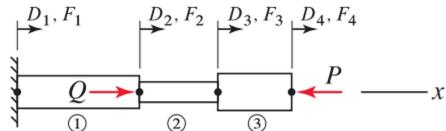


# HW1 Q1

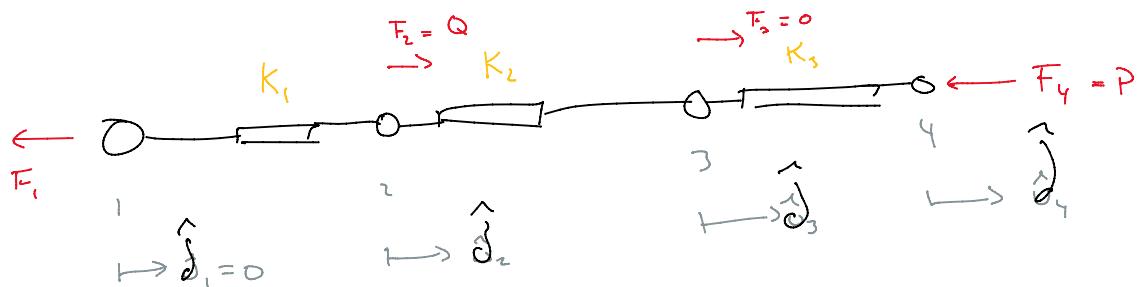
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For each of the following problem, formulate the global stiffness matrix  $K$ , and determine the global equilibrium equations  $[K]\{D\} = \{F\}$ . For Problem 1, solve the system of linear equilibrium equations you have derived and determine the nodal displacements, reactions at the support, and nodal forces.

## Problem 1 (60 pts)



$$\kappa_1 = \frac{A_1 E_1}{L_1} \quad \kappa_2 = \frac{A_2 E_2}{L_2} \quad \kappa_3 = \frac{A_3 E_3}{L_3}$$



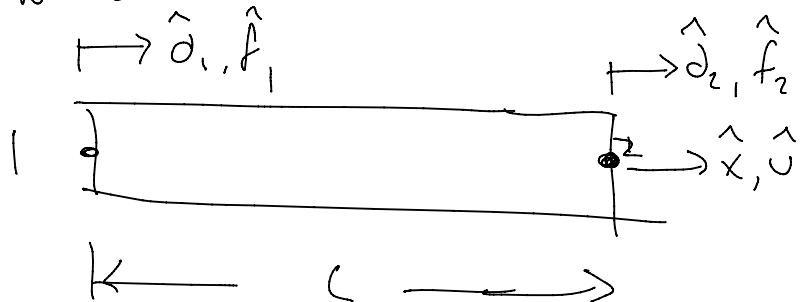
This problem has 3 elements, 4 nodes

Element	1	$\alpha$	nodes	1, 2
"	2	$\alpha$	"	2, 3
"	3	$\alpha$	"	3, 4
"				

In Equilibrium:

$$\sum F = 0 \Rightarrow \sum_{\text{# nodes}} f = F$$

We can analyze each element such that:



A Bar w/ length L, area A, Young's modulus E.

A bar w/ length  $L$ , area  $A$ , Young's modulus  $E$ .

Displacement function

$$\hat{u} = a_1 + a_2 \hat{x} \Rightarrow \begin{array}{l} \text{Linear interpolation} \\ \text{function for} \\ \text{element.} \end{array}$$

Matrix form

$$\hat{u} = \begin{bmatrix} 1, \hat{x} \end{bmatrix} = \begin{Bmatrix} a_1 \\ a_2 \end{Bmatrix}$$

As a function of nodal displacements:

$$\hat{u}(0) = \hat{d}_1 = a_1$$

$$\hat{u}(L) = \hat{d}_2 = a_1 + a_2 L \Rightarrow a_2 = \frac{\hat{d}_2 - \hat{d}_1}{L}$$

$$\hat{u} = \hat{d}_1 + \left( \frac{\hat{d}_2 - \hat{d}_1}{L} \right) \hat{x}$$

$$\hat{u} = \left( 1 - \frac{\hat{x}}{L} \right) \hat{d}_1 + \left( \frac{\hat{x}}{L} \right) \hat{d}_2$$

In matrix form:

$$\hat{u} = \begin{bmatrix} 1 & \hat{x} \\ 1 - \frac{\hat{x}}{L} & \frac{\hat{x}}{L} \end{bmatrix} \begin{Bmatrix} \hat{d}_1 \\ \hat{d}_2 \end{Bmatrix}$$

$$\hat{u} = \begin{bmatrix} N_1 & N_2 \end{bmatrix} \begin{Bmatrix} \hat{d}_1 \\ \hat{d}_2 \end{Bmatrix} = N \hat{d}$$

$$N_1 = 1 - \frac{\hat{x}}{L} \quad N_2 = \frac{\hat{x}}{L}$$

This is our shape function.

This is our shape function.

We then use the constitutive model

$$\sigma = E \epsilon$$

Strain - Stress relationship:

$$\epsilon = \frac{\partial \hat{u}}{\partial \hat{x}} = \frac{1}{\hat{J}} \left( N \hat{d} \right)$$

$\hat{J} \neq$  function of  $\hat{x}$

$$\text{Therefore, } \epsilon = \underbrace{\frac{\partial N}{\partial \hat{x}}}_{\text{Strain-disp.} = B} \hat{d} = B \hat{d}$$

$$\epsilon = \frac{1}{\hat{J}} \left[ \frac{\frac{\partial \hat{x}}{\partial \hat{1}}}{\hat{1}}, \frac{\frac{\partial \hat{x}}{\partial \hat{2}}}{\hat{2}} \right] \hat{d}$$

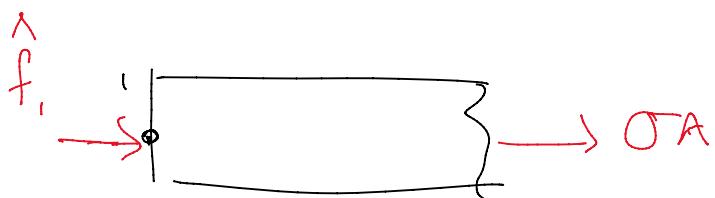
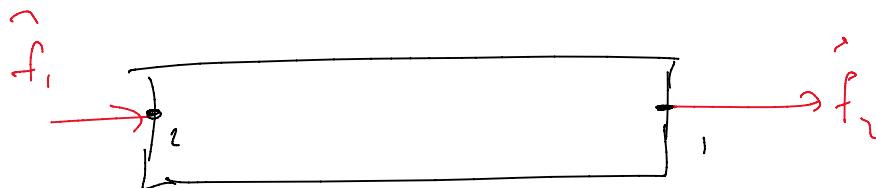
$$\epsilon = B \hat{d}$$

$$\text{Then, } \sigma = E B \hat{d}$$

$$\sigma = E \left[ \frac{1}{\hat{1}}, \frac{1}{\hat{2}} \right] \left\{ \begin{array}{c} \hat{d}_1 \\ \hat{d}_2 \end{array} \right\}$$

$$\sigma = E \frac{d_2 - d_1}{L}$$

For equilibrium,



$$\sum F = 0 = \hat{f}_1 + \hat{f}_2$$

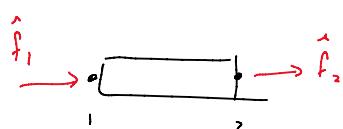
$$\sum F = 0 = \hat{f}_1 + \sigma A$$

$$\Rightarrow \hat{f}_1 = -\sigma A = -\frac{AE}{L} (\hat{d}_2 - \hat{d}_1)$$

$$\hat{f}_1 = \frac{AE}{L} (\hat{d}_1 - \hat{d}_2) = K (\hat{d}_1 - \hat{d}_2)$$

$$\hat{f}_2 = -\hat{f}_1 = K (\hat{d}_2 - \hat{d}_1) \quad \hookrightarrow k = \frac{AE}{L}$$

Then, we can apply this to our nodes 1, 2, 3, 4, elements ① ② ③. In our case, that means we can link  $\hat{\delta}_1, \hat{\delta}_2$  to  $\hat{f}_1, \hat{f}_2$  to  $\sigma$ . When we consider elements 1, 2, 3 continuously using stress-strain relationship



$\sigma = E B \hat{\delta}$   
In matrix form w/ some workaround:  $\hat{f}_1 \rightarrow \sigma \rightarrow \hat{f}_2$

$$K_1 \left[ \begin{array}{cc} +1 & -1 \\ -1 & +1 \end{array} \right] \left[ \begin{array}{c} \hat{\delta}_1 \\ \hat{\delta}_2 \end{array} \right] = \left[ \begin{array}{c} \hat{f}_1 \\ \hat{f}_2 \end{array} \right] \Rightarrow \sum F = 0 = \hat{f}_1 + \hat{f}_2 = \hat{f}_1 + \sigma A$$

$\hat{f}_1 = \sigma A = k_1 (\hat{\delta}_1 - \hat{\delta}_2)$

Equivalently to (in global eq.) for each node,  $\hat{f}_1 = k_1 (D_1 - D_2)$  DRiven by element 1, 2, 3

$$K_1 \left[ \begin{array}{ccccc} 1 & -1 & 0 & 0 & 0 \\ -1 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \end{array} \right] \left[ \begin{array}{c} D_1 \\ D_2 \\ D_3 \\ D_4 \end{array} \right] = \left[ \begin{array}{c} \hat{f}_1 \\ \hat{f}_2 \\ \hat{f}_3 \\ \hat{f}_4 \end{array} \right]$$

for element 1.  $\hat{f}_2 = k_1 (D_2 - D_1)$

w/ respect to element 1 so on, which is where the matrix comes from.

Element 2 equivalently models:

$$K_2 \left[ \begin{array}{ccccc} 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & -1 & 0 & 0 \\ 0 & -1 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \end{array} \right] \left[ \begin{array}{c} D_1 \\ D_2 \\ D_3 \\ D_4 \end{array} \right] = \left[ \begin{array}{c} \hat{f}_1 \\ \hat{f}_2 \\ \hat{f}_3 \\ \hat{f}_4 \end{array} \right] \left\{ \begin{array}{l} \hat{f}_1 = k_2 (D_1 - D_3) \\ \hat{f}_3 = k_2 (D_3 - D_2) \end{array} \right\}$$

Matrix form on left

And so on for elements 3

$$K_3 \left[ \begin{array}{ccccc} 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & -1 & 0 \\ 0 & 0 & -1 & 1 & 0 \end{array} \right] \left[ \begin{array}{c} D_1 \\ D_2 \\ D_3 \\ D_4 \end{array} \right] = \left[ \begin{array}{c} \hat{f}_1 \\ \hat{f}_2 \\ \hat{f}_3 \\ \hat{f}_4 \end{array} \right] \left\{ \begin{array}{l} \hat{f}_2 = k_3 (D_2 - D_4) \\ \hat{f}_4 = k_3 (D_4 - D_3) \end{array} \right\}$$

... however, this maps to:  $F = \hat{f}_1 + \hat{f}_2 + \hat{f}_3 + \hat{f}_4$

(3)

In global terms, however, this maps to:  $F = f_1 + f_2 + f_3$

$$\begin{bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{bmatrix} = \begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1+k_2 & -k_2 & 0 \\ 0 & -k_2 & k_2+k_3 & -k_3 \\ 0 & 0 & -k_3 & k_3 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \end{bmatrix}$$

$$k_1 = \frac{A_1 E_1}{L_1}$$

$$k_2 = \frac{A_2 E_2}{L_2}$$

$$k_3 = \frac{A_3 E_3}{L_3}$$

From the info. we know from our problem:

Global Stiffness Matrix

$$\begin{bmatrix} F_1 \\ Q \\ 0 \\ -P \end{bmatrix} = \begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1+k_2 & -k_2 & 0 \\ 0 & -k_2 & k_2+k_3 & -k_3 \\ 0 & 0 & -k_3 & k_3 \end{bmatrix} \begin{bmatrix} 0 \\ R_2 \\ D_3 \\ D_4 \end{bmatrix}$$

Multiplying our r.h.s:

$$\begin{bmatrix} F_1 \\ Q \\ 0 \\ -P \end{bmatrix} = \begin{bmatrix} k_1 \cdot 0 - k_1 \cdot D_2 + 0 \cdot D_3 + 0 \cdot D_4 \\ -k_1 \cdot 0 + (k_1+k_2)(D_1) - k_2 \cdot D_3 + 0 \cdot D_4 \\ 0 \cdot 0 - k_2 \cdot D_2 + (k_2+k_3)(D_3) - k_3 \cdot D_4 \\ 0 \cdot 0 + 0 \cdot D_1 - k_3 \cdot D_3 + k_3 \cdot D_4 \end{bmatrix}$$

$$\begin{bmatrix} F_1 \\ Q \\ 0 \\ -P \end{bmatrix} = \begin{bmatrix} -k_1 D_2 \\ k_1 D_1 + k_2 D_2 - k_2 D_3 \\ -k_2 D_2 + k_2 D_3 + k_3 D_3 - k_3 D_4 \\ -k_3 D_3 + k_3 D_4 \end{bmatrix}$$

From here, solve the system of equations:

$$F_1 = -k_1 D_2$$

$$\begin{aligned} Q &= k_1 D_1 + k_2 D_2 - k_2 D_3 \\ 0 &= -k_2 D_2 + k_2 D_3 + k_3 D_3 - k_3 D_4 \end{aligned} \quad \text{ADD}$$

$$Q = k_1 D_1 + k_3 D_3 - k_3 D_4 \quad \text{ADD}$$

$$\begin{aligned}
 \textcircled{5} \quad Q &= k_1 D_2 + \cancel{k_2 D_3} - \cancel{k_3 D_4} \\
 -P &= -\cancel{k_3 D_3} + \cancel{k_2 D_4} \\
 Q - P &= k_1 D_2 \Rightarrow F_1 = P - Q \\
 Q - P &= k_1 D_2 + k_2 D_3 - k_2 D_3 - k_3 D_3 + k_3 D_4 \\
 k_1 D_2 &= \cancel{k_1 D_2} + \cancel{k_2 D_2} - \cancel{k_2 D_3} - \cancel{k_3 D_3} + k_3 D_4 \\
 -k_2 D_2 + k_2 D_3 &= -k_3 D_3 + k_3 D_4
 \end{aligned}$$

$$F_1 + Q = k_2 D_2 - k_2 D_3$$

$$\begin{aligned}
 Q &= -k_2 D_2 + k_2 D_3 + \cancel{k_2 D_3} - \cancel{k_3 D_4} \\
 -P &= -\cancel{k_3 D_3} + \cancel{k_3 D_4}
 \end{aligned}$$

Determining Displacements

$$D_2 = \frac{F_1}{-k_1} \quad D_1 = 0$$

$$Q = D_3 (-k_2) + \frac{F_1}{-k_2} (k_1 + k_2)$$

$$\begin{bmatrix} F_1 \\ Q \\ 0 \\ -P \end{bmatrix} = \begin{bmatrix} -k_2 D_2 \\ k_1 D_2 + k_2 D_3 - k_3 D_3 \\ -k_2 D_2 + k_2 D_3 + k_3 D_3 - k_3 D_4 \\ -k_3 D_3 + k_3 D_4 \end{bmatrix}$$

$$\rightarrow \text{Subbing } D_2 = \frac{F_1}{-k_1}$$

$$D_3 = \frac{Q + \frac{F_1}{-k_1} (k_1 + k_2)}{-k_2}$$

$$0 = D_2 (-k_2) + D_3 (k_2 + k_3) + D_4 (-k_3)$$

$$D_2 \cdot k_2 - D_3 (k_2 + k_3) = D_4 (-k_3)$$

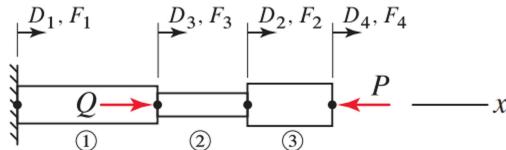
$$(-k_3) D_4 = \frac{F_1}{-k_2} (k_2) + \frac{[Q + \frac{F_1}{-k_1} (k_1 + k_2)] (k_2 + k_3)}{k_2}$$

$$D_4 = \frac{F_1}{-k_3} + \frac{[Q + \frac{F_1}{-k_1} (k_1 + k_2)] (k_2 + k_3)}{-k_2 k_3}$$

# HW1 Q2

Saturday, September 18, 2021 2:07 PM

## Problem 2 (20 pts)



$$\kappa_1 = \frac{A_1 E_1}{L_1} \quad \kappa_2 = \frac{A_2 E_2}{L_2} \quad \kappa_3 = \frac{A_3 E_3}{L_3}$$

$$F_1 = Q$$

$$F_3 = 0$$

$$\kappa_3 = 0$$

$$\rightarrow K_2$$

$$\kappa_2 = 0$$

$$F_4 = P$$



In matrix form w/ some boundary:

$$K_1 \begin{bmatrix} +1 & -1 \\ -1 & +1 \end{bmatrix} \begin{bmatrix} \hat{d}_1 \\ \hat{d}_2 \end{bmatrix} = \begin{bmatrix} \hat{f}_1 \\ \hat{f}_2 \end{bmatrix}$$

$$K_1 = \frac{A_1 E_1}{L_1}$$

Equivalent to

$$K_1 \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \end{bmatrix} = \begin{bmatrix} \hat{f}_1 \\ \hat{f}_2 \\ \hat{f}_3 \\ \hat{f}_4 \end{bmatrix} \quad \text{for element 1.}$$

For each node,

$$\hat{f}_1 = k_1(D_1 - D_2)$$

$$\hat{f}_2 = k_1(D_2 - D_1)$$

so on, which is where the matrix comes from.

Element 2 equivalently models:

$$K_2 \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 1 & -1 & 0 \\ 0 & -1 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \end{bmatrix} = \begin{bmatrix} \hat{f}_1 \\ \hat{f}_2 \\ \hat{f}_3 \\ \hat{f}_4 \end{bmatrix} \quad \left\{ \begin{array}{l} \hat{f}_1 = k_2(D_1 - D_3) \\ \hat{f}_3 = k_2(D_3 - D_1) \end{array} \right. \quad \text{Matrix form on left}$$

And so on for elements 3

$$K_3 \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \end{bmatrix} = \begin{bmatrix} \hat{f}_1 \\ \hat{f}_2 \\ \hat{f}_3 \\ \hat{f}_4 \end{bmatrix} \quad \left\{ \begin{array}{l} \hat{f}_2 = k_3(D_2 - D_4) \\ \hat{f}_4 = k_3(D_4 - D_2) \end{array} \right.$$

Then, the global stiffness matrix can be formed by adding elements 1, 2, 3:

$$[K] = \begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1+k_2 & -k_2 & 0 \\ 0 & -k_2 & k_2+k_3 & -k_3 \\ 0 & 0 & -k_3 & k_3 \end{bmatrix}$$

In global  $E_2$ , we can formulate:

$$\begin{bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{bmatrix} = \begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1+k_2 & -k_2 & 0 \\ 0 & -k_2 & k_2+k_3 & -k_3 \\ 0 & 0 & -k_3 & k_3 \end{bmatrix} \begin{bmatrix} D_1 \\ D_2 \\ D_3 \\ D_4 \end{bmatrix}$$

$k_1 = \frac{A_1 E_1}{L_1}$   
 $k_2 = \frac{A_2 E_2}{L_2}$   
 $k_3 = \frac{A_3 E_3}{L_3}$

We can plug in our knowns from the problem:

$$F_1 = Q, \quad F_2 = 0, \quad F_3 = -P, \quad D_1 = 0 \quad \text{to get:}$$

$$\begin{bmatrix} F_1 \\ Q \\ 0 \\ -P \end{bmatrix} = \begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1+k_2 & -k_2 & 0 \\ 0 & -k_2 & k_2+k_3 & -k_3 \\ 0 & 0 & -k_3 & k_3 \end{bmatrix} \begin{bmatrix} 0 \\ P_2 \\ D_3 \\ D_4 \end{bmatrix}$$

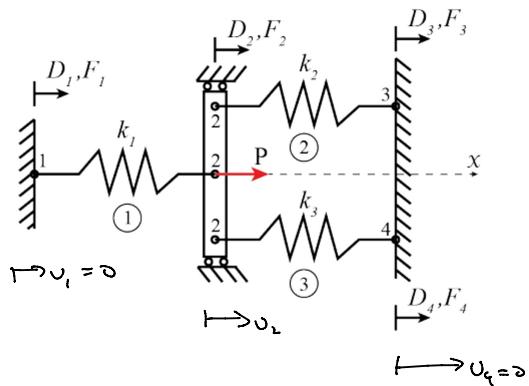
$$\{F\} = [K]\{D\}$$

# HW1 Q3

Saturday, September 18, 2021 2:07 PM

## Problem 3 (20 pts)

$$\rightarrow u_7 = 0$$



- Three elements, 4 nodes
  - Stiffness given 1:  $k_1$  2:  $k_2$  3:  $k_3$
  - Displacements
 

node 1	node 2	node 3	node 4
$u_1 = 0$	$u_2 = u_2$	$u_3 = u_4 = 0$	
  - Forces
 

Node 1	Node 2	Node 3	Node 4
$F_1$	$P$	$F_3$	$F_4$
- $F_E$  Analysis of Element ①

$$\hat{f}_1 = k_1 (u_1 - u_2)$$

$$\hat{f}_2 = k_1 (u_2 - u_1)$$

In matrix form

$$\begin{bmatrix} \hat{f}_1 \\ \hat{f}_2 \end{bmatrix} = k_1 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

For element ②

$$\hat{f}_3 = k_2 (u_2 - u_3)$$

$$\hat{f}_4 = k_3 (u_3 - u_4)$$

$$f_2 = k_2(u_2 - u_3)$$

$$\hat{f}_3 = k_2(u_3 - u_2)$$

In matrix form:

$$\begin{bmatrix} \hat{f}_2 \\ \hat{f}_3 \end{bmatrix} = k_2 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_2 \\ u_3 \end{bmatrix}$$

For element ③

$$\hat{f}_2 = k_3(u_2 - u_4)$$

$$\hat{f}_4 = k_3(u_4 - u_2)$$

In matrix form:

$$\begin{bmatrix} \hat{f}_2 \\ \hat{f}_4 \end{bmatrix} = k_3 \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_2 \\ u_4 \end{bmatrix}$$

Note: node 2 & 3, 4 then in global eq we can add elements 1, 2, 3 to get the global stiffness matrix:

$$\boxed{\begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1 + k_2 + k_3 & -k_2 & -k_3 \\ 0 & -k_2 & k_2 & 0 \\ 0 & -k_3 & 0 & k_3 \end{bmatrix}}$$

Assembling  $\begin{bmatrix} u \end{bmatrix} \{U\} = \{F\}$

$$\{U\} = \begin{pmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{pmatrix} = \begin{pmatrix} 0 \\ u_2 \\ 0 \\ 0 \end{pmatrix}$$

$$\{F\} = \begin{pmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{pmatrix} = \begin{pmatrix} F_1 \\ P \\ F_3 \\ F_4 \end{pmatrix}$$

$$\begin{pmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{pmatrix} \begin{pmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \end{pmatrix}$$

$$\text{Then } [k] \{u\} = \{F\}$$

$$\boxed{\begin{bmatrix} k_1 & -k_1 & 0 & 0 \\ -k_1 & k_1+k_2+k_3 & -k_2 & -k_3 \\ 0 & -k_2 & k_2 & 0 \\ 0 & -k_3 & 0 & k_3 \end{bmatrix} \begin{pmatrix} 0 \\ 1 \\ 0 \\ 0 \end{pmatrix} = \begin{pmatrix} F_1 \\ P \\ F_3 \\ F_4 \end{pmatrix}}$$