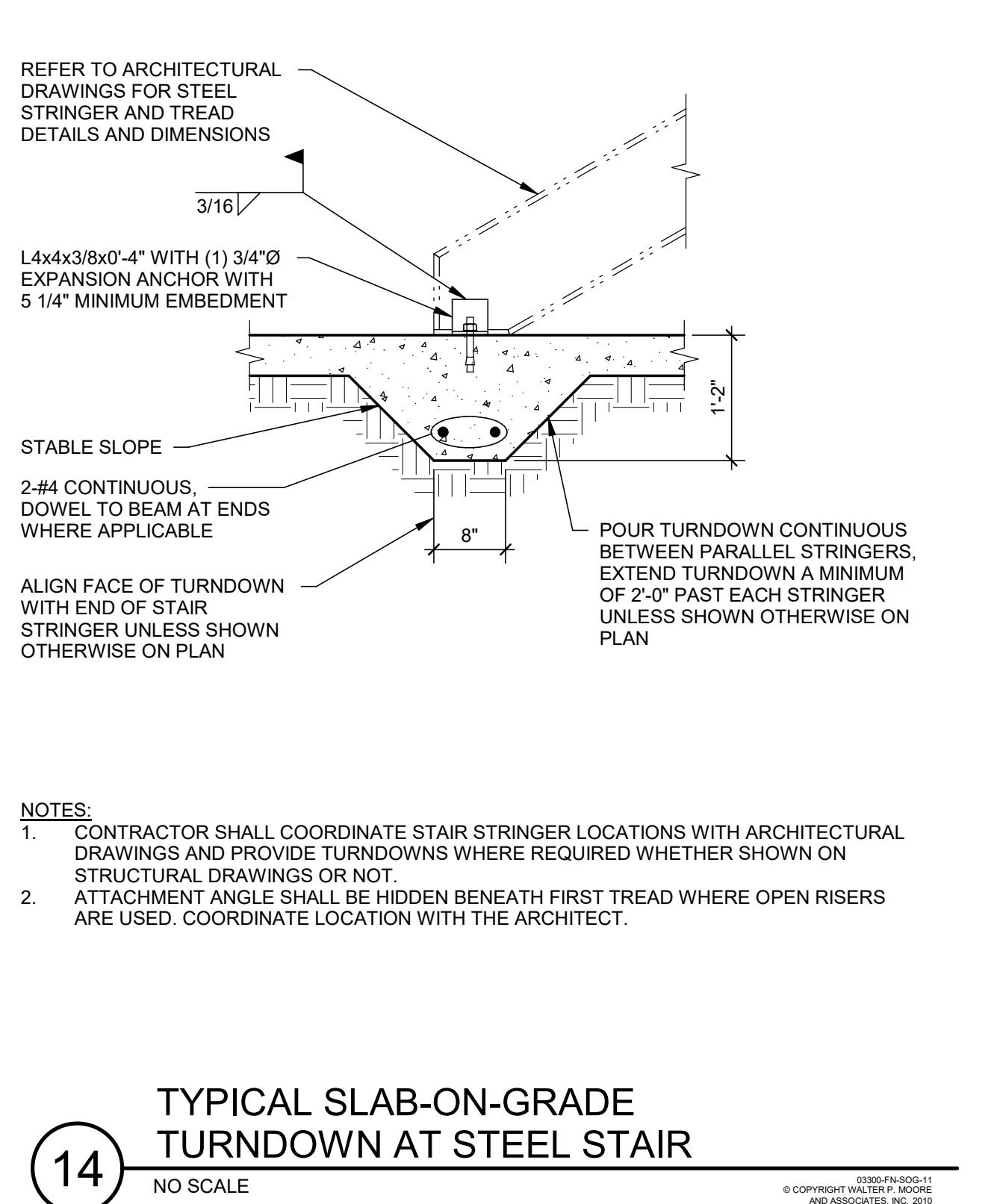
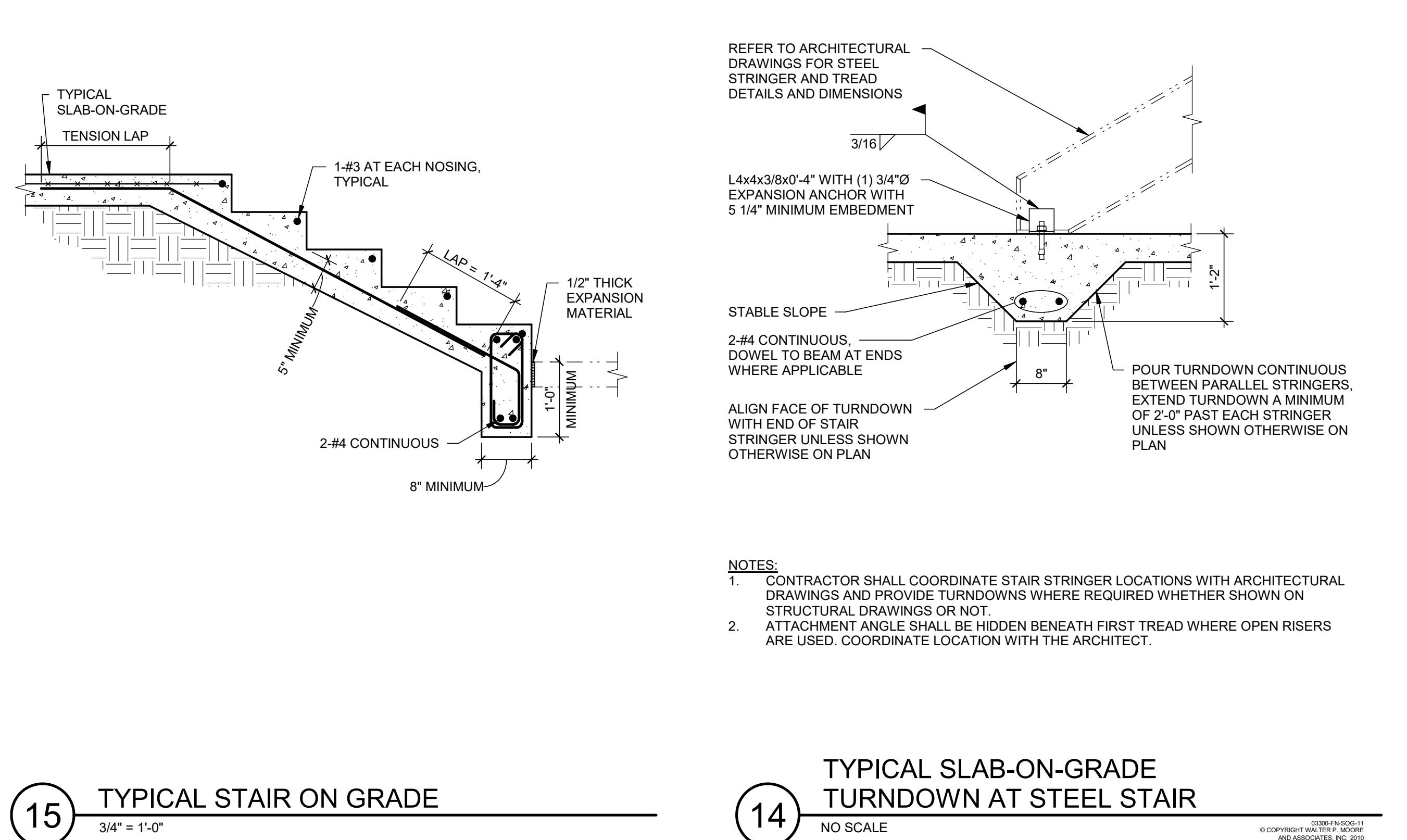
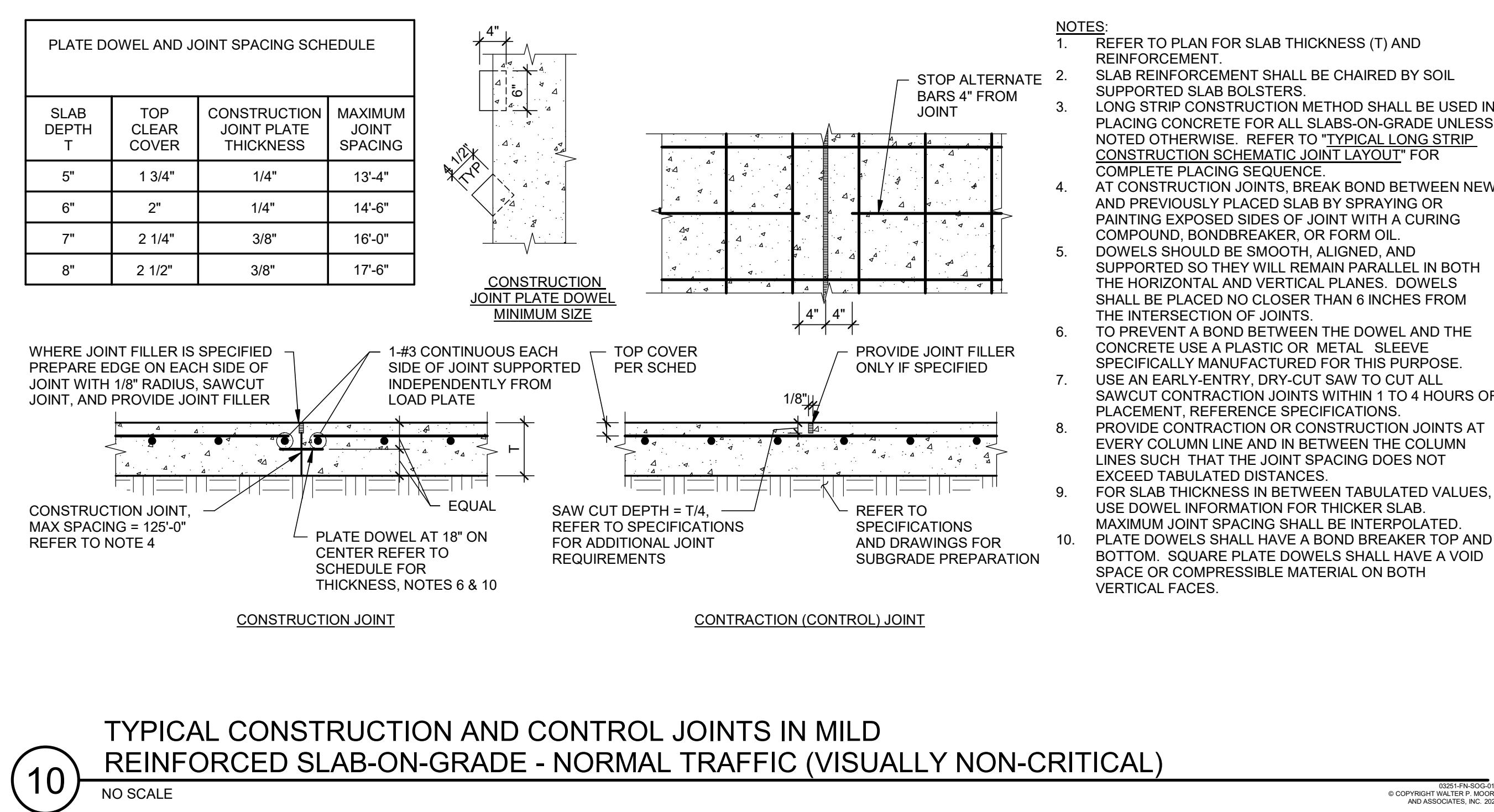
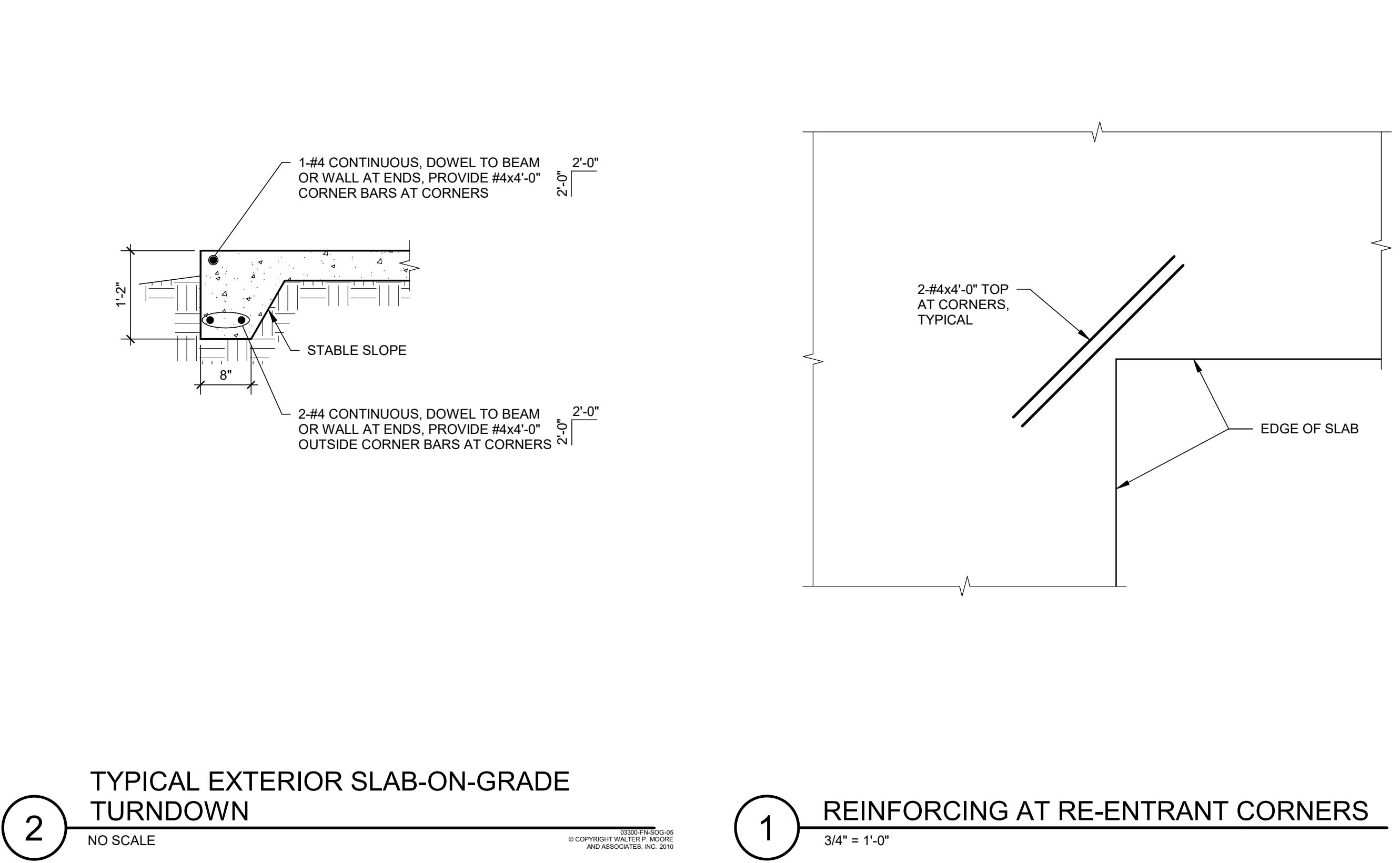
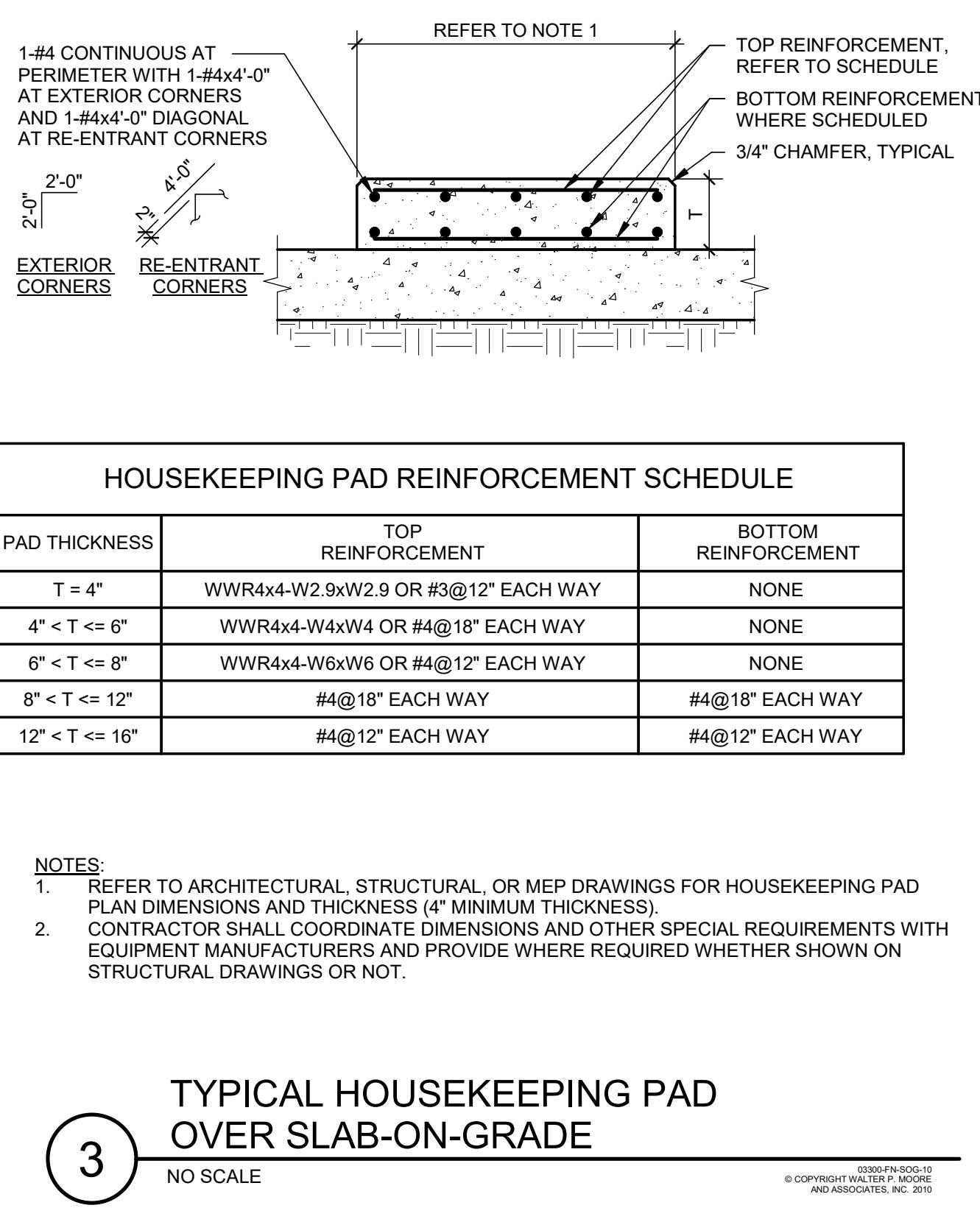
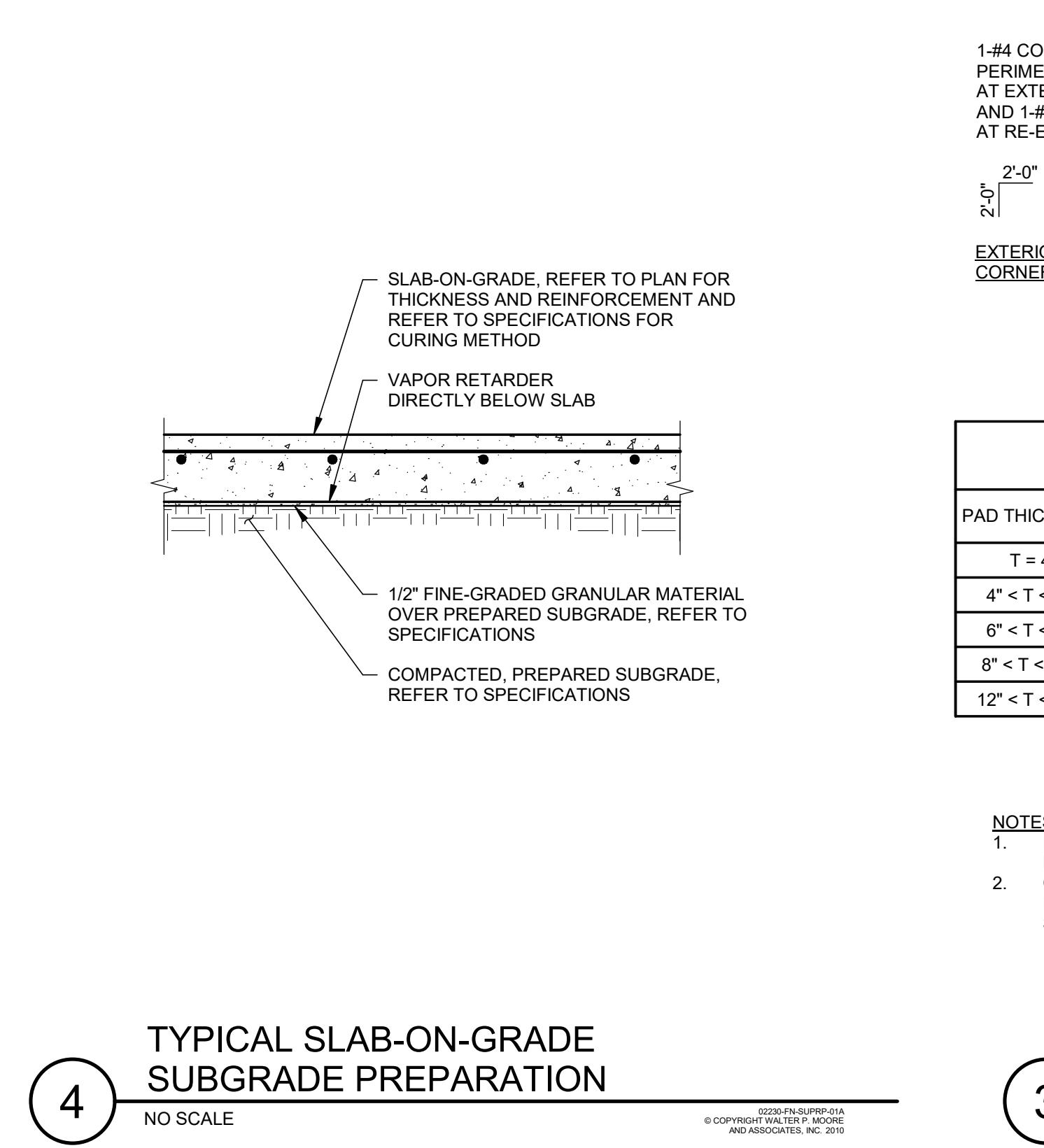
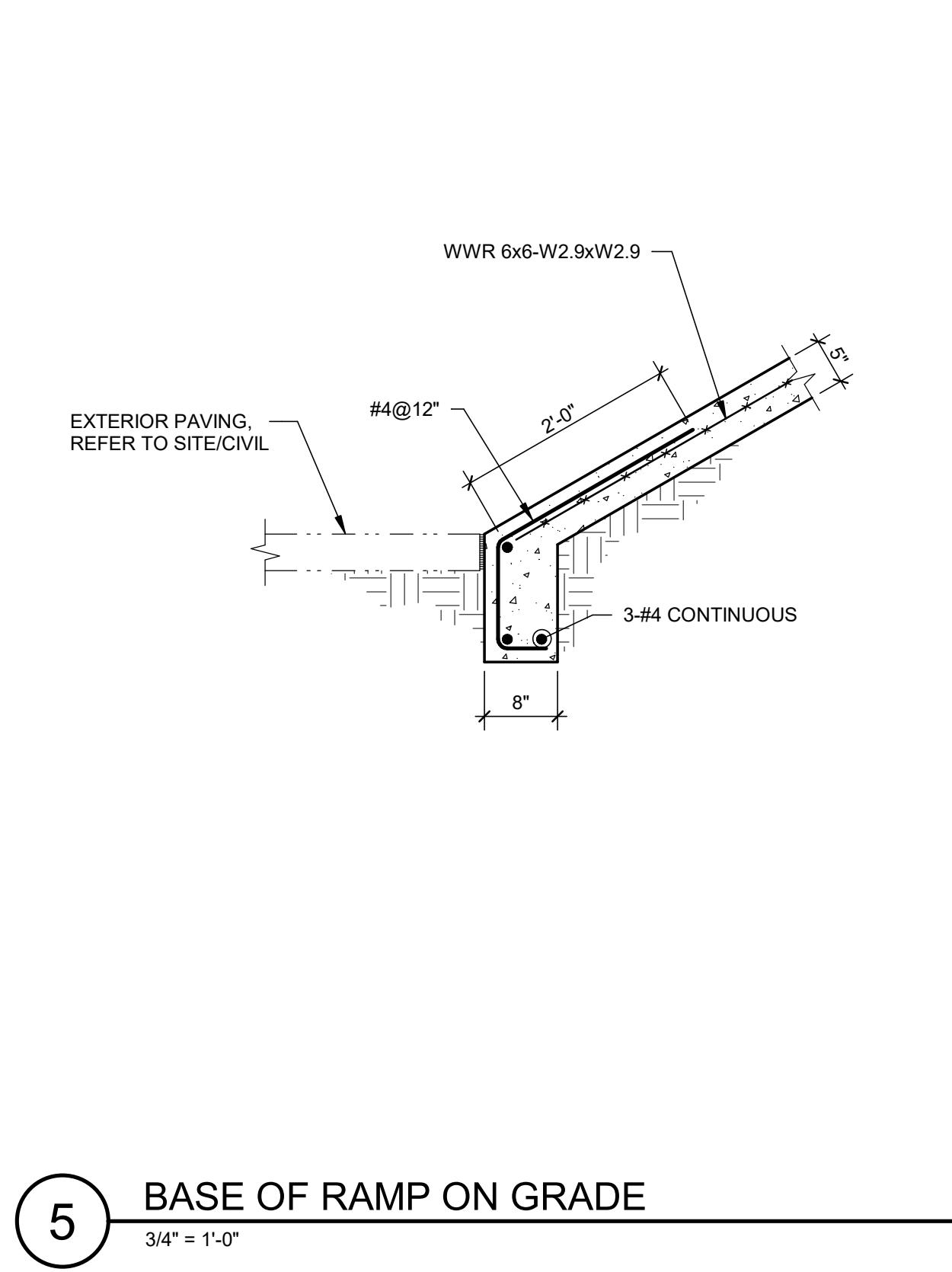
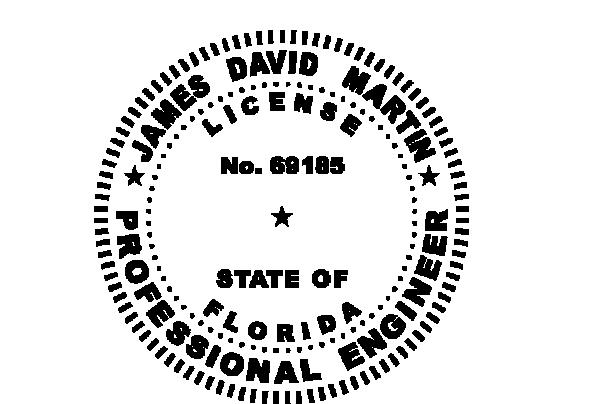


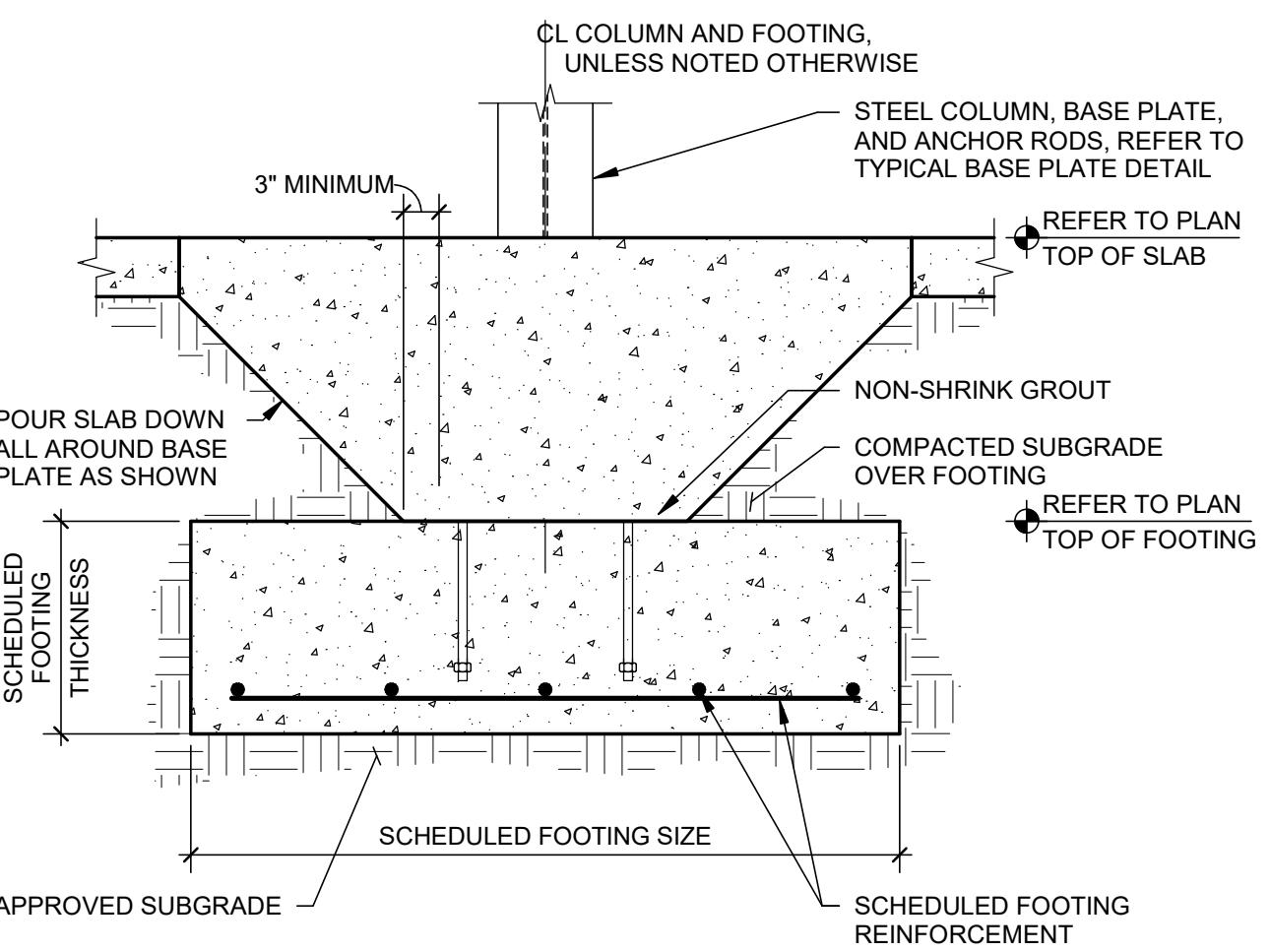
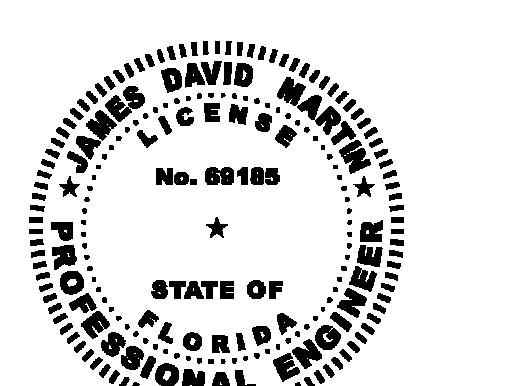
### 1 FRAMING PLAN - ROOF

3/32" = 1'-0"

#### PLAN NOTES

- TOP OF CONCRETE ELEVATION IS NOTED ON PLAN.
- FLOOR STRUCTURE CONSISTS OF 3 1/2" LIGHT WEIGHT CONCRETE ON 2" DEEP, 18 GAGE GALVANIZED (G90) COMPOSITE STEEL DECK (5 1/2" TOTAL THICKNESS). STEEL DECK SHALL BE PLACED WITH A TWO-Span CONDITION MINIMUM. NO SINGLE SPANS ARE ALLOWED WITHOUT WRITTEN APPROVAL OF ENGINEER-OF-RECORD. THE CONCRETE SLAB SHALL BE REINFORCED WITH WELDED SMOOTH WIRE REINFORCEMENT #4@12" AND #4@12" IN SPANS.
- ALL STEEL THAT IS PERMANENTLY EXPOSED TO THE EXTERIOR OR IS PERMANENTLY IN UNCONDITIONED SPACE SHALL BE HOT-DIPPED GALVANIZED.
- REFER TO SHEET SERIES S40X AND S41X FOR FLOOR FRAMING SECTIONS AND DETAILS.
- REFER TO SHEET SERIES S40X AND S41X FOR SHEAR WALL ELEVATIONS AND SCHEDULES.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR PENETRATIONS NOT SHOWN. REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS AT OPENINGS.
- GRAVITY CONNECTIONS ARE ONLY ALLOWED WHERE SHOWN. REFER TO S3.5X SERIES FOR PROPOSED LAYOUT.
- PLACE #4#16-0" @ 17" ACROSS STEEL GIRDERS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 14/402. PLACE #4#16-0" @ 17" ACROSS STEEL GIRDERS AT END OF CANTILEVERS.
- PLACE #4#25-0" @ 17" ACROSS STEEL BEAMS BELOW THE WELDED WIRE REINFORCEMENT PER DETAIL 11/402. PROVIDE 180 DEGREE HOOK ON END AT EDGE OF CANTILEVER.
- PROVIDE ANGLE OR LOOKER/KICKER AT EACH COLUMN ALONG GRID C AND E FOR DECK SUPPORT. REFER TO TYPICAL DETAILS.
- METAL PAN TREADS/RISERS, LANDINGS, STEEL STRINGERS, EMBED PLATES, HANDRAILS, AND ALL RELATED CONNECTIONS BY DELEGATED STAIR ENGINEER.
- EPX = EMBED PLATE TYPE "X". REFER TO S2/54.11 FOR EMBED PLATE ELEVATIONS.
- WINDOW WASHING EQUIPMENT LOCATIONS AND SIZES ARE SHOWN AS PER THE MANUFACTURER'S SPECIFICATIONS. THESE ARE NOT FINAL. ALL FINAL QUANTITIES AND LOCATIONS ARE TO BE DETERMINED BY THE WINDOW WASHING EQUIPMENT MANUFACTURER IN ACCORDANCE WITH APPLICABLE CODES AND REGULATIONS.



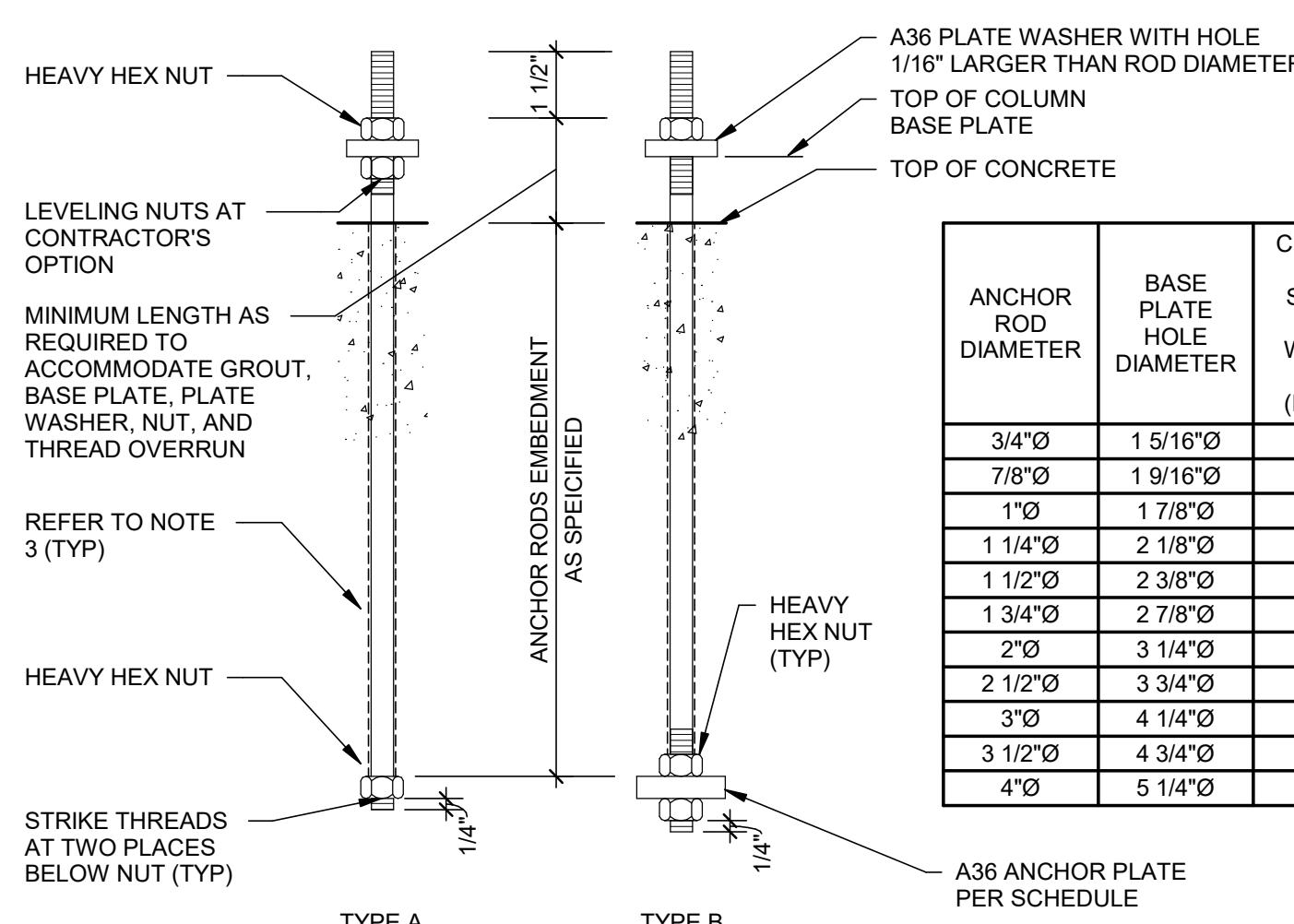
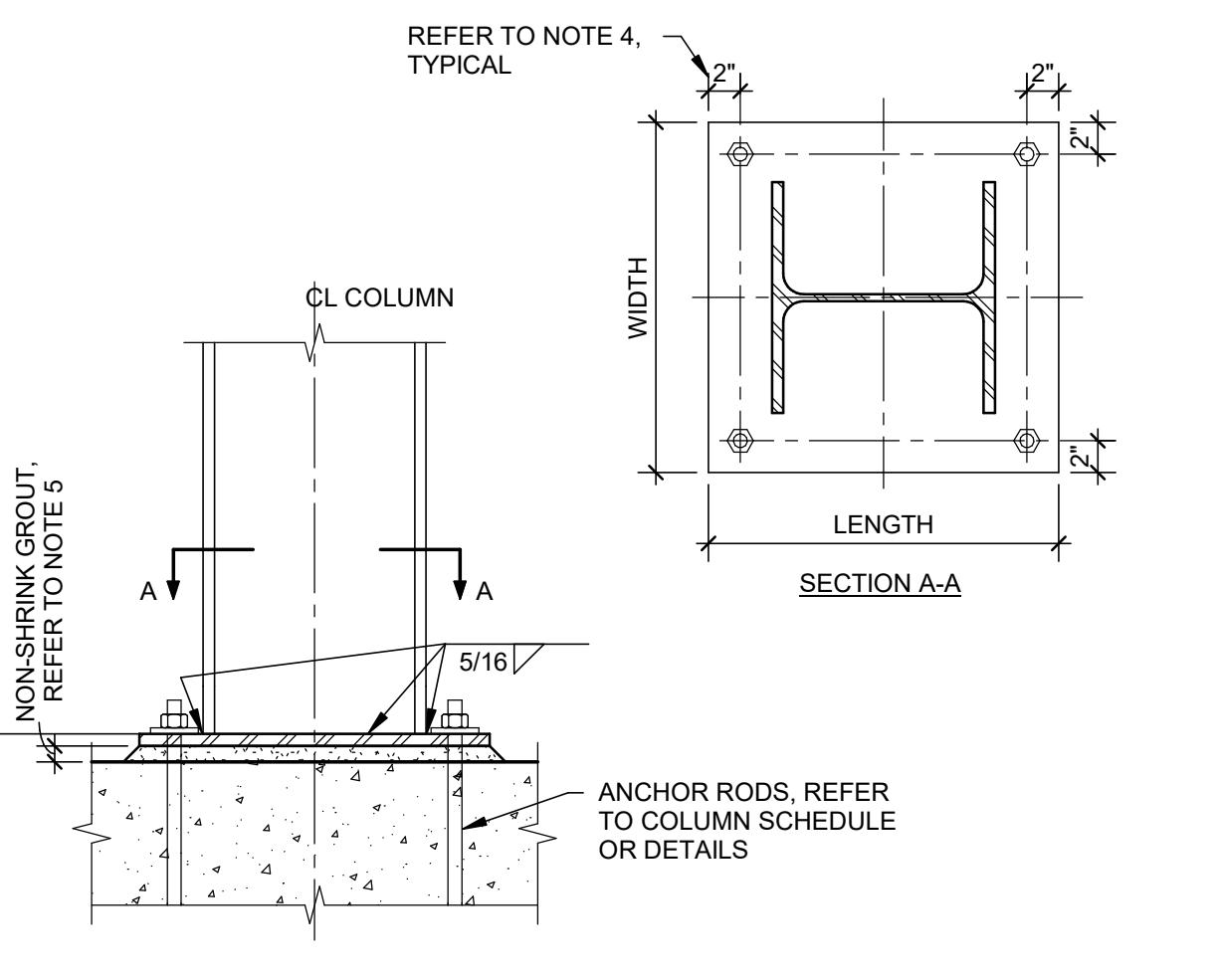


5 TYPICAL STEEL COLUMN ON SPREAD FOOTING  
NO SCALE

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BASE PLATE SCHEDULE						
MARK	PLATE SIZE	WELD SIZE	ANCHOR RODS	LAYOUT	REMARKS	
BP-1	1 1/4" x 20" x 1"-8"	5/16" FILLET	(4) 3/4" Ø x 1"-0" ASTM F1554, GR55	REFER TO 3/S302	TYPE "A" ANCHOR RODS	
BP-2	1 1/2" x 20" x 1"-8"	5/16" FILLET	(4) 3/4" Ø x 1"-0" ASTM F1554, GR55	REFER TO 3/S302	TYPE "A" ANCHOR RODS	
BP-3	1 3/8" x 23" x 1"-11"	5/16" FILLET	(4) 3/4" Ø x 1"-0" ASTM F1554, GR55	REFER TO 3/S302	TYPE "A" ANCHOR RODS	
BP-4	1 1/2" x 23" x 1"-11"	5/16" FILLET	(4) 3/4" Ø x 1"-0" ASTM F1554, GR55	REFER TO 3/S302	TYPE "A" ANCHOR RODS	
BP-5	1 3/4" x 23" x 1"-11"	5/16" FILLET	(4) 3/4" Ø x 1"-0" ASTM F1554, GR55	REFER TO 3/S302	TYPE "A" ANCHOR RODS	
BP-6	2" x 24" x 2"-0"	5/16" FILLET	(4) 3/4" Ø x 1"-0" ASTM F1554, GR55	REFER TO 3/S302	TYPE "A" ANCHOR RODS	
BP-7	1 1/4" x 18" x 1"-6"	5/16" FILLET	(6) 3/4" Ø x 1"-0" ASTM F1554, GR55	REFER TO 11/S312	TYPE "A" ANCHOR RODS	
BP-8	1 3/4" x 20" x 1"-10"	5/16" FILLET	(4) 3/4" Ø x 1"-0" ASTM F1554, GR55	REFER TO 3/S302	TYPE "A" ANCHOR RODS	
BP-9	2 1/4" x 24" x 2"-6"	3/8" FILLET	(6) 1 1/4" Ø x 1"-6" ASTM F1554, GR55	REFER TO 3/S302	GR 50 BP, TYPE "A" ANCHOR RODS	
BP-10	1 3/4" x 20" x 2"-2"	5/16" FILLET	(8) 1 1/4" Ø x 1"-4" ASTM F1554, GR55	REFER TO 11/S312	GR 50 BP, TYPE "A" ANCHOR RODS	

NOTES:  
1. ALL BASE PLATE MATERIAL TO BE A36, UNLESS NOTED OTHERWISE.  
2. REFER TO ST100X SERIES FOR BASE PLATE MARKS.  
3. LENGTH SPECIFIED IN ANCHOR ROD COLUMN IS EMBEDMENT LENGTH.  
REFER TO 2/S302 FOR TOTAL LENGTH.



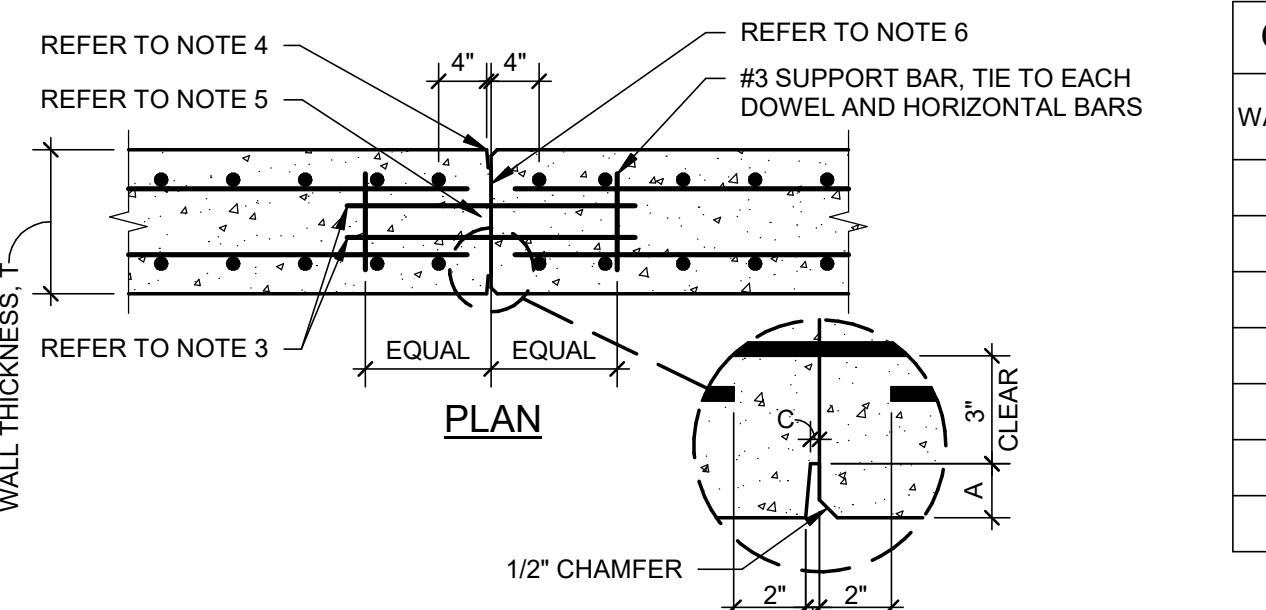
ANCHOR ROD DIAMETER	BASE PLATE HOLE DIAMETER	CIRCULAR OR SQUARE PLATE WASHER TYPE (INCHES)		F1554 GRADE 36		F1554 GRADE 55		F1554 GRADE 105	
		PLATE WASHER THICKNESS (INCHES)	ANCHOR PLATE SIZE (INCHES)	PLATE WASHER THICKNESS (INCHES)	ANCHOR PLATE SIZE (INCHES)	PLATE WASHER THICKNESS (INCHES)	ANCHOR PLATE SIZE (INCHES)	PLATE WASHER THICKNESS (INCHES)	ANCHOR PLATE SIZE (INCHES)
3/4"Ø	1 5/16"	2	1/4	-	-	5/8	1 2/2" x 1/2"	-	-
7/8"Ø	2 1/2"	2 1/2	5/16	5/8	1 2/2" x 1/2"	-	-	-	-
1"Ø	1 7/8"	3	3/8	-	-	-	-	-	-
1 1/4"Ø	2 1/8"	3 1/2	1/2	5/8	3/4 x 2 1/2" x 3/4"	3/4	3/4 x 3/4 x 3/4"	7/8	7/8 x 3/4"
1 1/2"Ø	2 3/8"	4	1/2	-	-	-	-	-	-
1 3/4"Ø	2 7/8"	4 1/2	5/8	5/8	1 1/4" x 1 1/2" x 1/2"	1 1/4	1 1/4" x 3/4 x 3/4"	7/8	7/8 x 3/4 x 3/4"
2"Ø	3 1/4"	5	3/4	-	-	-	-	-	-
2 1/2"Ø	3 3/4"	5 1/2	7/8	-	-	-	-	-	-
3"Ø	4 1/4"	6 1/2	1	1 1/4	2 1/4" x 1/2" x 1/2"	2 1/4	2 1/4" x 1/2" x 1/2"	1 1/2	1 1/2" x 1/2" x 1/2"
3 1/2"Ø	4 3/4"	7	1 1/4	-	-	-	-	-	-
4"Ø	5 1/4"	8	1 1/2	-	-	-	-	-	-

NOTES:  
1. REFER TO COLUMN SCHEDULE FOR ANCHOR ROD SIZE, TYPE, AND EMBEDMENT. ALL ANCHOR RODS SHALL BE TYPE "A" UNLESS NOTED OTHERWISE.  
2. UNLESS NOTED OTHERWISE, ALL ANCHOR ROD NUTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD.  
3. ANCHOR PLATE IS NOT REQUIRED AT TYPE "A" ANCHOR RODS.

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3 TYPICAL WIDE FLANGE COLUMN BASE CONNECTION  
NO SCALE

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NOTES:  
1. WHERE CONSTRUCTION JOINT LOCATIONS ARE NOT SHOWN ON THE DRAWINGS, SUBMIT PROPOSED JOINT LOCATIONS FOR APPROVAL TO THE CONTRACTOR. JOINTS SHALL BE SPACED NO CLOSER THAN 5'-0" FROM CORNERS AND PILASTERS. COORDINATE JOINT LOCATIONS WITH ARCHITECT.  
2. THIS DETAIL IS NOT TO BE USED IN SHEAR WALLS AND WHERE WALLS SPAN HORIZONTALLY TO SUPPORT LATERAL AND/OR VERTICAL LOADS. REFER TO PLANS FOR LOCATIONS WHERE WALLS ARE SO DESIGNED.  
3. PROVIDE SMOOTH DOWEL BARS AT SAME SPACING AS HORIZONTAL REINFORCEMENT. REFER TO SCHEDULE FOR NUMBER, SIZE, AND LENGTH. LIGHTLY COAT EXPOSED END OF DOWEL WITH PARAFFIN-BASED LUBRICANT, ASPHALT EMULSION, FORM OIL, OR GREASE OR USE A PLASTIC COATED DOWEL AND COAT EXPOSED END OF DOWEL WITH A CURE-IN-THE-WALL COMPOUND. DO NOT PLACEMENT OF DOWEL OR USE A PLASTIC COATED DOWEL AS IT MAY CAUSE ADDITIONAL DEFLECTION DUE TO HEAT GENERATION.  
4. PROVIDE CONTINUOUS VERTICAL GROOVE FORMED IN EACH FACE OF WALL. FORM GROOVE WITH 3/4" X 6" LONG HEAD STUD ANCHORS.  
5. PROVIDE CONTINUOUS WATERSTOP AT BELOW GRADE CONDITIONS AND WHERE SHOWN ON ARCHITECTURAL DRAWINGS.  
6. BREAK JOINT BETWEEN NEW AND PREVIOUSLY PLACED CONCRETE BY SPRAYING OR PAINTING EXPOSED SIDE WITH A CURING COMPOUND, BOND BREAKER, OR FORM OIL.  
7. REFER TO "TYPICAL VERTICAL CONTROL JOINT AT CONCRETE WALL (VISUALLY CRITICAL)" FOR CONTROL JOINT DETAIL.

10 TYPICAL VERTICAL CONSTRUCTION JOINT AT CONCRETE WALL (VISUALLY CRITICAL)  
NO SCALE

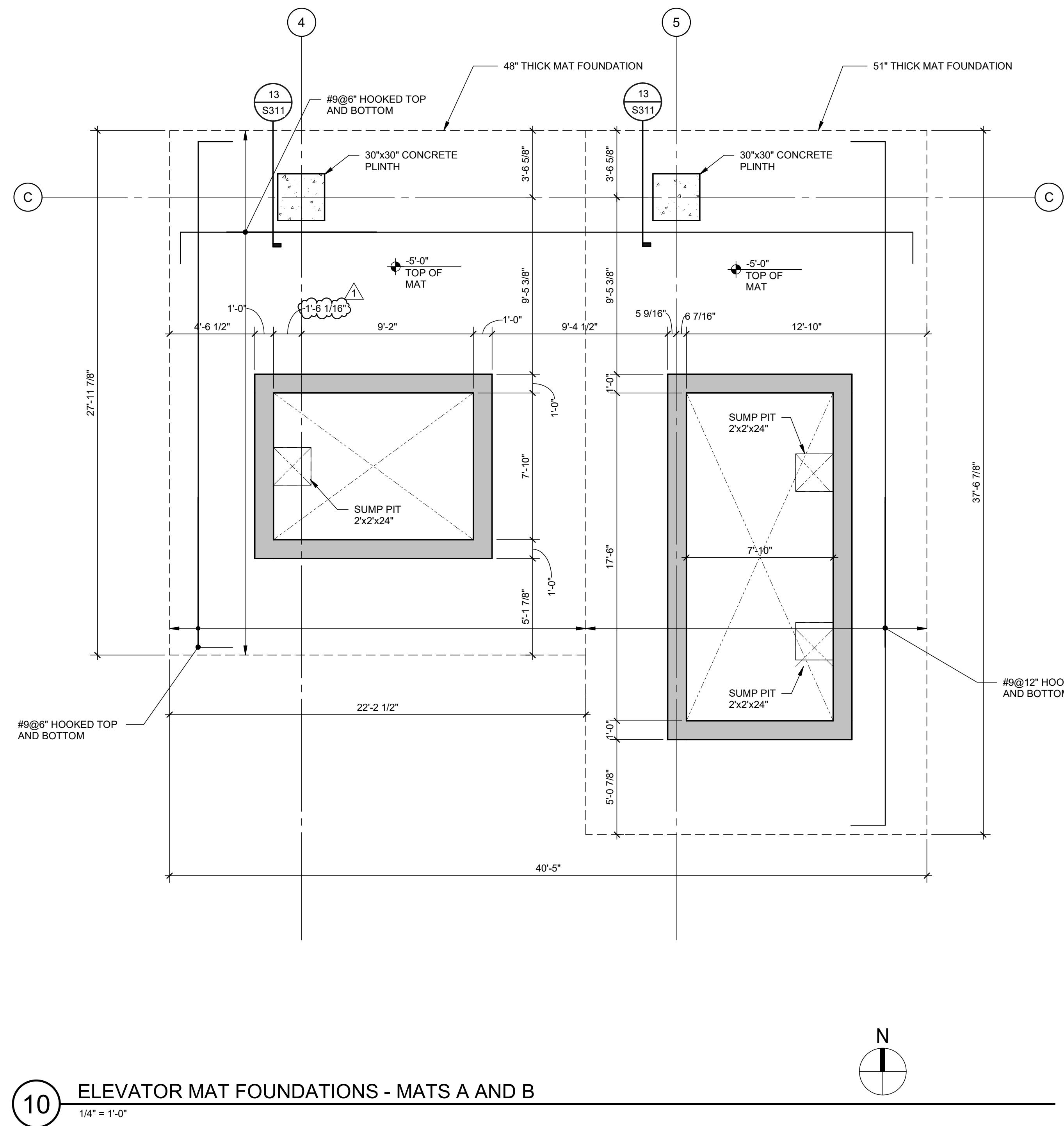
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GROOVE DIMENSION SCHEDULE		
WALL THICKNESS	A	B
8"≤T<12"	1"	3/8"
12"≤T<16"	1 1/2"	3/8"
16"≤T<22"	2"	9/16"
22"≤T<26"	2 1/2"	9/16"
26"≤T<36"	3"	3/4"
36"≤T<42"	4"	7/8"
T≥42"	5"	1 3/8"

ADDITIONAL DOWEL SCHEDULE		
HORIZONTAL BAR SIZE	DOWELS	
T<12"	(2) 1/2"Ø x 2"-0"	(2) 3/8"Ø x 2"-0"
T≥12"	(2) 1/2"Ø x 2"-0"	(2) 3/8"Ø x 2"-0"
#4	(1) 1/2"Ø x 2"-0"	(2) 3/8"Ø x 2"-0"
#5	(1) 1/2"Ø x 2"-0"	(2) 3/8"Ø x 2"-0"
#6	(1) 3/4"Ø x 3"-2"	(2) 1/2"Ø x 2"-0"
#7	(1) 7/8"Ø x 4"-4"	(2) 5/8"Ø x 2"-6"
#8	(1) 7/8"Ø x 4"-4"	(2) 5/8"Ø x 2"-6"
#9	NA	(2) 3/4"Ø x 3"-2"
#10	NA	(2) 7/8"Ø x 4"-4"
#11	NA	(2) 1"Ø x 6"-0"

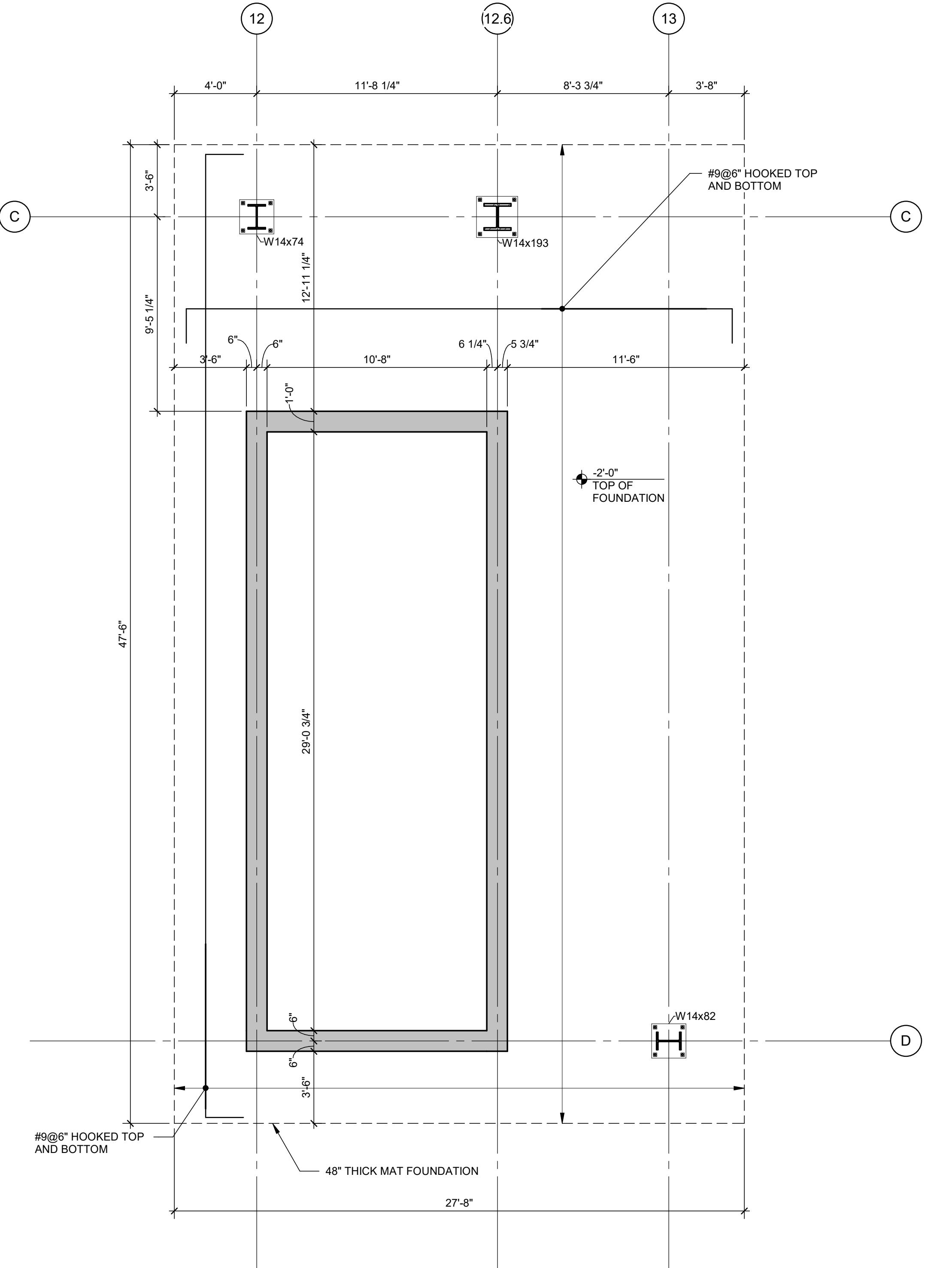
NOTES:  
1. LOCATE VERTICAL CONTROL JOINTS ACCORDING TO THE FOLLOWING CRITERIA:  
A. WHERE SHOWN ON ARCHITECTURAL OR STRUCTURAL DRAWINGS  
B. AT 20'-0" MAXIMUM SPACING ALONG A CONTINUOUS LENGTH OF WALL  
C. WHERE CHANGES IN WALL THICKNESS OR HEIGHT OCCUR FROM CORNERS OR PILASTERS  
D. AT ABUTTMENT CHANGES IN WALL THICKNESS OR HEIGHT  
2. WHERE CONTROL JOINT LOCATIONS ARE NOT SHOWN ON THE DRAWINGS, SUBMIT WALL JOINT LOCATIONS FOR APPROVAL PRIOR TO DETAILING REINFORCEMENT AND A MINIMUM OF TWO WEEKS PRIOR TO CONCRETE PLACEMENT.  
3. THIS DETAIL IS NOT TO BE USED IN SHEAR WALLS AND WHERE WALLS SPAN HORIZONTALLY TO SUPPORT LATERAL AND/OR VERTICAL LOADS. REFER TO PLANS FOR LOCATIONS WHERE WALLS ARE SO DESIGNED.  
4. PROVIDE SMOOTH DOWEL BARS AT SAME SPACING AS HORIZONTAL REINFORCEMENT. REFER TO SCHEDULE FOR NUMBER, SIZE, AND LENGTH. LIGHTLY COAT ONE END OF DOWEL WITH PARAFFIN-BASED LUBRICANT, ASPHALT EMULSION, FORM OIL, OR GREASE OR USE A PLASTIC COATED DOWEL AND COAT EXPOSED END OF DOWEL WITH A CURE-IN-THE-WALL COMPOUND.  
5. PROVIDE CONTINUOUS VERTICAL GROOVE FORMED IN EACH FACE OF WALL. FORM GROOVE WITH 3/4" X 6" LONG HEAD STUD ANCHORS.  
6. PROVIDE CONTINUOUS WATERSTOP AT BELOW GRADE CONDITIONS AND WHERE SHOWN ON ARCHITECTURAL DRAWINGS.  
7. REFER TO "TYPICAL VERTICAL CONTROL JOINT AT CONCRETE WALL (VISUALLY CRITICAL)" FOR CONTROL JOINT DETAIL.

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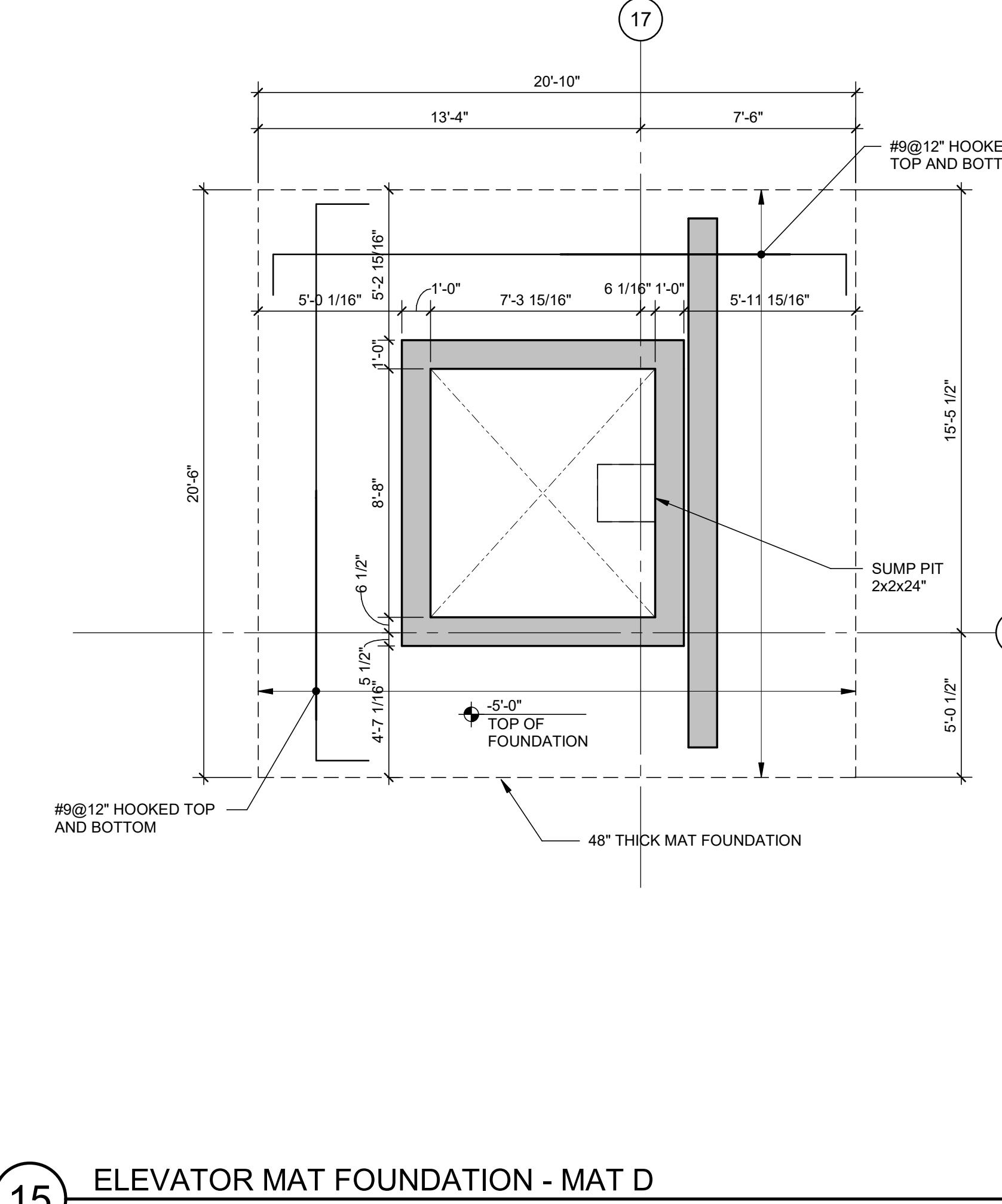
10 ELEVATOR MAT FOUNDATIONS - MATS A AND B

1/4" = 1'-0"

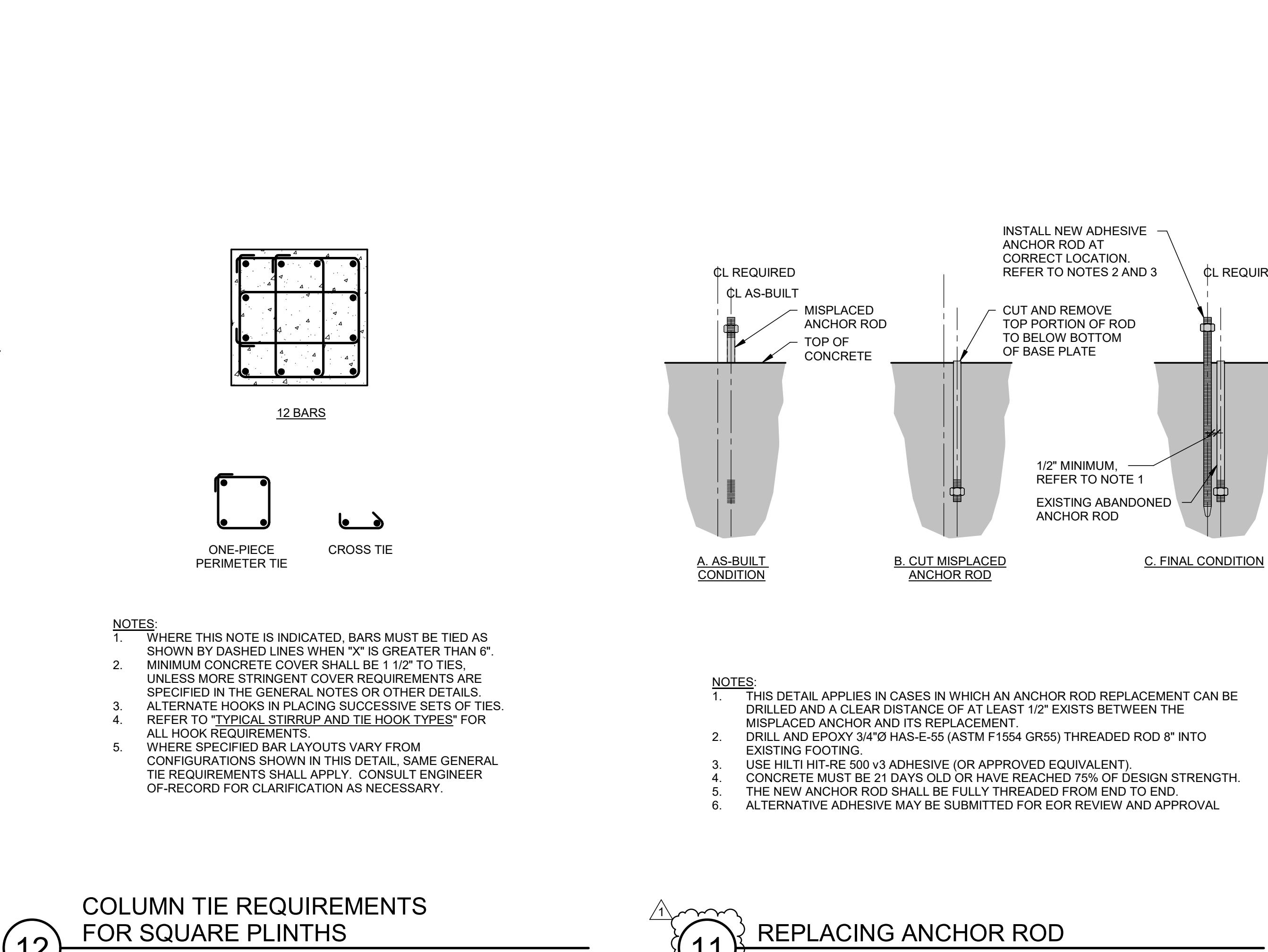


7 STAIRWELL MAT FOUNDATION - MAT C

1/4" = 1'-0"

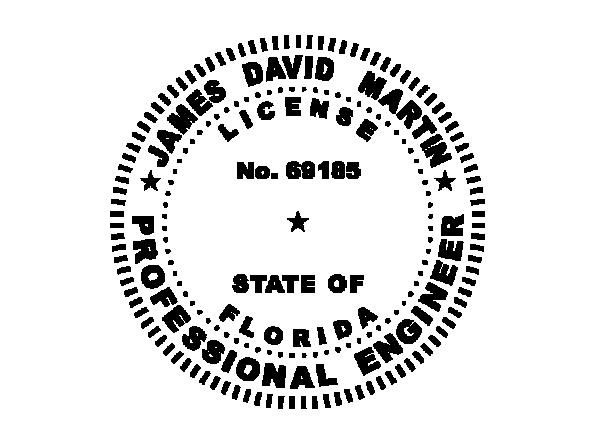


15 ELEVATOR MAT FOUNDATION - MAT D



**Sarasota County  
Administration Center**

Project No.	22.23005.00
Drawn By	BD
Checked By	JDM
Date	09/08/2023
Revisions:	
1 ASI-02	05/17/2024



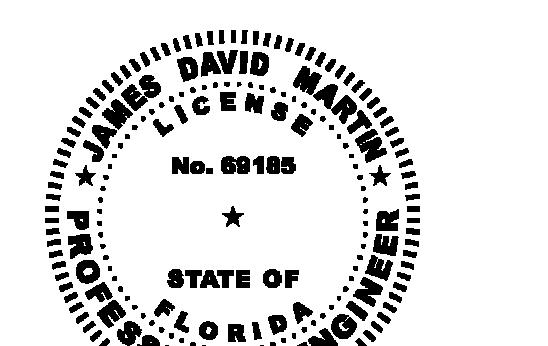
To the best of the Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S31**  
FOUNDATION  
DETAILS



Hellmuth, Obata & Kassabaum, P.C.  
One Tampa City Center, Suite 1800  
Tampa, FL 33602 USA  
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To the best of my Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

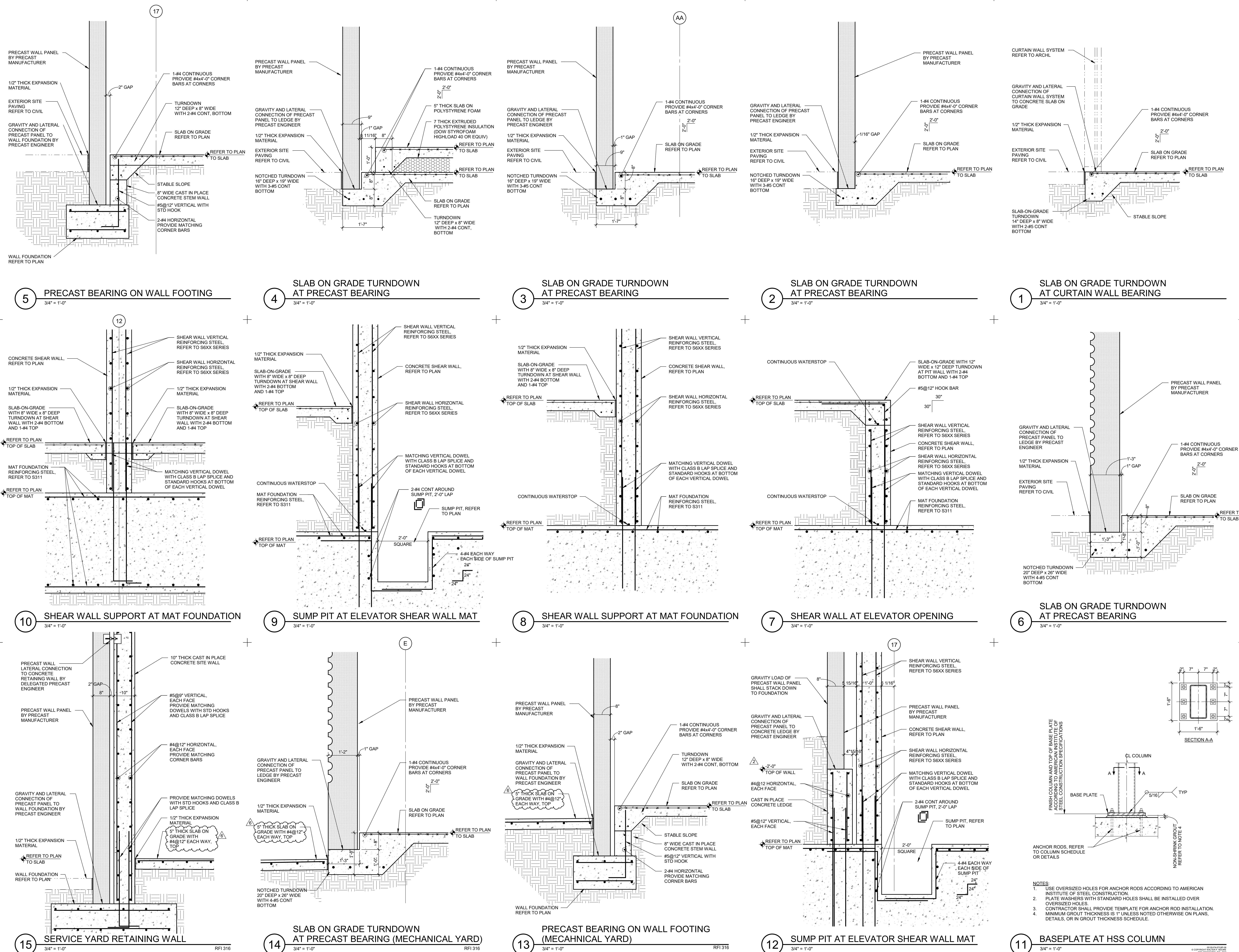
James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-2204-049  
Certificate of Authorization No. 3818

No. 69185  
\* STATE OF FLORIDA PROFESSIONAL ENGINEER

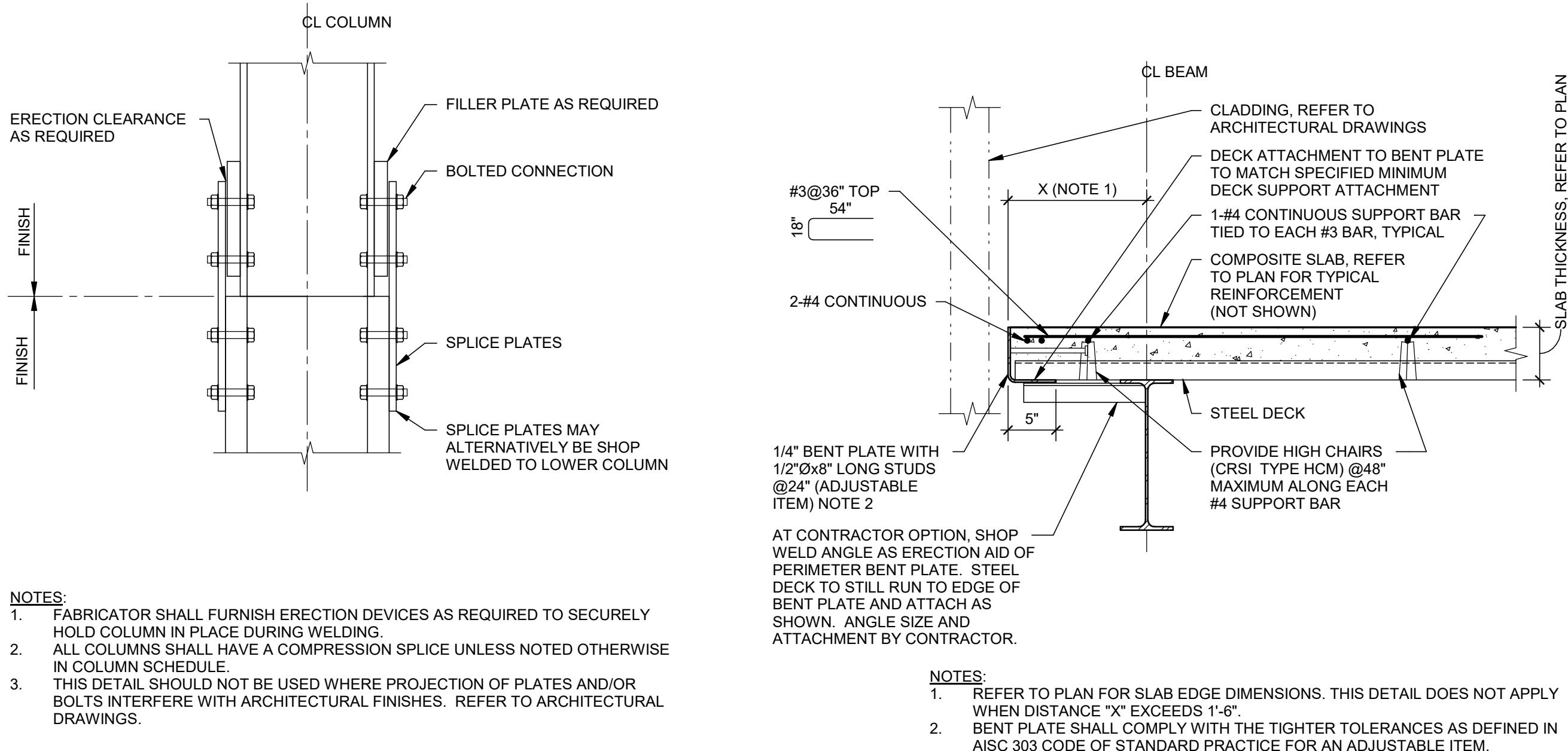
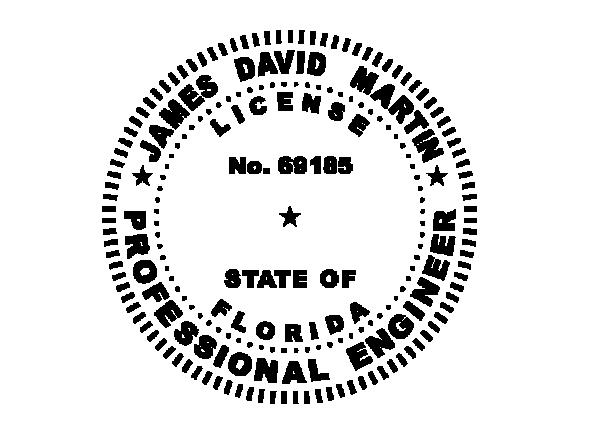
CONFORMED SET  
02/14/2024

S312  
FOUNDATION DETAILS

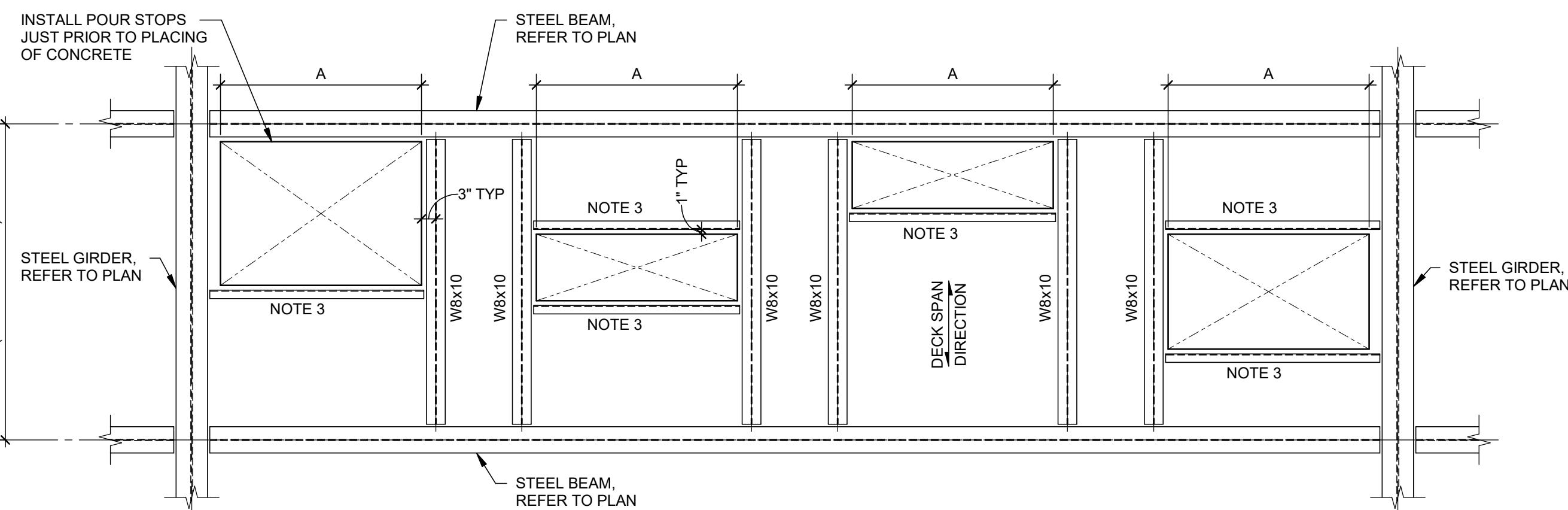
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scale as required  
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5 WIDE FLANGE COLUMN COMPRESSION SPLICE FOR COLUMNS OF SAME SERIES  
NO SCALE

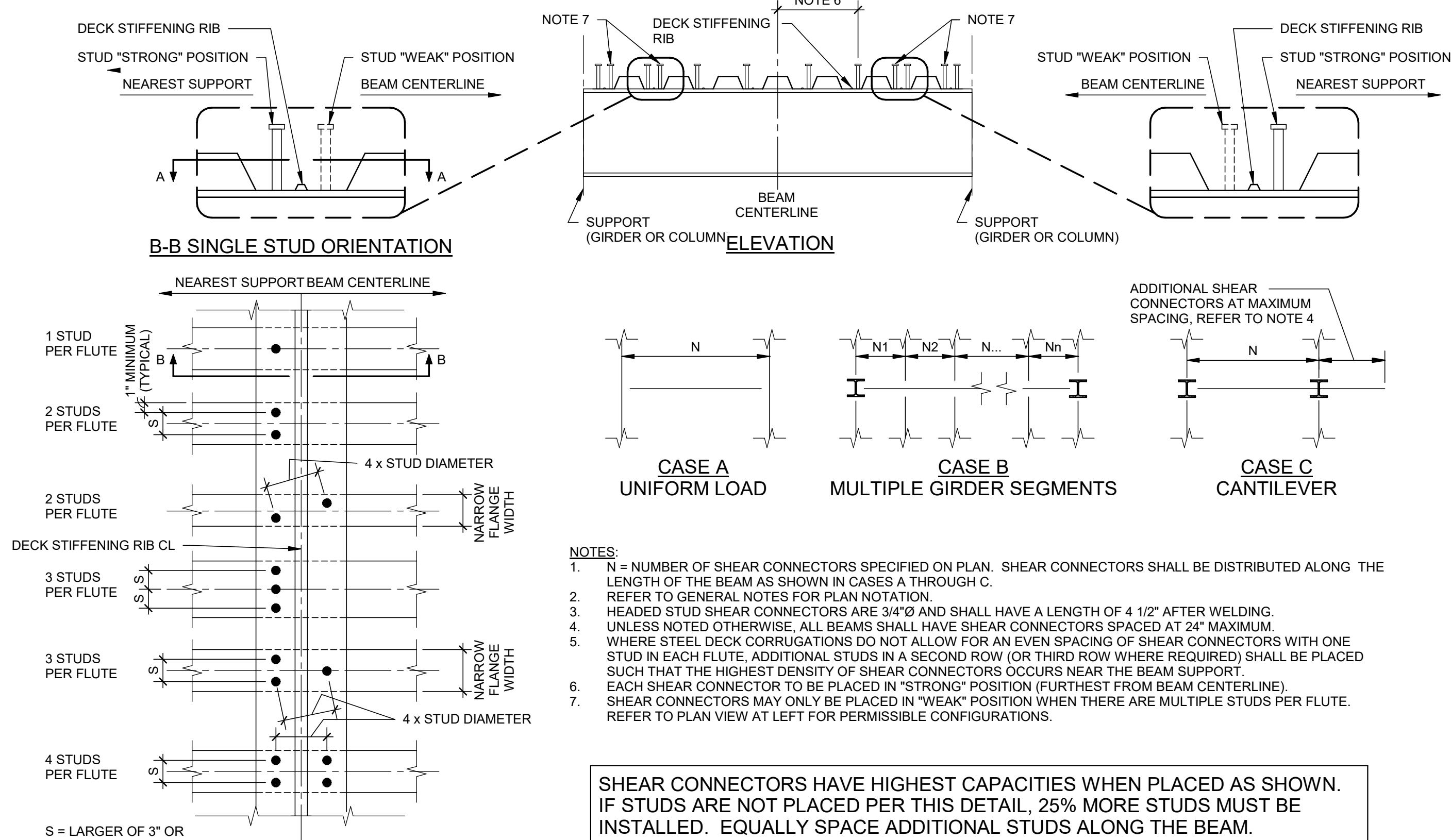


4 COMPOSITE SLAB EDGE CONDITION  
NO SCALE

NOTES:  
1. COORDINATE OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.  
2. "A" IS THE OPENING DIMENSION IN THE DIRECTION PERPENDICULAR TO THE DECK SPAN. THIS DETAIL IS APPLICABLE FOR "A" DIMENSIONS GREATER THAN 4'-0" AND UP TO 8'-0". REFER TO PLAN FOR FRAMING OF OPENINGS GREATER THAN 8'-0".  
3. WHERE THIS NOTE IS INDICATED, FRAMING MEMBER SHALL BE C5x6.7 FOR 2" DECK SYSTEMS AND C6x8.2 FOR 3" DECK SYSTEMS.

4. DECK SHALL BE CONTINUOUS OVER FRAMED OPENINGS. DO NOT CUT OPENINGS UNTIL CONCRETE HAS BEEN PLACED AND IMMEDIATELY BEFORE OPENINGS ARE NEEDED.  
5. THIS DETAIL SHOWS TYPICAL OPENING CONFIGURATIONS. VERIFY FRAMING WITH ENGINEER FOR SPECIAL CASES OR WHERE DIMENSIONS EXCEED MAXIMUM DIMENSIONS SHOWN IN THIS DETAIL. DO NOT PLACE SHEAR CONNECTORS ON W8, C5, OR C6 FRAMING MEMBERS SHOWN IN THIS DETAIL. ATTACH DECK PER SPECIFICATIONS.  
6. COORDINATE OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

10 TYPICAL FRAMED OPENINGS IN COMPOSITE SLAB (4'-0" < A <= 8'-0")





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Fort Myers, FL 33901

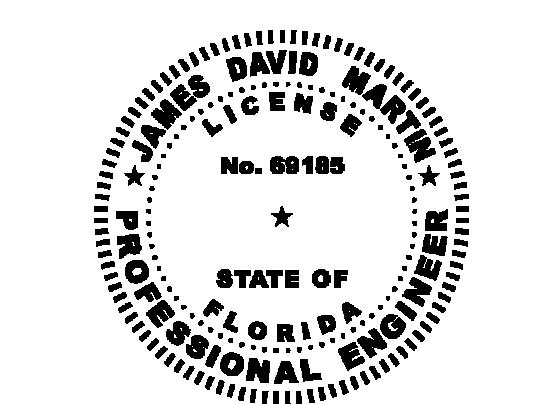
**Stantec**  
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6920 Professional Pkwy.  
Sarasota, FL 34240  
Walter P. Moore  
Structural Engineers  
201 East Kennedy Blvd., Suite 700  
Tampa, FL 33602

**walter p moore**  
Sieben Acoustics  
Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607

**Sarasota County  
Administration Center**  
1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

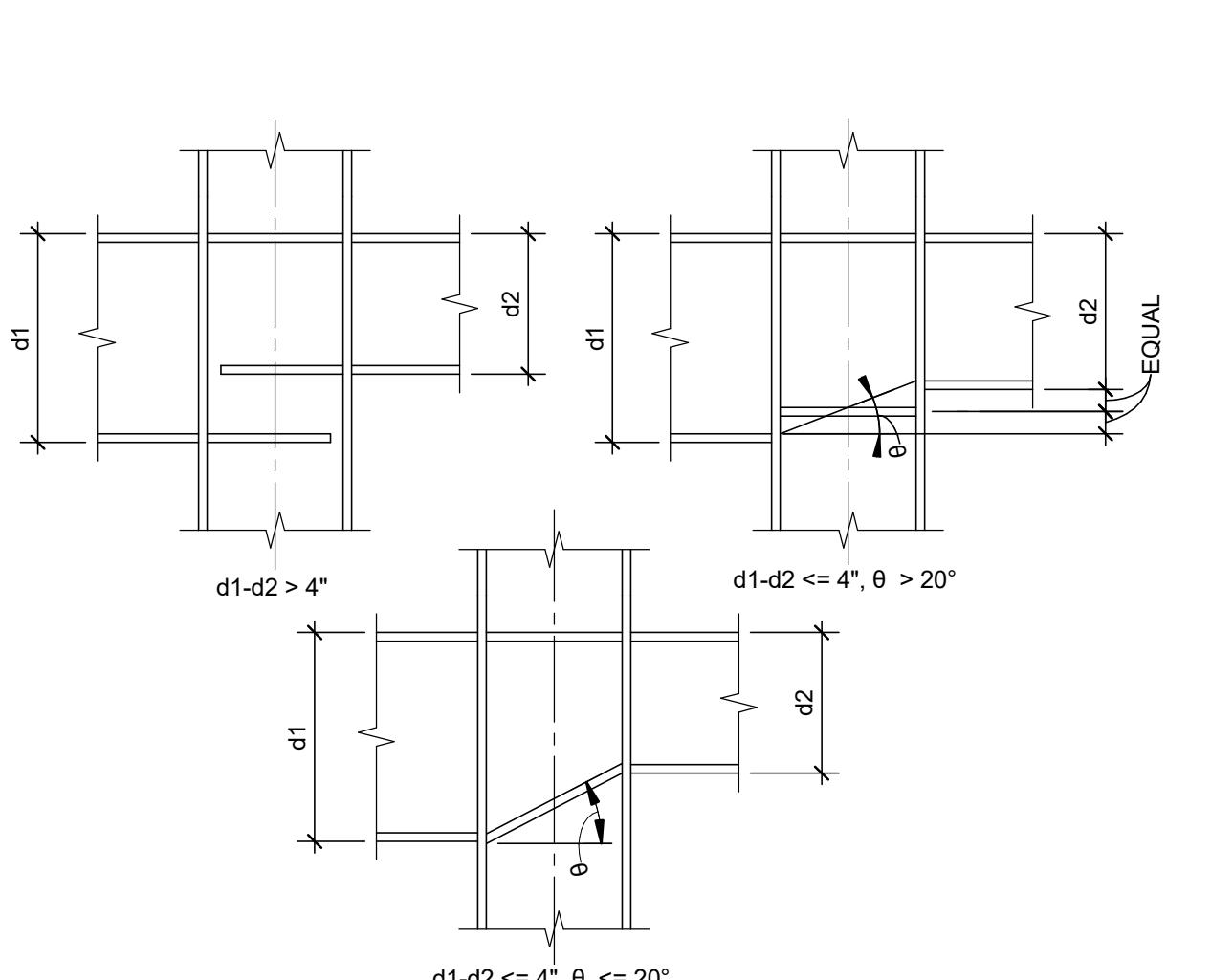
Revisions:



To the best of the Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

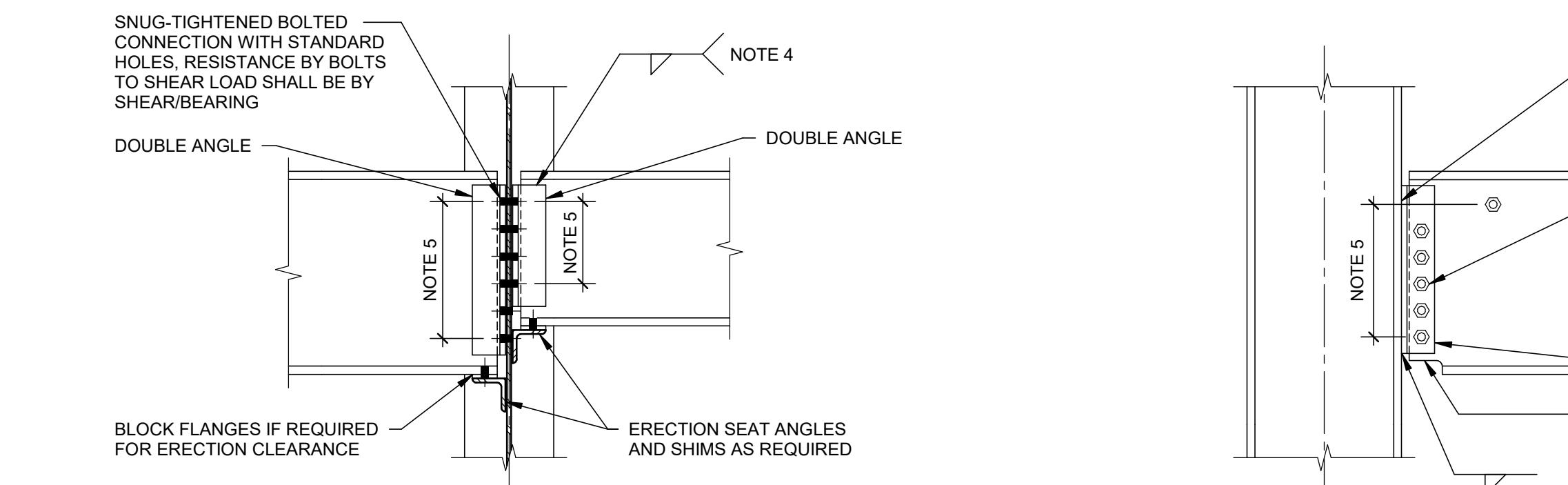
James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-2204-04  
Certificate of Authorization No. 3818



NOTES:  
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.  
2. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.  
3. WHEN BEAM FRAMES ON ONLY ONE SIDE OF COLUMN OR WHEN BEAM REACTIONS ARE SIGNIFICANTLY DIFFERENT, STIFFEN COLUMN WEB AS REQUIRED.

#### TYPICAL COLUMN STIFFENER ARRANGEMENT FOR OPPOSING BEAMS OF DIFFERENT DEPTHS

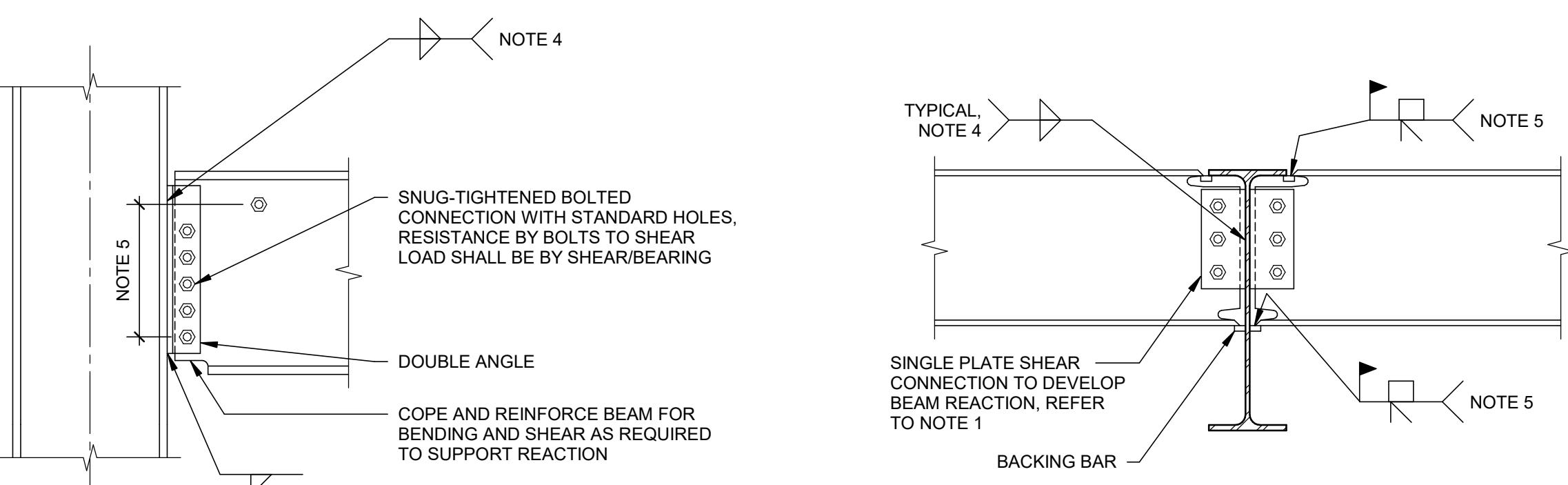
5 NO SCALE



NOTES:  
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.  
2. REFER TO PLAN FOR CONNECTION DESIGN REACTIONS.  
3. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.  
4. SHOP WELD ON 3 SIDES TO DEVELOP BEAM REACTION, TYPICAL.  
5. CONNECTION LENGTH SHALL BE AT LEAST 1/2 OF THE BEAM DEPTH.

#### TYPICAL SHEAR CONNECTION AT COLUMN WEB

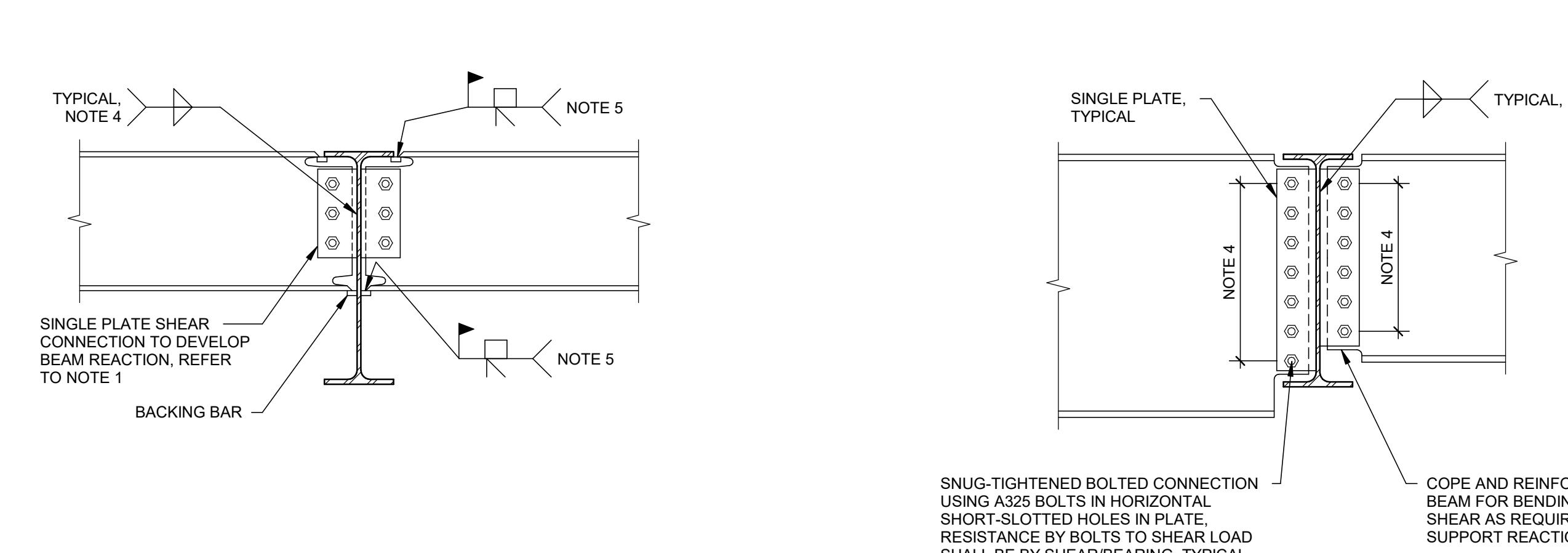
4 NO SCALE



NOTES:  
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.  
2. REFER TO PLAN FOR CONNECTION DESIGN REACTIONS.  
3. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.  
4. MINIMUM FILLET WELD SIZE FOR SINGLE PLATE SHEAR CONNECTIONS SHALL BE 5/8 TIMES THE PLATE THICKNESS.  
5. IF PROVIDED BY FABRICATOR DRAWINGS, PARTIAL PENETRATION WELD TO DEVELOP MOMENT CAN BE USED IN LIEU OF COMPLETE JOINT PENETRATION WELD. FOR THIS CASE, WELD SHALL BE REQUIRED TO DEVELOP THE BEAM FLANGE FORCE COMPUTED AS FOLLOWS:  
•  $P_u = \mu M_u / 3D_f$ , WHERE  
•  $\mu$  = DESIGN MOMENT (KIP-FEET).  
•  $D_f$  = BEAM DEPTH (INCHES).  
•  $P_u$  = BEAM FLANGE FORCE (KIPS).

#### TYPICAL SHEAR CONNECTION AT COLUMN FLANGE

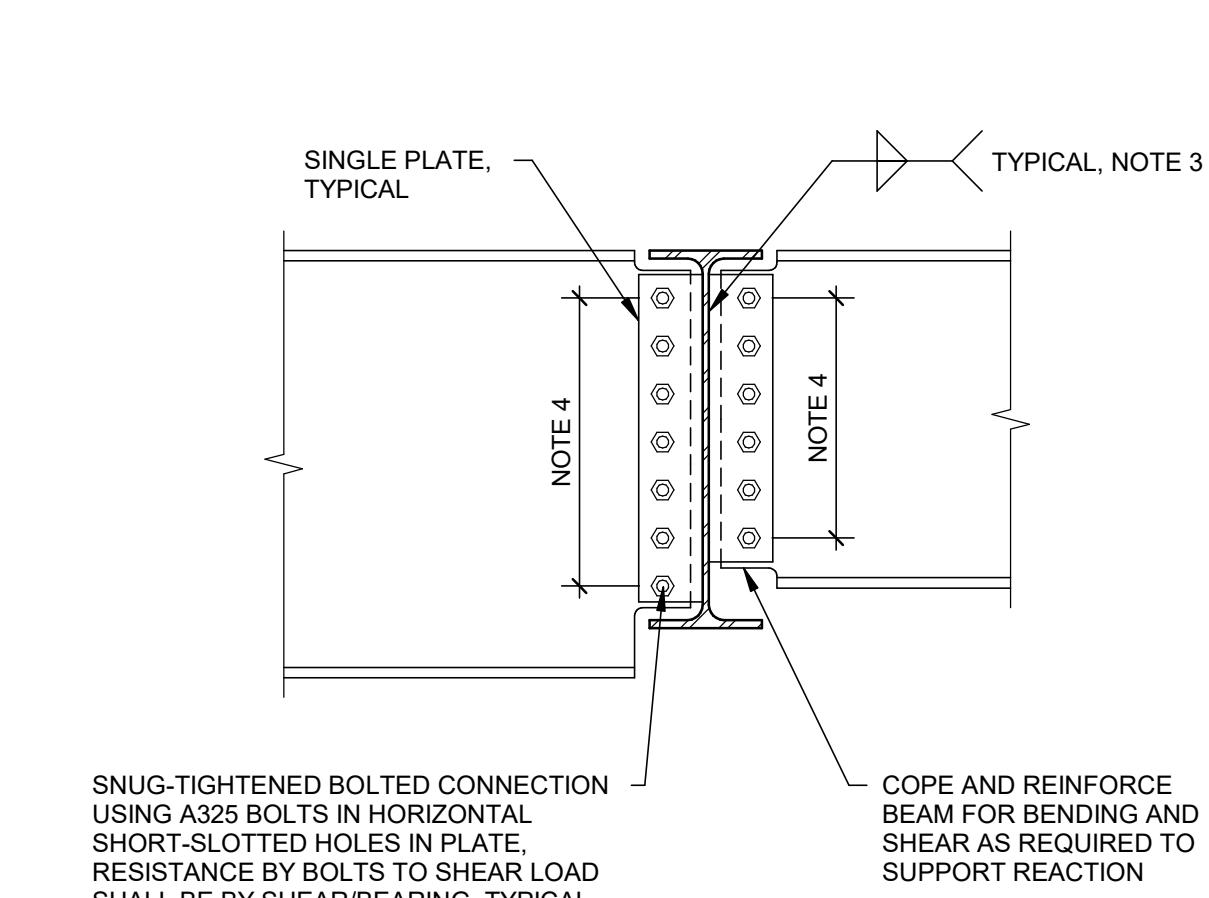
3 NO SCALE



NOTES:  
1. BOLTS IN WEB CONNECTION MAY BE SNUG-TIGHTENED OR PRETENSIONED A325 OR A490 BOLTS WITH HORIZONTAL SHORT-SLOTTED HOLES IN PLATE. IF BOLTS ARE TO BE PRETENSIONED, SNUG-TIGHTEN BOLTS PRIOR TO WELDING FLANGES AND PRETENSION AFTER WELDING FLANGES.  
2. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.  
3. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION LRFD MANUAL WHERE APPLICABLE.  
4. MINIMUM FILLET WELD SIZE FOR SINGLE PLATE SHEAR CONNECTIONS SHALL BE 5/8 TIMES THE PLATE THICKNESS.  
5. IF PROVIDED BY FABRICATOR DRAWINGS, PARTIAL PENETRATION WELD TO DEVELOP MOMENT CAN BE USED IN LIEU OF COMPLETE JOINT PENETRATION WELD. FOR THIS CASE, WELD SHALL BE REQUIRED TO DEVELOP THE BEAM FLANGE FORCE COMPUTED AS FOLLOWS:  
•  $P_u = \mu M_u / 3D_f$ , WHERE  
•  $\mu$  = DESIGN MOMENT (KIP-FEET).  
•  $D_f$  = BEAM DEPTH (INCHES).  
•  $P_u$  = BEAM FLANGE FORCE (KIPS).

#### TYPICAL BEAM-TO-BEAM MOMENT CONNECTION

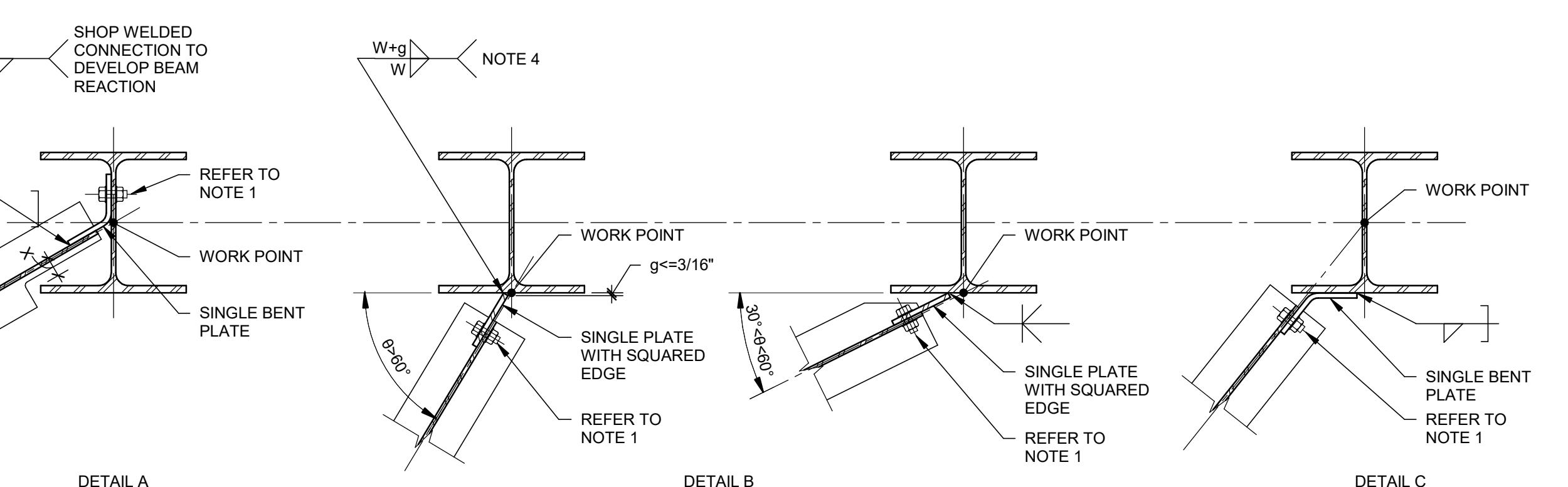
2 NO SCALE



NOTES:  
1. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN CRITERIA.  
2. PROVIDE PREDESIGNED CONNECTIONS AS SHOWN IN AMERICAN INSTITUTE OF STEEL CONSTRUCTION MANUAL WHERE APPLICABLE.  
3. MINIMUM FILLET WELD SIZE SHALL BE 5/8 TIMES THE PLATE THICKNESS.  
4. CONNECTION LENGTH SHALL BE AT LEAST 1/2 OF THE SUPPORTED BEAM DEPTH.

#### TYPICAL BEAM-TO-BEAM SHEAR CONNECTION

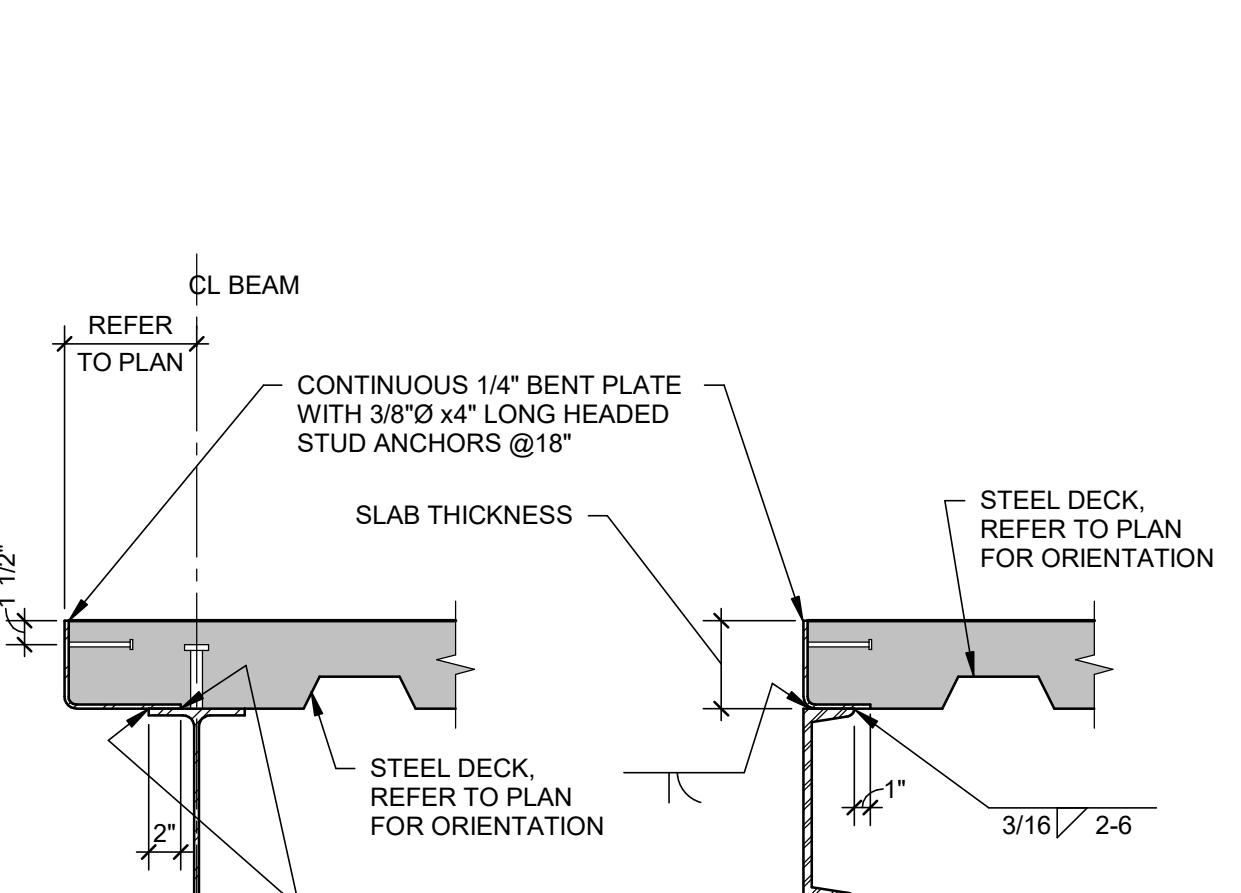
1 NO SCALE



NOTES:  
1. PROVIDE REMOVABLE VERTICAL FORMED BULKHEAD SET IN LOWER FLUTE. BULKHEAD MUST EXTEND FULL DEPTH OF SLAB WITH SLAB REINFORCEMENT EXTENDED THROUGH IT.  
2. CONSTRUCTION JOINTS PARALLEL TO DECK FLUTES SHALL BE LOCATED NO CLOSER THAN 5'-0" FROM CENTERLINE OF THE NEAREST GIRDERS.  
3. CONSTRUCTION JOINTS PERPENDICULAR TO DECK FLUTES SHALL BE LOCATED AT THE CENTER OF THE STEEL DECK SPAN.

#### TYPICAL CONSTRUCTION JOINT IN CONCRETE SLAB ON STEEL DECK

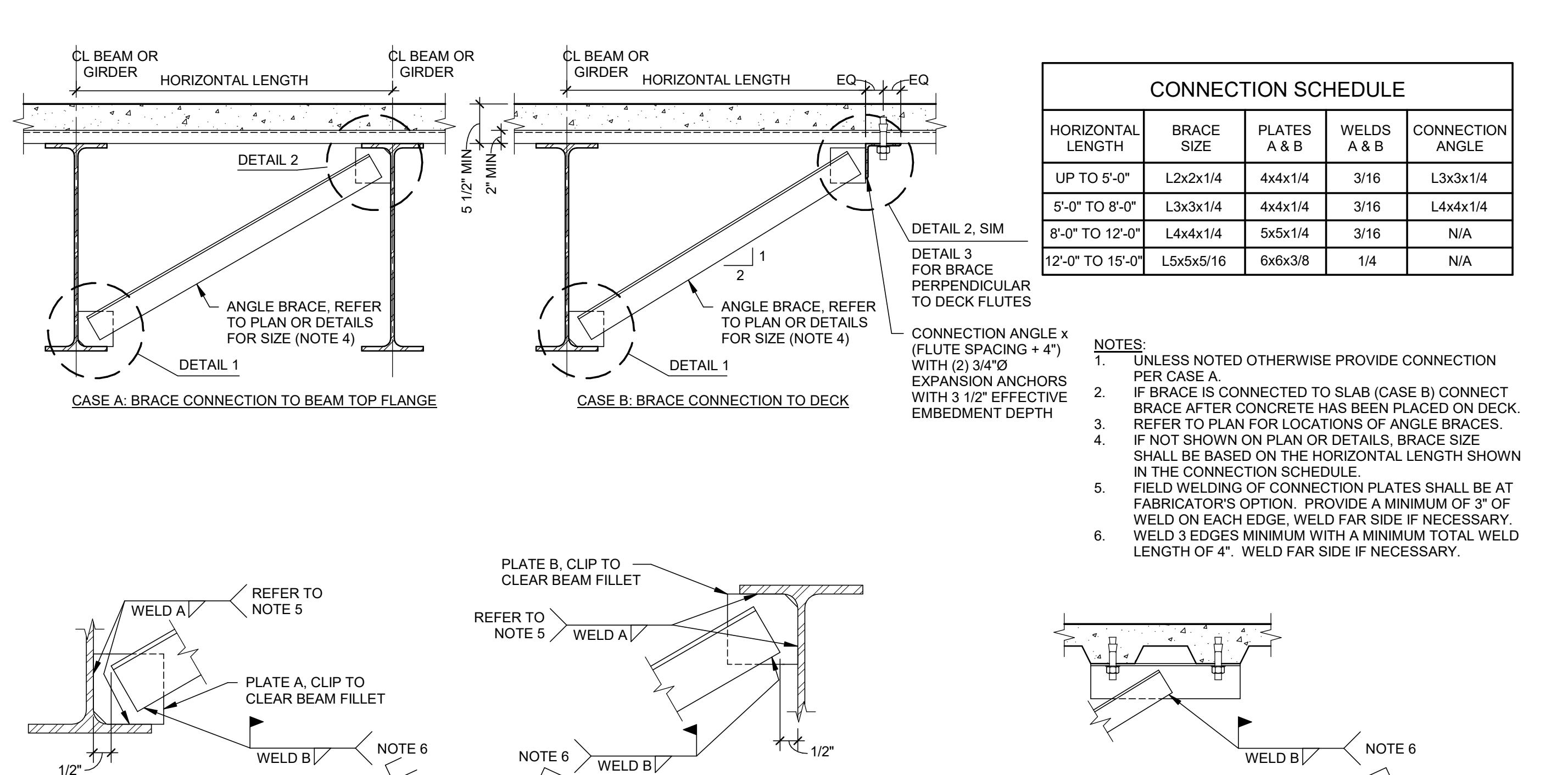
10 NO SCALE



NOTES:  
1. REFER TO STAIR SHOP DRAWINGS FOR CONNECTION OF STAIR STRINGER TO BEAM OR CHANNEL.

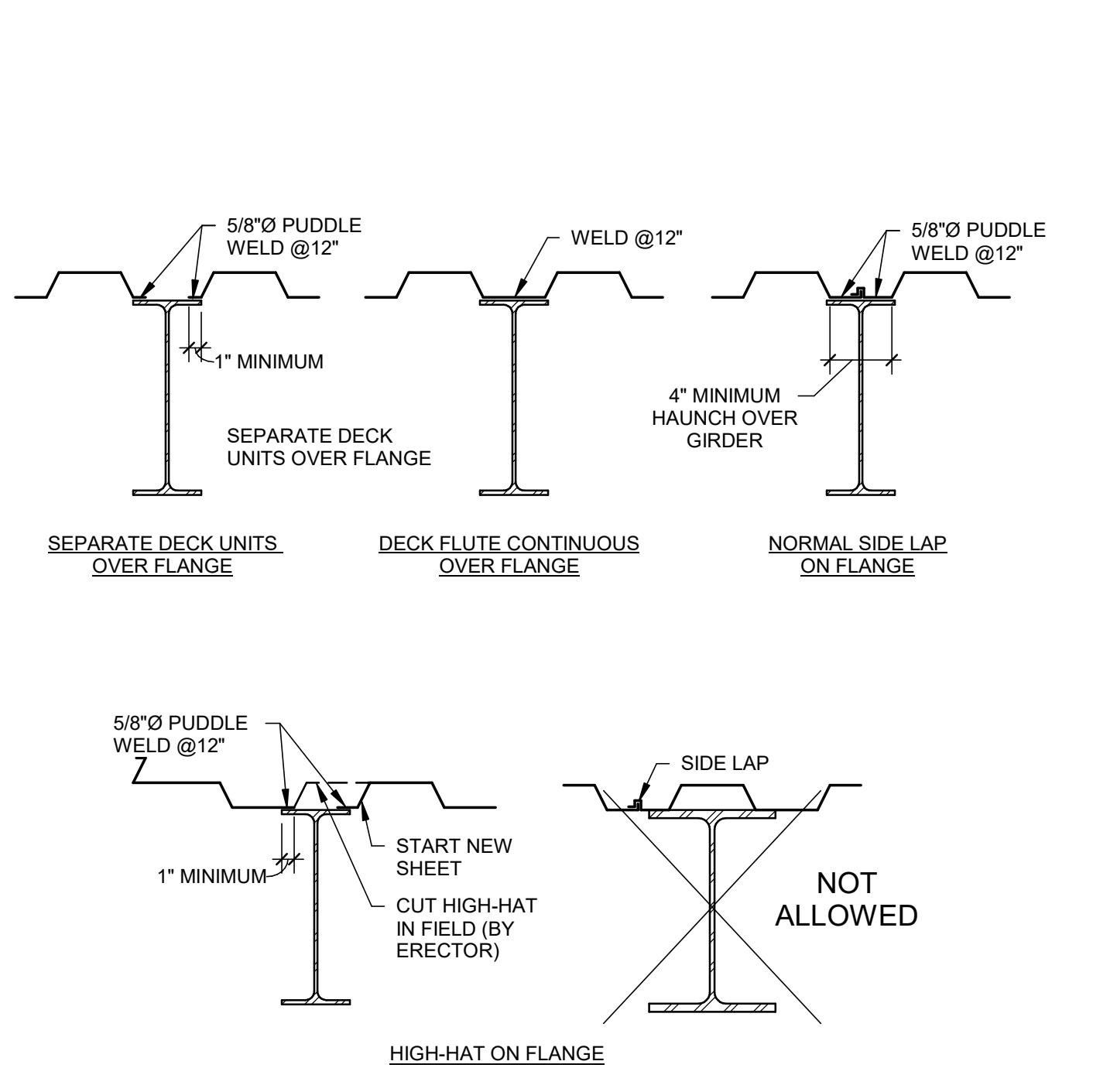
#### TYPICAL COMPOSITE SLAB LANDING FOR STEEL STAIR

15 NO SCALE



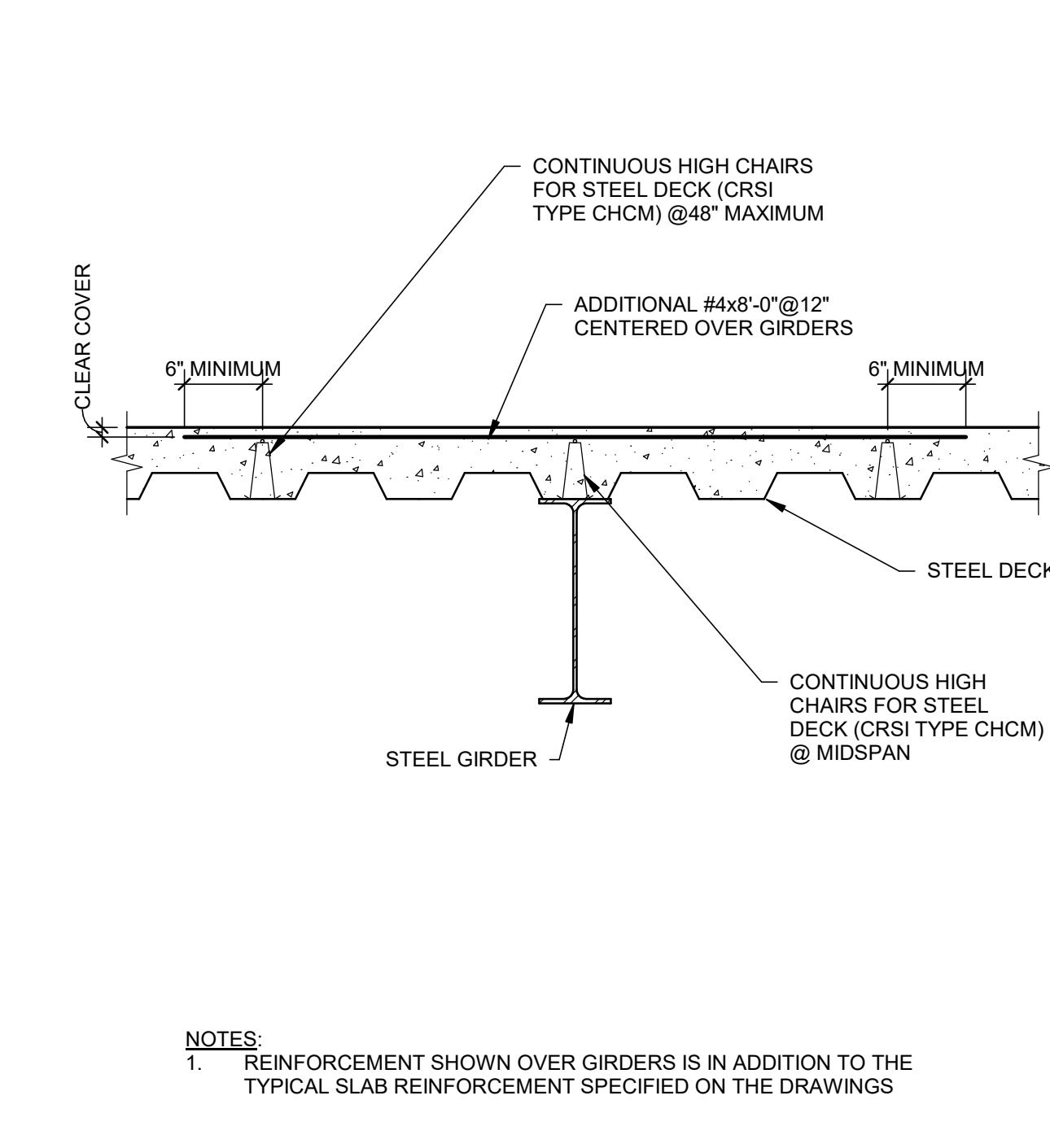
#### TYPICAL BEAM BOTTOM FLANGE BRACE (COMPOSITE CONSTRUCTION)

14 NO SCALE



#### TYPICAL COMPOSITE STEEL DECK DETAILS AT GIRDER

12 NO SCALE



#### TYPICAL ADDITIONAL REINFORCEMENT IN COMPOSITE SLABS AT INTERIOR GIRDERS

11 NO SCALE



Hellmuth, Obata & Kassabaum, P.C.  
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Fort Myers, FL 33901

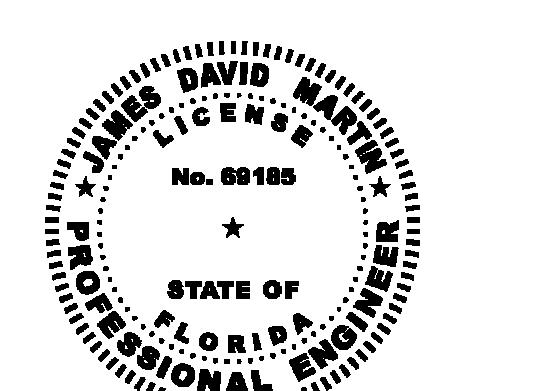
**Stantec** Civil & Landscape  
6920 Professional Pkwy.  
Sarasota, FL 34240

Walter P. Moore  
Structural Engineers  
201 East Kennedy Blvd., Suite 700  
Tampa, FL 33602

**SIEBEIN ACOUSTIC**  
Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions: 1 ASI-02  
05/17/2024

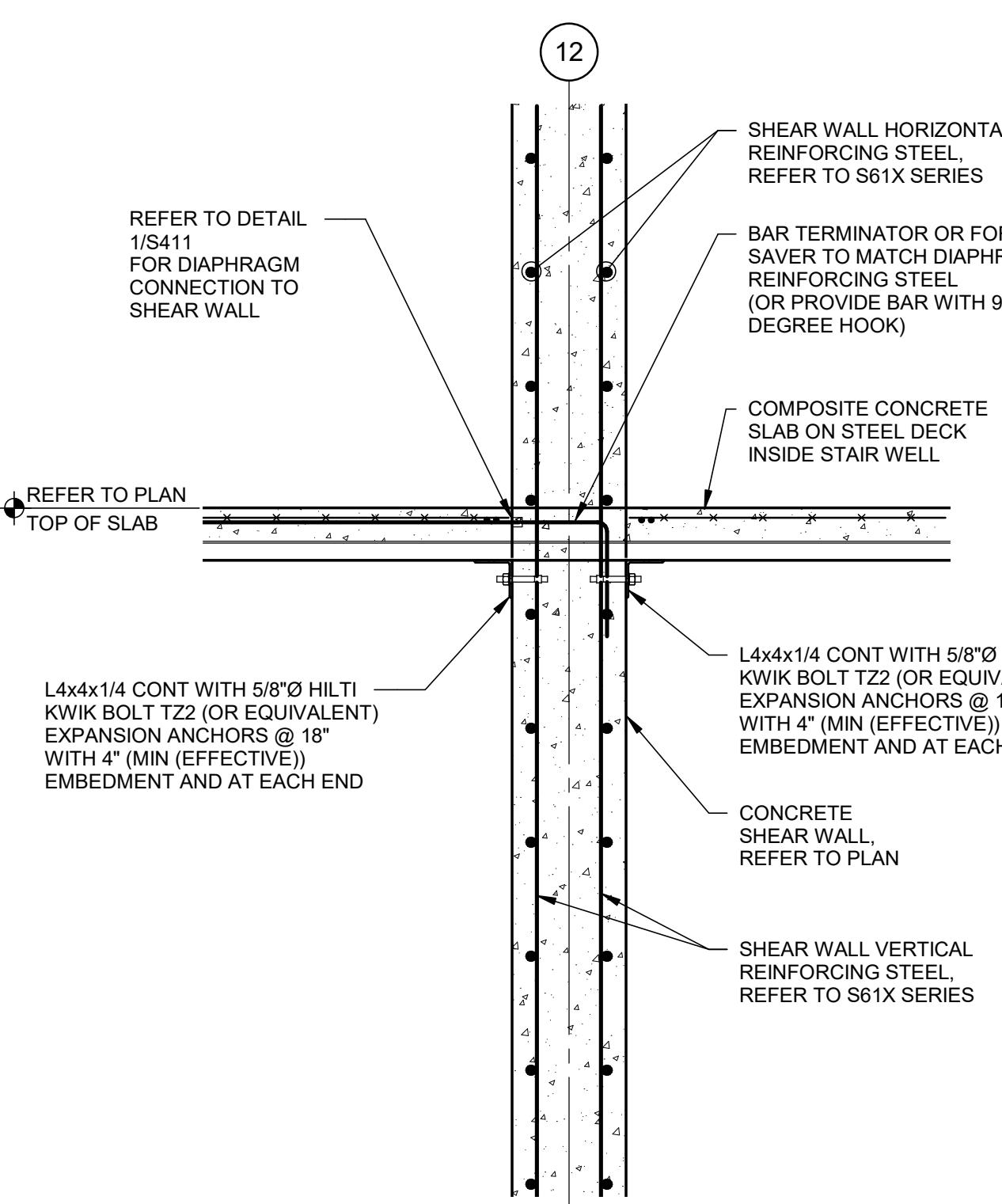


To the best of my Engineer's knowledge, the plans and specifications comply with the building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 503 and 603 of Florida Statutes.

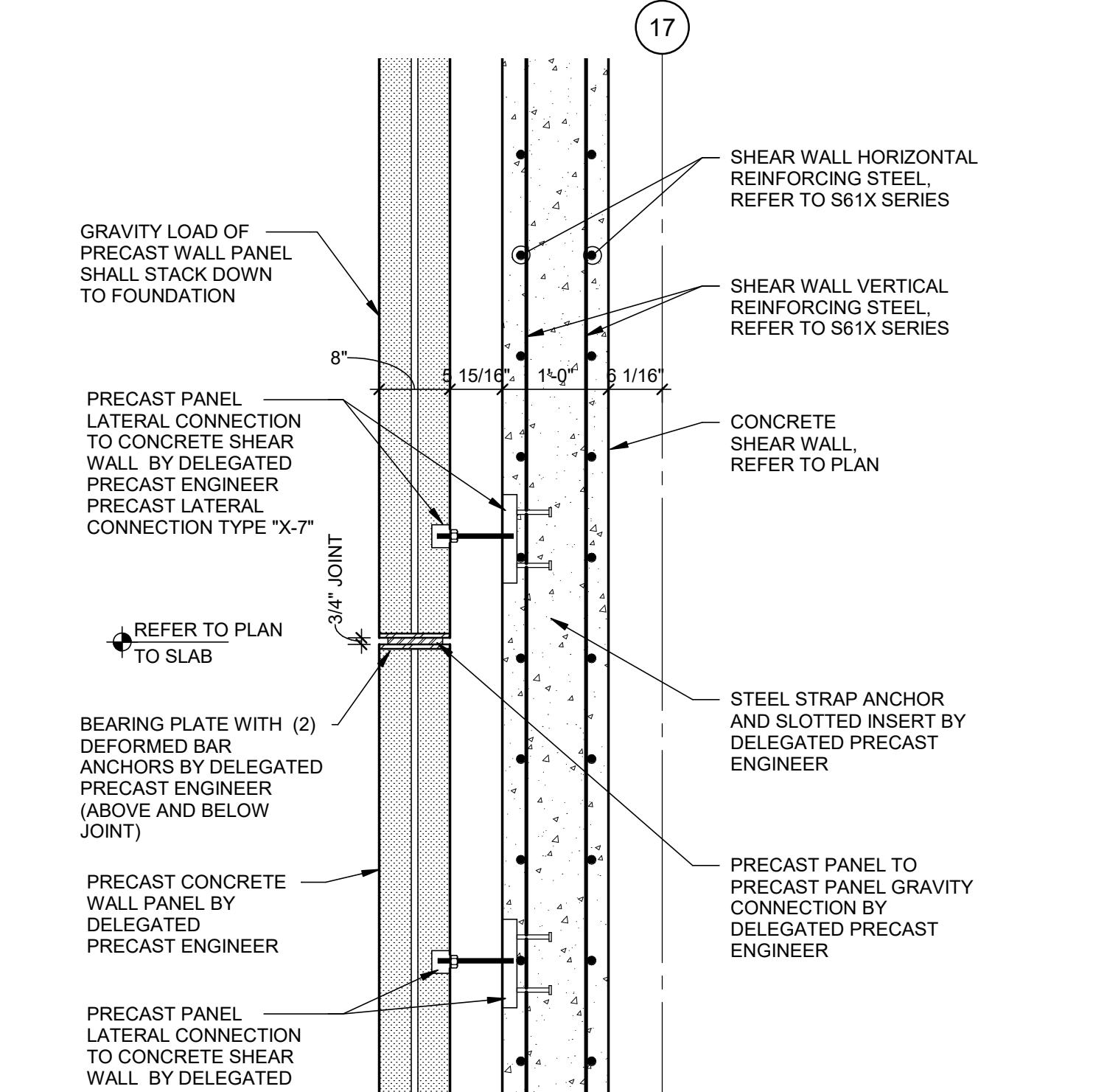
CONFIRMED SET 02/14/2024

**S411**  
STEEL FRAMING DETAILS

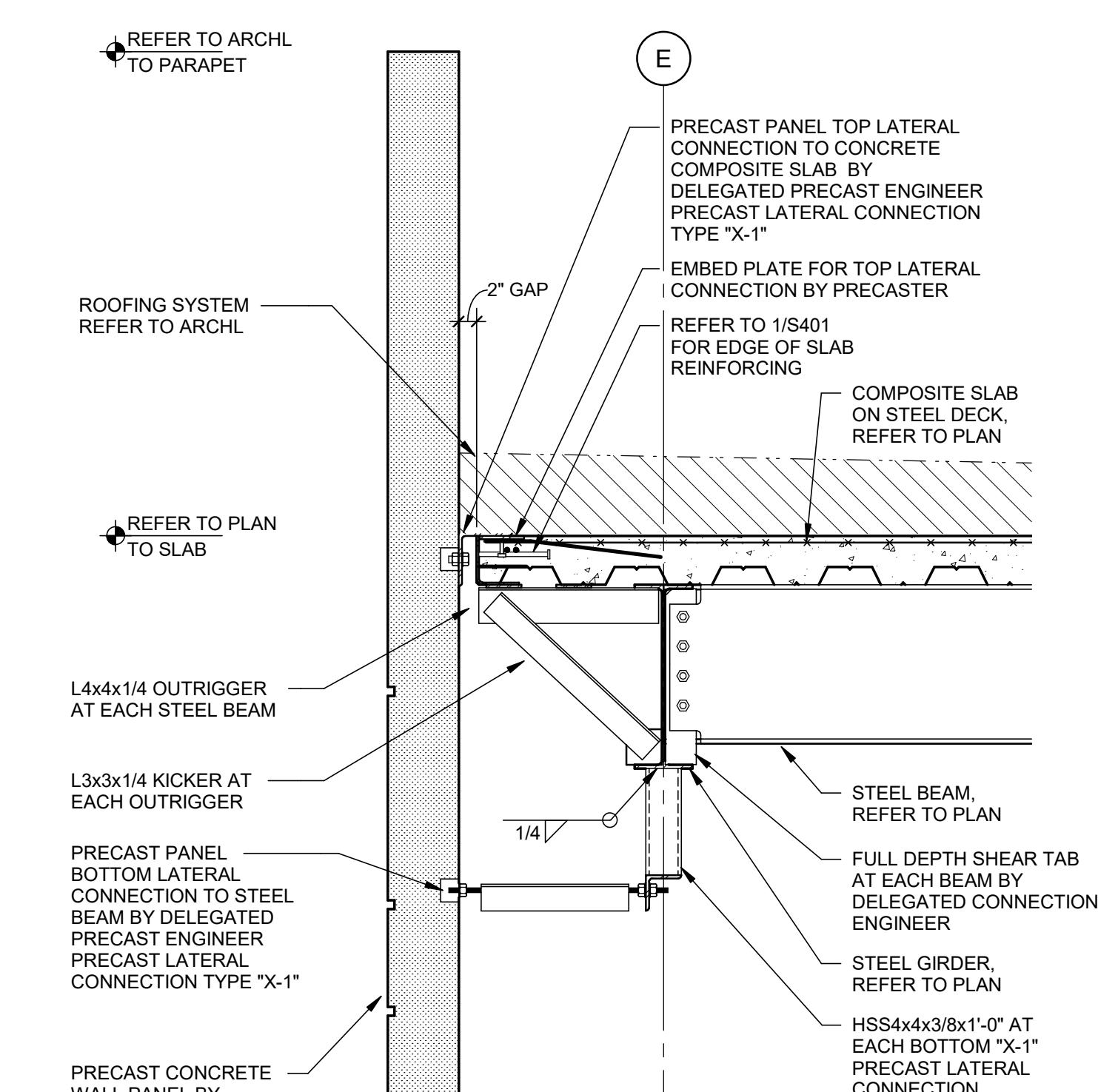
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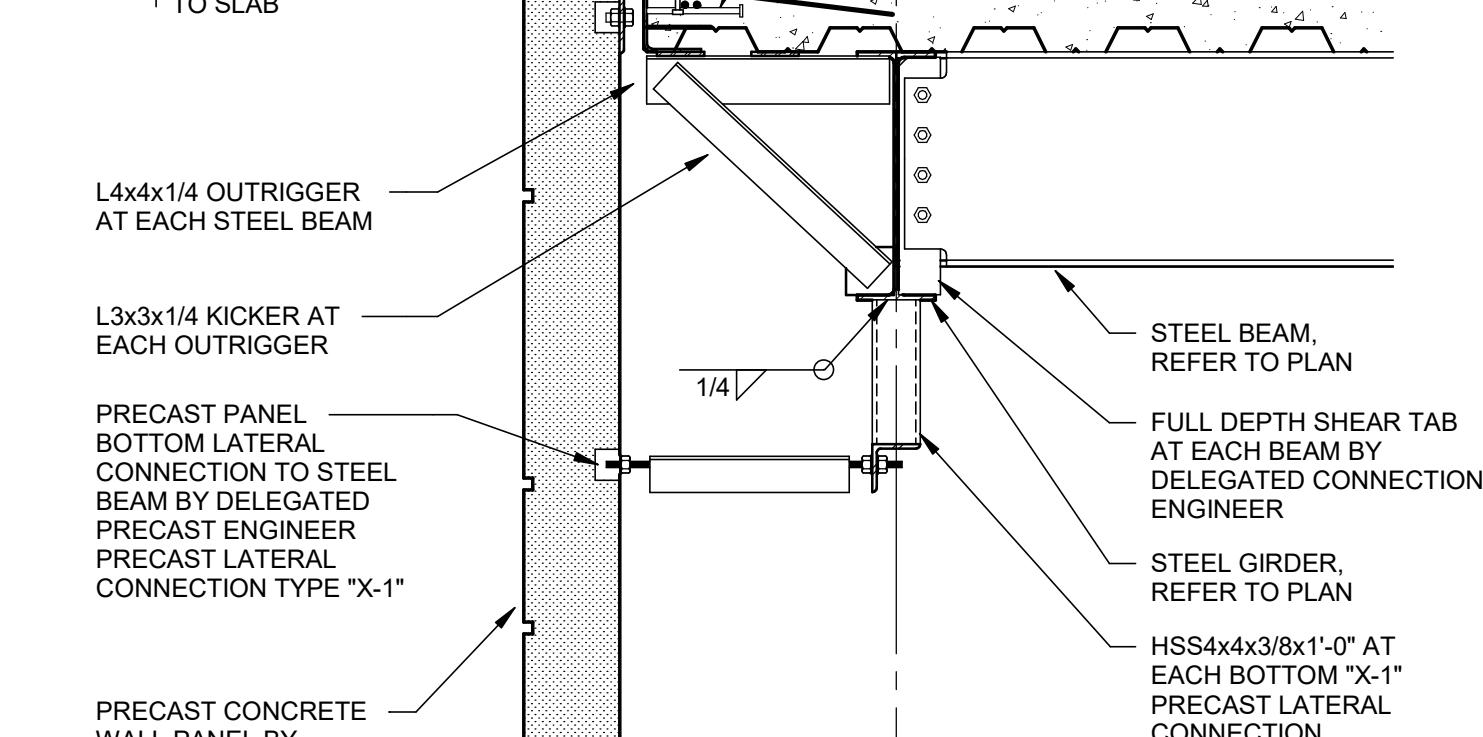
**5 FLOOR DIAPHRAGM TO SHEAR WALL CONNECTION AT STAIR LANDING**  
3/4" = 1'-0"



**10 PRECAST WALL PANEL LATERAL CONNECTION TO SHEAR WALL**  
3/4" = 1'-0"



**15 COMPOSITE SLAB EDGE CONDITION AT ROOF**  
3/4" = 1'-0"



**14 COMPOSITE SLAB EDGE CONDITION AT ROOF**  
3/4" = 1'-0"



**13 EDGE OF COMPOSITE SLAB**  
3/4" = 1'-0"



**12 EMBED PLATE ELEVATIONS**  
1" = 1'-0"



**11 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
3/4" = 1'-0"



**10 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
3/4" = 1'-0"



**9 ROOF COMPOSITE SLAB TO CONCRETE SHEAR WALL CONNECTION**  
3/4" = 1'-0"



**8 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
3/4" = 1'-0"



**7 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
3/4" = 1'-0"



**6 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
3/4" = 1'-0"



**5 STEEL BEAM CONNECTION TO SHEAR WALL AT WALL OPENING**  
3/4" = 1'-0"



**4 STEEL BEAM CONNECTION TO SHEAR WALL AT WALL OPENING**  
3/4" = 1'-0"



**3 STEEL BEAM CONNECTION TO CONCRETE SHEAR WALL**  
1" = 1'-0"



**2 STEEL GIRDER CONNECTION TO CONCRETE SHEAR WALL**  
1" = 1'-0"



**1 FLOOR DIAPHRAGM TO SHEAR WALL CONNECTION**  
1" = 1'-0"



**12 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**11 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**10 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**9 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**8 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**7 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**6 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**5 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**4 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**3 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**2 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**1 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**12 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**11 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**10 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**9 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**8 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**7 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**6 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**5 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**4 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**3 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**2 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"



**1 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"

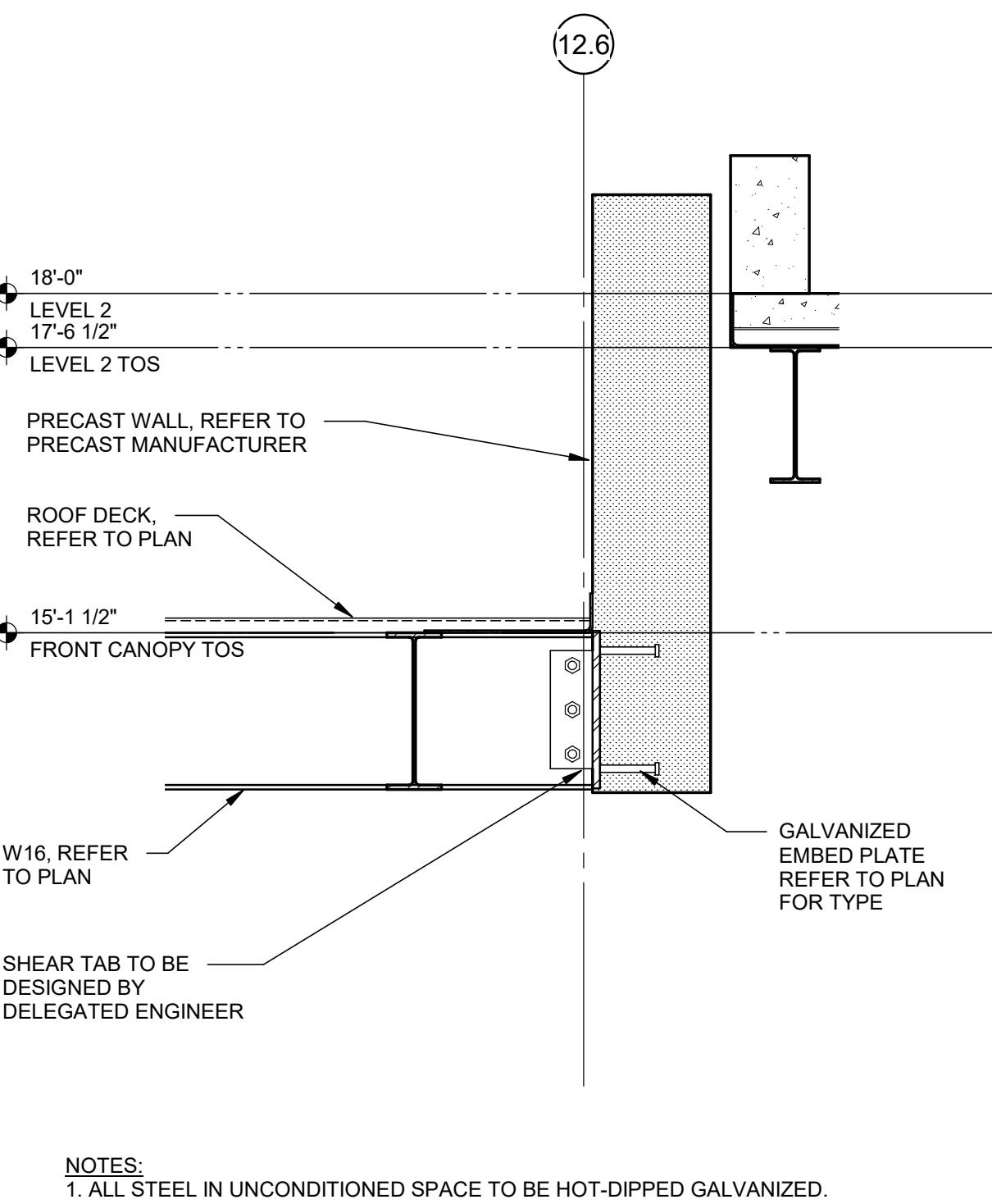
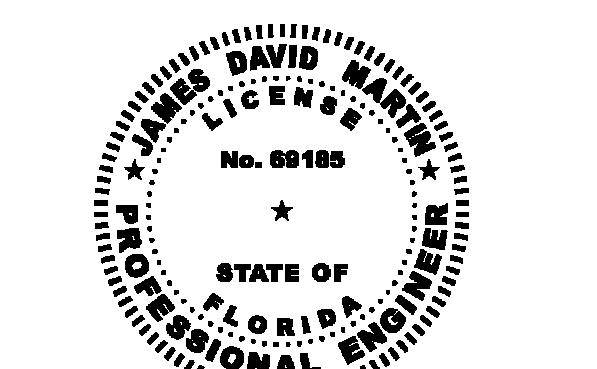


**12 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"

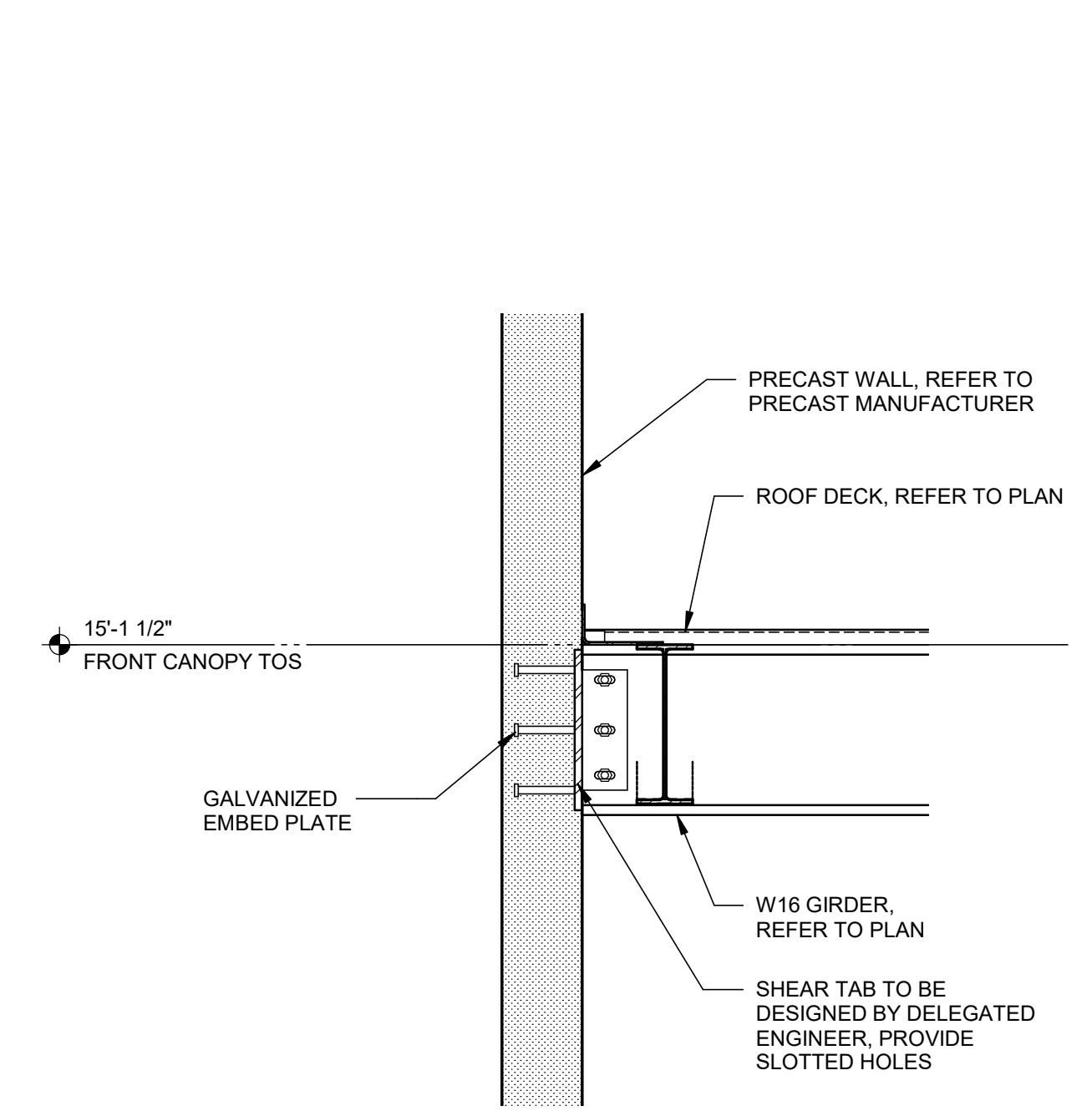


**11 EDGE OF COMPOSITE SLAB AT ELEVATOR OVERRUN**  
1" = 1'-0"

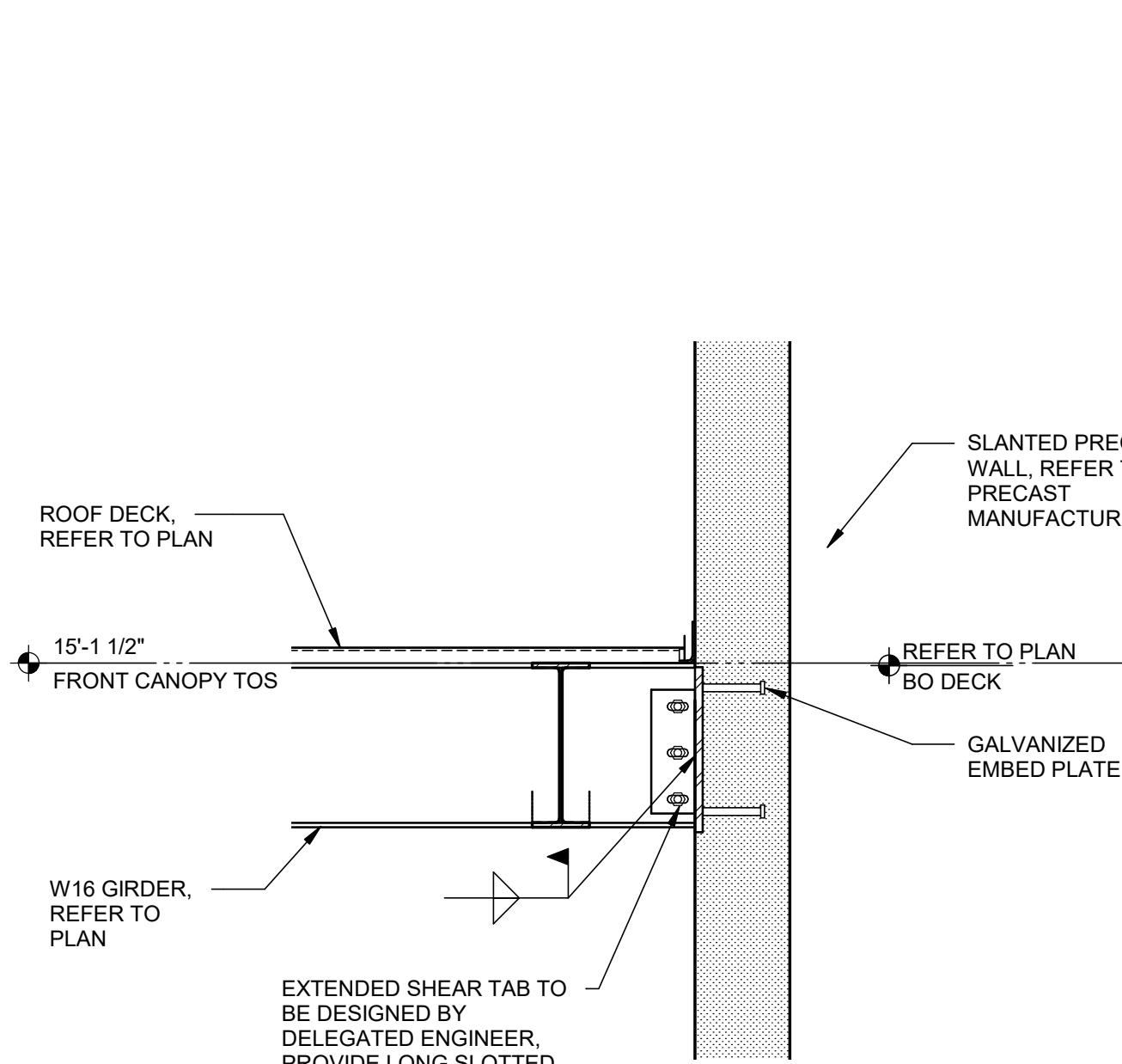




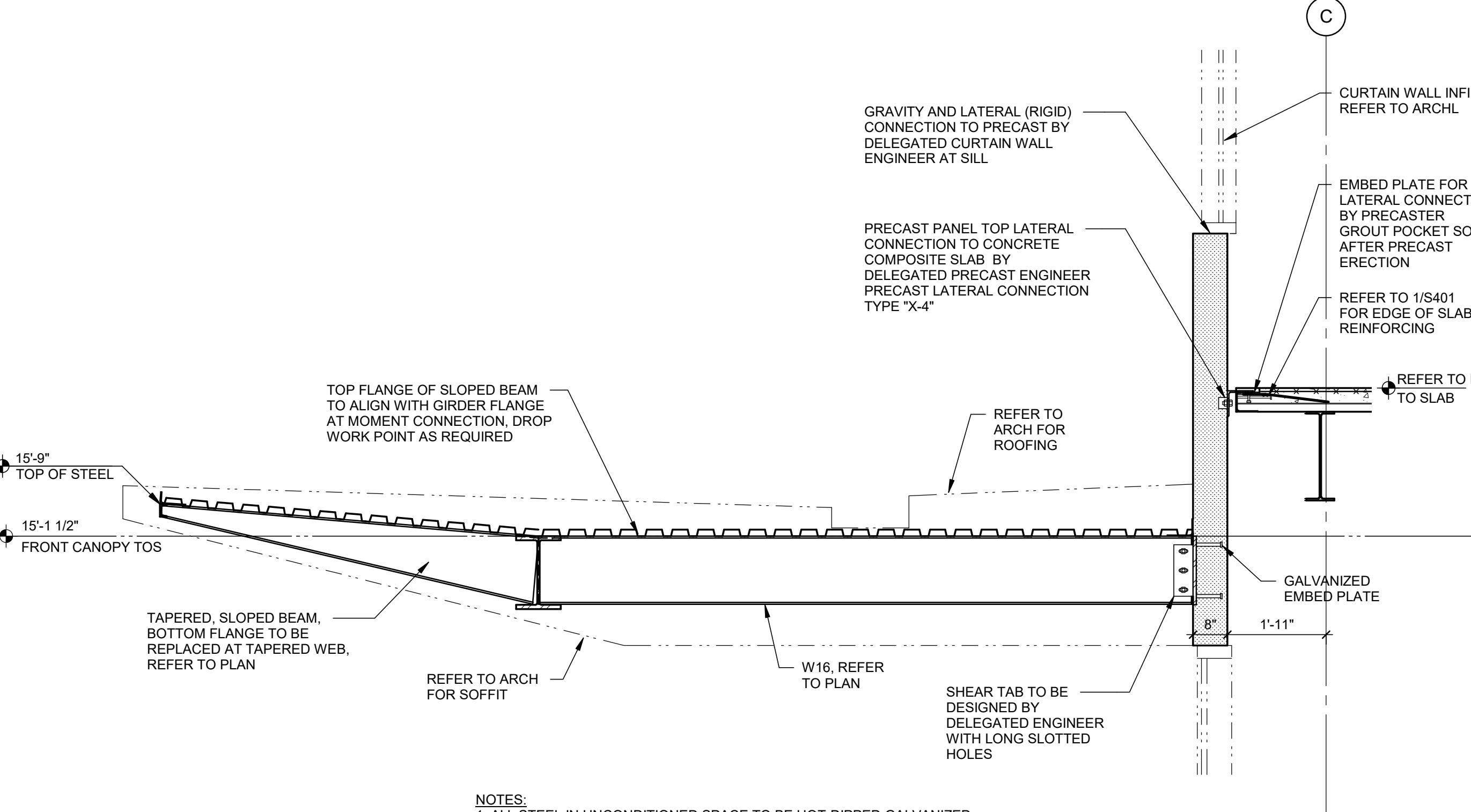
5 FRONT CANOPY BEAM TO PRECAST  
3/4" = 1'-0"



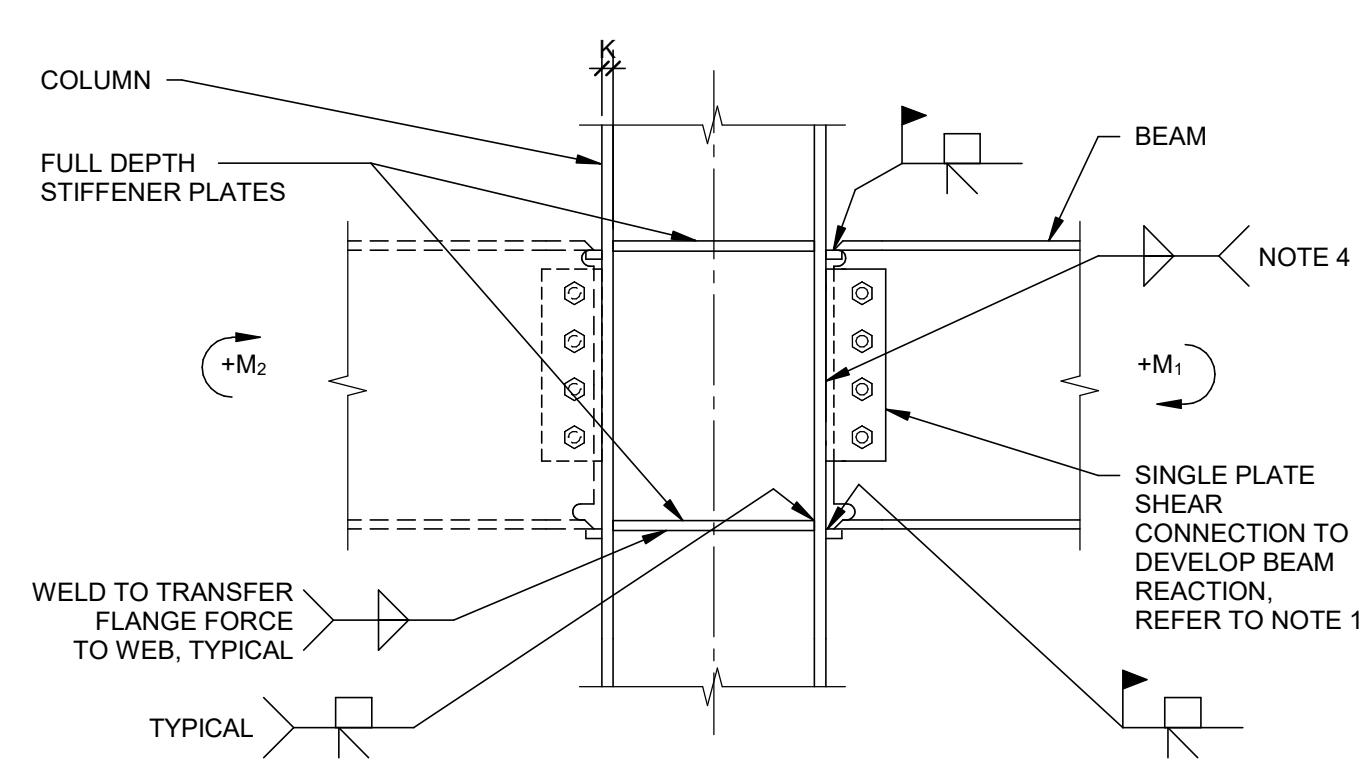
4 FRONT CANOPY GIRDER  
TO PRECAST WALL  
3/4" = 1'-0"



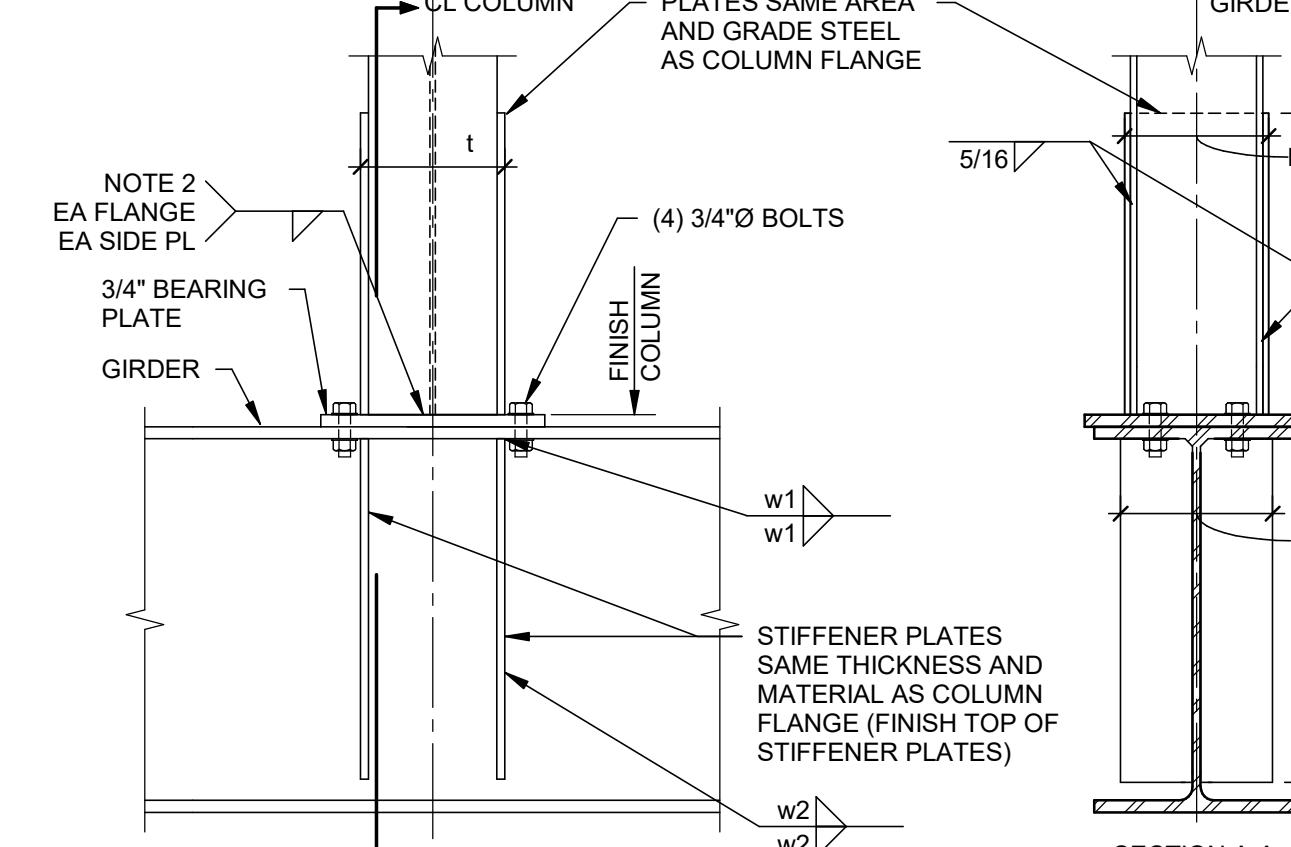
3 FRONT CANOPY GIRDER  
TO SLANTED PRECAST WALL  
3/4" = 1'-0"



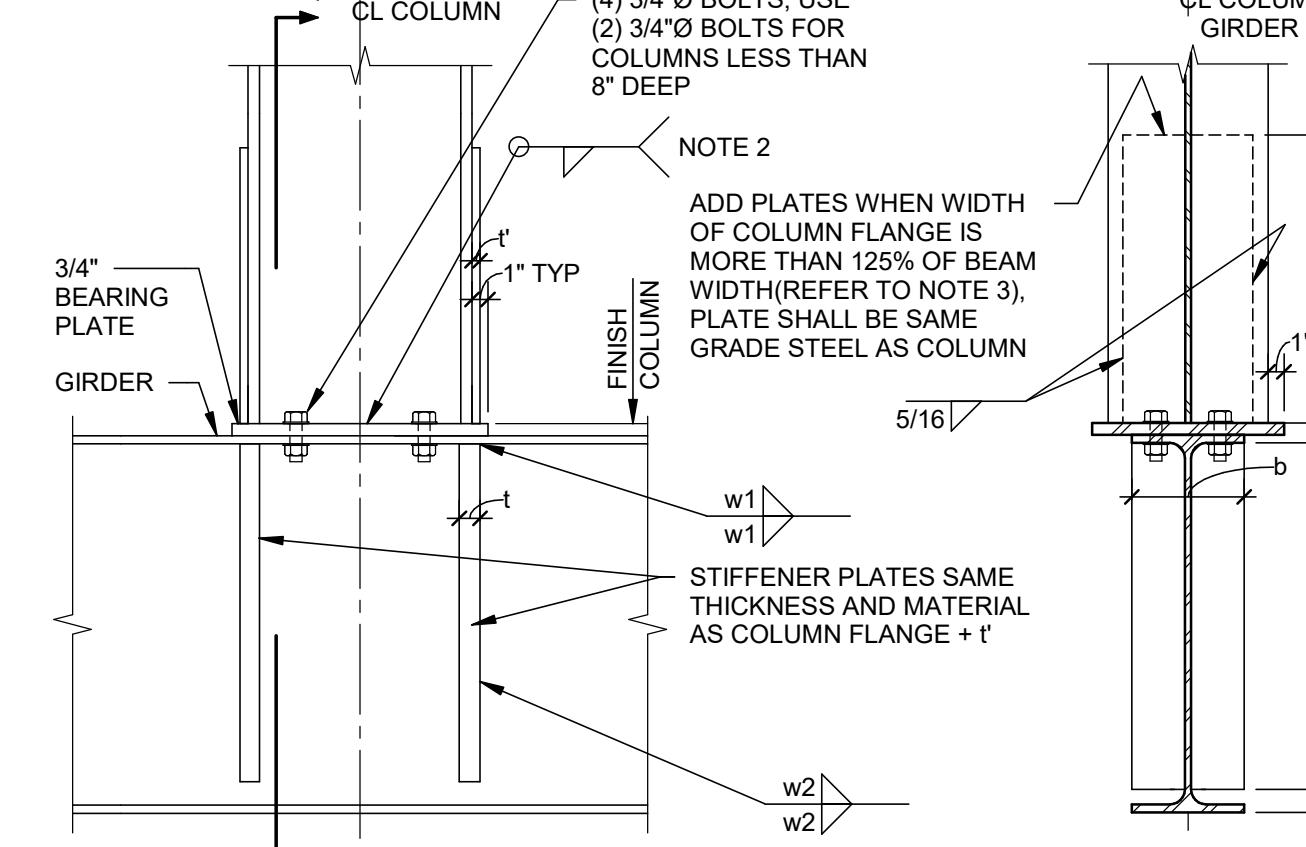
2 FRONT CANOPY BEAM SECTION  
1/2" = 1'-0"



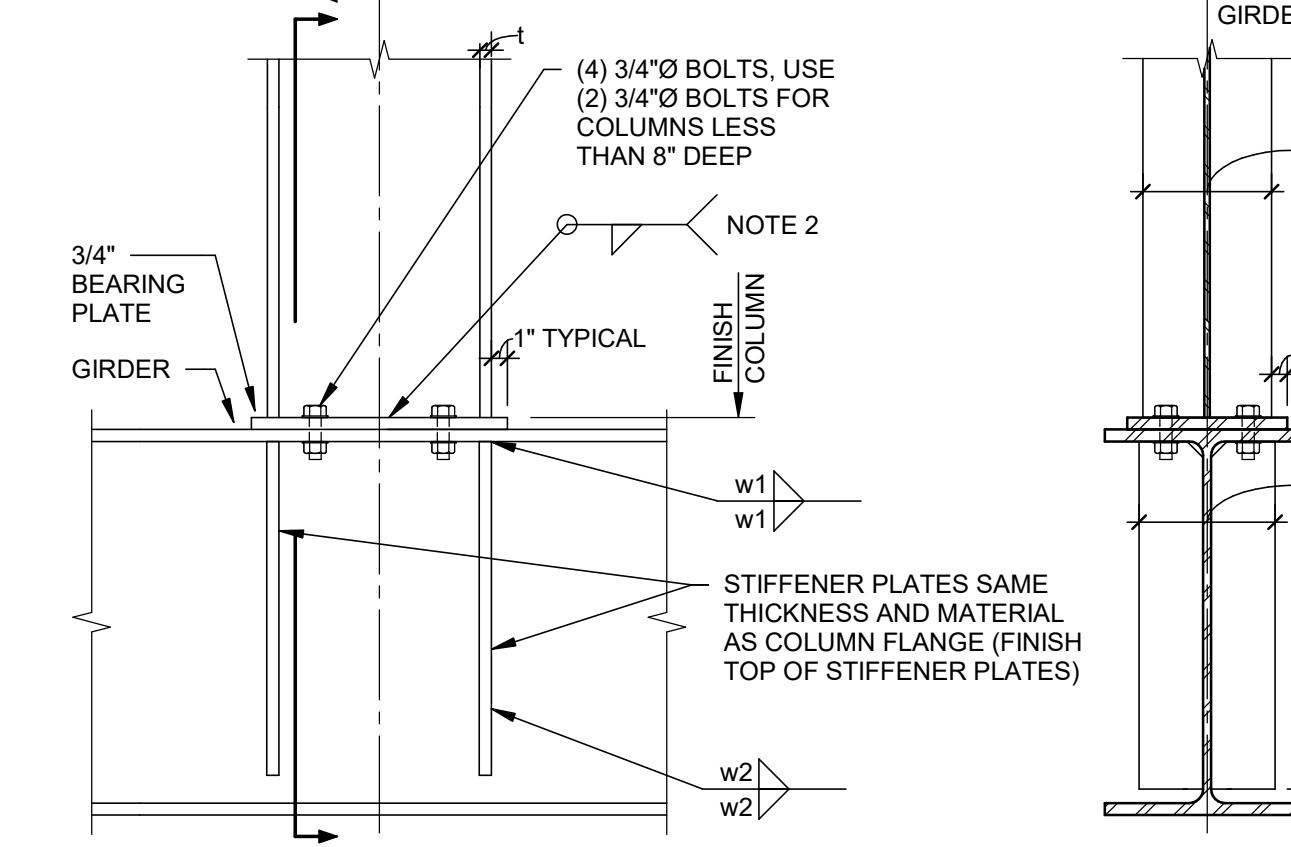
10 TYPICAL MOMENT CONNECTION  
AT COLUMN FLANGE  
NO SCALE



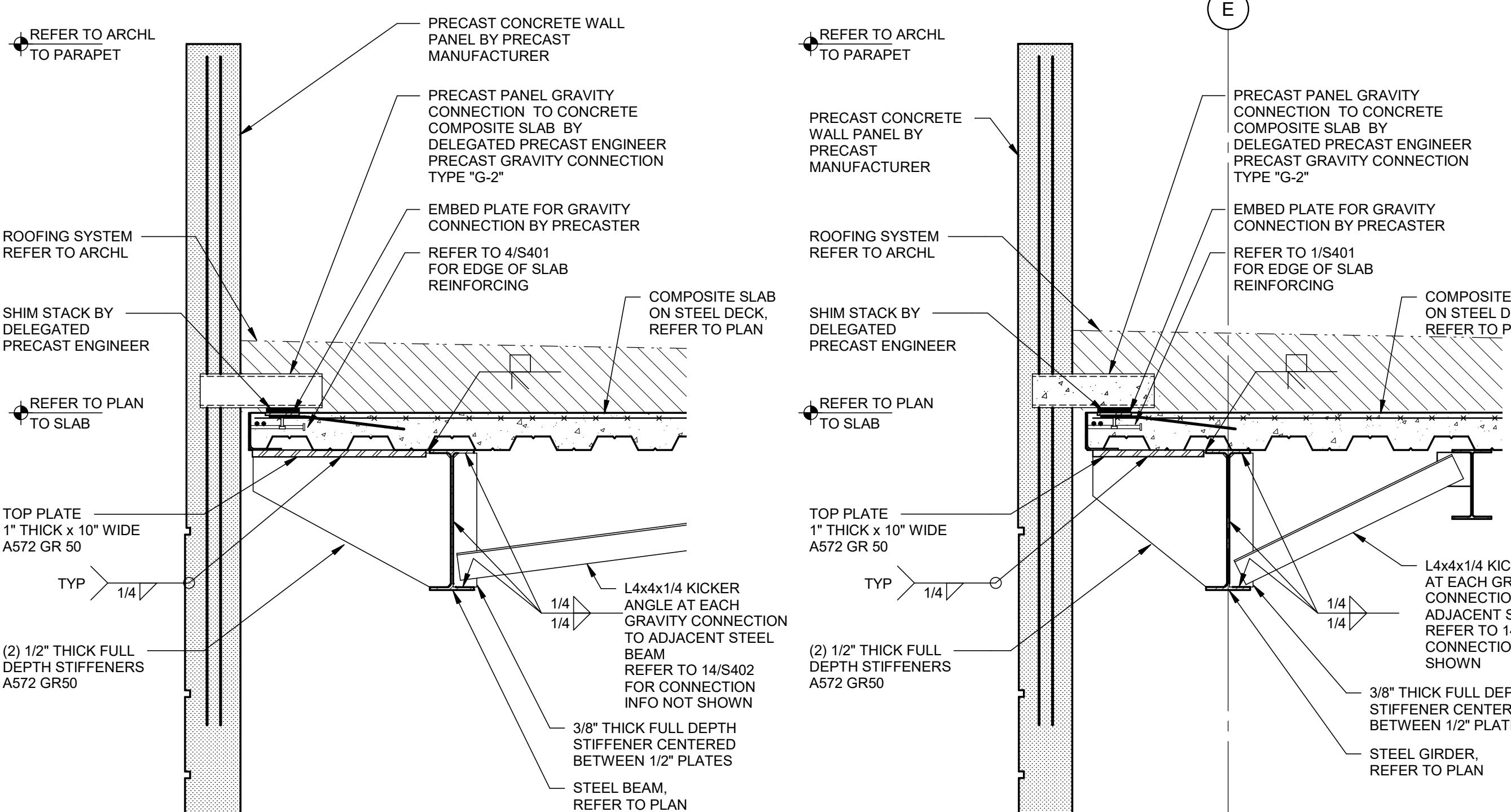
9 STEEL WIDE FLANGE COLUMN TRANSFER  
GIRDER WIDER THAN COLUMN DEPTH  
NO SCALE



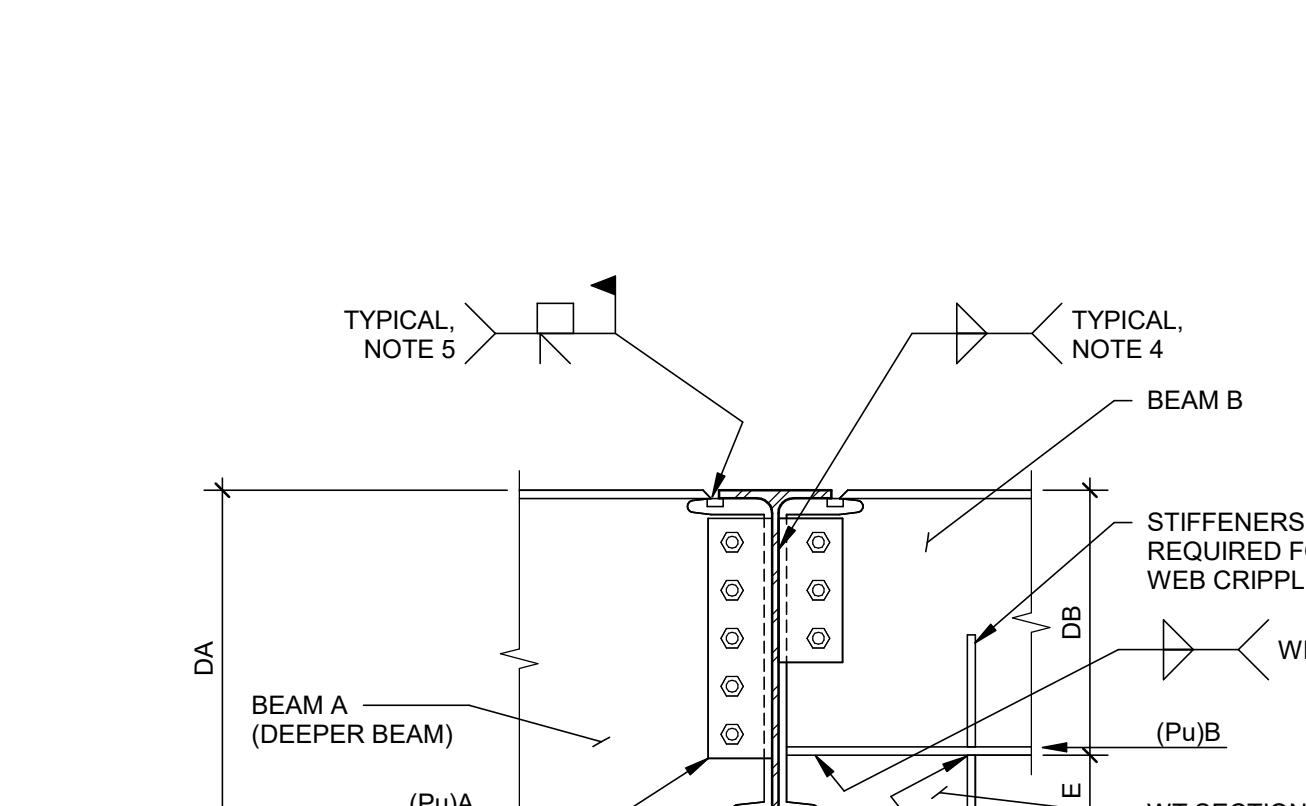
8 STEEL WIDE FLANGE COLUMN TRANSFER  
GIRDER NARROWER THAN COLUMN WIDTH  
NO SCALE



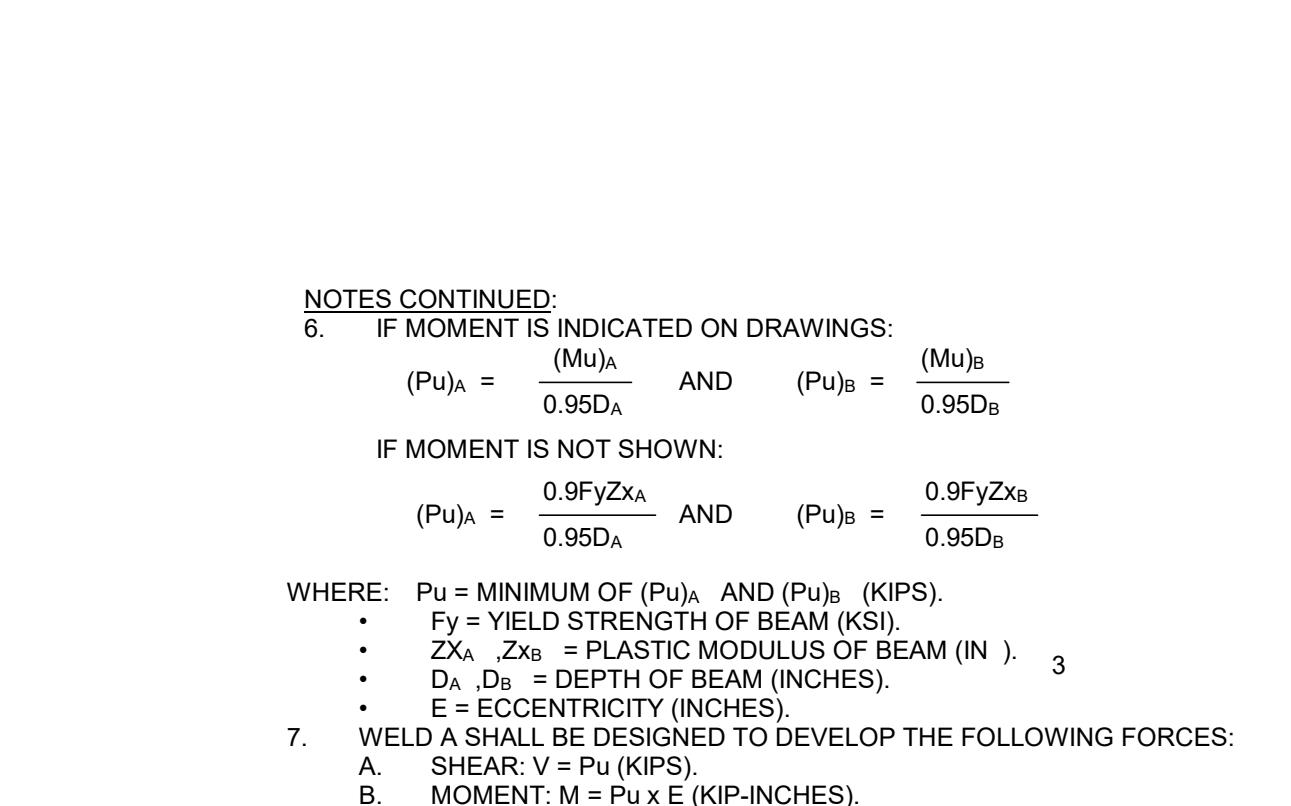
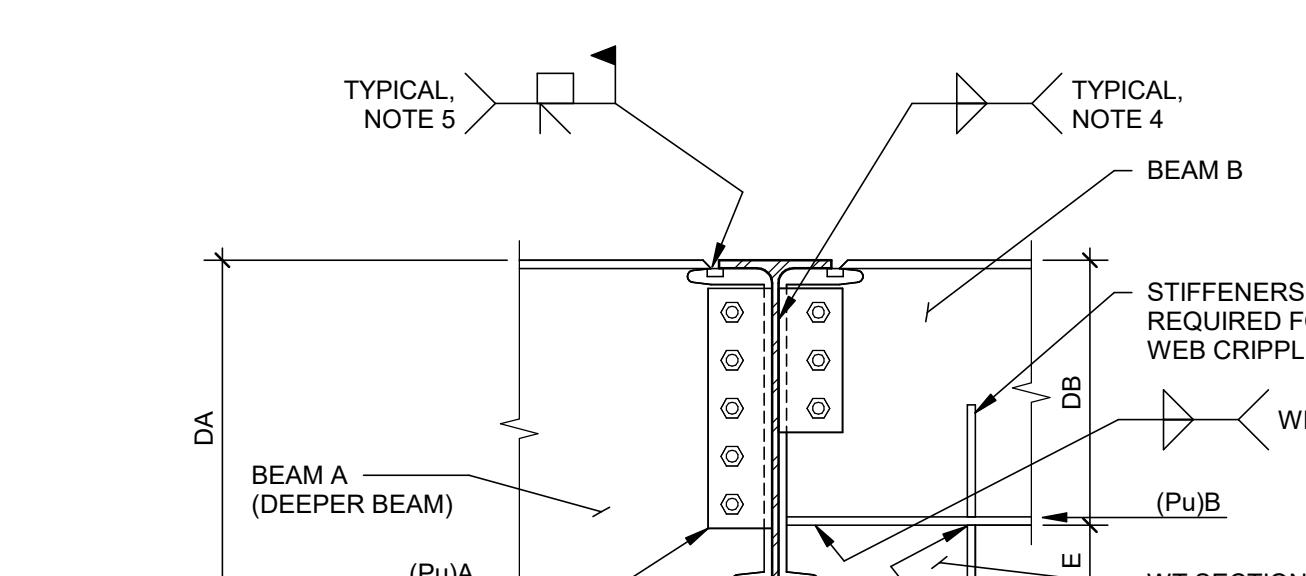
7 STEEL WIDE FLANGE COLUMN TRANSFER  
GIRDER WIDER THAN COLUMN WIDTH  
NO SCALE



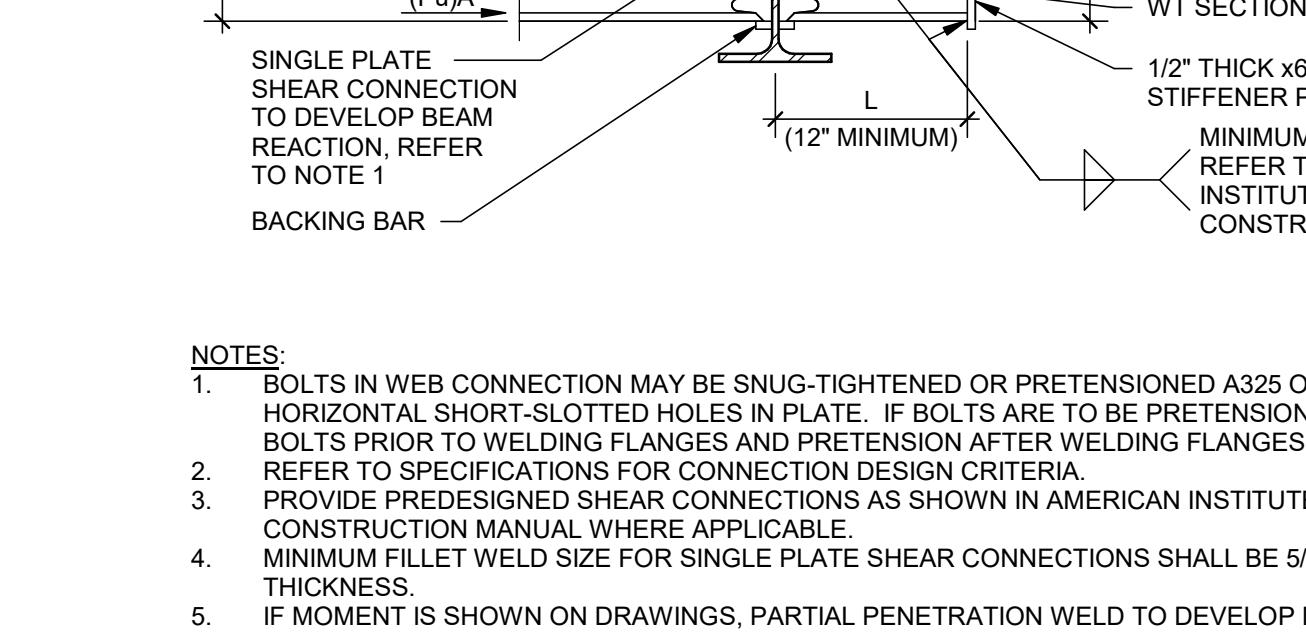
15 COMPOSITE SLAB EDGE CONDITION AT ROOF  
3/4" = 1'-0"



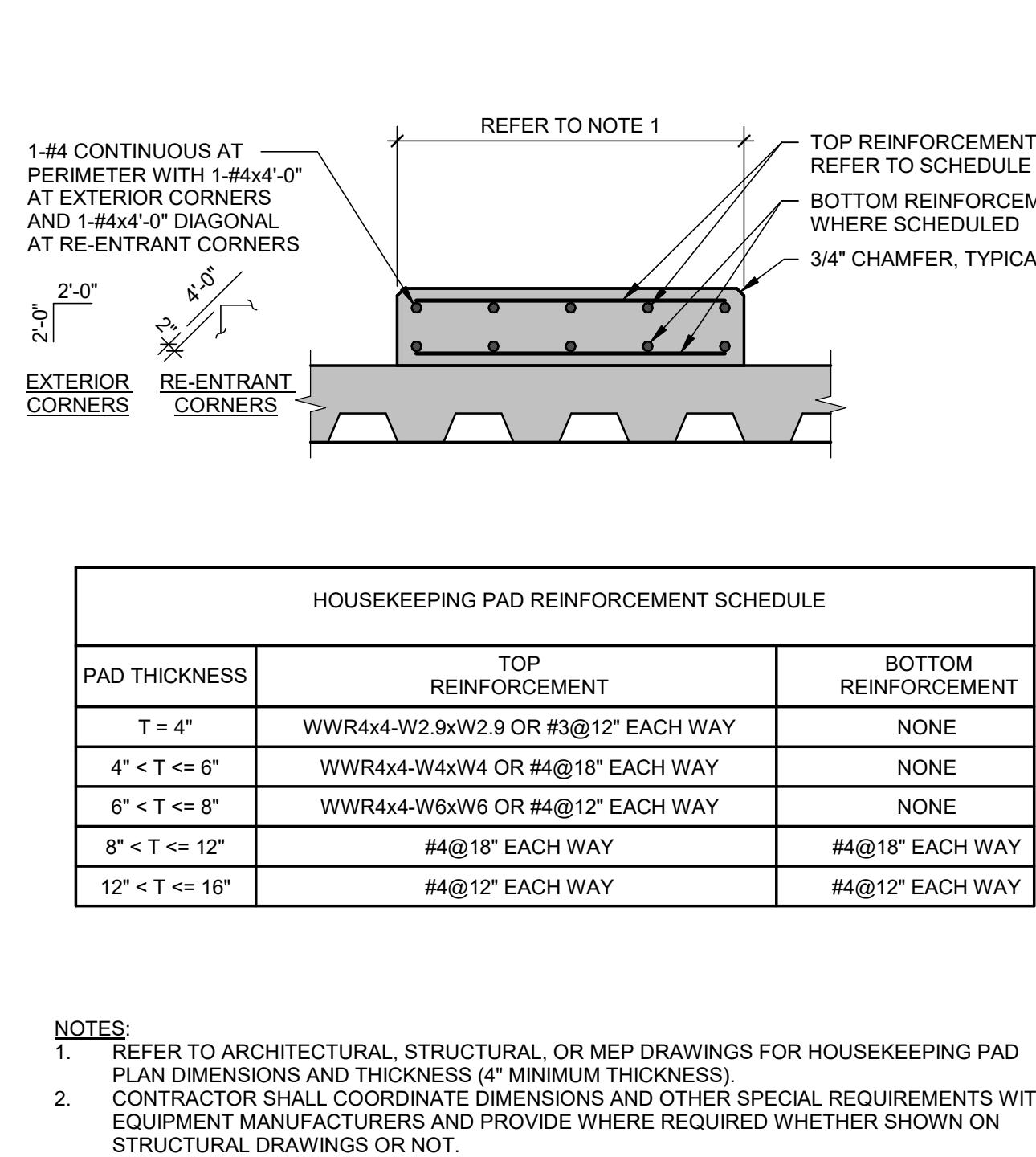
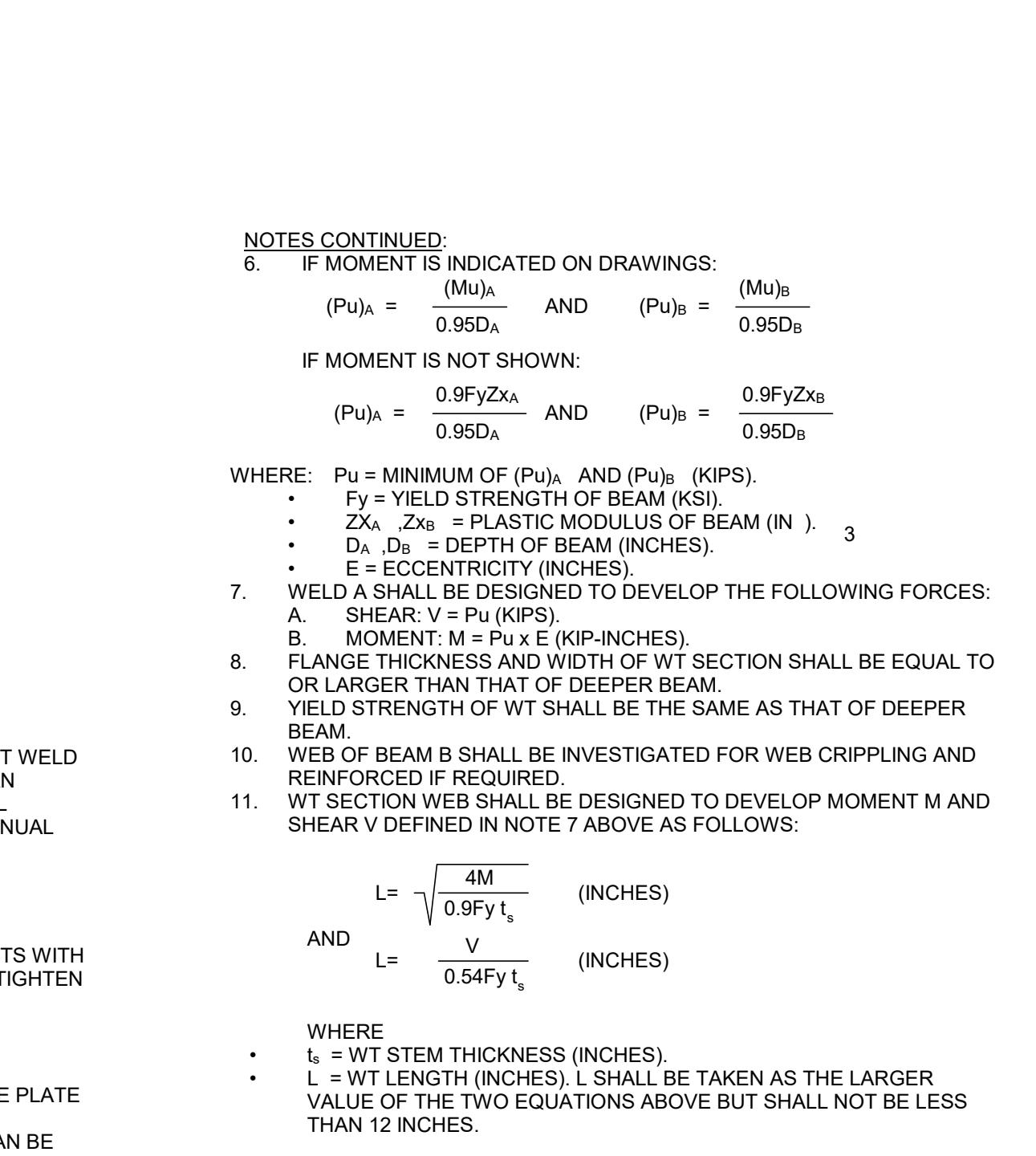
8 STEEL WIDE FLANGE COLUMN TRANSFER  
GIRDER NARROWER THAN COLUMN WIDTH  
NO SCALE




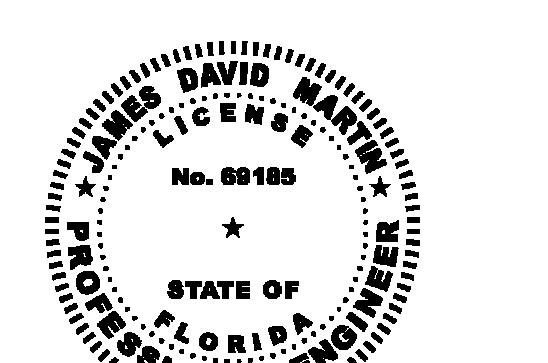
6 PARAPET LATERAL FRAMING - NORTH ROOF  
3/4" = 1'-0"

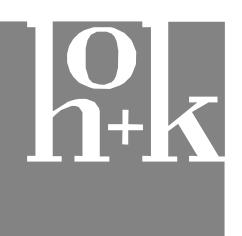


13 TYPICAL BEAM-TO-BEAM MOMENT CONNECTION  
NO SCALE



11 TYPICAL HOUSEKEEPING PAD  
OVER STRUCTURAL SLAB  
NO SCALE





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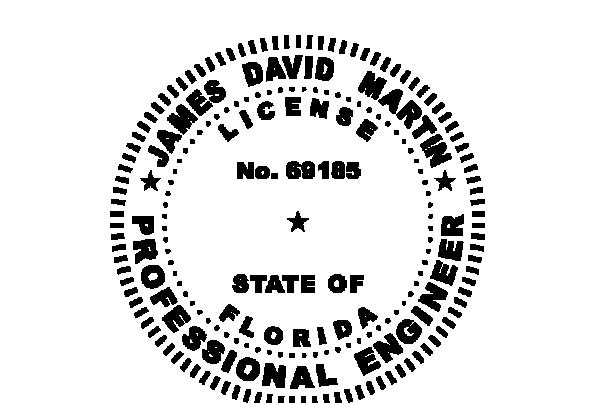
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Tampa, FL 33602

**SIEBEIN** ACOUSTIC  
625 NW 60th St, Suite C  
Gainesville, FL 32607

Sarasota County Administration Center  
1 Apex Road Sarasota, Florida 34240

Project No. 22.2900.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
2 ASI-02 05/17/2024  
3 ASI-03 06/07/2024  
4 ASI-05 08/01/2024  
6 ASI-07 09/26/2024



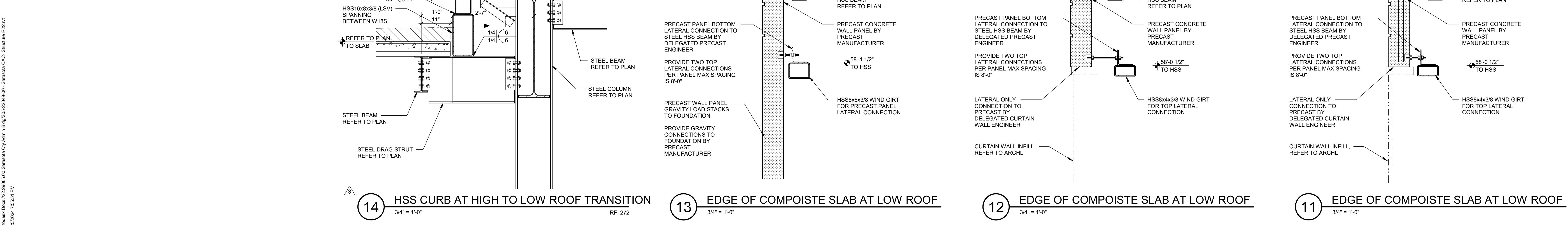
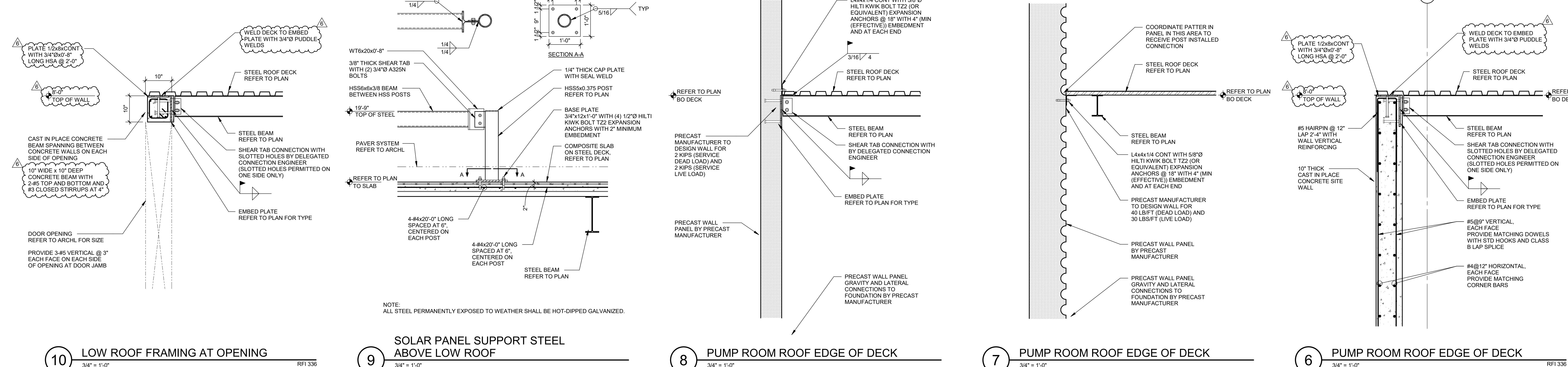
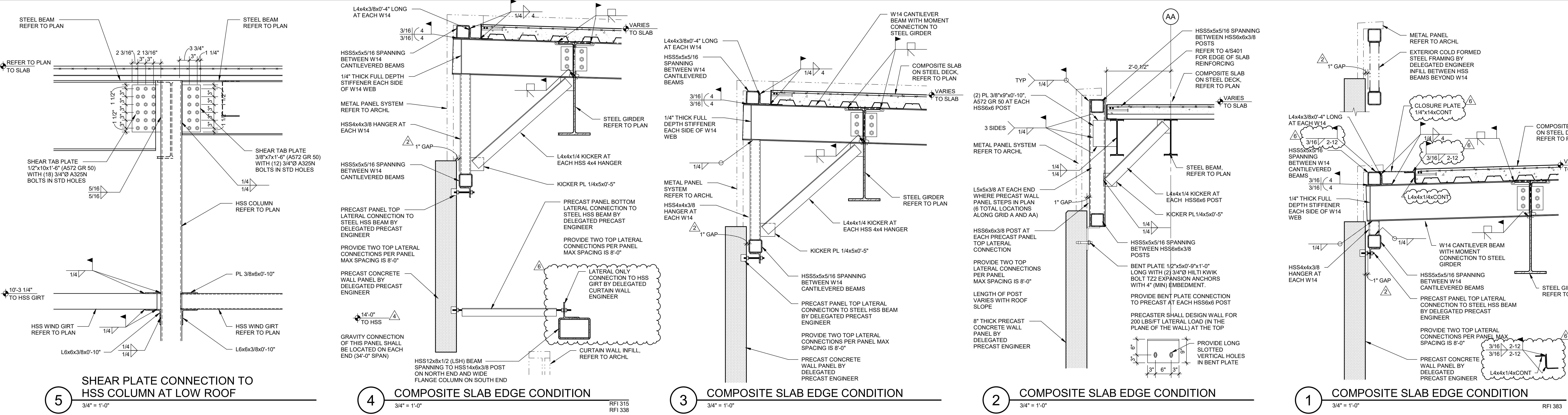
James David Martin, P.E., FL# 69185  
WPM Project No. S03-2204-00  
Certificate of Authorization No. 3818

To the best of the Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S414**  
STEEL FRAMING DETAILS

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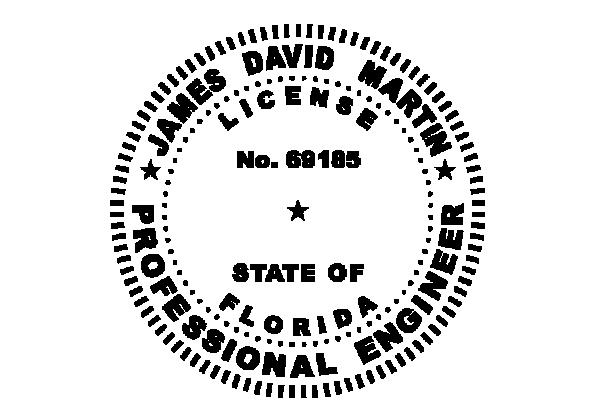


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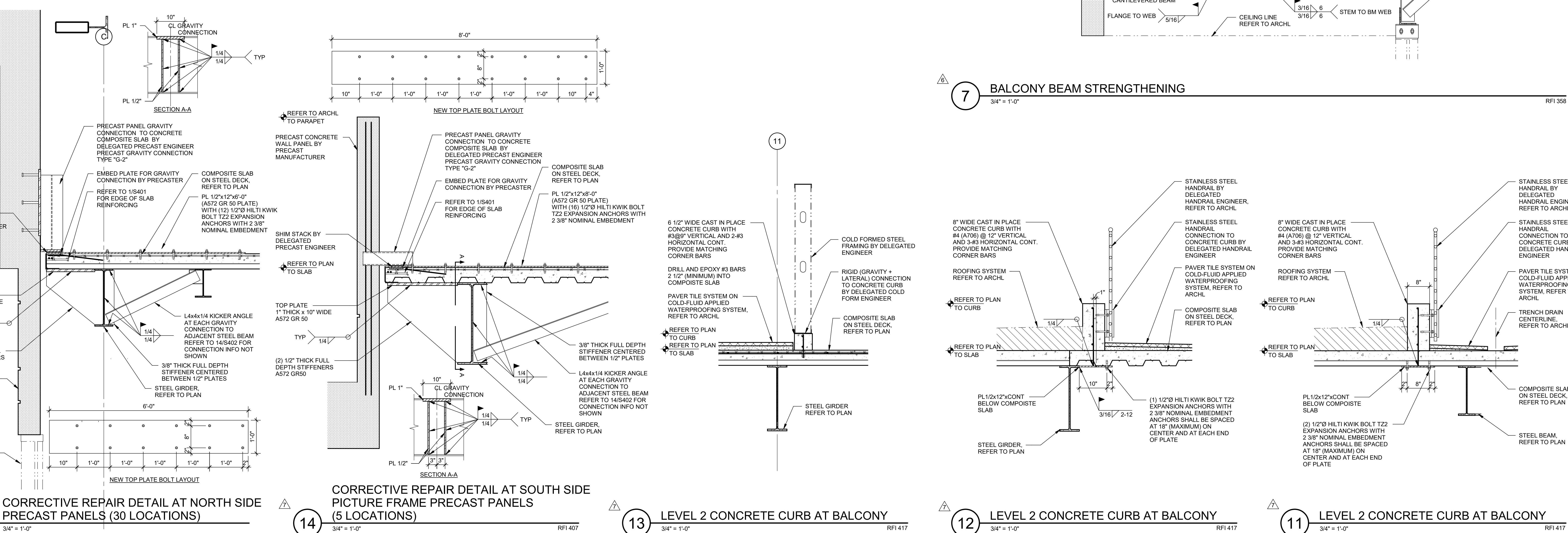
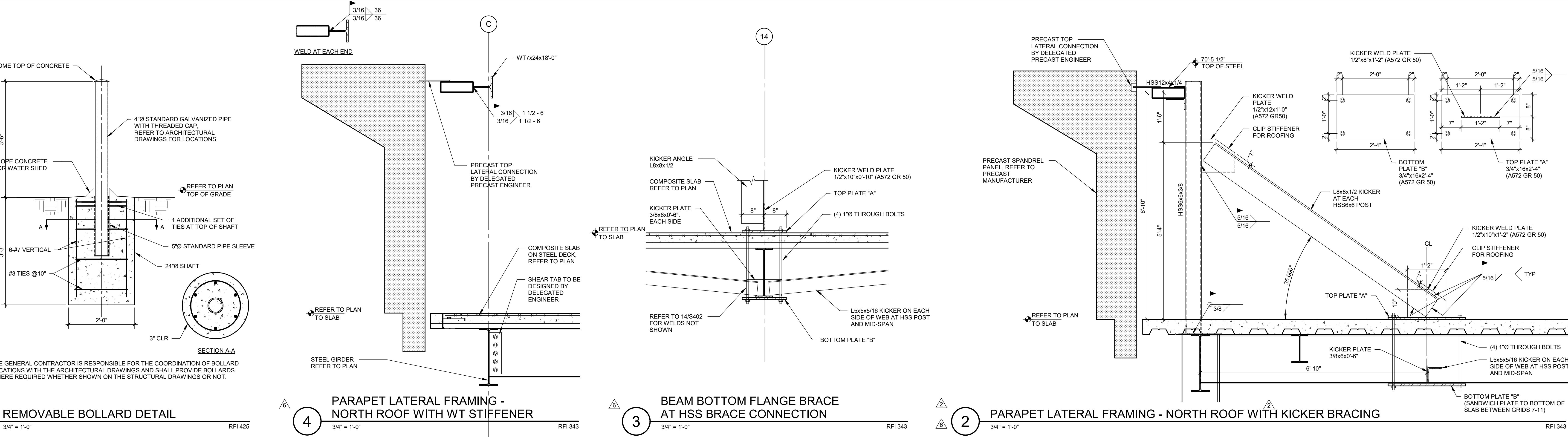
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Drawn By: BD  
Checked By: JDM  
Date: 09/08/2023

Revisions:  
2 ASI-02 05/17/2024  
6 ASI-07 09/26/2024  
7 ASI-09 10/25/2024  
8 ASI-10 11/22/2024



To the best of the Engineer's knowledge, the plans and specifications comply with all minimum building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 503 and 603 of Florida Statutes.





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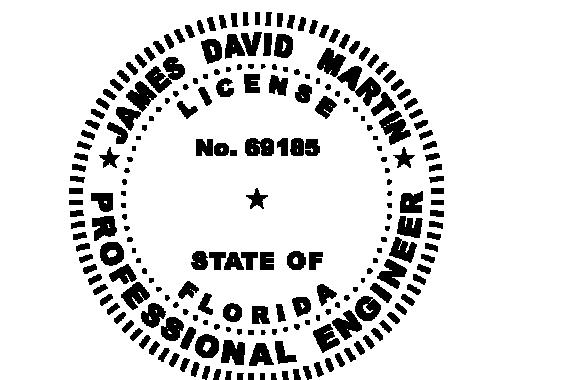
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Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:



James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22045-00  
Certificate of Authorization No. 663 of

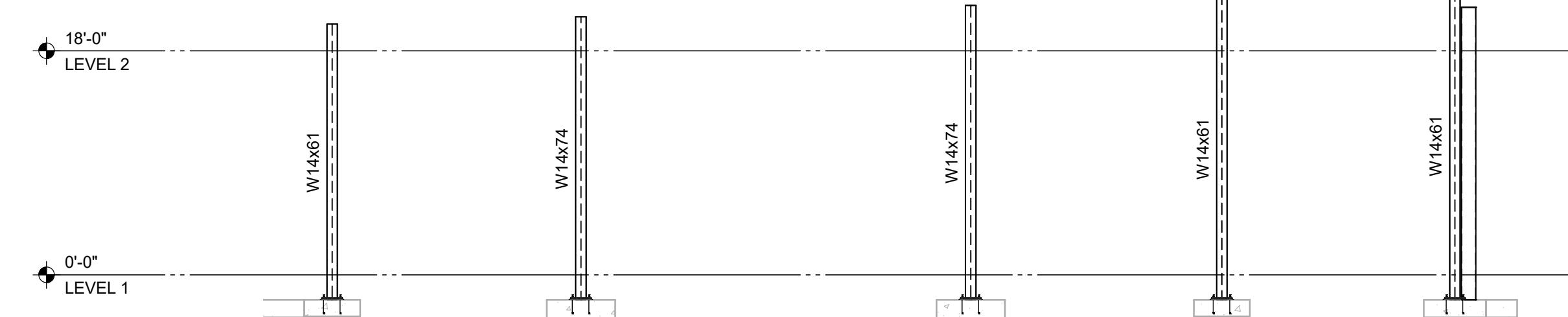
To the best of the Engineer's knowledge, the plans and specifications comply with all applicable laws, including codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

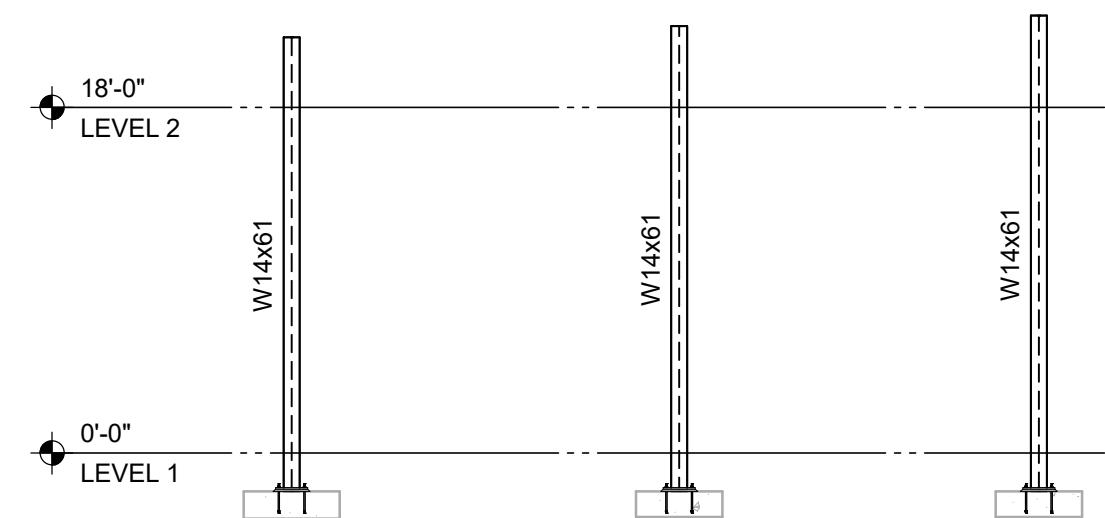
**S511**  
STEEL COLUMN  
FRAME ELEVATIONS

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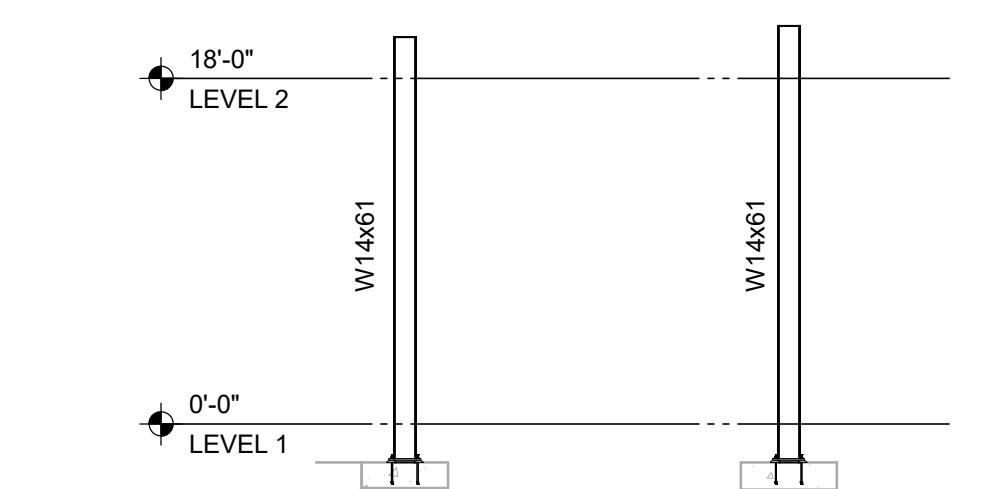
COLUMN ELEVATION NOTES:  
1. ALL MEMBERS SHALL BE  $F_y = 50$  KSI STEEL.  
2. PROVIDE COMPRESSION SPLICES FOR ALL COLUMN UNLESS NOTED OTHERWISE.  
3. REFER TO DETAILS AND PLANS FOR TOP ELEVATION OF COLUMNS.  
4. REFER TO 4/302 FOR BASE PLATE SCHEDULE INCLUDING ANCHOR ROD SIZES,  
EMBEDMENT, AND TYPES.  
5. ALL EXTERIOR AND INTERIOR COLUMN SPLICES ARE TO BE 48" (MINIMUM) ABOVE  
FINISH FLOOR.  
6. ALL STEEL COLUMNS ON GRID C AND E ARE TO HAVE 1" THICK CAP PLATE FOR  
PARAPET SUPPORT STEEL ATTACHMENT. WELD CAP PLATE TO COLUMN USING 3/8"  
THICK FILLET WELD ON EACH FLANGE AND BOTH SIDES OF WEB.



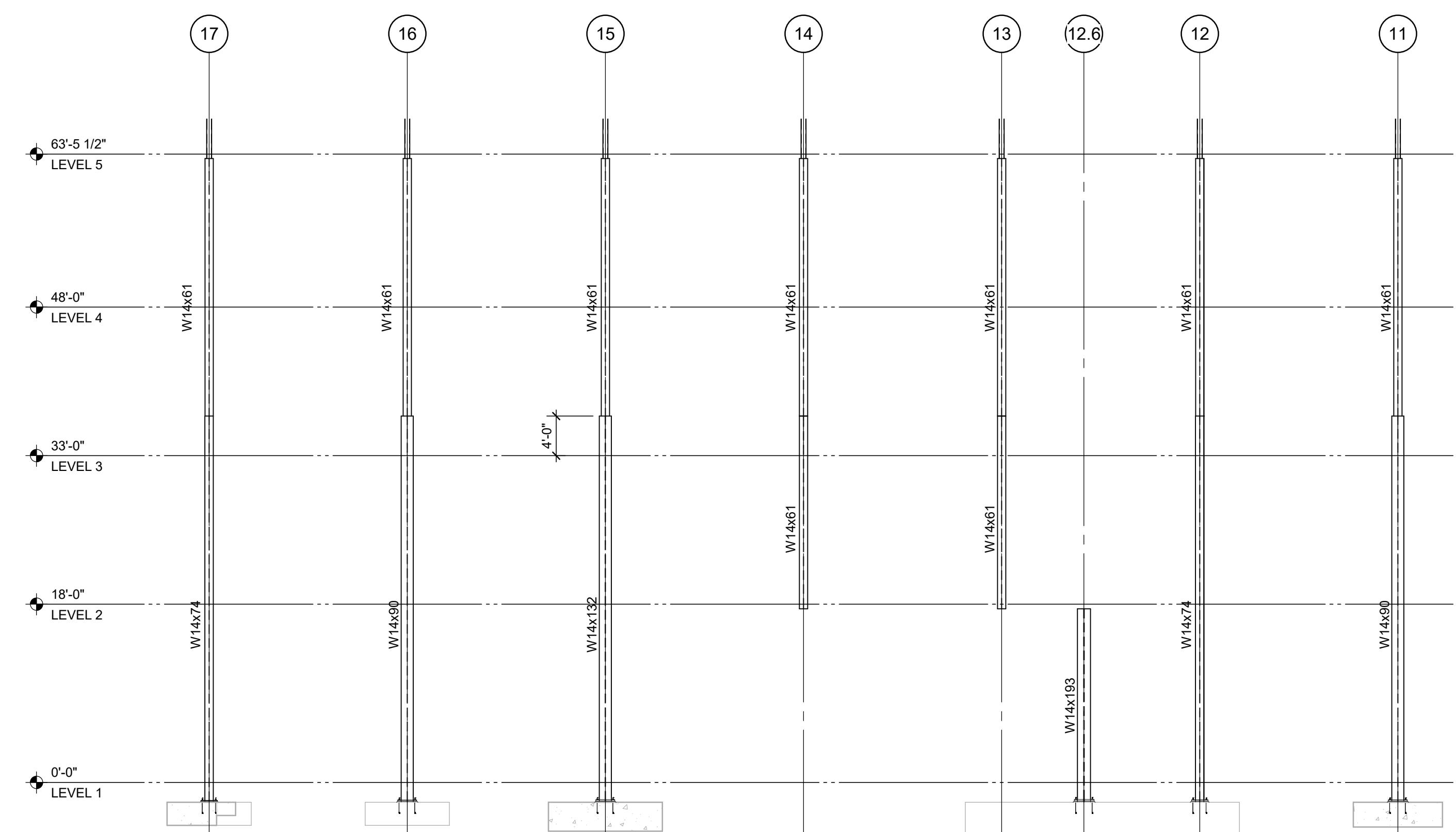
3 GRID B - COLUMNS ELEVATION



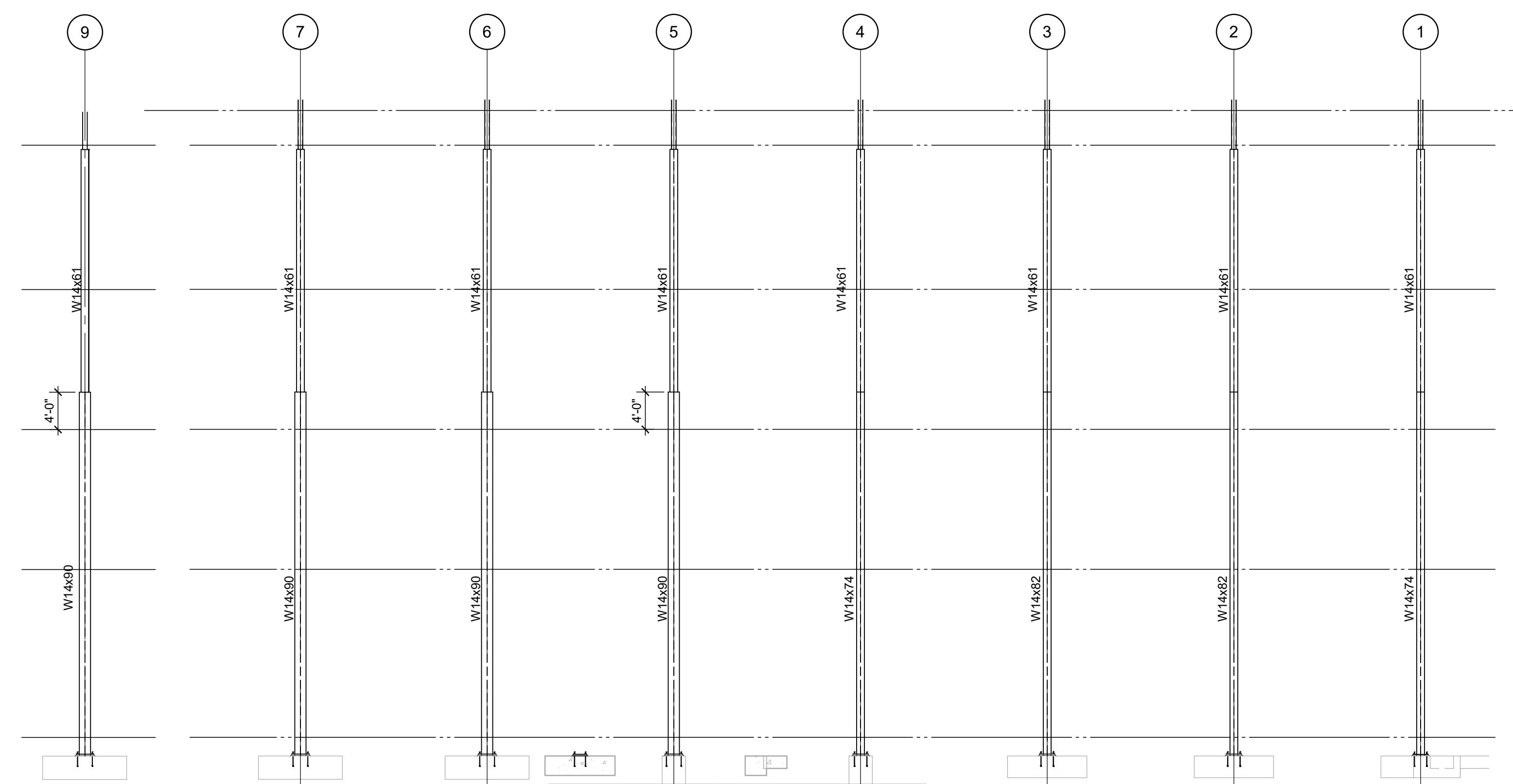
2 GRID AA - COLUMNS ELEVATION



1 GRID A - COLUMNS ELEVATION



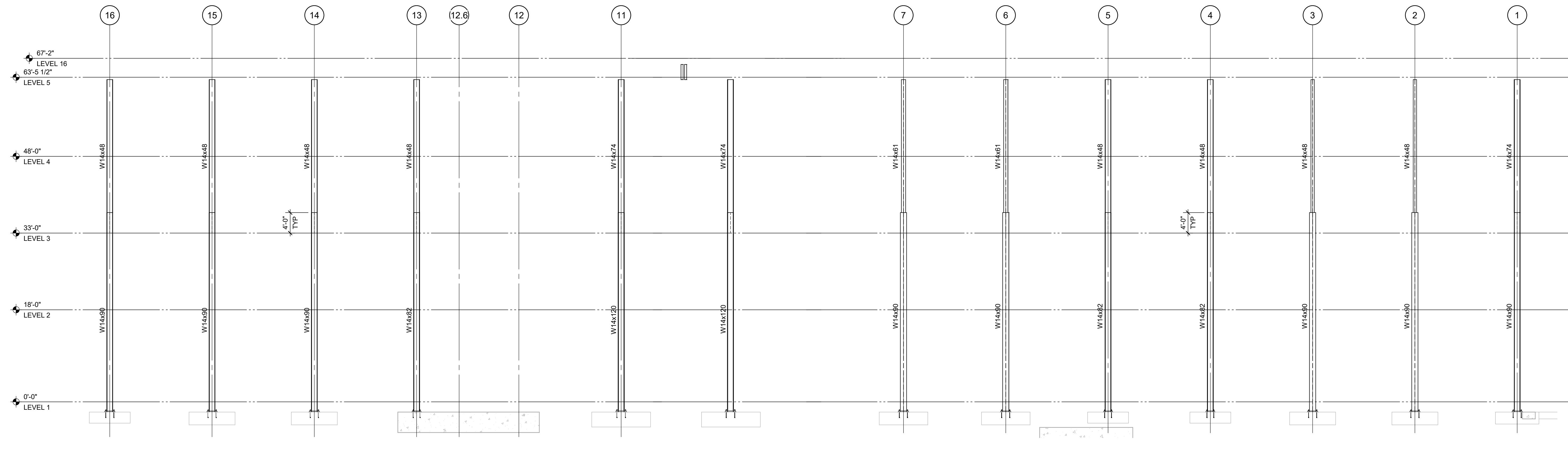
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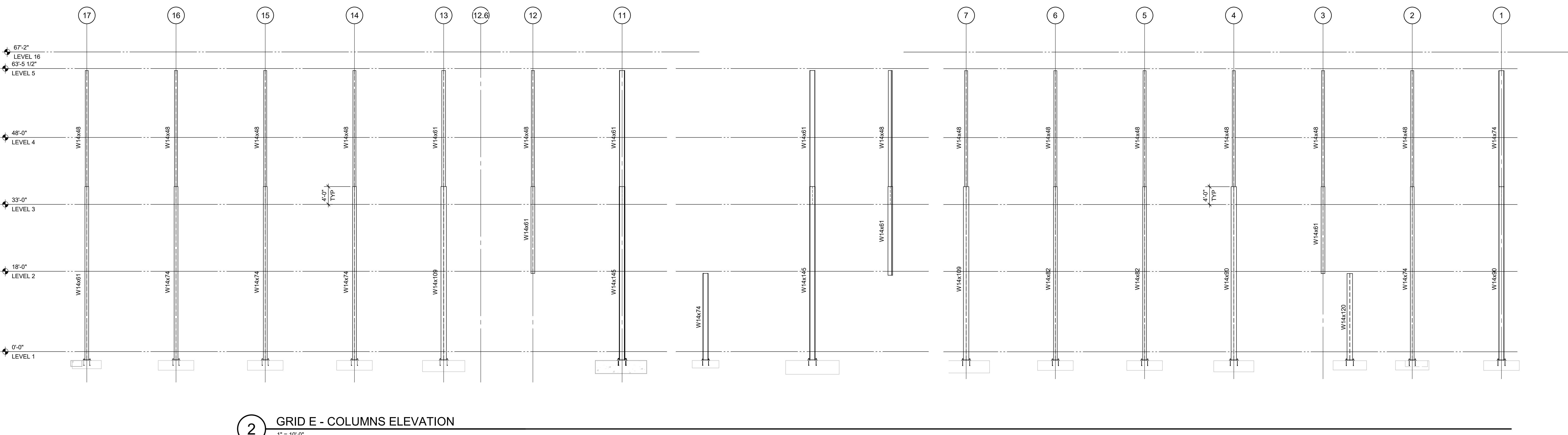


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COLUMN ELEVATION NOTES:  
1. ALL MEMBERS SHALL BE Fy = 50 KSI STEEL.  
2. PROVIDE COMPRESSION SPLICES FOR ALL COLUMN UNLESS NOTED OTHERWISE.  
REFER TO 5.5A01.  
3. REFER TO 5.5A01 AND PLANS FOR TOP ELEVATION OF COLUMNS.  
4. REFER TO 5.5A01 FOR BASE PLATE SCHEDULE INCLUDING ANCHOR ROD SIZES,  
EMBEDMENT, AND TYPES.  
5. ALL PERIMETER AND INTERIOR COLUMN SPLICES ARE TO BE 48" (MINIMUM) ABOVE  
FLOOR FLOORS.  
6. ALL STEEL COLUMNS ON GRID C AND E ARE TO HAVE 1" THICK CAP PLATE FOR  
PARAPET SUPPORT STEEL ATTACHMENT. WELD CAP PLATE TO COLUMN USING 3/8"  
THICK FILLET WELD ON EACH FLANGE AND BOTH SIDES OF WEB.

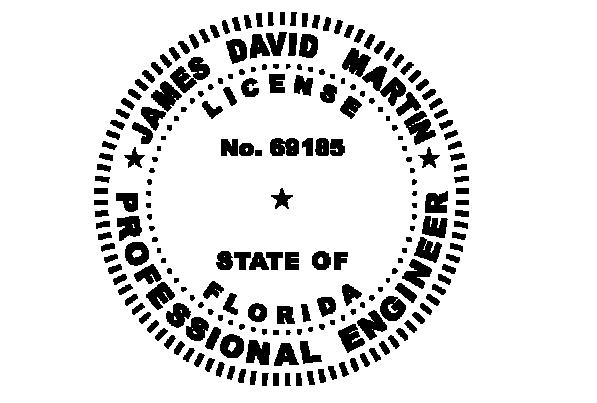


Sarasota County  
Administration Center

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:



James David Martin, P.E. FL P.E. No. 66185  
WPM Project No. S05-2204-040  
Certificate of Authorization No. 66185  
To the best of the Engineer's knowledge, the plans and specifications comply with all applicable laws, including  
codes and applicable fire safety standards as determined by  
local authority in accordance with Chapter 553 and 603 of  
Florida Statutes.

CONFORMED SET  
02/14/2024

**S512**  
STEEL COLUMN  
FRAME ELEVATIONS



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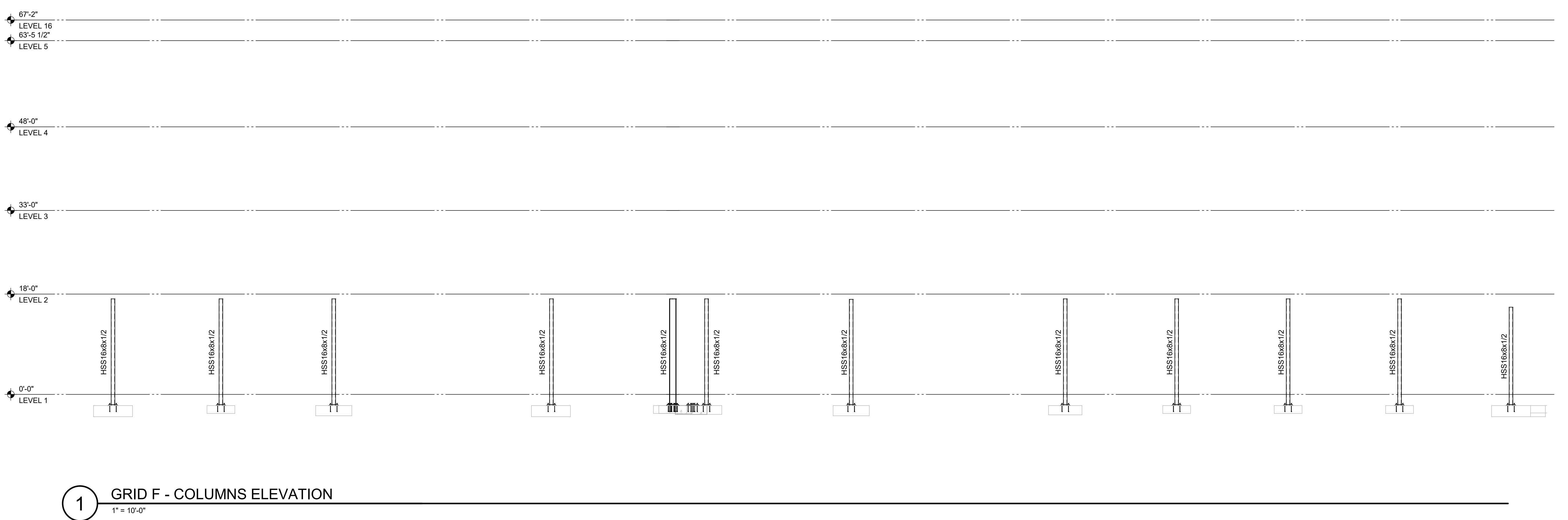
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Sarasota, FL 34240

**Walter P. Moore**  
Structural Engineers  
201 East Kennedy Blvd., Suite 700  
Tampa, FL 33602

**SIEBEIN**  
Sieben Acoustics  
Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607



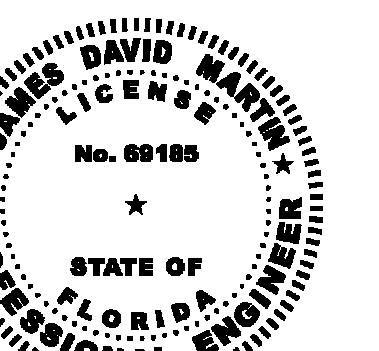
COLUMN ELEVATION NOTES:  
1. ALL MEMBERS SHALL BE Fy = 50 KSI STEEL.  
2. PROVIDE COMPRESSION SPLICES FOR ALL COLUMN UNLESS NOTED OTHERWISE.  
REFER TO 5 IS401.  
3. REFER TO 5 IS401 AND PLANS FOR TOP ELEVATION OF COLUMNS.  
4. REFER TO 5 IS401 FOR BASE PLATE SCHEDULE INCLUDING ANCHOR ROD SIZES,  
EMBEDMENT, AND TYPES.  
5. ALL PERIMETER AND INTERIOR COLUMN SPLICES ARE TO BE 48" (MINIMUM) ABOVE  
FLOOR FLOORS.  
6. ALL STEEL COLUMNS ON GRID C AND E ARE TO HAVE 1" THICK CAP PLATE FOR  
PARAPET SUPPORT STEEL ATTACHMENT. WELD CAP PLATE TO COLUMN USING 3/8"  
THICK FILLET WELD ON EACH FLANGE AND BOTH SIDES OF WEB.

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Project No. 22.29005.00  
Drawn By BD  
Checked By  
Date 09/08/2023

Revisions:  
B PERMIT DOCUMENTS 10/09/2023



James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22045-09  
Certificate of Authorization No. 663 of  
To the best of the Engineer's knowledge, the plans and  
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CONFORMED SET  
02/14/2024

**S513**  
STEEL COLUMN  
FRAME ELEVATIONS

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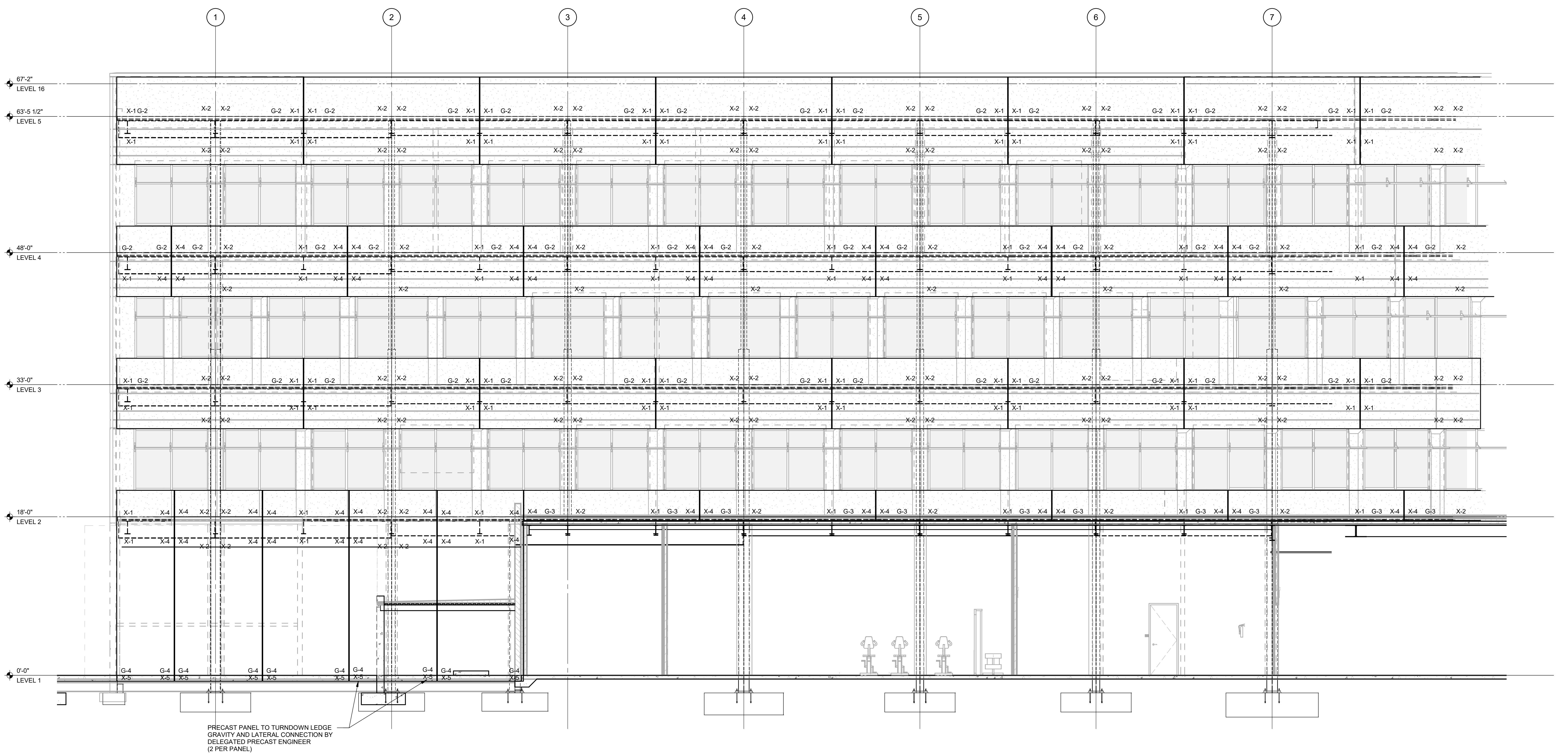
**Stantec** Civil & Landscape  
Sarasota, FL 34240

**Walter P. Moore** Structural Engineers  
201 East Kennedy Blvd., Suite 700  
Tampa, FL 33602

**SIEBEIN ACOUSTIC** Sieben Acoustics  
Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607

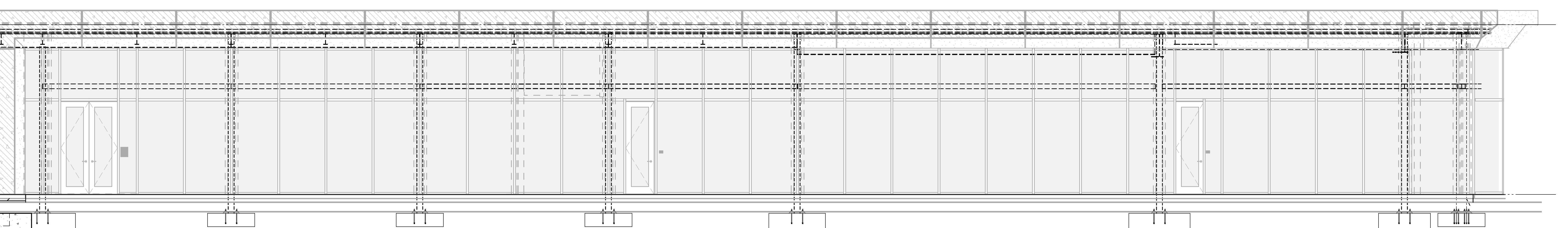
**Sarasota County  
Administration Center**

1 Apex Road  
Sarasota, Florida 34240



**1** BUILDING ELEVATION

3/16" = 1'-0"



**2** BUILDING ELEVATION

3/16" = 1'-0"

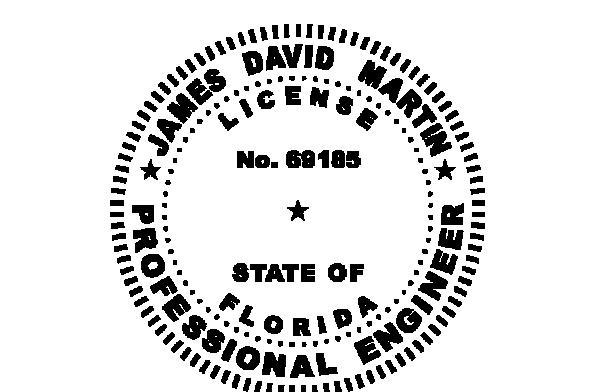
NOTES:  
XX = PRECAST LATERAL CONNECTION TO STRUCTURE BY DELEGATED PRECAST ENGINEER  
GX = PRECAST GRAVITY CONNECTION TO STRUCTURE BY DELEGATED PRECAST ENGINEER

PRECAST CONCRETE SUBCONTRACTOR, STEEL SUBCONTRACTOR, AND STEEL DETAILER SHALL COORDINATE LOCATIONS OF ALL PRECAST GRAVITY AND LATERAL CONNECTIONS WITH PRECAST CONCRETE SHOP DRAWINGS.

PRECAST CONCRETE SUBCONTRACTOR SHALL PROVIDE SHOP DRAWINGS TO STEEL FABRICATOR AND STEEL DETAILER IN A TIMELY MANNER SO THAT ALL PRECAST PREWELD ASSEMBLIES CAN BE MADE IN THE STEEL FABRICATOR'S SHOP.

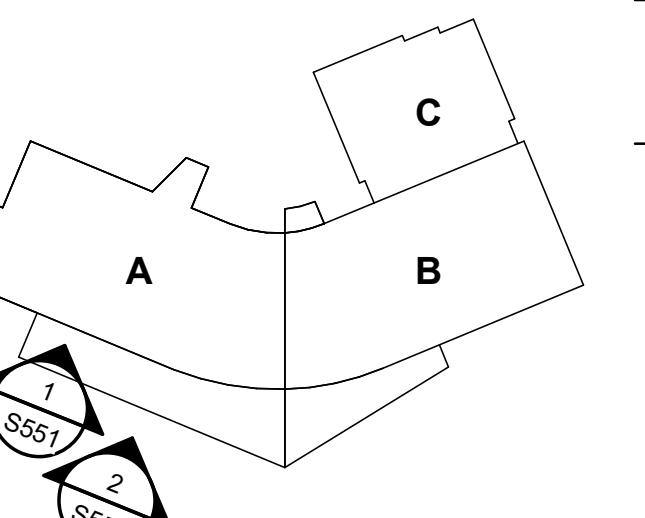
Project No.: 22.29005.00  
Drawn By: BD  
Checked By: JDM  
Date: 09/08/2023

Revisions:



James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-2204-09  
Certificate of Authorization No. 663 of  
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Florida Statutes.

CONFIRMED SET  
02/14/2024



**S551**  
ARCHITECTURAL  
PRECAST - SOUTH  
ELEVATION AREA A

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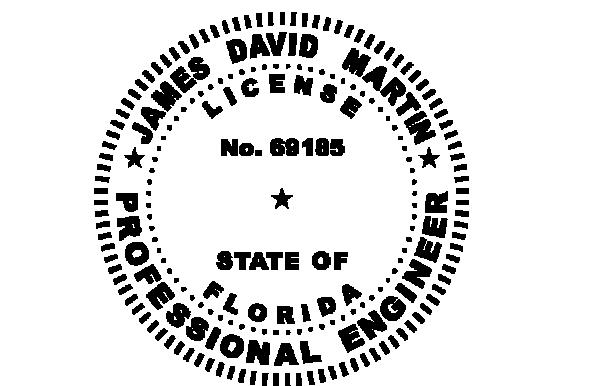


**Sarasota County Administration Center**

1 Apex Road  
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Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
A Pre-GMP ASI #1 09/27/2023

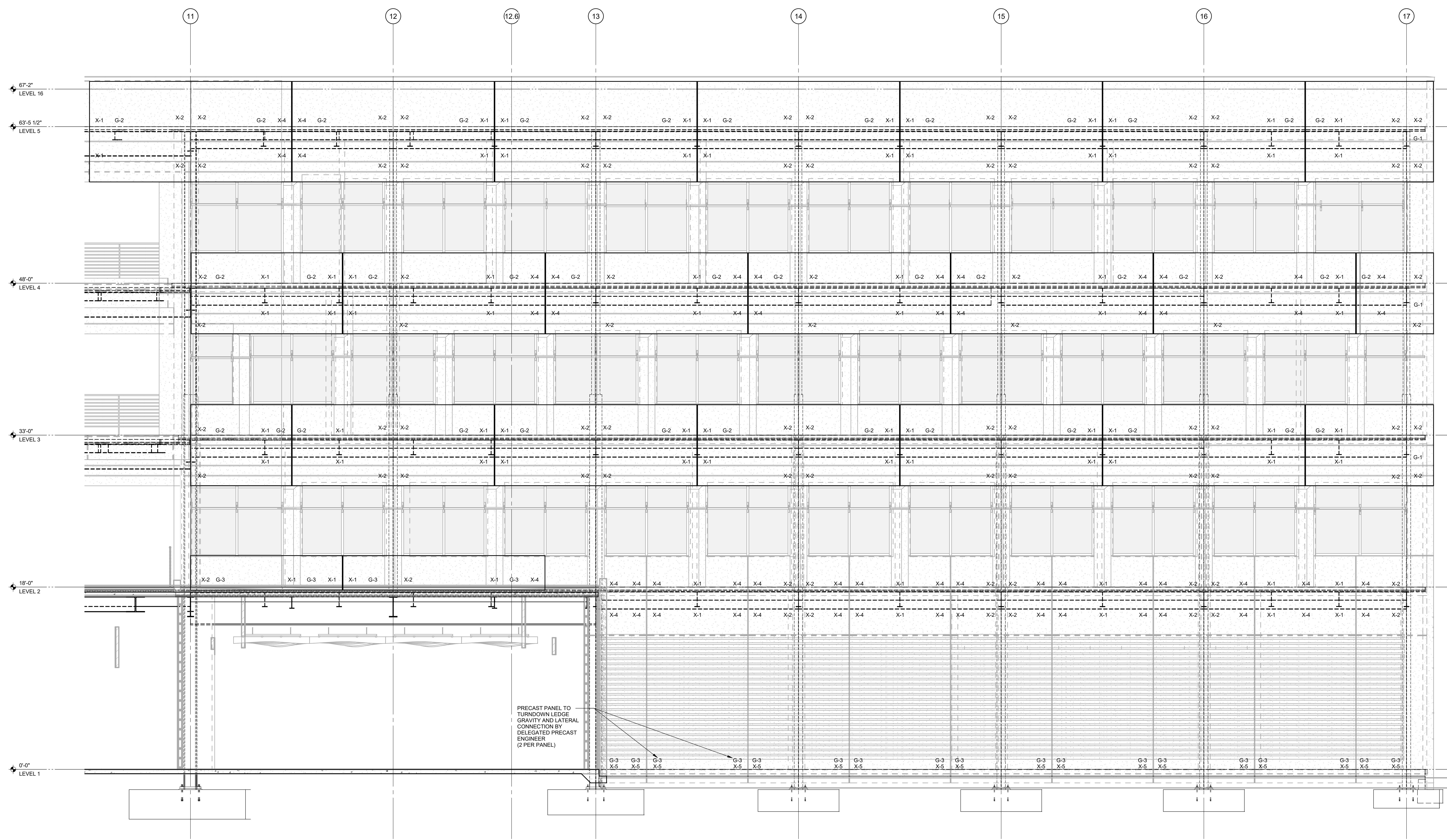


James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-2204-00  
Certificate of Authorization No. 663 of

To the best of the Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFIRMED SET  
02/14/2024

**S552**  
ARCHITECTURAL  
PRECAST - SOUTH  
ELEVATION AREA B



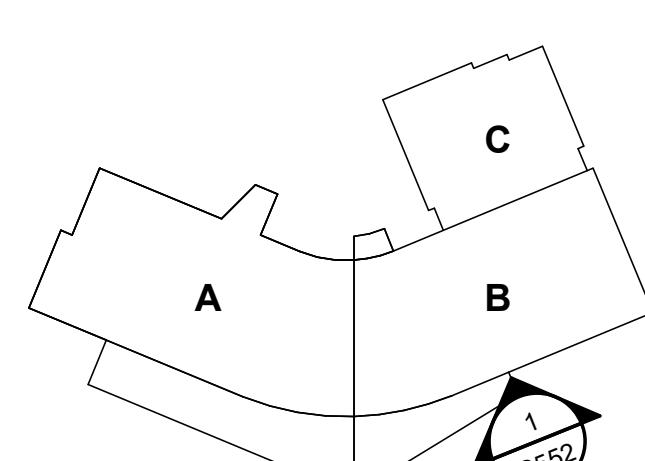
**BUILDING ELEVATION**

1

NOTES:  
X-X = PRECAST LATERAL CONNECTION TO STRUCTURE BY DELEGATED PRECAST ENGINEER  
G-X = PRECAST GRAVITY CONNECTION TO STRUCTURE BY DELEGATED PRECAST ENGINEER

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Walter P. Moore  
Structural Engineers  
201 East Kennedy Blvd., Suite 700  
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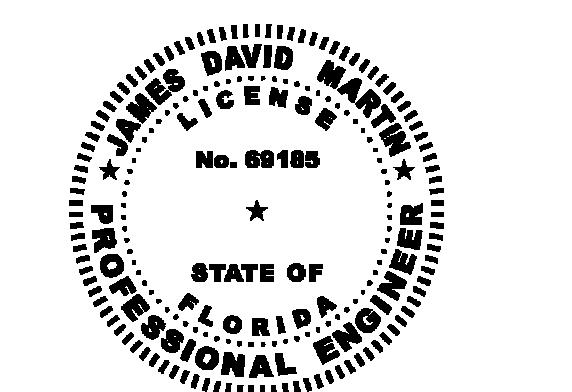
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Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607

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Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

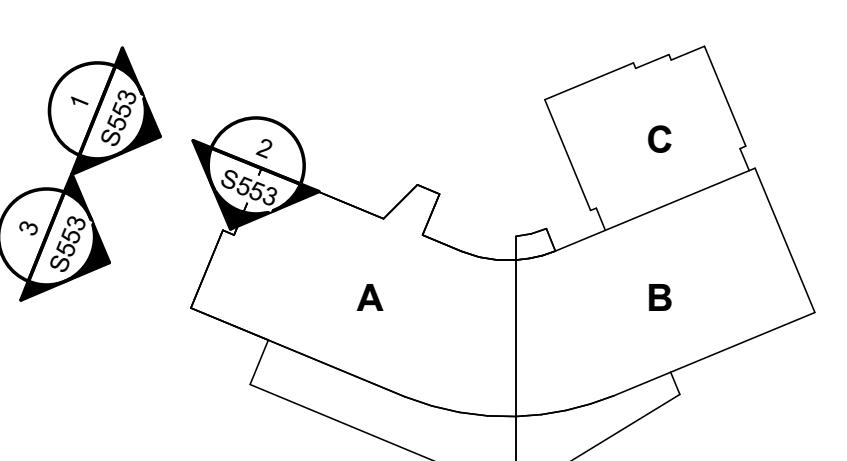
Revisions:



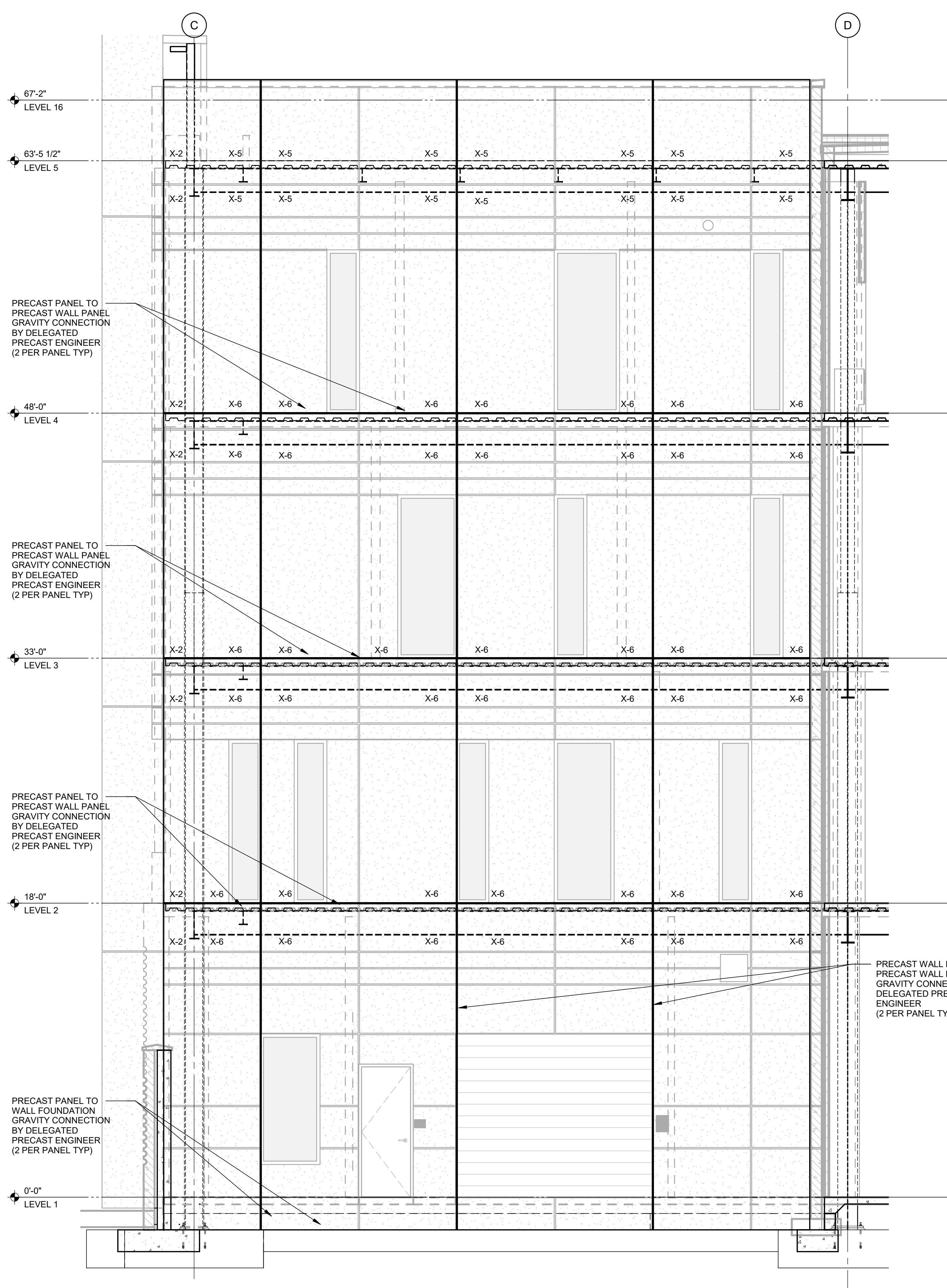
James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22040-00  
Certificate of Authorization No. 663 of  
To the best of the Engineer's knowledge, the plans and  
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CONFORMED SET  
02/14/2024

**S553**  
ARCHITECTURAL  
PRECAST - WEST  
ELEVATION AREA A



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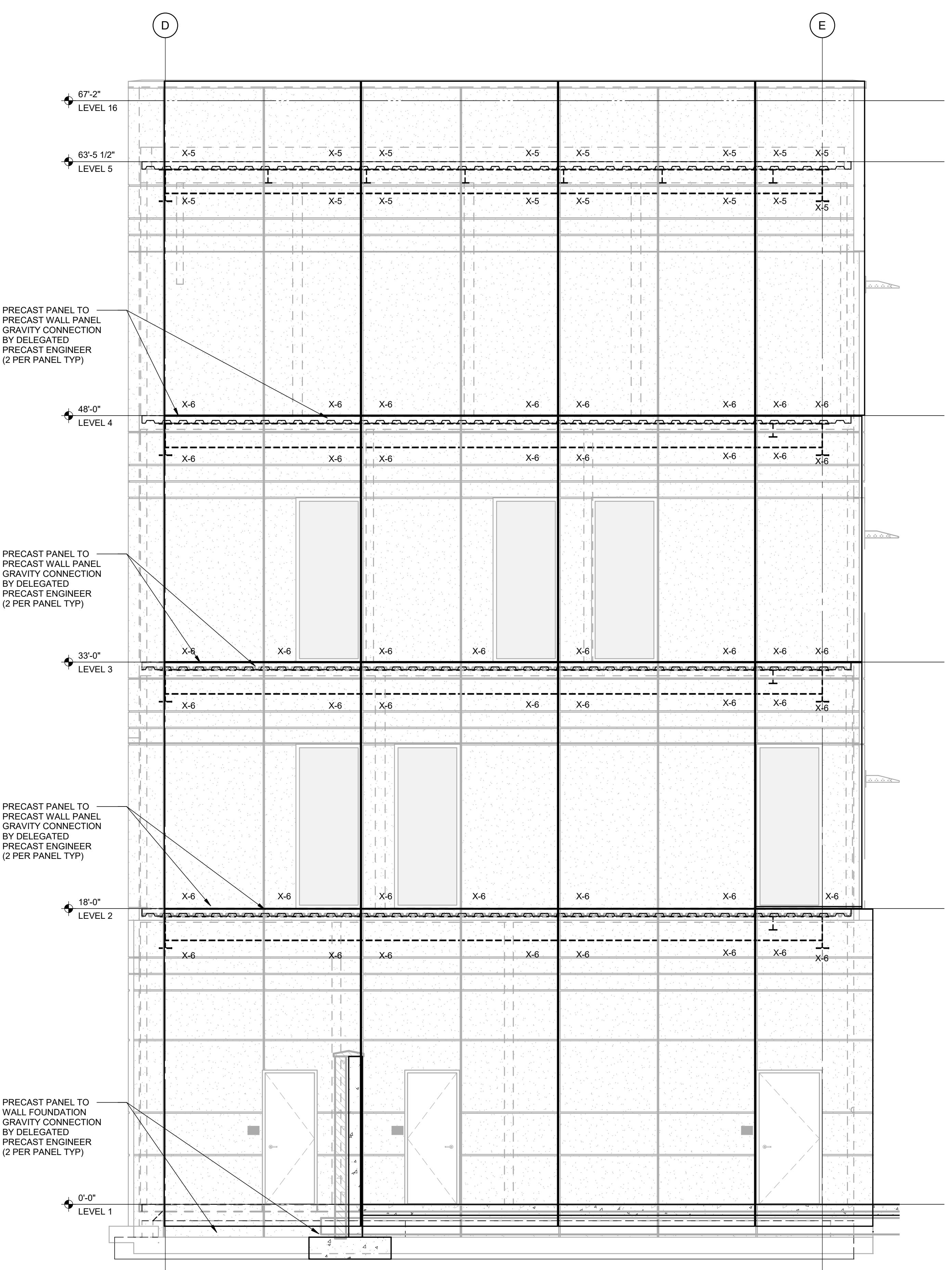
1 BUILDING ELEVATION  
1/4" = 1'-0"

NOTES:  
X-X = PRECAST LATERAL CONNECTION TO STRUCTURE BY DELEGATED PRECAST ENGINEER  
G-X = PRECAST GRAVITY CONNECTION TO STRUCTURE BY DELEGATED PRECAST ENGINEER

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PRECAST CONCRETE SUBCONTRACTOR SHALL PROVIDE SHOP DRAWINGS TO STEEL  
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ASSEMBLIES CAN BE MADE IN THE STEEL FABRICATOR'S SHOP.

2 BUILDING ELEVATION  
1/4" = 1'-0"



3 BUILDING ELEVATION  
1/4" = 1'-0"



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Walter P. Moore  
Structural Engineers  
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Tampa, FL 33602

**SIEBEIN ACOUSTIC**  
Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607

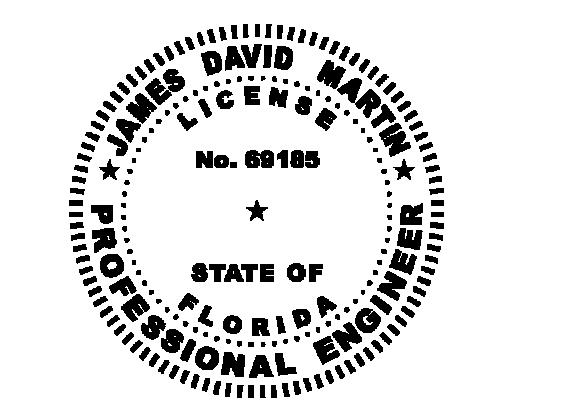
PRECAST PANEL TO  
PRECAST WALL PANEL  
GRAVITY CONNECTION  
BY DELEGATED  
PRECAST ENGINEER

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1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:

A Pre-GMP ASI #1 09/27/2023



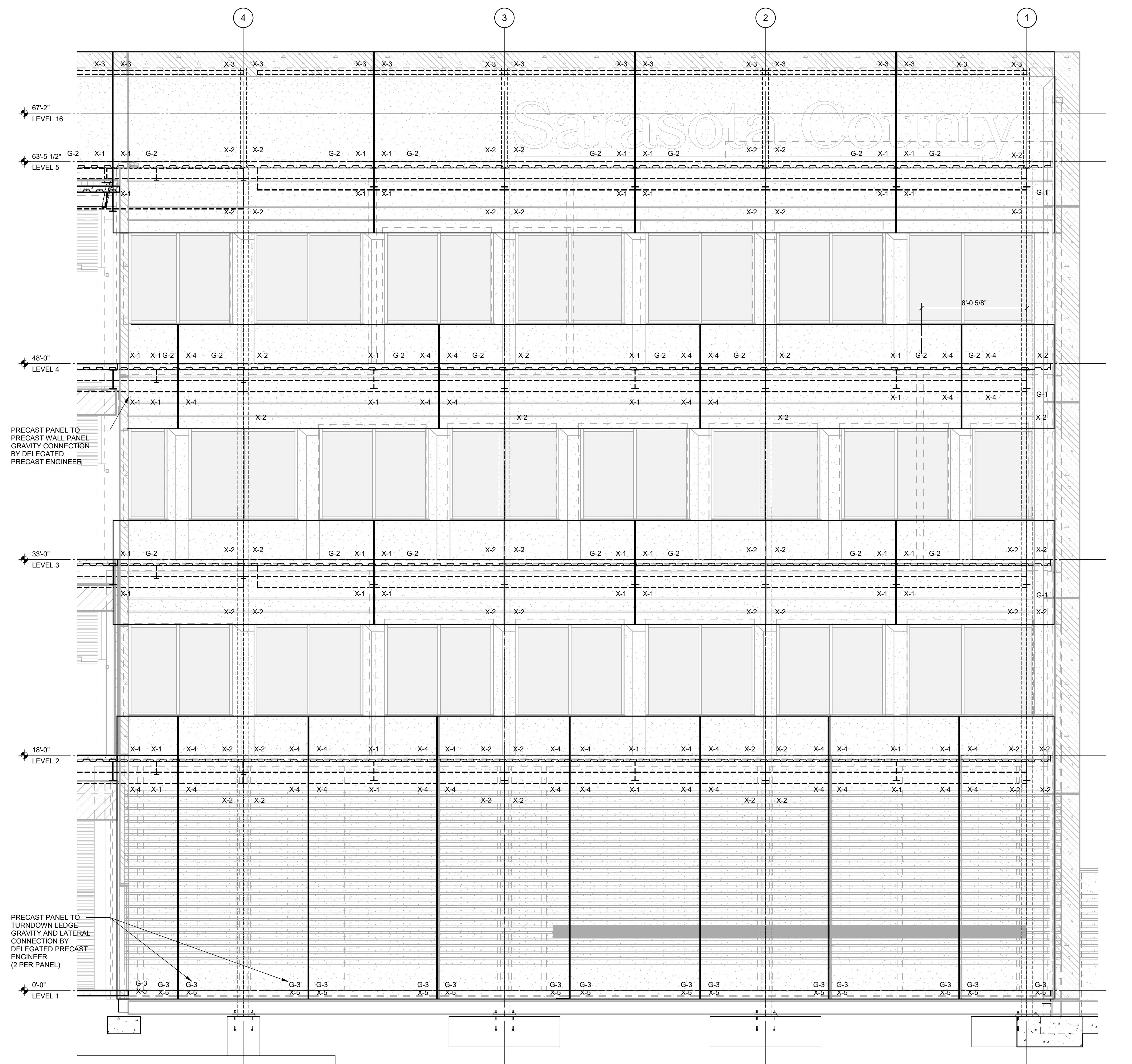
James David Martin, P.E. FL P.E. No. 69185  
WPM Project No. S05-22045-00  
Certificate of Authorization No. 69185

To the best of the Engineer's knowledge, the plans and specifications comply with all applicable laws, including codes and applicable fire safety standards as determined by local authority in accordance with Chapter 653 and 653 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S554**  
ARCHITECTURAL  
PRECAST - NORTH  
ELEVATION AREA A

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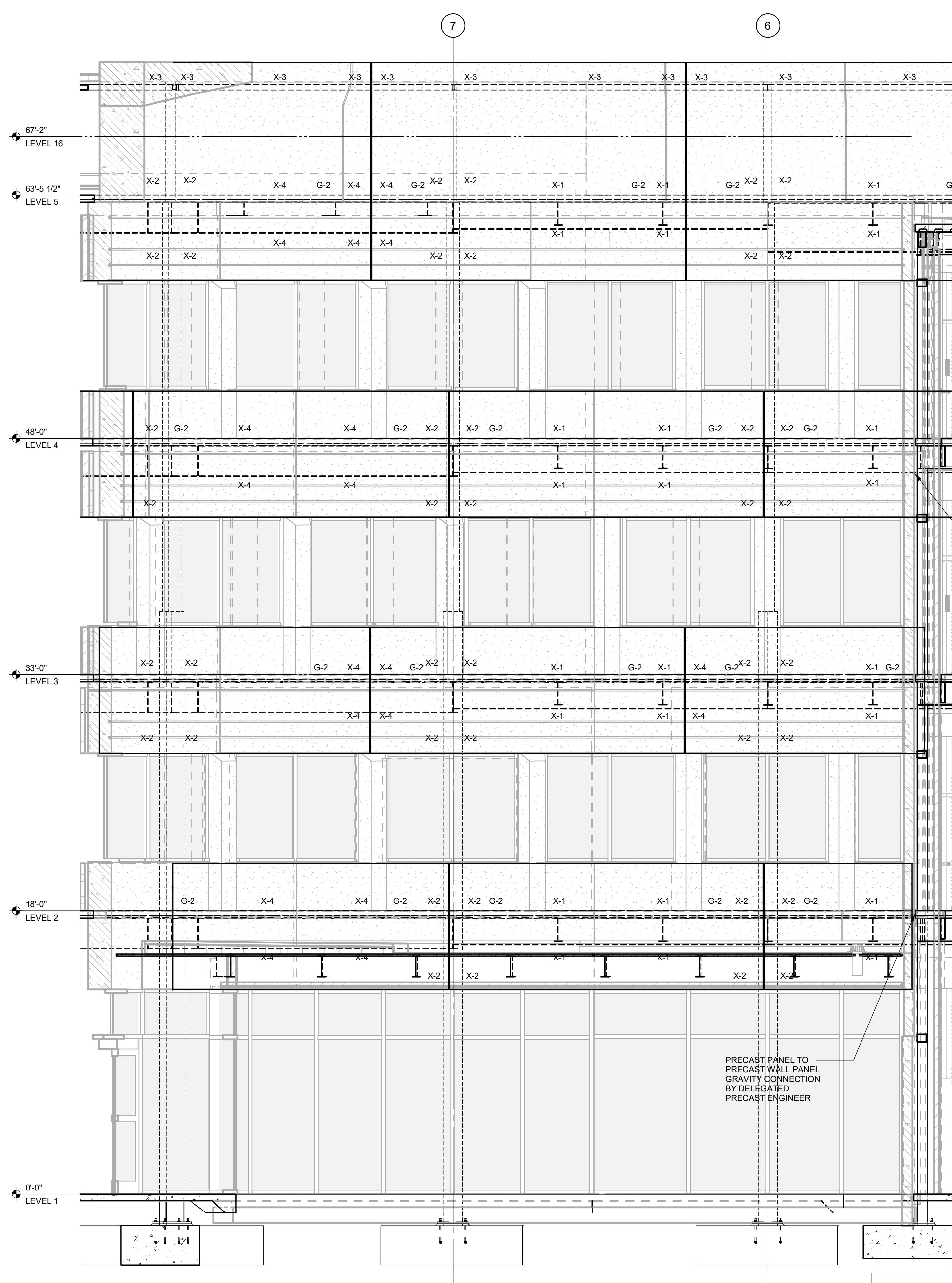
1 BUILDING ELEVATION

1/4" = 1'-0"

NOTES:  
X-X = PRECAST LATERAL CONNECTION TO STRUCTURE BY DELEGATED PRECAST ENGINEER  
G-X = PRECAST GRAVITY CONNECTION TO STRUCTURE BY DELEGATED PRECAST ENGINEER

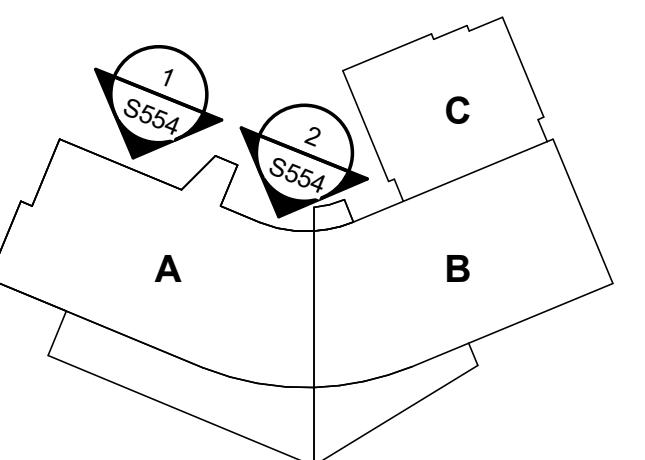
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2 BUILDING ELEVATION

1/4" = 1'-0"



CONFORMED SET  
02/14/2024

**S554**  
ARCHITECTURAL  
PRECAST - NORTH  
ELEVATION AREA A

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**walter p moore**

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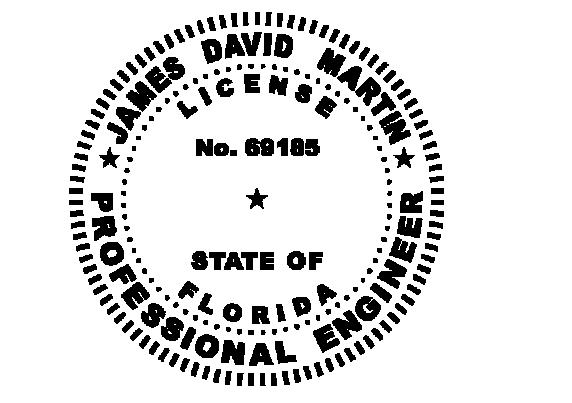
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Acoustics  
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Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:  
A Pre-GMP ASI #1 09/27/2023

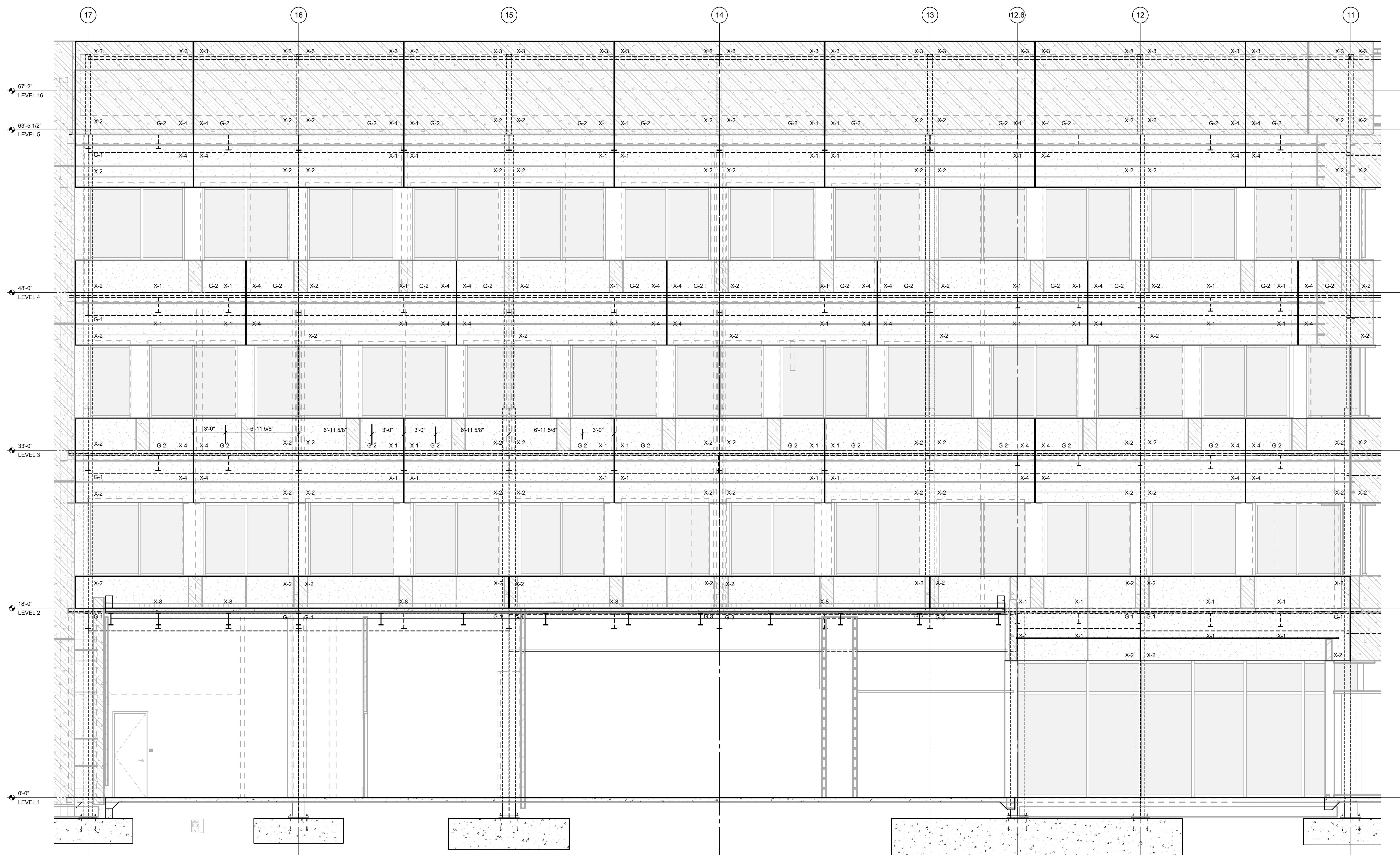


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WPM Project No. S05-22045-00  
Certificate of Authorization No. 663 of  
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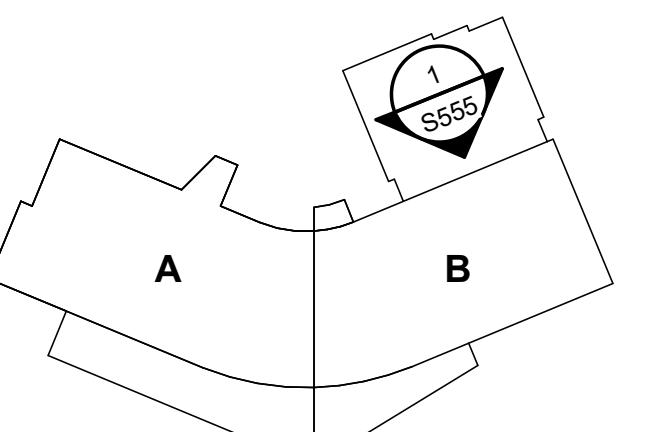
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02/14/2024

**S555**  
ARCHITECTURAL  
PRECAST - NORTH  
ELEVATION AREA B

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**1** BUILDING ELEVATION  
1/4" = 1'-0"





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**Walter P. Moore** Structural Engineers  
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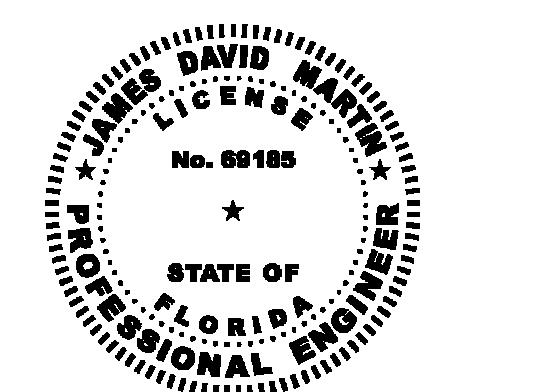
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Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

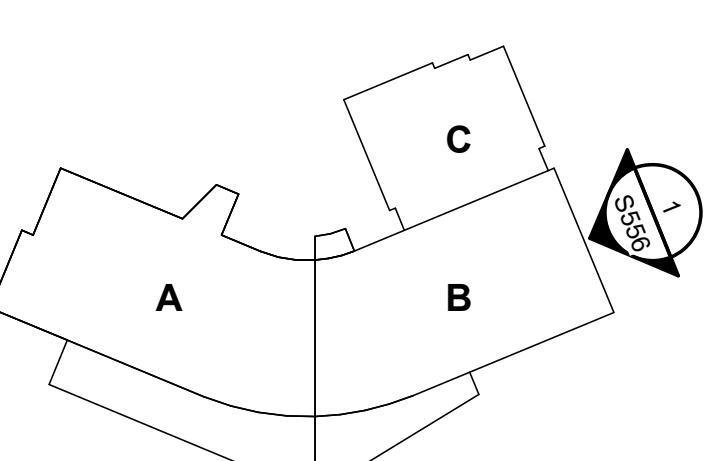
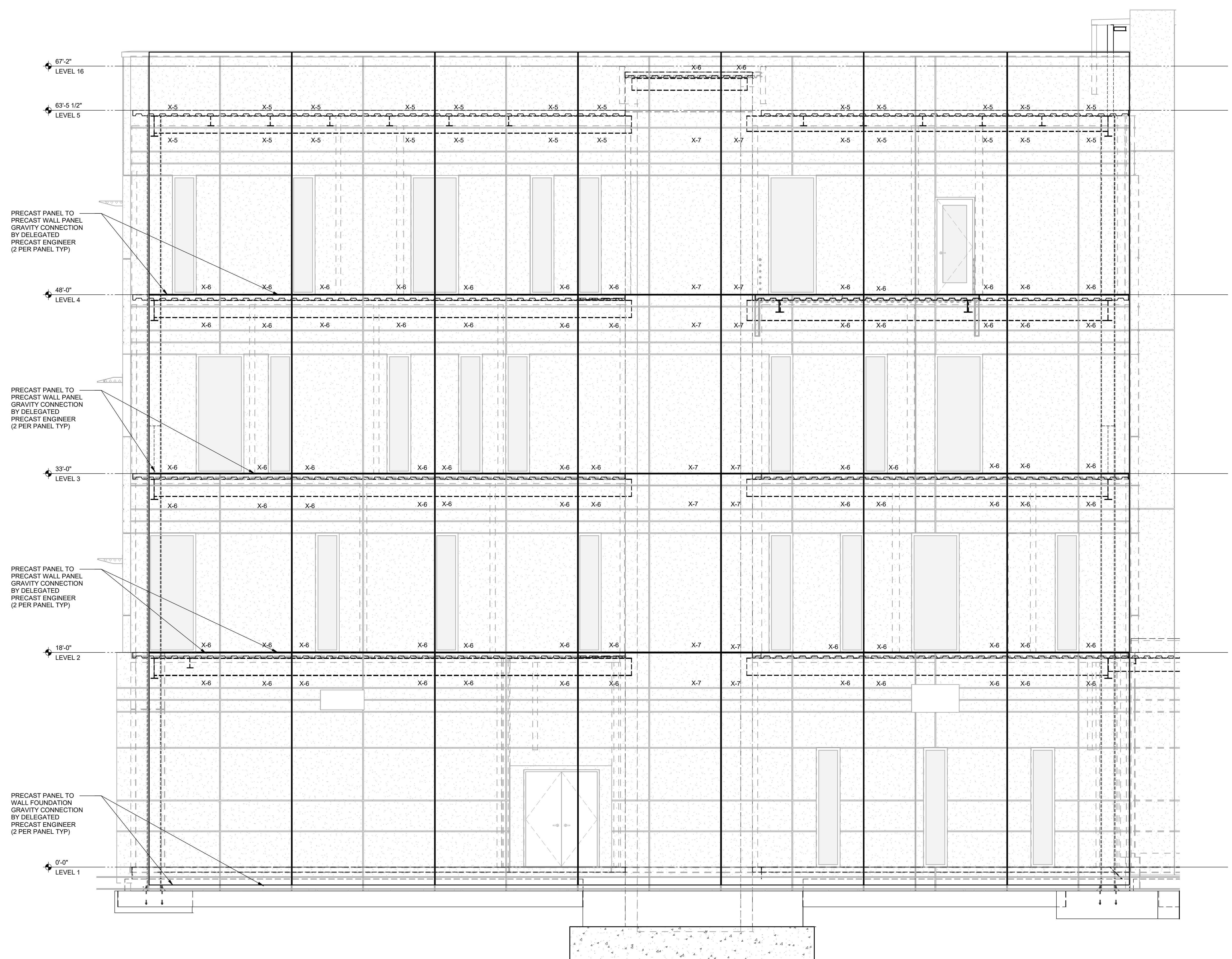
Revisions:



James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22040-00  
Certificate of Authorization No. 663 of  
To the best of the Engineer's knowledge, the plans and  
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codes and applicable fire safety standards as determined  
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Florida Statutes.

**CONFORMED SET**  
02/14/2024

**S556**  
ARCHITECTURAL  
PRECAST - EAST  
ELEVATION AREA B





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Sarasota, FL 34240

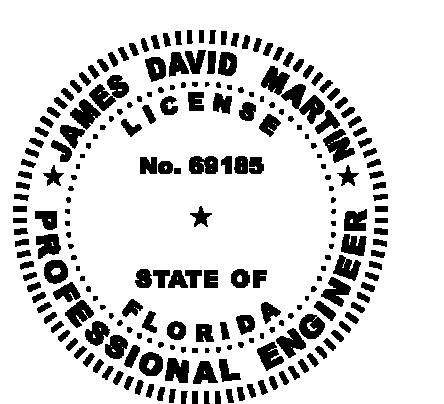
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Structural Engineers  
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**SIEBEIN**  
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Architectural Acoustics  
Sieben Acoustics  
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Gainesville, FL 32607

**Sarasota County  
Administration Center**

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Sarasota, Florida 34240

RFI 295  
Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

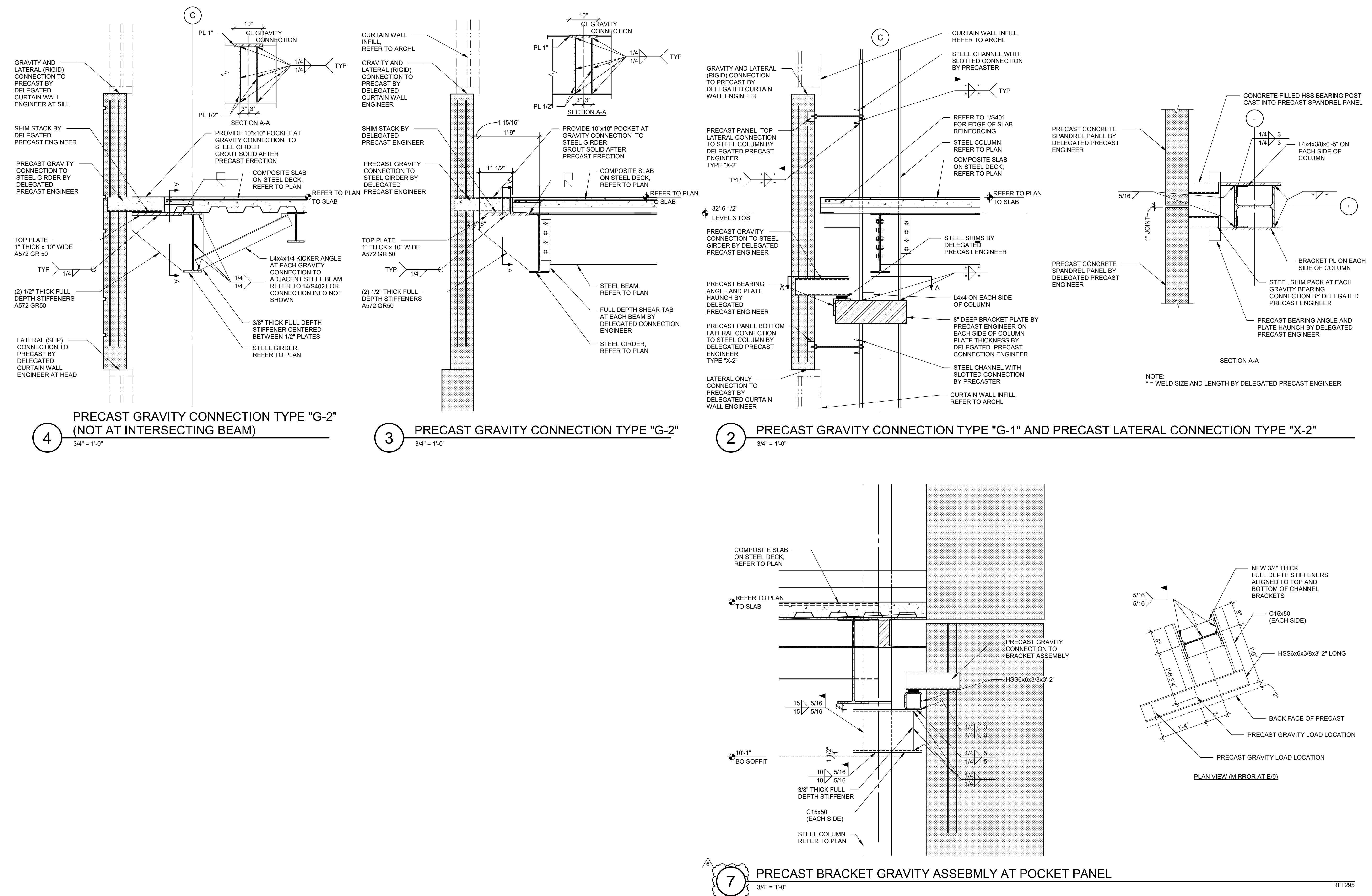


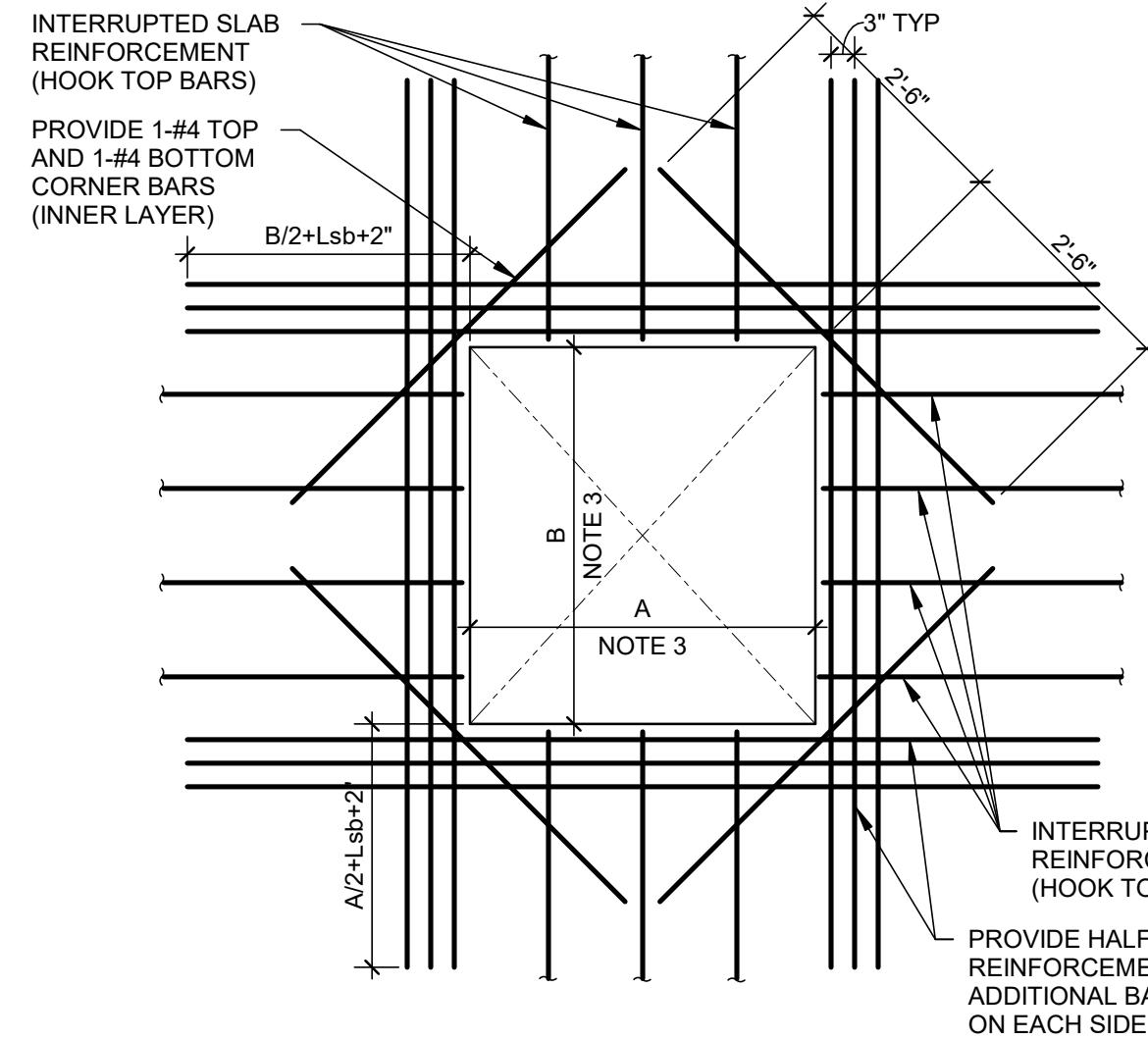
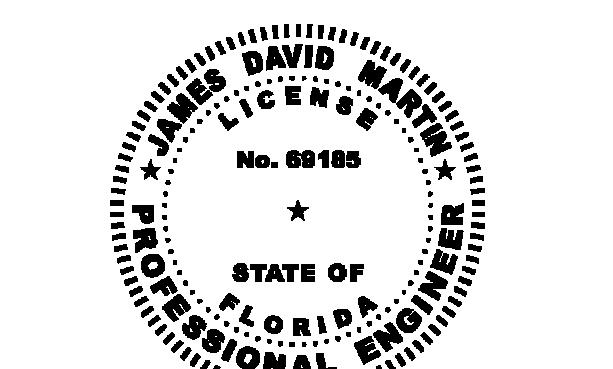
James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22045-00  
Certificate of Authorization No. 663 of  
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CONFORMED SET  
02/14/2024

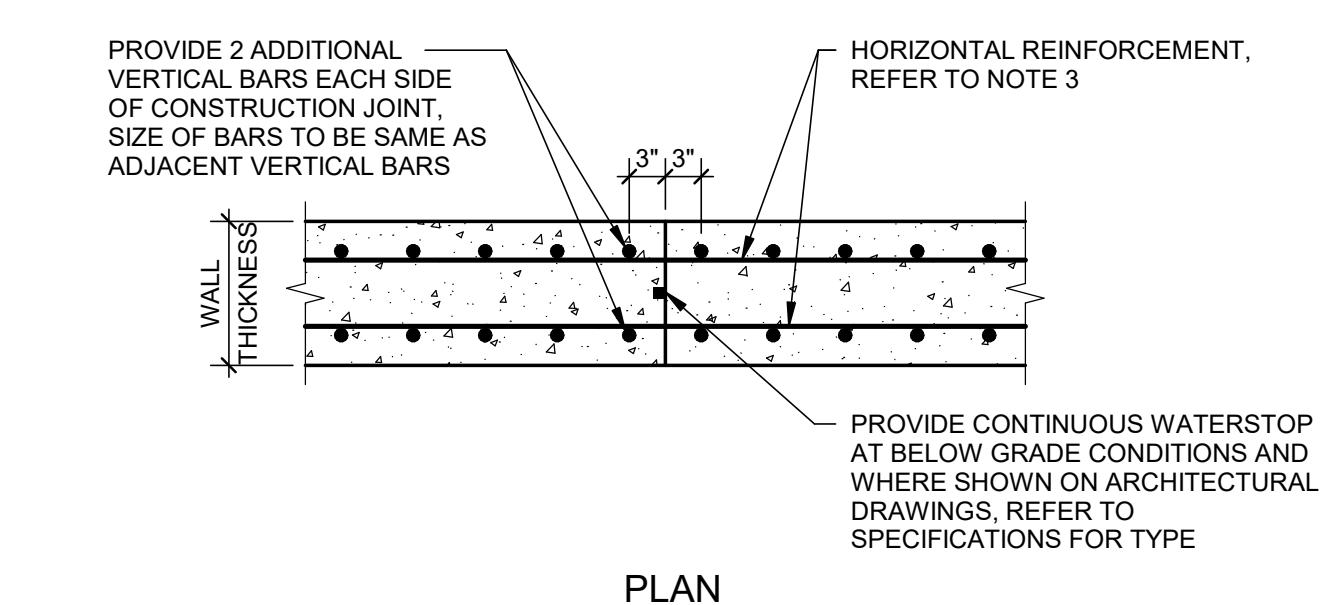
**S557**  
ARCHITECTURAL  
PRECAST -  
FRAMING DETAILS

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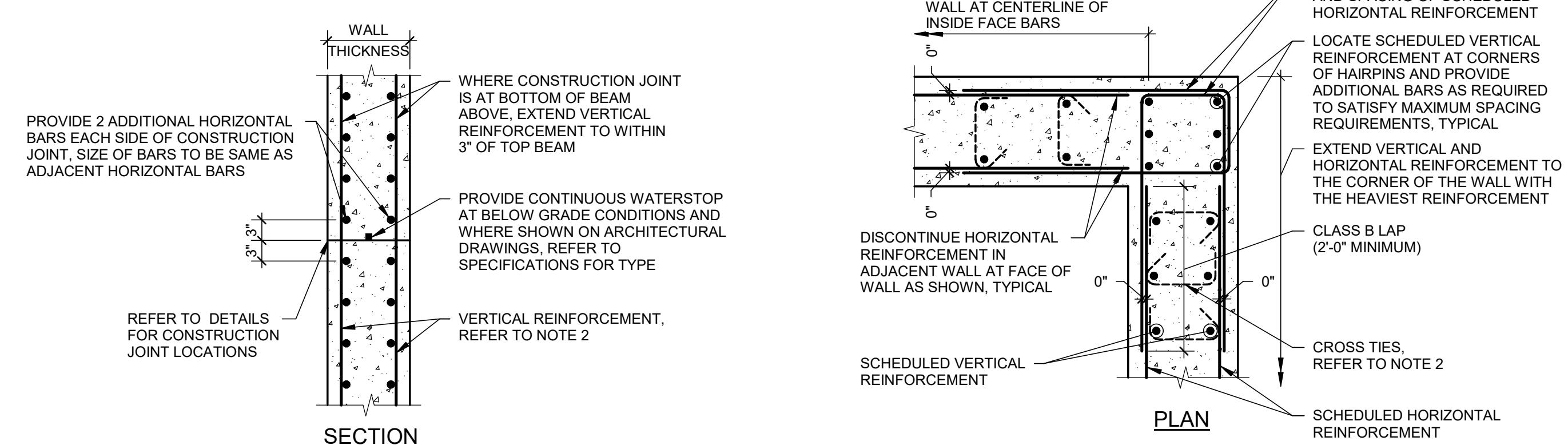




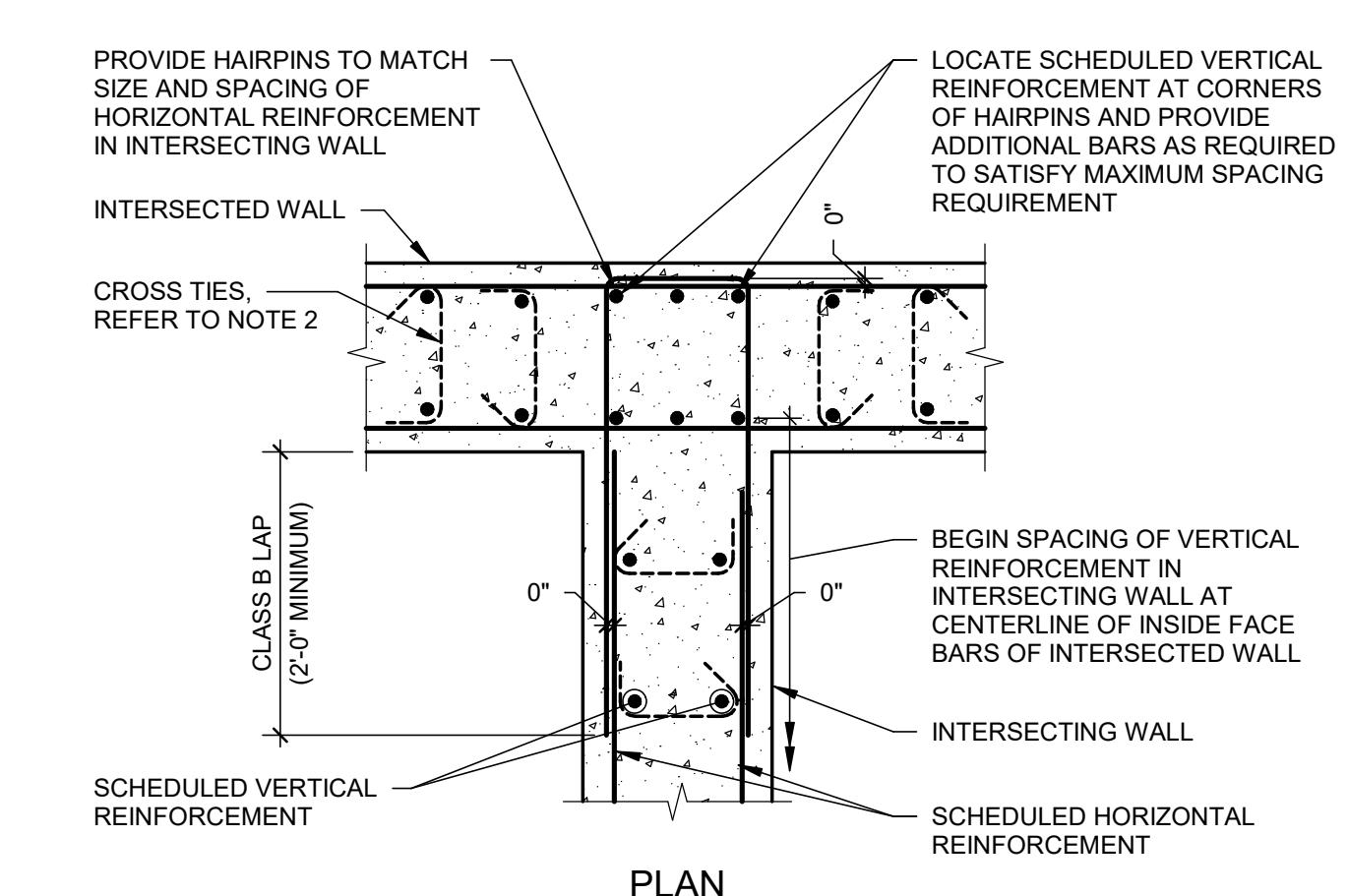
5 TYPICAL OPENING  
IN MILD REINFORCED CONCRETE SLAB  
NO SCALE



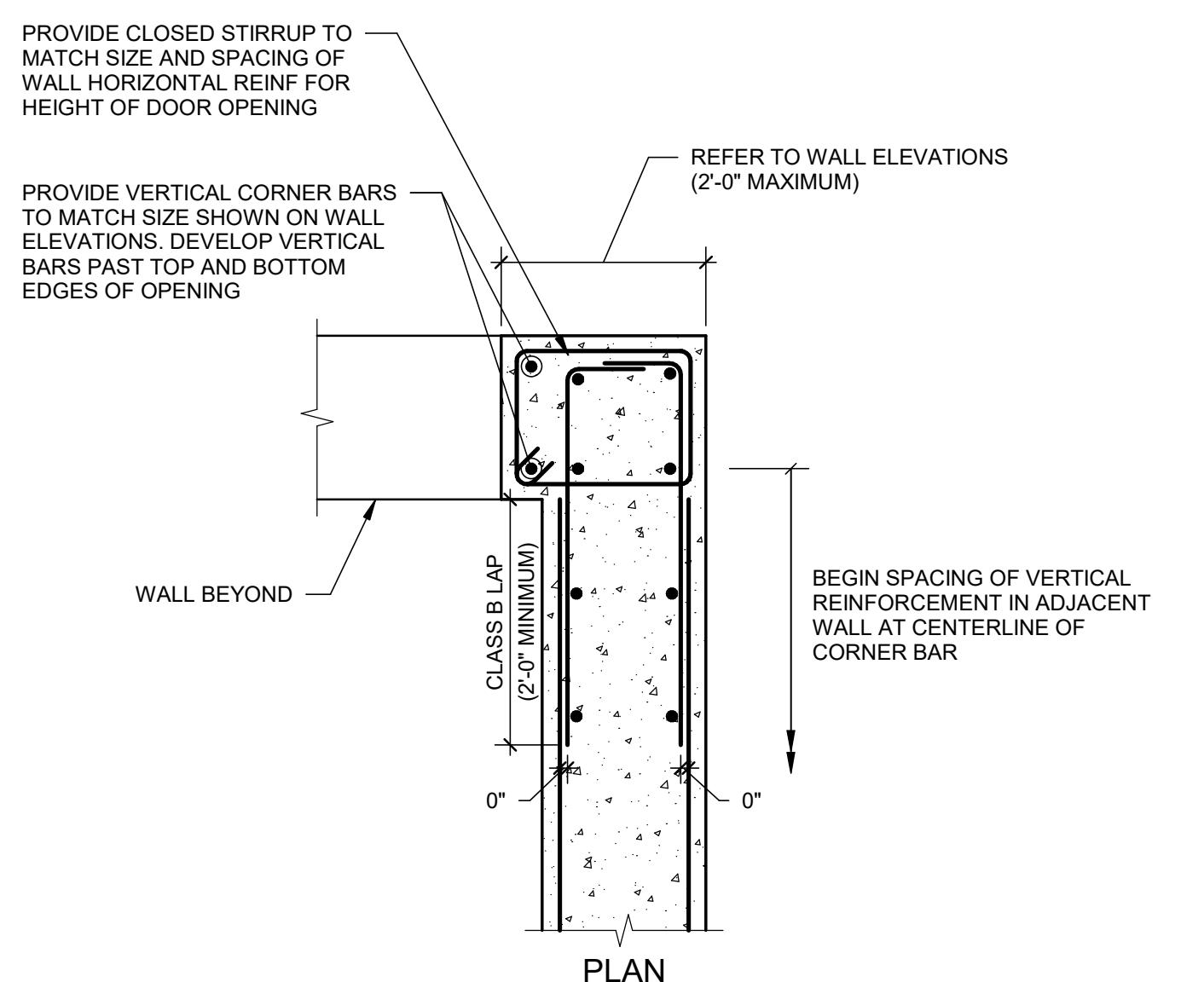
4 TYPICAL VERTICAL CONSTRUCTION JOINT AT  
CONCRETE WALL (VISUALLY NON-CRITICAL)  
NO SCALE



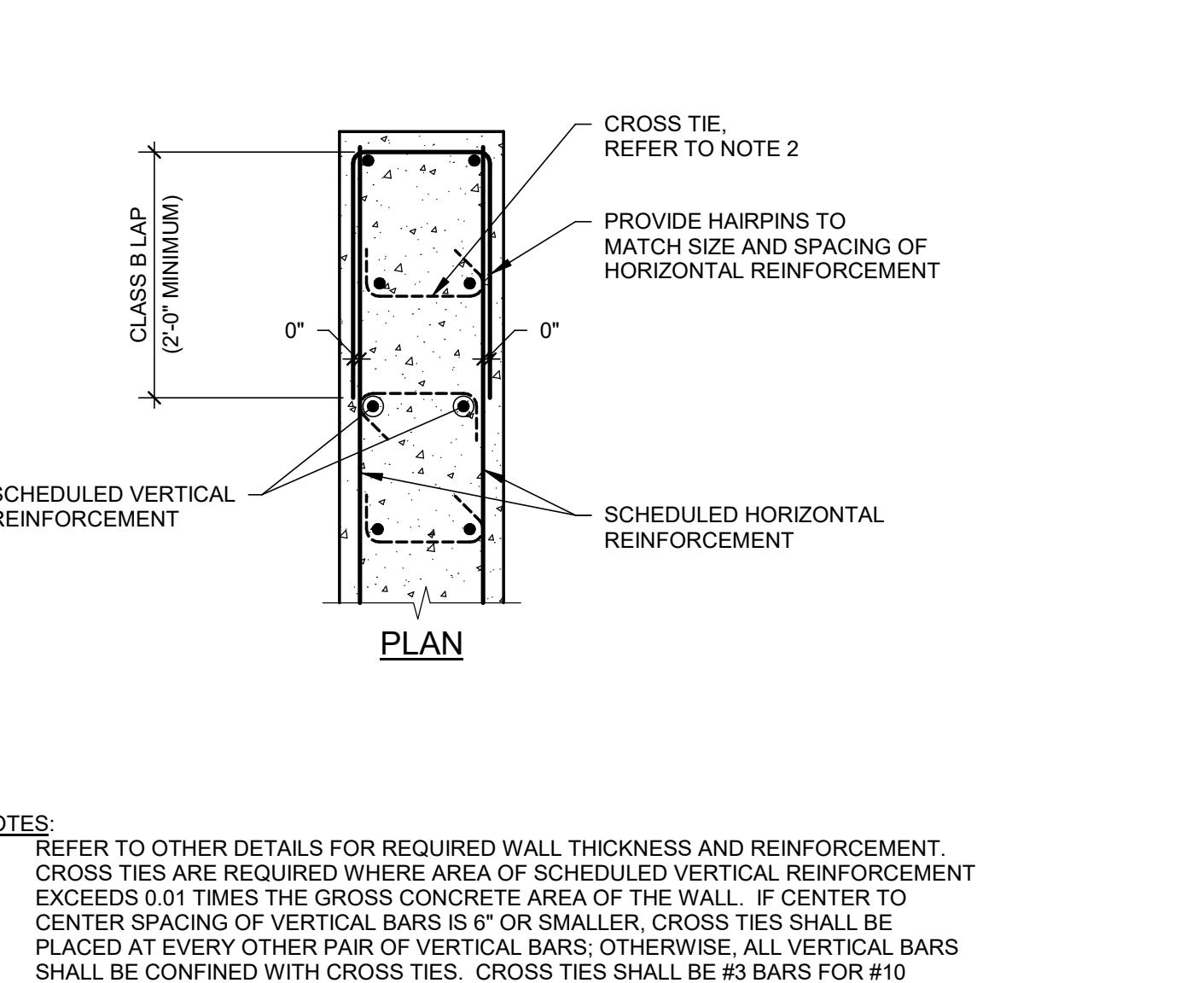
3 TYPICAL HORIZONTAL CONSTRUCTION JOINT  
AT CONCRETE WALL  
NO SCALE



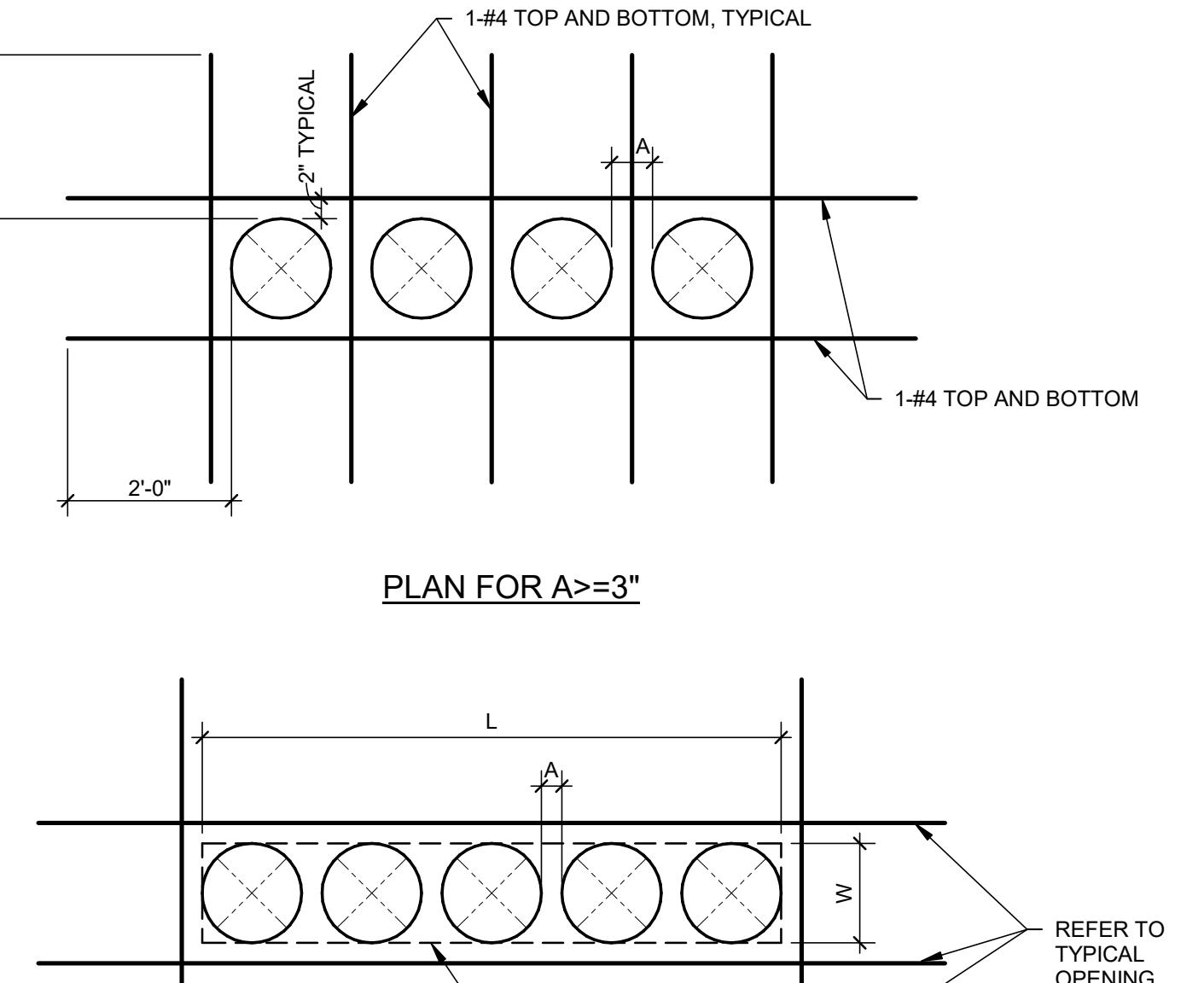
2 TYPICAL REINFORCEMENT SPLICE  
AT CONCRETE SHEAR WALL CORNER  
NO SCALE



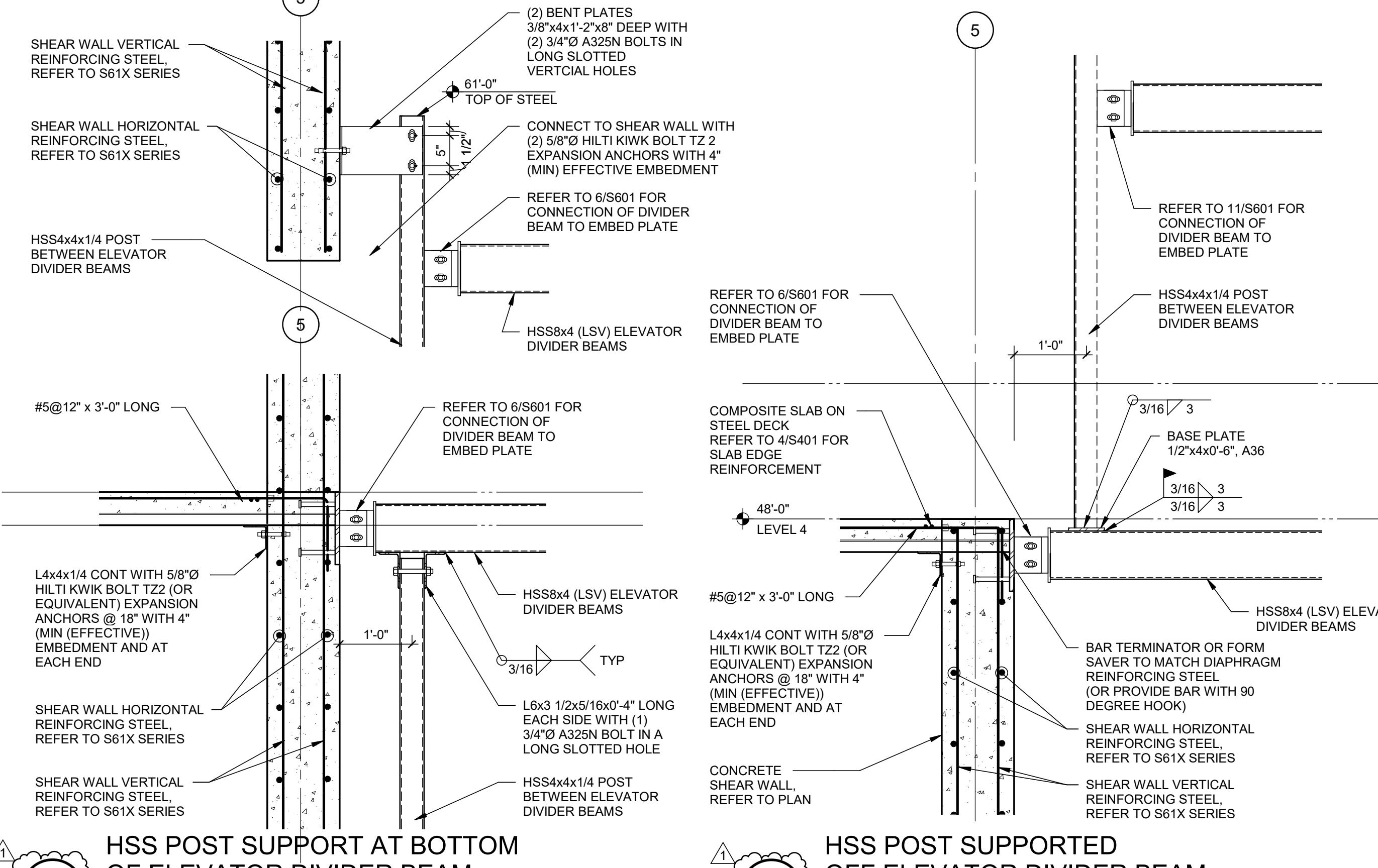
10 TYPICAL WALL RETURN  
REINFORCEMENT AT DOOR OPENINGS  
3/4" = 1'-0"



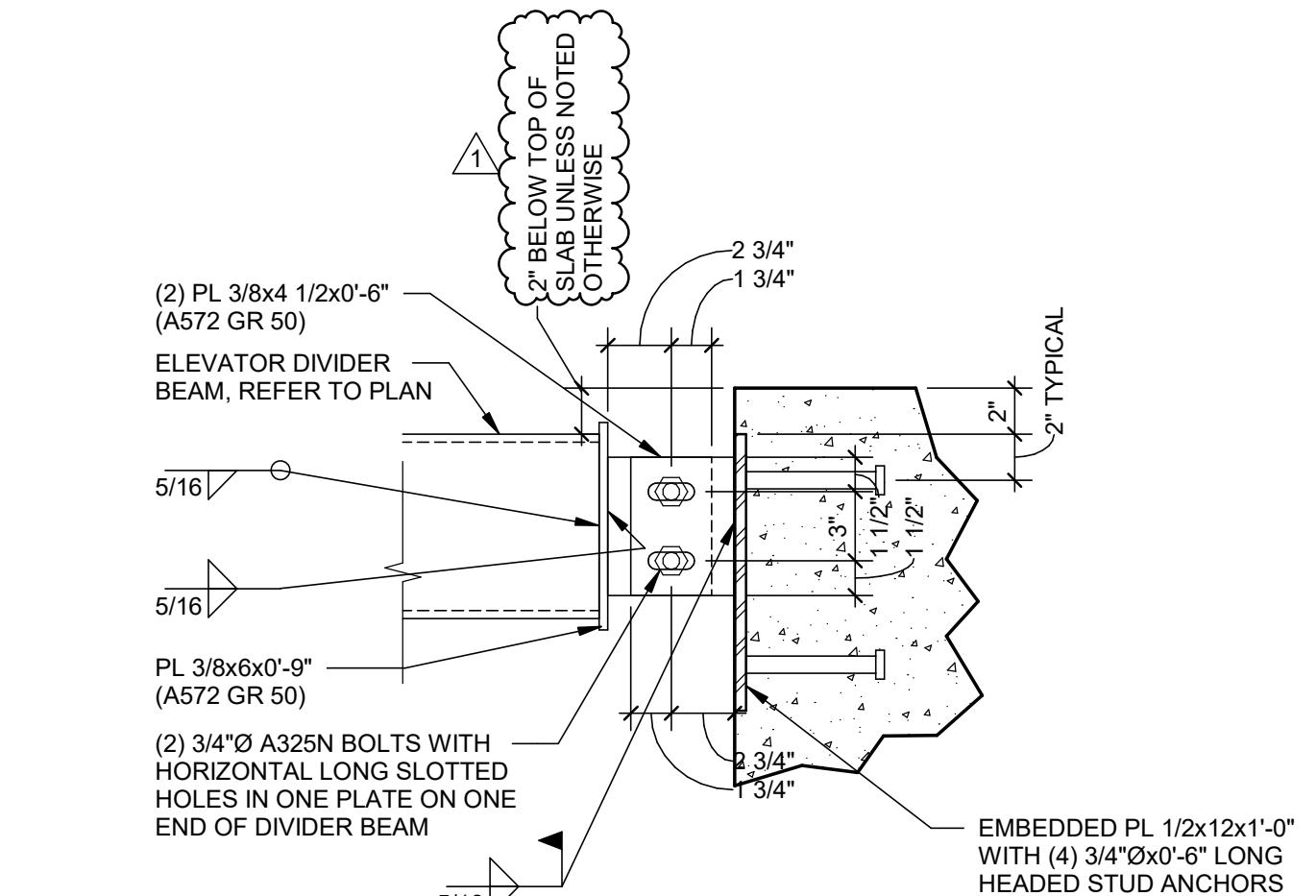
9 TYPICAL REINFORCEMENT SPLICE  
AT CONCRETE SHEAR WALL END  
NO SCALE



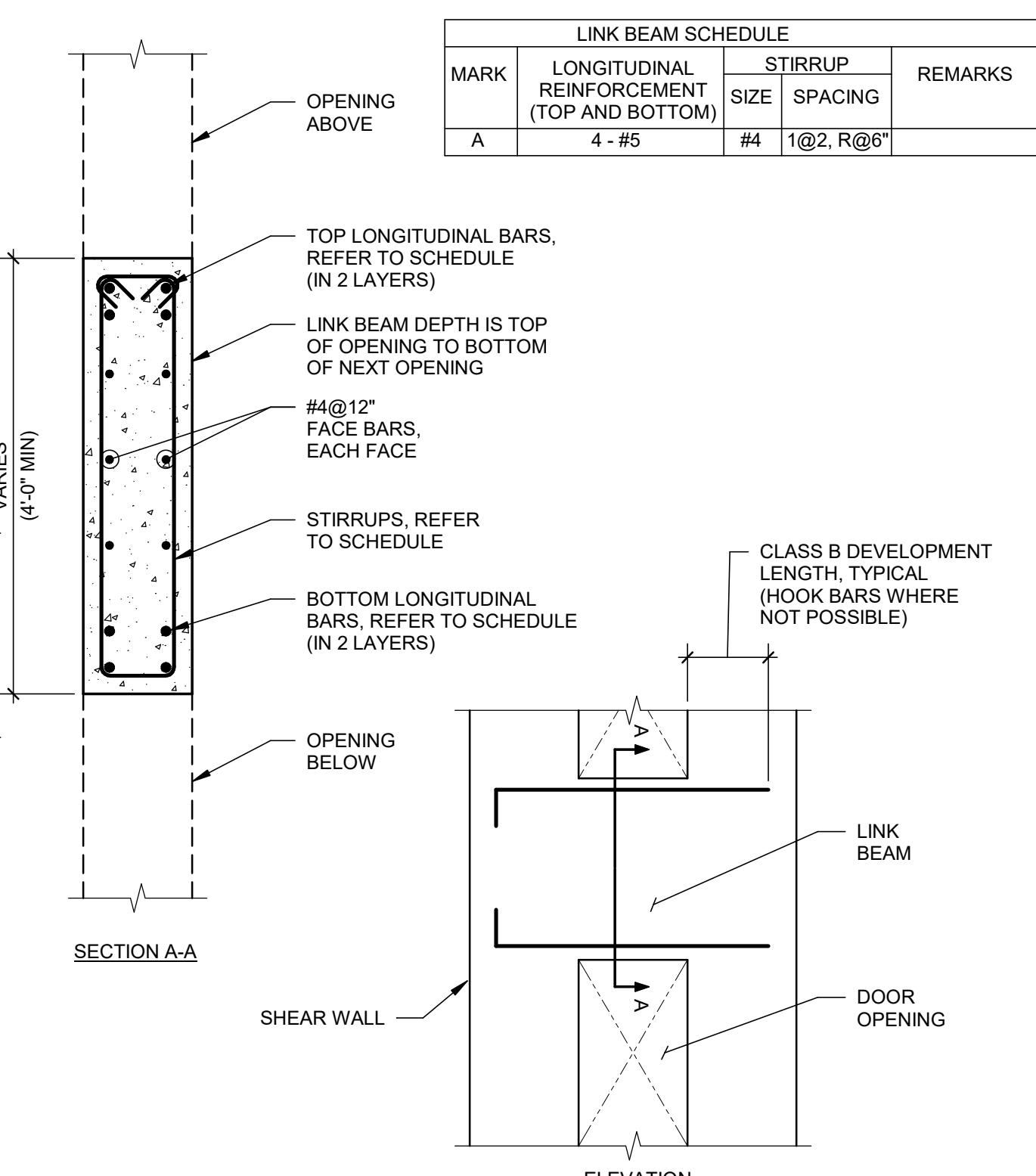
8 ADDITIONAL REINFORCEMENT AROUND PIPE SLEEVES  
NO SCALE



13 HSS POST SUPPORT AT BOTTOM  
OF ELEVATOR DIVIDER BEAM  
3/4" = 1'-0"



6 TYPICAL ELEVATOR DIVIDER BEAM  
CONNECTION TO CONCRETE BEAM OR WALL  
1 1/2" = 1'-0"



11 CONCRETE SHEAR WALL LINK BEAMS  
3/4" = 1'-0"



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Sarasota, FL 34240

Walter P. Moore  
Structural Engineers  
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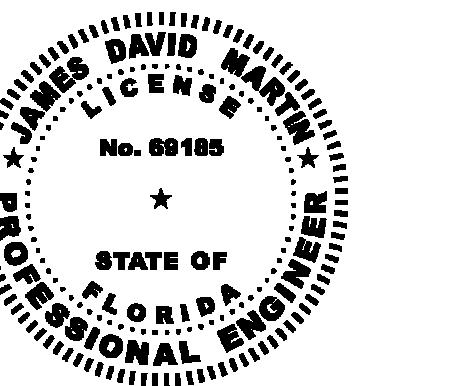
**SIEBEIN** Sieben Acoustics  
Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607

## Sarasota County Administration Center

1 Apex Road  
Sarasota, Florida 34240

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions: 1 ASI-02  
05/17/2024



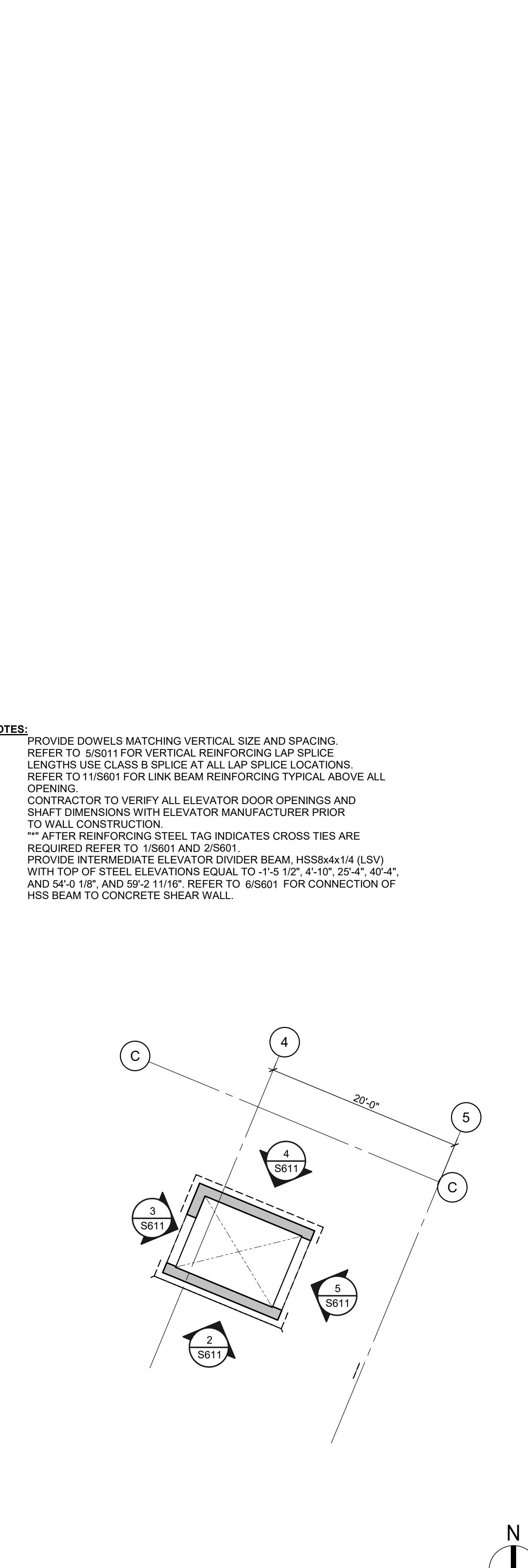
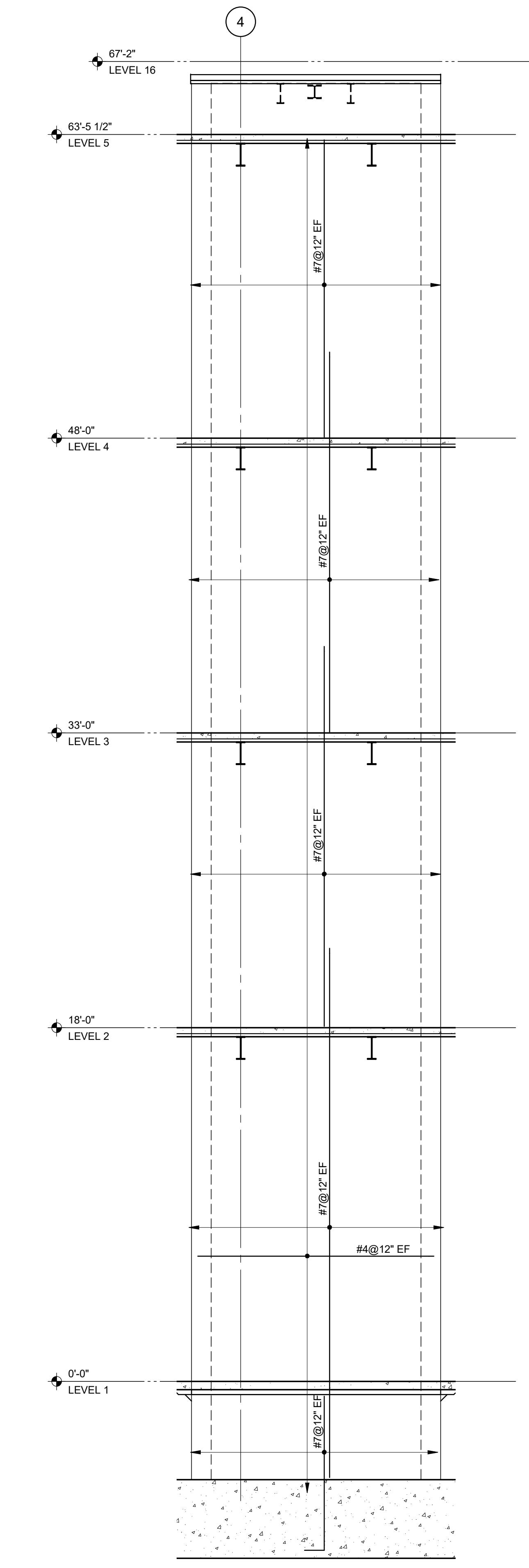
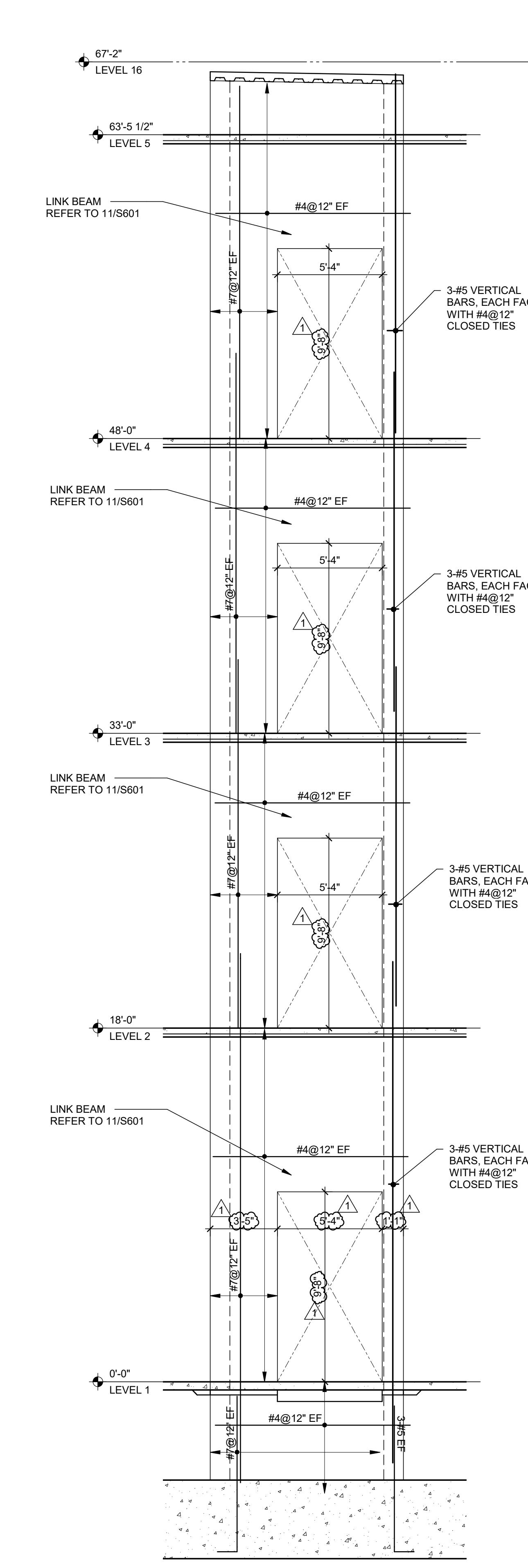
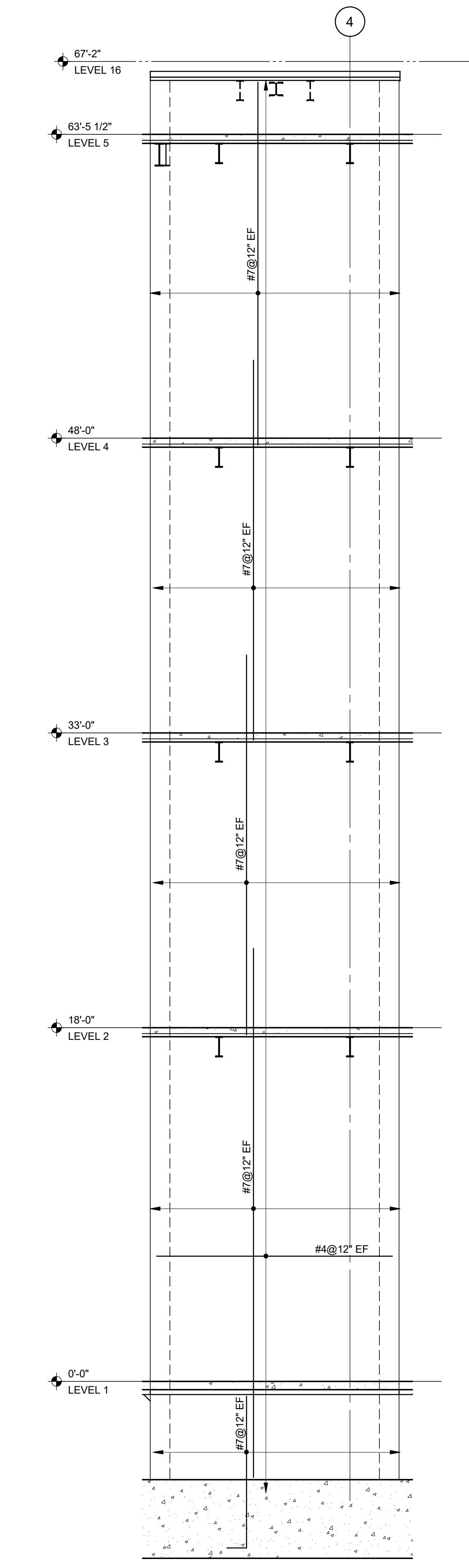
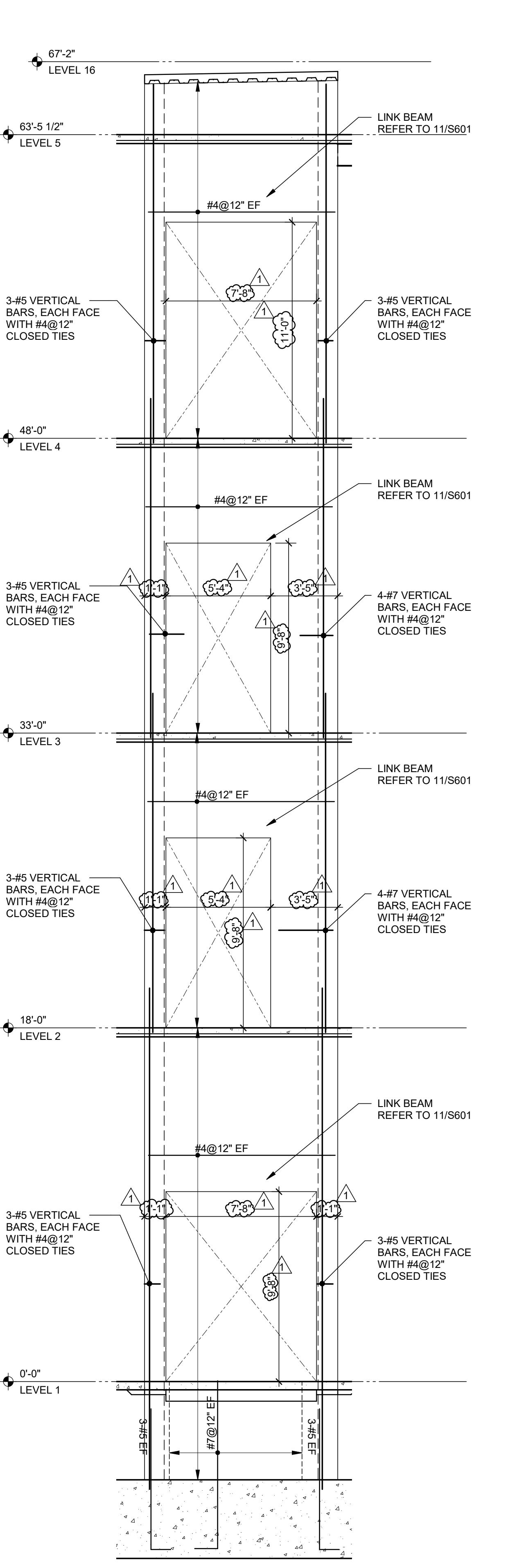
James David Martin, P.E., FL PE No. 69185  
WPM Project No. S05-22040-00  
Certificate of Authorization No. 3818  
To the best of the Engineer's knowledge, the plans and specifications comply with all applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S611**  
CONCRETE SHEAR WALL ELEVATIONS



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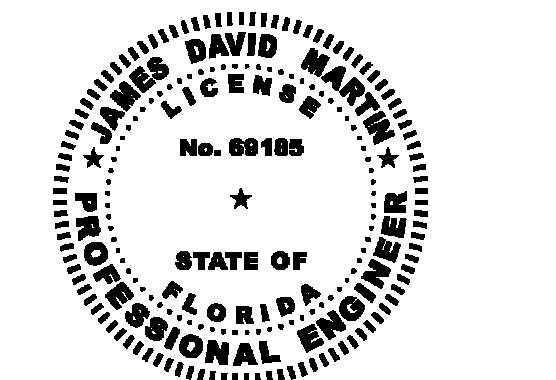


# Sarasota County Administration Center

# Sarasota County Administration

Project No.	22.29005.00
Drawn By	BD
Checked By	JDM
Date	09/08/2023

## Revisions:



James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-22049-00  
Certificate of Authorization No. 3818

best of the Engineer's knowledge, the plans and specifications comply with the applicable minimum building and applicable fire safety standards as determined by authority in accordance with Chapter 553 and 663 of

Authority in accordance with Chapter 333 and 663 of  
Statutes.

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**CONFORMED SET**

02/14/202

S612

# **6012**

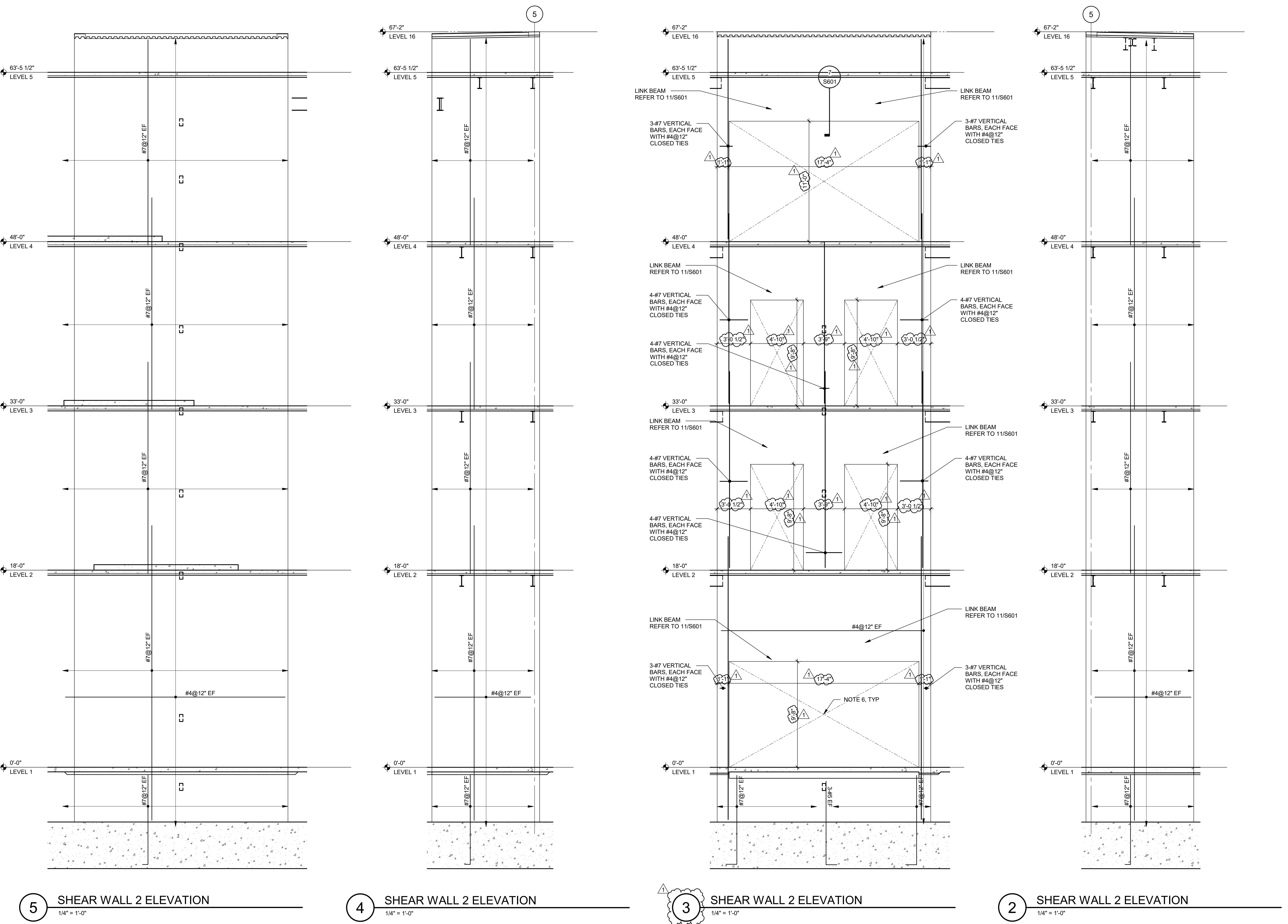
## **CONCRETE SHEAR**

## CONCRETE SHEAR WALL ELEVATIONS

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Sarasota, FL 34240

**Walter P. Moore** Structural Engineers  
201 East Kennedy Blvd., Suite 700  
Tampa, FL 33602

**SIEBEIN ACOUSTIC** Sieben Acoustics  
Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607

## Sarasota County Administration Center

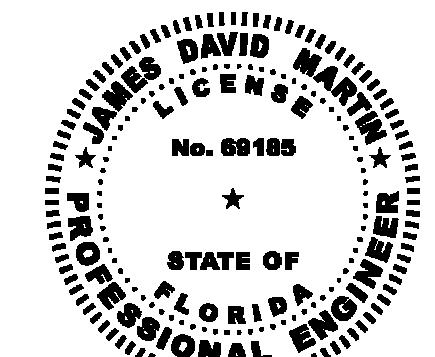
1 Apex Road  
Sarasota, Florida 34240

Autodesk Docs (7/22/2009 5:00 Sunmetra City Admin Bldg/S613/2204840 - Sarasota Cr/C - Structure R22.v4)

Project No. 22.29005.00  
Drawn By BD  
Checked By JDM  
Date 09/08/2023

Revisions:

1 ASI-02  
05/17/2024



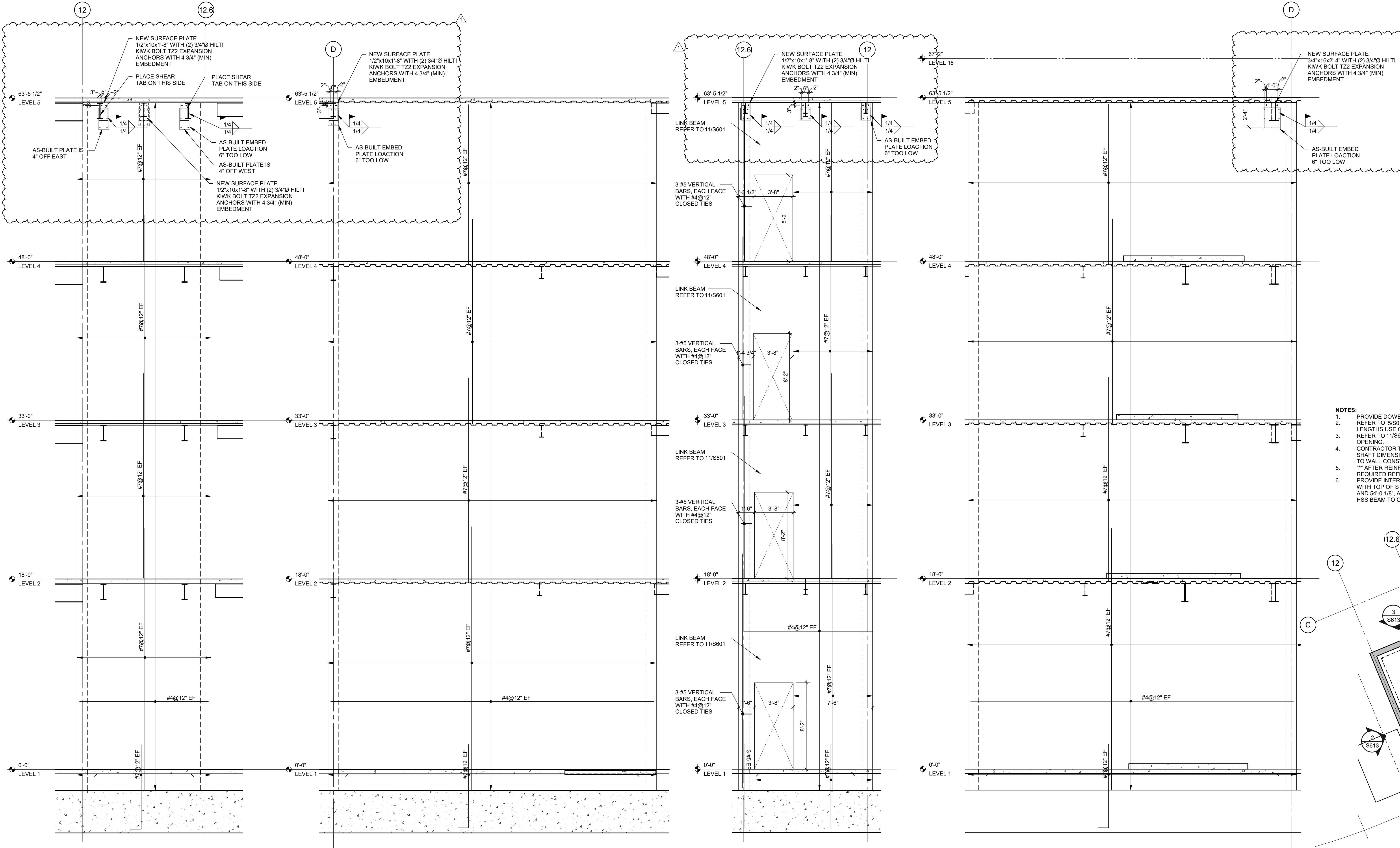
To the best of my knowledge, the plans and specifications comply with all applicable codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

CONFORMED SET  
02/14/2024

**S613**

CONCRETE SHEAR WALL ELEVATIONS

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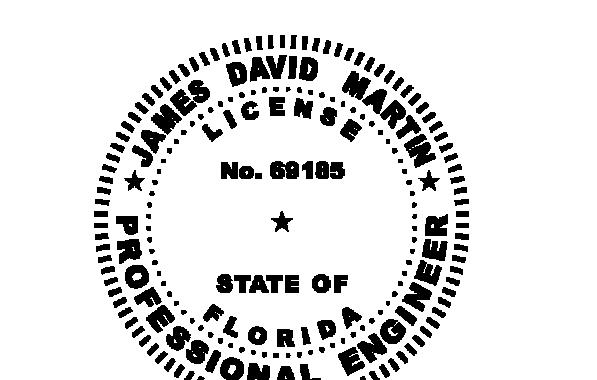
Walter P. Moore  
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201 East Kennedy Blvd., Suite 700  
Tampa, FL 33602

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Acoustics  
625 NW 60th St, Suite C  
Gainesville, FL 32607

**Sarasota County  
Administration Center**  
1 Apex Road  
Sarasota, Florida 34240

Project No.: 22.29005.00  
Drawn By: BD  
Checked By: JDM  
Date: 09/08/2023

Revisions:  
7 ASI-09 10/25/2024

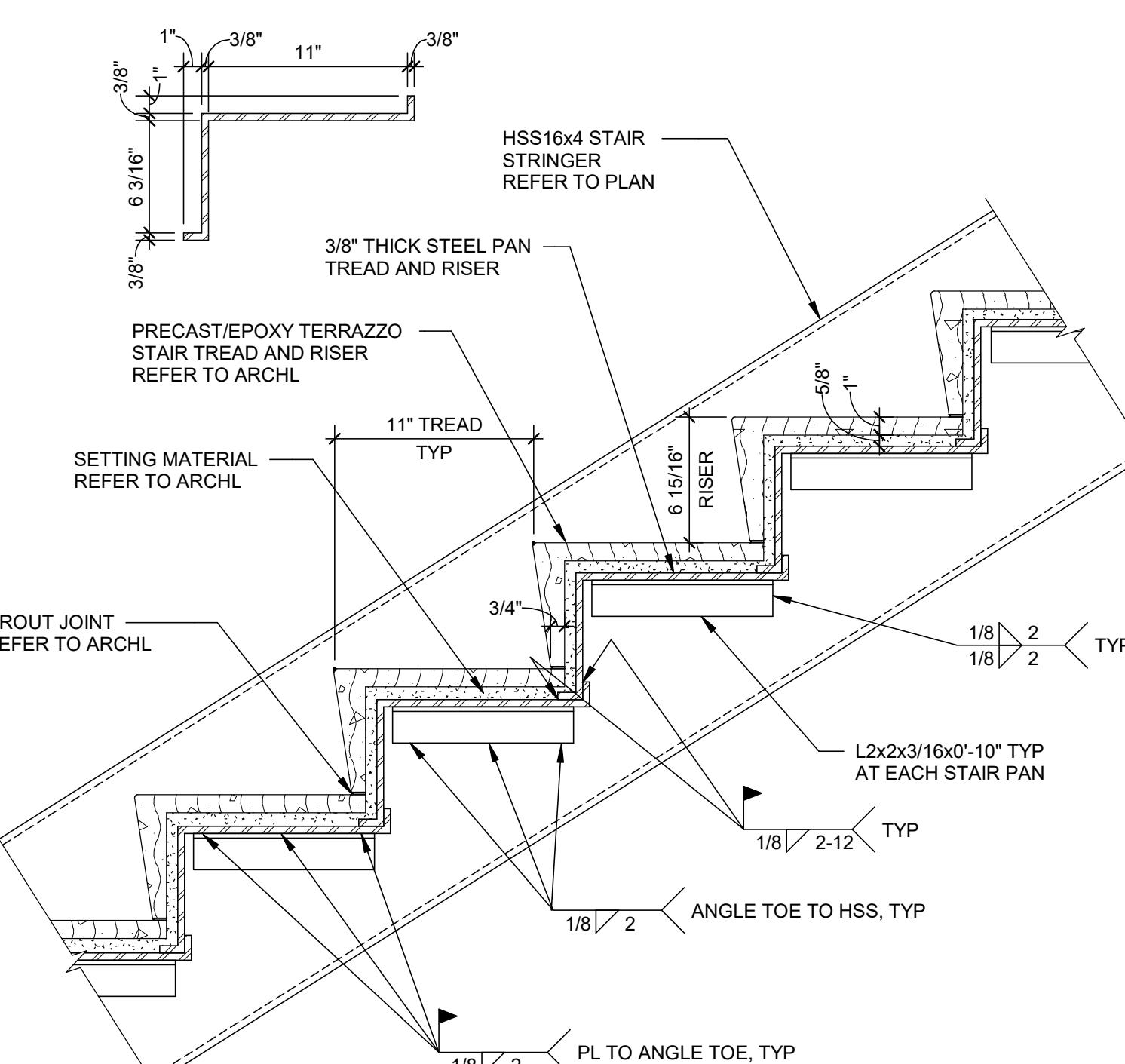


James David Martin, P.E. FL PE No. 69185  
WPM Project No. S05-2204-049  
Certificate of Authorization No. 3818  
To the best of the Engineer's knowledge, the plans and specifications comply with applicable building codes and applicable fire safety standards as determined by local authority in accordance with Chapter 553 and 603 of Florida Statutes.

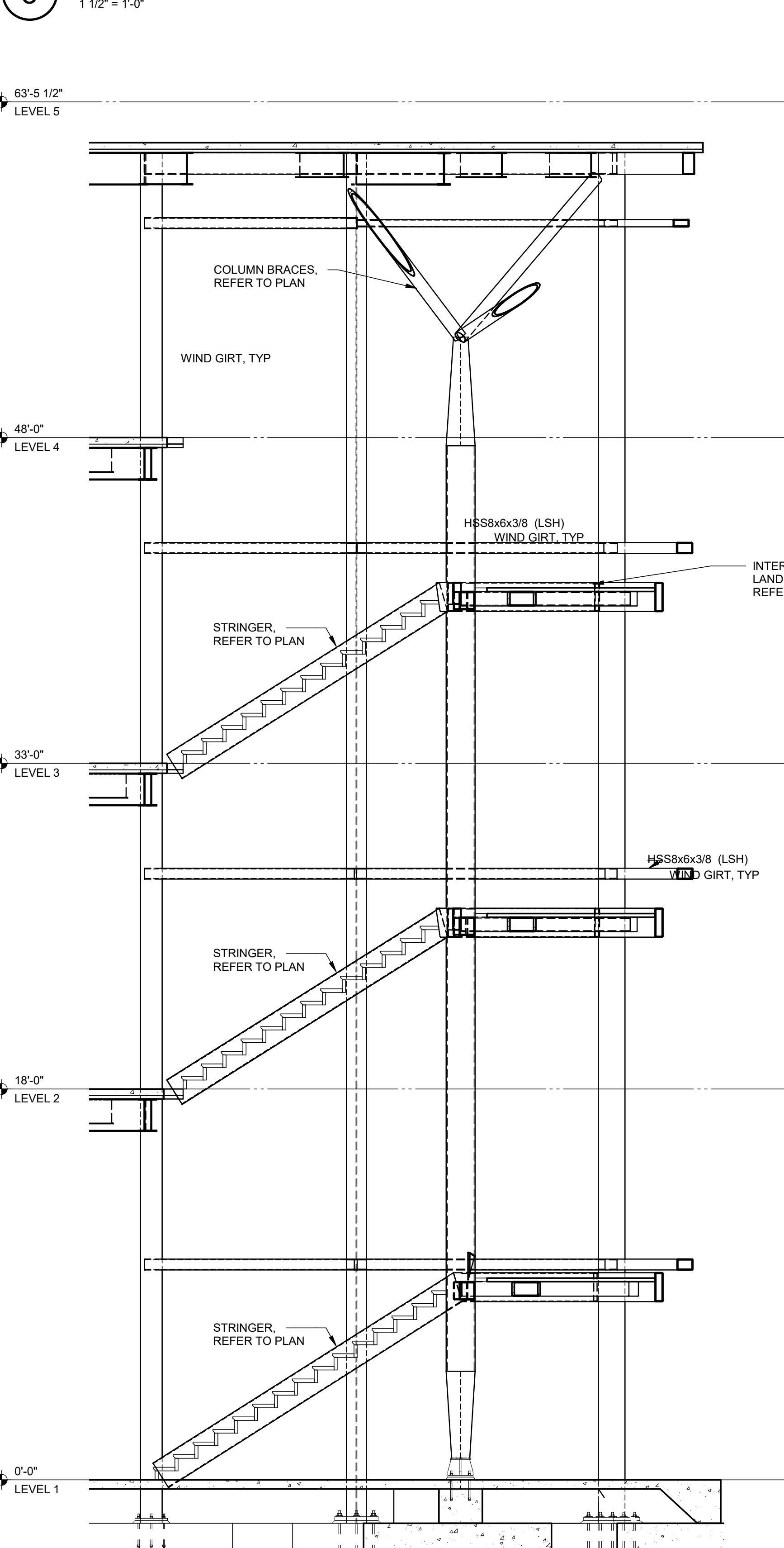
CONFORMED SET  
02/14/2024

**S701**  
MONUMENTAL STAIR

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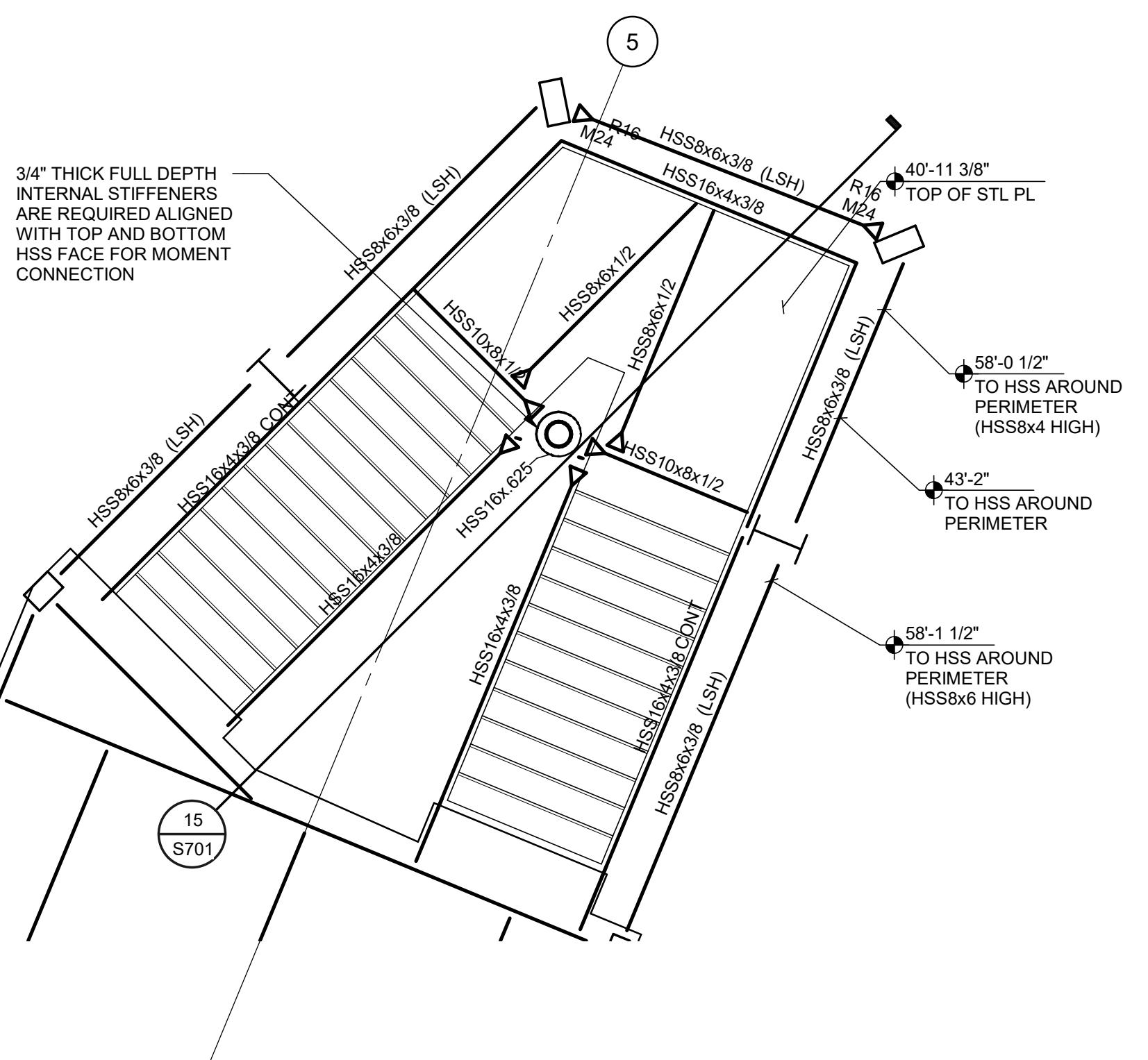


5 TYPICAL STAIR TREAD AND RISER



15 MONUMENTAL STAIR ELEVATION

1/4" = 1'-0"



- MAIN ENTRY STAIR ARCHITECTURALLY EXPOSED STRUCTURAL STEEL NOTES:
- THE FOLLOWING STRUCTURAL STEEL ELEMENTS AND CONNECTIONS ARE TO BE SUPPLIED AND ERECTED PER AES3: MAIN ENTRY STAIR STRINGERS, LANDINGS, TREADS AND RISERS.
  - AESS 3: STRUCTURAL STEEL DESIGNED AS 'AES3 IN THE CONTRACT DOCUMENTS AND CONFORMING TO ANSI/AISC 303-16, CHAPTER 10 DEFINITION OF AES3. THESE ARE FEATURE ELEMENTS VIEWED AT A DISTANCE LESS THAN 10 FEET. THE ART OF METALWORKING IS INTENDED TO BE VISIBLE TO THE VIEWER.
  - 3/4" THICK FULL DEPTH INTERNAL STIFFENERS ARE REQUIRED ALIGNED WITH TOP AND BOTTOM HSS FACE FOR MOMENT CONNECTION
  - THE TOLERANCE ON OVERALL PROFILE DIMENSIONS OF WELDED BUILT-UP MEMBERS SHALL BE ONE-HALF OF THAT SPECIFIED IN AWS D1.1/D1.1M: 2015 STRUCTURAL WELDING CODE -STEEL (AWS D1).
  - PROVIDE HIDDEN PIECE MARKS THAT MAY BE FULLY REMOVED AFTER FABRICATION
  - FABRICATE AES3 WITH EXPOSED SURFACES SMOOTH, SQUARE AND OF SURFACE QUALITY CONSISTENT WITH THE APPROVED MOCK UP.
  - GRIND PROJECTIONS AT BUTT AND PLUG WELDS TO BE SMOOTH WITH THE ADJACENT SURFACE.
  - ORIENTATION OF HSS SEAMS SHALL BE AS SHOWN.
  - COPES, MITERS, AND CUTS IN SURFACES EXPOSED TO VIEW SHALL HAVE A MAXIMUM GAP OF 1/8" IN AN OPEN JOINT. IF THE GAP IS SHOWN TO BE IN CONTACT, THE CONTACT SHALL BE UNIFORM WITHIN 1/16".
  - MILL MARKS SHALL NOT BE EXPOSED TO VIEW. IF IT IS NOT POSSIBLE TO HIDE MILL MARKS, THEN THE MILL MARKS ARE TO BE REMOVED BY APPROPRIATE LENGTH CUTTING OF MILL MATERIAL. IF THIS IS NOT POSSIBLE, THE FABRICATOR SHALL REMOVE THE MILL MARK, GRIND, AND FILL THE SURFACE TO BE CONSISTENT WITH THE APPROVED MOCK UP.
  - THE MATCHING OF ABUTTING CROSS SECTIONS IS REQUIRED
  - PROVIDE SURFACE PREPARATIONS TO SSPC-SP3. COORDINATE THE REQUIRED SURFACE PROFILE WITH THE APPROVED PAINT SUBMITTAL PRIOR TO BEGINNING SURFACE PREPARATION.
  - AESS ACCEPTANCE- THE ARCHITECT SHALL OBSERVE THE AES3 STEEL IN THE SHOP AT A VIEWING DISTANCE CONSISTENT WITH THE FINAL INSTALLATION AND DETERMINE ACCEPTABILITY BASED ON THE QUALIFICATION DATA AND SUBMITTALS. THE TESTING AND INSPECTION AGENCY SHALL HAVE NO RESPONSIBILITY FOR ENFORCING THE REQUIREMENTS OF THIS SECTION.
  - AESS ERECTION TOLERANCES: ERECT TO STANDARD FRAME TOLERANCES FOR STRUCTURAL STEEL PER CHAPTER 7 OF ANSI/AISC 303-16.
  - FIELD WELDING: WELD PROFILE, QUALITY, AND FINISH SHALL BE CONSISTENT WITH MOCK-UPS APPROVED PRIOR TO FABRICATION.
  - PROVIDE A CONTINUOUS APPEARANCE TO ALL WELDED JOINTS INCLUDING TACK WELDS. PROVIDE JOINT FILLER AT INTERMITTENT WELDS.

15 FRAMING PLAN - LEVEL 4 - STAIR PARTIAL PLAN

1/4" = 1'-0"

8 FRAMING PLAN - LEVEL 3 - STAIR PARTIAL PLAN

1/4" = 1'-0"

13 FRAMING PLAN - LEVEL 2 - STAIR PARTIAL PLAN

1/4" = 1'-0"

12 STAIR ISOMETRIC

1/4" = 1'-0"

