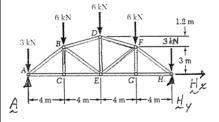


Determine the force in each member of the Gambrel roof truss shown. State whether each member is in tension or compression.

## **SOLUTION**

### **FBD Truss:**



$$\rightarrow \Sigma F_r = 0$$
:  $\mathbf{H}_r = 0$ 

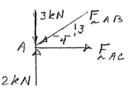
By symmetry:  $\mathbf{A} = \mathbf{H}_y = 12 \text{ kN}$ 

By inspection of joints C and G,

$$F_{CE} = F_{AC}$$
 and  $F_{BC} = 0$ 

$$F_{EG} = F_{GH}$$
 and  $F_{FG} = 0$ 

Joint *A*:



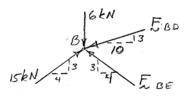
† 
$$\Sigma F_y = 0$$
: 12 kN − 3 kN −  $\frac{3}{5}F_{AB} = 0$   $F_{AB} = 15.00$  kN C ◀

$$F_{AB} = 15.00 \text{ kN C} \blacktriangleleft$$

→ 
$$\Sigma F_x = 0$$
:  $F_{AC} - \frac{4}{5} (15 \text{ kN}) = 0$   $F_{AC} = 12.00 \text{ kN T}$ 

$$F_{AC} = 12.00 \text{ kN T} \blacktriangleleft$$

Joint *B*:



$$\longrightarrow \Sigma F_x = 0: \frac{4}{5} (15 \text{ kN}) - \frac{10}{10.44} F_{BD} - \frac{4}{5} F_{BE} = 0$$

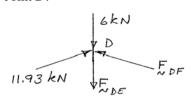
$$\uparrow \Sigma F_y = 0: \frac{3}{5} (15 \text{ kN}) - 6 \text{ kN} - \frac{3}{10.44} F_{BD} + \frac{3}{5} F_{BE} = 0$$

Solving yields

$$F_{BD} = 11.93 \text{ kN C} \blacktriangleleft$$

$$F_{BE} = 0.714 \text{ kN C} \blacktriangleleft$$

Joint *D*:



$$\rightarrow$$
  $\Sigma F_x = 0$ : by symmetry

$$F_{DF} = 11.93 \text{ kN C} \blacktriangleleft$$

$$\uparrow \Sigma F_y = 0$$
:  $-F_{DE} - 6 \text{ kN} + 2 \frac{3}{10.44} (11.93 \text{ kN}) = 0$ 

$$F_{DE} = 0.856 \text{ kN T} \blacktriangleleft$$

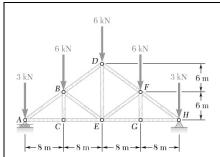
By symmetry: 
$$F_{EF} = F_{BE}$$
 so  $F_{EF} = 0.714 \text{ kN C} \blacktriangleleft$ 

$$F_{FH} = F_{AB} \qquad \qquad F_{FH} = 15.00 \; \mathrm{kN} \; \; \mathrm{C} \; \blacktriangleleft \label{eq:FH}$$

$$F_{GH} = F_{AC}$$
  $F_{GH} = 12.00 \text{ kN T} \blacktriangleleft$ 

From above 
$$F_{CE} = F_{AC}$$
  $F_{CE} = 12.00 \text{ kN T} \blacktriangleleft$ 

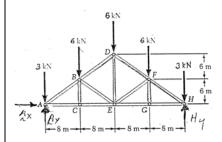
$$F_{EG} = F_{GH}$$
  $F_{EG} = 12.00 \text{ kN T} \blacktriangleleft$ 



Determine the force in each member of the Howe roof truss shown. State whether each member is in tension or compression.

# **SOLUTION**

#### **FBD Truss:**

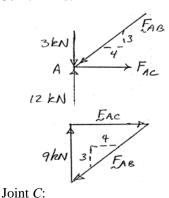


$$\rightarrow \Sigma F_x = 0$$
:  $\mathbf{A}_x = 0$ 

By symmetry:  $\mathbf{A}_{v} = \mathbf{H}_{v} = 12 \text{ kN}$ 

and  $F_{FH}=F_{AB}; F_{GH}=F_{AC}; F_{FG}=F_{BC}$   $F_{DF}=F_{BD}; F_{EF}=F_{BE}; F_{EG}=F_{CE}$ 

## **Joint FBDs:**



$$\frac{9 \text{ kN}}{3} = \frac{F_{AC}}{4} = \frac{F_{AB}}{5}$$

$$F_{AC} = 12.00 \text{ kN T}$$

$$F_{AB} = 15.00 \text{ kN C} \blacktriangleleft$$

so 
$$F_{FH} = 15.00 \text{ kN C} \blacktriangleleft$$

$$F_{GH} = 12.00 \text{ kN T} \blacktriangleleft$$

$$F_{BC} = 0; F_{CE} = 12 \text{ kN}$$

$$F_{BC} = 0 = F_{FG} \blacktriangleleft$$

$$F_{CE} = 12.00 \text{ kN T} \blacktriangleleft$$

$$F_{EG} = 12.00 \text{ kN T} \blacktriangleleft$$

Note: 
$$\alpha = \tan^{-1} \frac{4}{3}$$
 so  $\sin \alpha = 0.8$ 

$$\beta = 2\tan^{-1}\frac{3}{4} \qquad \text{so} \qquad \sin\beta = 0.96$$

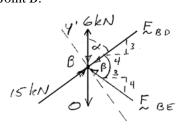
$$\sum F_{y^1} = 0: -6 \text{ kN } \sin \alpha + F_{BE} \sin \beta = 0$$

$$F_{BE} = 5.00 \text{ kN C}$$

so 
$$F_{EF} = 5.00 \text{ kN C}$$

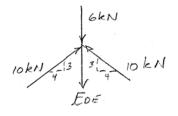
$$\uparrow \Sigma F_y = 0: -6 \text{ kN} - \frac{3}{5} F_{BD} + \frac{3}{5} F_{BE} + \frac{3}{5} (15 \text{ kN}) = 0$$

# Joint B:



# **PROBLEM 6.10 CONTINUED**

Joint *D*:

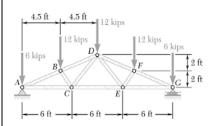


$$\frac{3}{5}F_{BD} = \frac{3}{5}(5 \text{ kN}) + \frac{3}{5}(15 \text{ kN}) - 6 \text{ kN} \qquad F_{BD} = 10.00 \text{ kN C}$$

so 
$$F_{DF} = 10.00 \text{ kN C} \blacktriangleleft$$

$$\frac{3}{5}F_{BD} = \frac{3}{5}(5 \text{ kN}) + \frac{3}{5}(15 \text{ kN}) - 6 \text{ kN} \qquad F_{BD} = 10.00 \text{ kN C} \blacktriangleleft$$
so  $F_{DF} = 10.00 \text{ kN C} \blacktriangleleft$ 

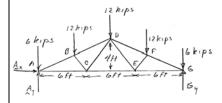
$$\uparrow \Sigma F_y = 0: -6 \text{ kN} + 2\left(\frac{3}{5}10 \text{ kN}\right) - F_{DE} = 0 \qquad F_{DE} = 6.00 \text{ kN T} \blacktriangleleft$$



Determine the force in each member of the Fink roof truss shown. State whether each member is in tension or compression.

### **SOLUTION**

**FBD Truss:** 



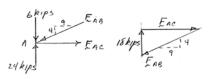
$$\rightarrow \Sigma F_x = 0$$
:  $\mathbf{A}_x = 0$ 

By symmetry:  $\mathbf{A}_y = \mathbf{G}_y = 24 \text{ kips}$ 

also; 
$$F_{FG}=F_{AB};\ F_{EG}=F_{AC};\ F_{EF}=F_{BC}$$
 
$$F_{DF}=F_{BD};\ F_{DE}=F_{CD}$$

### **Joint FBDs:**

Joint *A*:



$$\frac{18 \text{ kips}}{4} = \frac{F_{AC}}{9} = \frac{F_{AB}}{\sqrt{97}};$$

$$F_{AC} = 40.5 \text{ kips T} \blacktriangleleft$$

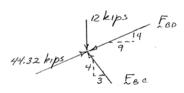
$$F_{AB} = 44.32 \text{ kips}$$

$$F_{AB} = 44.3 \text{ kips C} \blacktriangleleft$$

so 
$$F_{FG} = 44.3 \text{ kips C} \blacktriangleleft$$

$$F_{EG} = 40.5 \text{ kips T} \blacktriangleleft$$

Joint *B*:



$$ightharpoonup \Sigma F_x = 0: \frac{9}{\sqrt{97}} (44.32 \text{ kips} - F_{BD}) - \frac{3}{5} F_{BC} = 0$$

$$\uparrow \Sigma F_y = 0: \frac{4}{\sqrt{97}} (44.32 \text{ kips} - F_{BD}) + \frac{4}{5} F_{BC} - 12 = 0$$

Solving:

$$F_{BC} = 11.25 \text{ kips } \mathbf{C} \blacktriangleleft$$

$$F_{BD} = 36.9 \text{ kips } C \blacktriangleleft$$

so 
$$F_{EF} = 11.25 \text{ kips } C \blacktriangleleft$$

$$F_{DF} = 36.9 \text{ kips C} \blacktriangleleft$$

# **PROBLEM 6.11 CONTINUED**

$$\uparrow \Sigma F_y = 0: \frac{4}{5} (11.25 \text{ kips}) - \frac{4}{5} F_{CD} = 0$$

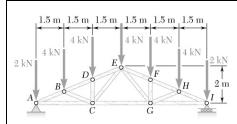
$$F_{CD} = 11.25 \text{ kips T} \blacktriangleleft$$

Joint *C*:

$$F_{DE} = 11.25 \text{ kips T} \blacktriangleleft$$

$$\rightarrow \Sigma F_x = 0$$
:  $F_{CE} + 2\left[\frac{3}{5}(11.25 \text{ kips})\right] - 40.5 \text{ kips} = 0$ 

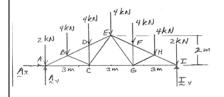
$$F_{CE} = 27.0 \text{ kips T} \blacktriangleleft$$



Determine the force in each member of the fan roof truss shown. State whether each member is in tension or compression.

# **SOLUTION**

#### **FBD Truss:**



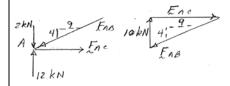
$$\rightarrow \Sigma F_x = 0$$
:  $\mathbf{A}_x = 0$ 

By symmetry:  $\mathbf{A}_{v} = \mathbf{I}_{v} = 12 \text{ kN}$ 

 $F_{AB} = F_{HI}; F_{AC} = F_{GI}; F_{BC} = F_{GH}$  $F_{RD} = F_{FH}; F_{DC} = F_{FG}; F_{DE} = F_{FF}$  $F_{CE} = F_{EG}$ 

## Joint FBDs:

#### Joint *A*:



$$\frac{10 \text{ kN}}{4} = \frac{F_{AC}}{9} = \frac{F_{AB}}{\sqrt{97}}$$

$$F_{AC} = 22.5 \text{ kN T} \blacktriangleleft$$

so 
$$F_{GI} = 22.5 \text{ kN T} \blacktriangleleft$$

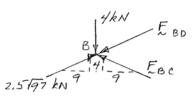
$$F_{AB} = 2.5\sqrt{97} \text{ kN}$$

so 
$$F_{GI}=22.5~\mathrm{kN}~\mathrm{T}$$
  $\blacktriangleleft$  
$$F_{AB}=2.5\sqrt{97}~\mathrm{kN}$$
 
$$F_{AB}=24.6~\mathrm{kN}~\mathrm{C}$$

so 
$$F_{HI} = 24.6 \text{ kN C} \blacktriangleleft$$

Joint *B*:

Joint *D*:



$$\rightarrow \Sigma F_x = 0: 22.5 \text{ kN} - \frac{9}{\sqrt{97}} (F_{BD} + F_{BC}) = 0$$

Solving:

$$F_{RD} = 19.70 \text{ kN C} \blacktriangleleft$$

so 
$$F_{FH} = 19.70 \text{ kN C} \blacktriangleleft$$

and 
$$F_{BC} = 4.92 \text{ kN C} \blacktriangleleft$$

so 
$$F_{GH} = 4.92 \text{ kN C} \blacktriangleleft$$

By inspection: 
$$F_{DE} = 19.70 \text{ kN C} \blacktriangleleft$$

By inspection: 
$$F_{DE} = 19.70 \text{ kN C}$$

so 
$$F_{EF} = 19.70 \text{ kN C} \blacktriangleleft$$

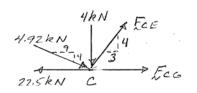
and 
$$F_{CD} = 4.00 \text{ kN C} \blacktriangleleft$$

so 
$$F_{FG} = 4.00 \text{ kN C} \blacktriangleleft$$

# **PROBLEM 6.12 CONTINUED**

$$\rightarrow \Sigma F_x = 0: -22.5 \text{ kN} + \frac{9}{\sqrt{97}} (4.92 \text{ kN}) + \frac{3}{5} F_{CE} + F_{CG} = 0$$

$$\uparrow \Sigma F_y = 0: -4 \text{ kN} - \frac{4}{\sqrt{97}} (4.92 \text{ kN}) + \frac{4}{5} F_{CE} = 0$$

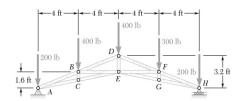


Solving:

$$F_{CE} = 7.50 \text{ kN T} \blacktriangleleft$$

so 
$$F_{EG} = 7.50 \text{kN T} \blacktriangleleft$$

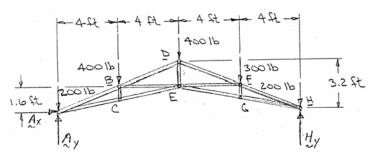
and 
$$F_{CG} = 13.50 \text{ kN T} \blacktriangleleft$$



Determine the force in each member of the roof truss shown. State whether each member is in tension or compression.

#### **SOLUTION**

#### **FBD** Truss:



$$(\Sigma M_A = 0: (16 \text{ ft}) H_y - (16 \text{ ft}) (200 \text{ lb}) - (12 \text{ ft}) (300 \text{ lb}) - (8 \text{ ft}) (400 \text{ lb}) - (4 \text{ ft}) (400 \text{ lb}) = 0$$

$$\mathbf{H}_{v} = 725 \text{ lb} \dagger$$

$$\Sigma F_{y} = 0$$
:  $A_{y} - 200 \text{ lb} - 400 \text{ lb} - 400 \text{ lb} - 300 \text{ lb} - 200 \text{ lb} + 725 \text{ lb} = 0$ 

$$\mathbf{A}_{y} = 775 \text{ lb}$$

$$\rightarrow \Sigma F_x = 0$$
:  $\mathbf{A}_x = 0$ 

# **Joint FBDs:**

Joint *A*:

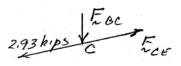
$$\frac{200lb}{A} = \frac{575 \text{ lb}}{1} = \frac{F_{AC}}{\sqrt{26}} = \frac{F_{AB}}{\sqrt{29}}$$

$$F_{AB} = 3096.5 \text{ lb}; \ F_{AB} = 3.10 \text{ kips } \text{ C} \blacktriangleleft$$

$$F_{AC}$$
 = 2931.9 lb;  $F_{AC}$  = 2.93 kips T

## **PROBLEM 6.13 CONTINUED**

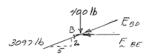
Joint *C*:



By inspection:

 $F_{CE} = 2.93 \text{ kips T} \blacktriangleleft$ 

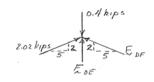
Joint *B*:



† Σ
$$F_y = 0$$
:  $\frac{2}{\sqrt{29}}$  (3097 lb) – 400 lb –  $\frac{2}{\sqrt{29}}$   $F_{BD} = 0$   $F_{BD} = 2020.0$  lb;  $F_{BD} = 2.02$  kips C ◀

→ Σ
$$F_x = 0$$
:  $\frac{5}{\sqrt{29}}$  (3097 – 2020) lb –  $F_{BE} = 0$   $F_{BE} = 1000.0$  lb;  $F_{BE} = 1.000$  kip C  $\blacktriangleleft$ 

Joint *D*:



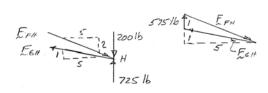
$$\rightarrow \Sigma F_x = 0: \frac{5}{\sqrt{29}} (2.02 \text{ kips} - F_{DF}) = 0$$

 $F_{DF} = 2.02 \text{ kips } C \blacktriangleleft$ 

$$\int \Sigma F_y = 0$$
:  $F_{DE} + 2\frac{2}{\sqrt{29}} (2.02 \text{ kips}) - 0.4 \text{ kips} = 0$ 

 $F_{DE} = 1.100 \text{ kips } \text{ C} \blacktriangleleft$ 

Joint *H*:



$$\frac{525 \text{ lb}}{1} = \frac{F_{FH}}{\sqrt{29}} = \frac{F_{GH}}{\sqrt{26}}$$

 $F_{FH} = 2827 \text{ lb}; \ F_{FH} = 2.83 \text{ kips } \text{ C} \blacktriangleleft$ 

 $F_{GH} = 2677 \text{ lb}; \ F_{GH} = 2.68 \text{ kips T} \blacktriangleleft$ 

Joint *G*:



By inspection:

 $F_{EG} = 2.68 \text{ kips T} \blacktriangleleft$ 

 $F_{FG} = 0$ 

# **PROBLEM 6.13 CONTINUED**

Joint *F*:

$$\uparrow \Sigma F_y = 0$$
:  $\frac{2}{\sqrt{29}} (2.83 \text{ kips} - F_{DF}) - 0.3 \text{ kips} = 0$   $F_{DF} = 2.02 \text{ kips}$ 

$$\rightarrow \Sigma F_x = 0$$
:  $F_{EF} + \frac{5}{\sqrt{29}} (2.02 \text{ kips} - 2.827 \text{ kips}) = 0$ 

$$F_{EF} = 0.750 \text{ kips C} \blacktriangleleft$$