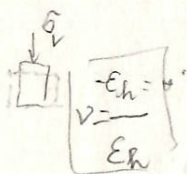


Principles; based on the definition of Poisson ratio

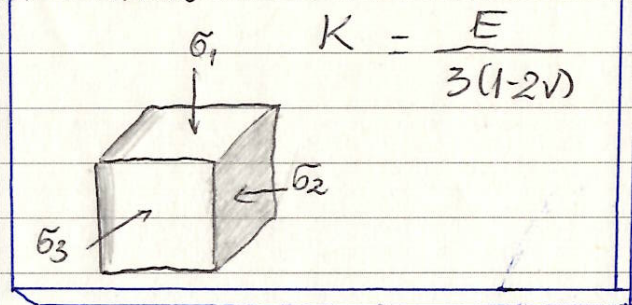


Assumption:

+ Isotropic

+ Homogeneous

1 Prove



* Only σ_1 :

$$d\epsilon_1 = \frac{d\sigma_1}{E}; d\epsilon_2 = -\frac{\nu}{E} d\sigma_1; d\epsilon_3 = -\frac{\nu}{E} d\sigma_1$$

* Only σ_2 :

$$d\epsilon_1 = -\frac{\nu}{E} d\sigma_2; d\epsilon_2 = \frac{d\sigma_2}{E}; d\epsilon_3 = -\frac{\nu}{E} d\sigma_2$$

* Only σ_3 :

$$d\epsilon_1 = -\frac{\nu}{E} d\sigma_3; d\epsilon_2 = -\frac{\nu}{E} d\sigma_3; d\epsilon_3 = \frac{d\sigma_3}{E}$$

* Sum:

$$d\epsilon_1 = \frac{1}{E} d\sigma_1 - \frac{\nu}{E} d\sigma_2 - \frac{\nu}{E} d\sigma_3$$

$$d\epsilon_2 = -\frac{\nu}{E} d\sigma_1 + \frac{1}{E} d\sigma_2 - \frac{\nu}{E} d\sigma_3$$

$$d\epsilon_3 = -\frac{\nu}{E} d\sigma_1 - \frac{\nu}{E} d\sigma_2 + \frac{1}{E} d\sigma_3$$

* Isotropic compression: $d\sigma_1 = d\sigma_2 = d\sigma_3$

$$\Rightarrow d\epsilon_1 = d\epsilon_2 = d\epsilon_3 = \frac{1-2\nu}{E} d\sigma$$

$$\Rightarrow d\sigma_{1,2,3} = \frac{E}{1-2\nu} d\epsilon_{1,2,3}$$

$$K = f(E, \nu)$$

$\nu = 0.5 \Rightarrow$ incompressible material

$$\Rightarrow dp = \frac{E}{3(1-2\nu)} d\epsilon_v \quad \left\{ \begin{array}{l} dp = \frac{1}{3} (d\sigma_1 + d\sigma_2 + d\sigma_3) \\ d\epsilon_v = d\epsilon_1 + d\epsilon_2 + d\epsilon_3 \end{array} \right.$$

K (bulk modulus)

2. Derive 3D elastic stiffness matrix:

Derive the relation between stress σ and strain ϵ via ν , E , G

• Shear strain:

$$\gamma_{xy} = \gamma_{yx} = \frac{\sigma_{xy}}{G} = \frac{\sigma_{yx}}{G}$$

$$\gamma_{yz} = \gamma_{zy} = \frac{\sigma_{yz}}{G} = \frac{\sigma_{zy}}{G}$$

$$\gamma_{zx} = \gamma_{xz} = \frac{\sigma_{zx}}{G} = \frac{\sigma_{xz}}{G}$$

• Also, from (1), we have the equations of $d\epsilon_x, d\epsilon_y, d\epsilon_z$

$$\begin{bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{zx} \end{bmatrix} = \begin{bmatrix} 1/E & -\nu/E & -\nu/E & 0 & 0 & 0 \\ -\nu/E & 1/E & -\nu/E & 0 & 0 & 0 \\ -\nu/E & -\nu/E & 1/E & 0 & 0 & 0 \\ 0 & 0 & 0 & 1/G & 0 & 0 \\ 0 & 0 & 0 & 0 & 1/G & 0 \\ 0 & 0 & 0 & 0 & 0 & 1/G \end{bmatrix} \begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{yz} \\ \sigma_{zx} \end{bmatrix}$$

Elastic stiffness matrix

• Stiffness matrix: $\sigma = D^e \epsilon$

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{yz} \\ \sigma_{zx} \end{bmatrix} = \begin{bmatrix} E'(1-\nu) & E'\nu & E'\nu & 0 & 0 & 0 \\ E'\nu & E'(1-\nu) & E'\nu & 0 & 0 & 0 \\ E'\nu & E'\nu & E'(1-\nu) & 0 & 0 & 0 \\ 0 & 0 & 0 & G & 0 & 0 \\ 0 & 0 & 0 & 0 & G & 0 \\ 0 & 0 & 0 & 0 & 0 & G \end{bmatrix} \begin{bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{yz} \\ \epsilon_{zx} \end{bmatrix}$$

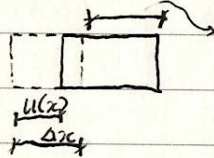
Elastic Stiffness Matrix

Prove: $\epsilon_x = \frac{\partial u}{\partial x}$; $\epsilon_y = \frac{\partial v}{\partial y}$; $\epsilon_z = \frac{\partial w}{\partial z}$

Strain $\epsilon(x)$ \in position

Displacement $u(x)$ \in position

Question: How do we get strain field from displacement



$$u(x+\Delta x) = u(x) + \Delta x \frac{\partial u}{\partial x} + O(\Delta x^2)$$

$$\epsilon_x = \frac{\Delta l}{l} = \frac{1}{\Delta x} (L' - L)$$

$$L' = \Delta x + u(x+\Delta x) - u(x)$$

$$= \Delta x + \left[u(x) + \Delta x \frac{\partial u}{\partial x} + O(\Delta x^2) \right] - u(x)$$

$$= \Delta x + \Delta x \frac{\partial u}{\partial x} + O(\Delta x^2)$$

$$L = \Delta x$$

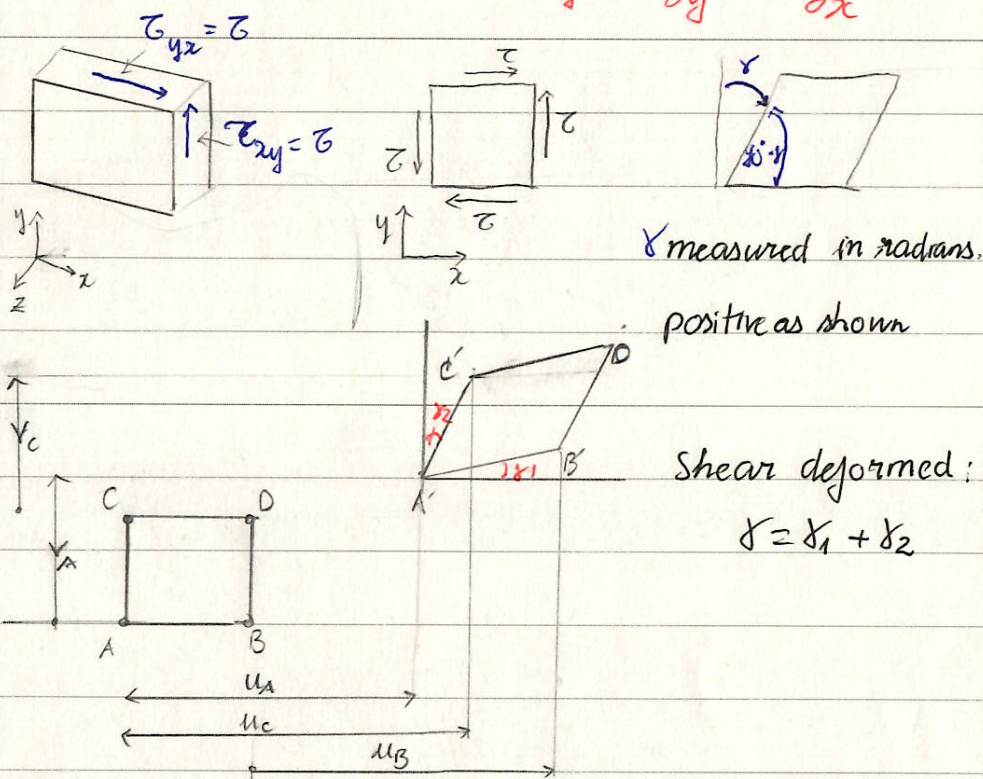
$$\Rightarrow \epsilon_x = \frac{L - L'}{\Delta x} = \frac{\partial u}{\partial x} + O(\Delta x) \approx \frac{\partial u}{\partial x}$$

Similarly: $\epsilon_y = \frac{\partial v}{\partial y}$; $\epsilon_z = \frac{\partial w}{\partial z}$

Normal strain measures changes in length along a specific direction

"Shear strain measures changes in angles with respect to two specific directions as the material deforms in response to shear stress"

Prove : Shear strain: $\gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}$



$$\tan \gamma_1 = \frac{v'_B - v'_A}{u'_B - u'_A} = \frac{\Delta v_{BA}}{\Delta x + \Delta u_{BA}}; \tan \gamma_2 = \frac{u'_C - u'_A}{v'_C - v'_A} = \frac{\Delta u_{CA}}{\Delta y + \Delta v_{CA}}$$

Assume infinitesimal strain

$$\Rightarrow \left\{ \begin{array}{l} \gamma_1 \ll 1; \gamma_2 \ll 1; \tan \gamma_1 \approx \gamma_1; \tan \gamma_2 \approx \gamma_2 \\ \Delta u_{BA} \ll \Delta x \Rightarrow \Delta x + \Delta u_{BA} \approx \Delta x \\ \Delta v_{CA} \ll \Delta y \Rightarrow \Delta y + \Delta v_{CA} \approx \Delta y \end{array} \right.$$

$$\gamma_{xy,av} = \gamma = \gamma_1 + \gamma_2 = \frac{\Delta v_{BA}}{\Delta x} + \frac{\Delta u_{CA}}{\Delta y} = \frac{\partial v}{\partial x} + \frac{\partial u}{\partial y}$$

To define the shear strain γ_{xy} at one point P we pass the limit in the average strain expression by shrinking both dimensions Δx and Δy to zero:

$$\Rightarrow \gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}$$