

CAM CLAY MODEL: NC SOIL

* Associated plastic flow rule

$$\dot{\epsilon}_p = \Delta \frac{\partial f}{\partial \sigma}$$

$\dot{\epsilon}_p$: plastic strain increment tensor

σ : effective stress tensor

Δ : plastic multiplier, > 0 , at the time of loading

- To express this isotropic nature of f_σ in a way that is easier to understand:

$$\left\{ \begin{array}{l} \dot{\epsilon}_v^p = \Delta \frac{\partial f}{\partial p} \\ \dot{\epsilon}_s^p = \Delta \frac{\partial f}{\partial q} \end{array} \right.$$

Volumetric strain at critical state: $\dot{\epsilon}_v^p = 0$ at $q = Mp$

Original Cam clay:

$$\dot{\epsilon}_v^p = 0 \Rightarrow \frac{\partial f}{\partial p} = 0 \Leftrightarrow \left\{ f = \frac{N-T}{v_0} \frac{q}{Mp} + \frac{\lambda-K}{v_0} \ln \frac{p}{p_0} - \epsilon_v^p \right.$$

$$\Leftrightarrow \frac{N-T}{M} \left(\frac{-q}{p^2} \right) + (\lambda-K) \frac{1}{p} = 0$$

$$\Leftrightarrow N-T = \boxed{M \frac{p}{q}} (\lambda-K)$$

$\frac{p}{q} \rightarrow 1$

$$\Leftrightarrow N-T = \lambda - K$$

Modify Cam clay:

$$\dot{\epsilon}_v^p = 0 \Leftrightarrow \frac{\partial f}{\partial p} = 0 \left\{ f = \frac{1}{v_0} \left[\frac{N-T}{\ln 2} \ln \left\{ 1 + \left(\frac{q}{Mp} \right)^2 \right\} + (\lambda-K) \ln \frac{p}{p_0} \right] - \epsilon_v^p \right.$$

$$\Leftrightarrow \frac{N-T}{\ln 2} \frac{1}{1 + \left(\frac{q}{Mp} \right)^2} 2 \frac{q}{Mp} \frac{-q}{Mp} \frac{1}{p} + (\lambda-K) \frac{1}{p} = 0$$

$$\Leftrightarrow N-T = \ln 2 (\lambda-K)$$