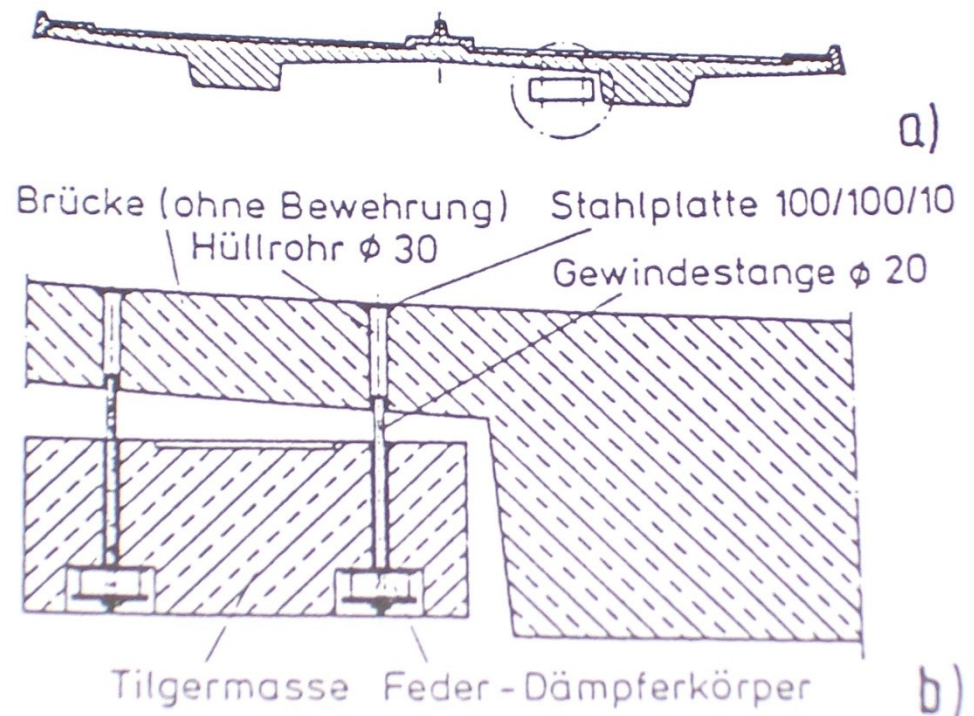


Tuned Mass Dampers

a mass that is connected to a structure by a spring and a damping element without any other support, in order to reduce vibration of the structure.



Tuned mass dampers are mainly used in the following applications:

tall and slender free-standing structures (bridges, pylons of bridges, chimneys, TV towers) which tend to be excited dangerously in one of their mode shapes by wind,



stairs, spectator stands, pedestrian bridges excited by marching or jumping people. These vibrations are usually not dangerous for the structure itself, but may become very unpleasant for the people,



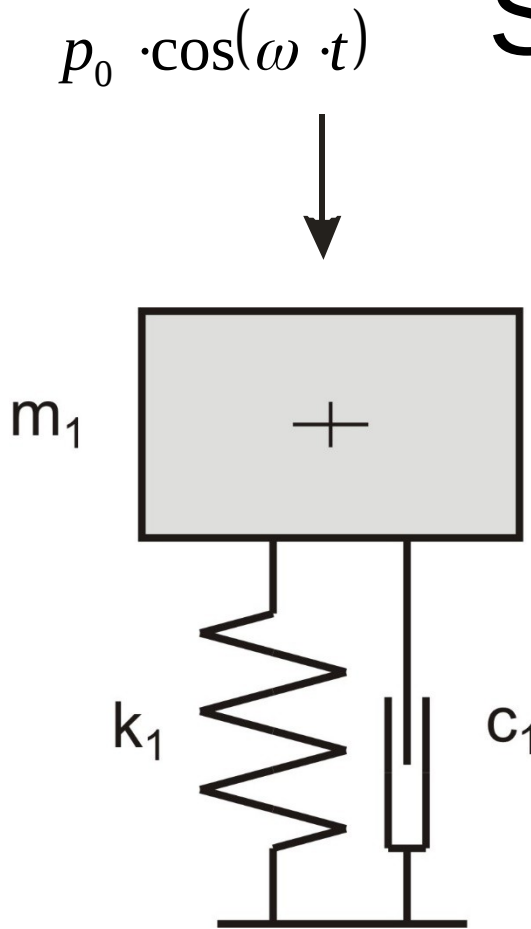
steel structures like factory floors excited in one of their natural frequencies by machines , such as screens, centrifuges, fans etc.,



ships exited in one of their natural frequencies by the main engines or even by ship motion.



SDOF System



$$u_1 = \frac{p_0}{k_1} \cdot \frac{1}{\sqrt{(1 - \beta^2)^2 + (2 \cdot \xi \cdot \beta)^2}} \cdot \sin(\omega \cdot t + \alpha)$$

eigenfrequency:

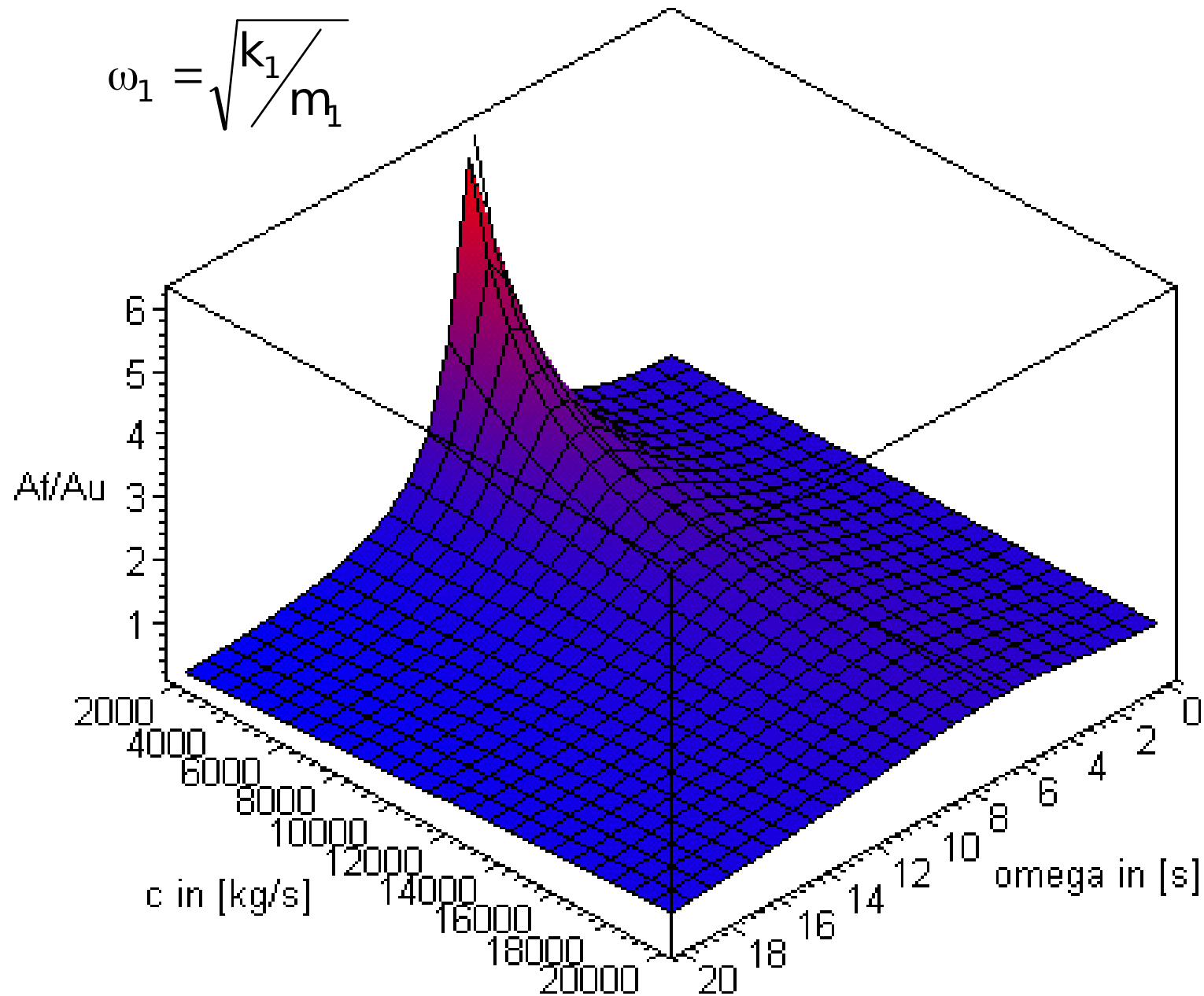
$$\omega_1 = \sqrt{\frac{k_1}{m_1}}$$

damping ratio of Lehr:

$$\xi = \frac{c_1}{2 \cdot m_1 \cdot \omega_1}$$

Amplitudenverhältnis in Abhängigkeit von Frequenz und Dämpfung

$$\omega_1 = \sqrt{k_1 / m_1}$$



- Thin structures with low damping have a high peak in their amplification if the frequency of excitation is similar to eigenfrequency
- → High dynamic forces and deformations

Solutions:

- **Strengthen the structure to get a higher eigenfrequency**
- **Application of dampers**
- **Application of tuned mass dampers**

- **Strengthen the structure to get a higher eigenfrequency**

Eigenfrequency of a beam: $f_1 = \frac{\pi^2}{2} \cdot \frac{\pi}{L^2} \sqrt{\frac{EI}{m}}$

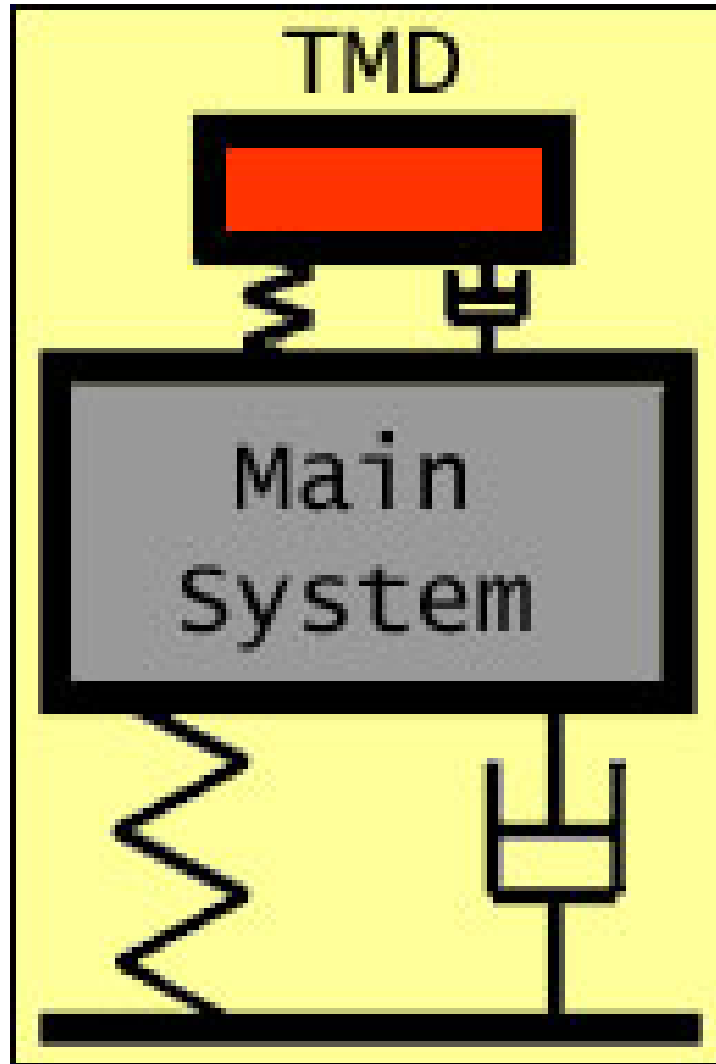
Doubling the stiffness only leads to multiplication of the eigenfrequency by about 1.4.

Most dangerous eigenfrequencies for human excitation: 1.8 - 2.4 Hz

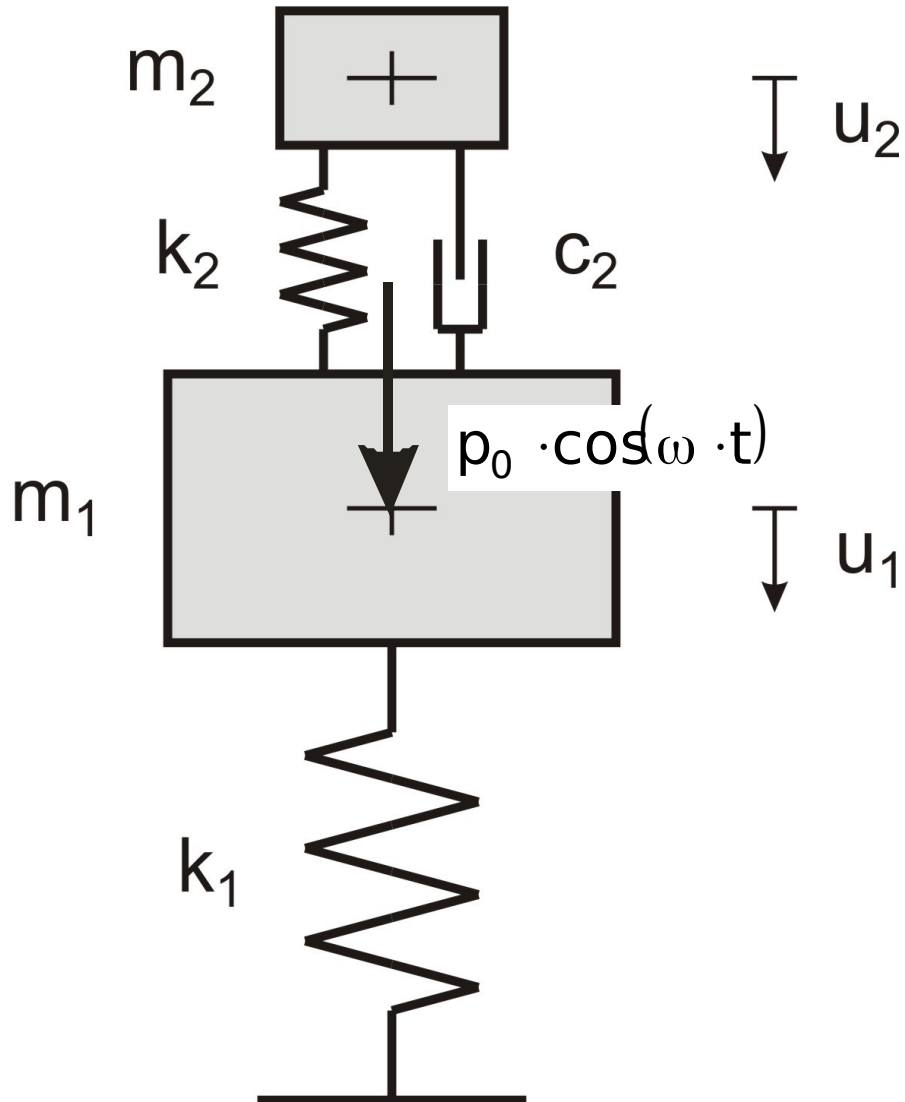
•Application of dampers



- **Application of tuned mass dampers**



2 DOF System



differential equations:

$$m_1 \cdot \ddot{u}_1 + k_1 \cdot u_1 + k_2 \cdot (u_1 - u_2) + c_2 \cdot (\dot{u}_1 - \dot{u}_2) = p_0 \cdot \cos(\omega \cdot t)$$

$$m_2 \cdot \ddot{u}_2 + k_2 \cdot (u_2 - u_1) + c_2 \cdot (\dot{u}_2 - \dot{u}_1) = 0$$

solution:

$$u_1 = C_1 \cdot \cos(\omega \cdot t) + C_2 \cdot \sin(\omega \cdot t)$$

$$u_2 = C_3 \cdot \cos(\omega \cdot t) + C_4 \cdot \sin(\omega \cdot t)$$

$$u_{1,\max} = \sqrt{C_1^2 + C_2^2}$$

$$u_{2,\max} = \sqrt{C_3^2 + C_4^2}$$

linear equation system by derivation of the solution
and application to the differential equations:

$$C_1(-m_1 \omega^2 + k_1 + k_2) + C_2(c \omega) + C_3(-k_2) + C_4(-c \omega) = p_0$$

$$C_1(-c \omega) + C_2(-m_1 \omega^2 + k_1 + k_2) + C_3(c \omega) + C_4(-k_2) = 0$$

$$C_1(-k_2) + C_2(-c \omega) + C_3(-m_2 \omega^2 + k_2) + C_4(c \omega) = 0$$

$$C_1(c \omega) + C_2(-k_2) + C_3(-c \omega) + C_4(-m_2 \omega^2 + k_2) = 0$$

$$C := [p_0 (m_1 \omega^2 + k_1 + k_2) \quad -p_0 c \omega \quad -p_0 k_2 \quad p_0 c \omega]$$

$$\frac{u_{1,\max}}{u_{1,\text{stat}}} = \sqrt{\frac{4 \cdot \xi^2 \cdot \beta^2 + (\beta^2 - \alpha^2)^2}{4 \cdot \xi^2 \cdot \beta^2 (\beta^2 - 1 + \mu \cdot \beta^2)^2 [\mu \cdot \alpha^2 \cdot \beta^2 - (\beta^2 - 1) \cdot (\beta^2 - \alpha^2)]^2}}$$

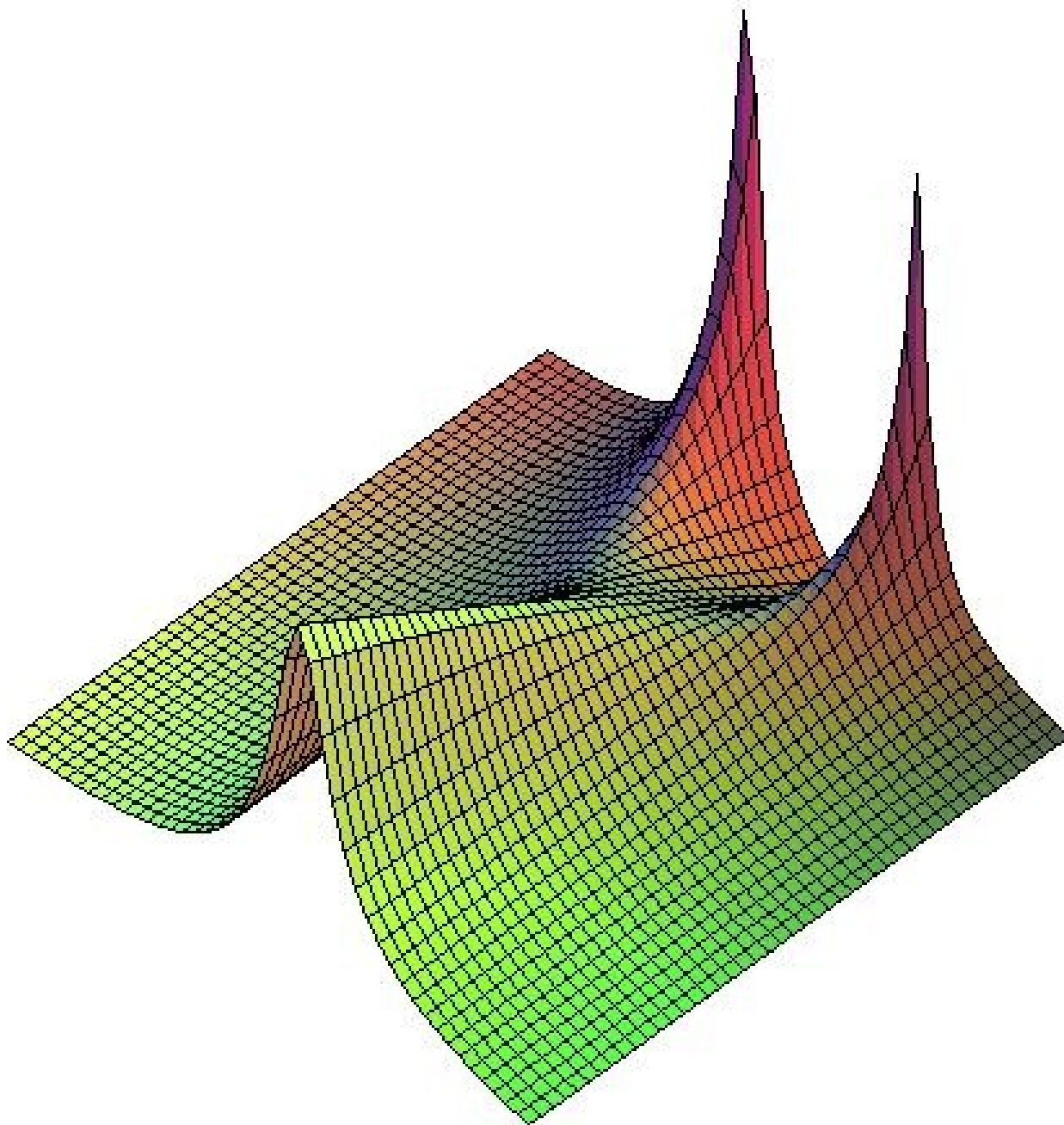
static deformation: $u_{1,\text{stat}} = \frac{p_0}{k_1}$

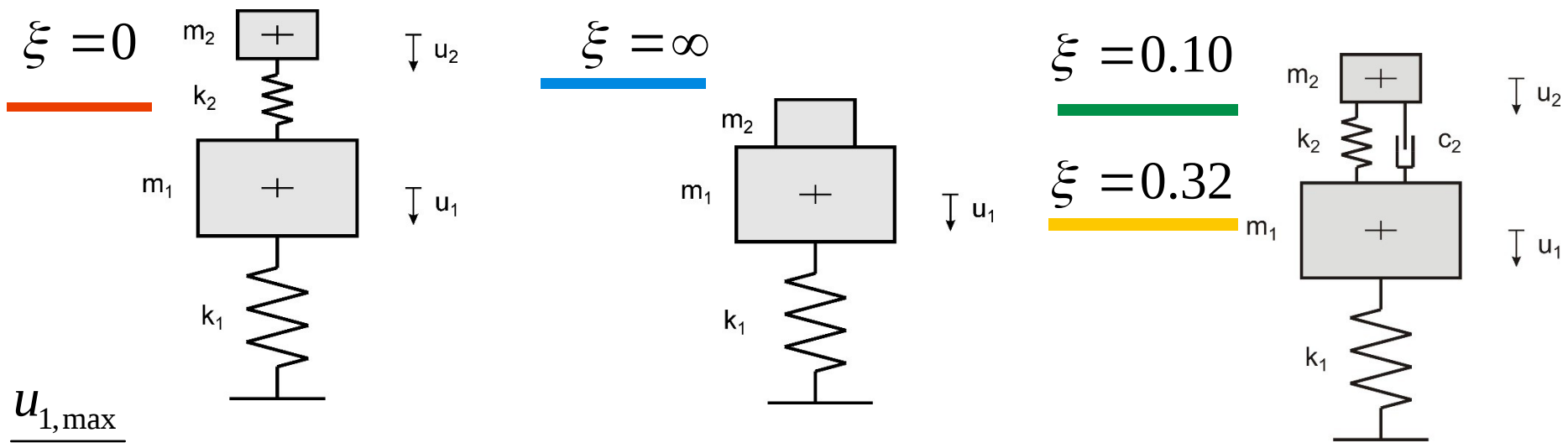
eigenfrequencies: $\omega_1 = \sqrt{\frac{k_1}{m_1}} \quad \omega_2 = \sqrt{\frac{k_2}{m_2}}$

ratio of frequencies: $\alpha = \frac{\omega_2}{\omega_1} \quad \beta = \frac{\omega}{\omega_1}$

Damping ratio of Lehr: $\xi = \frac{c_2}{2 \cdot m_2 \cdot \omega_2}$

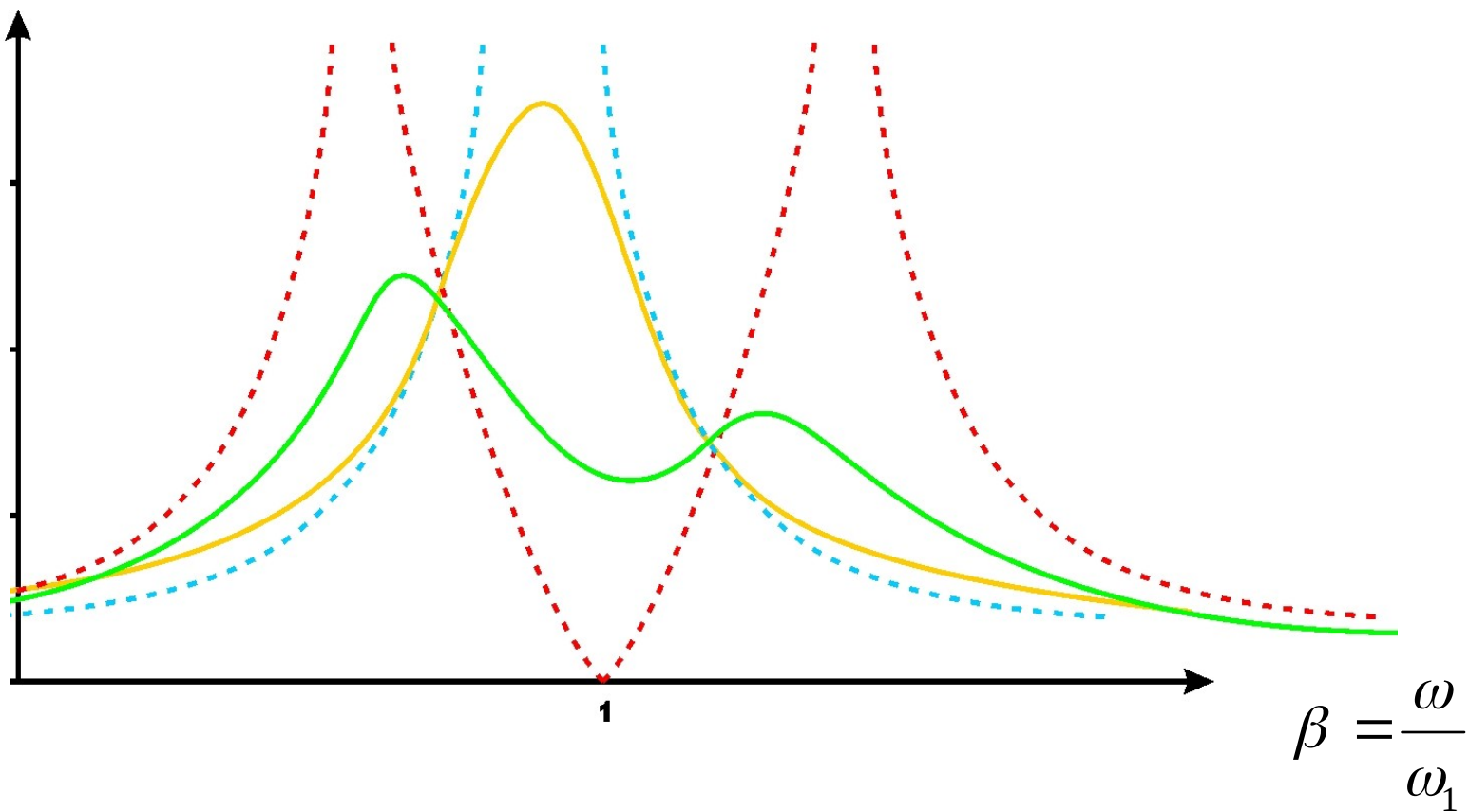
mass ratio: $\mu = \frac{m_2}{m_1}$



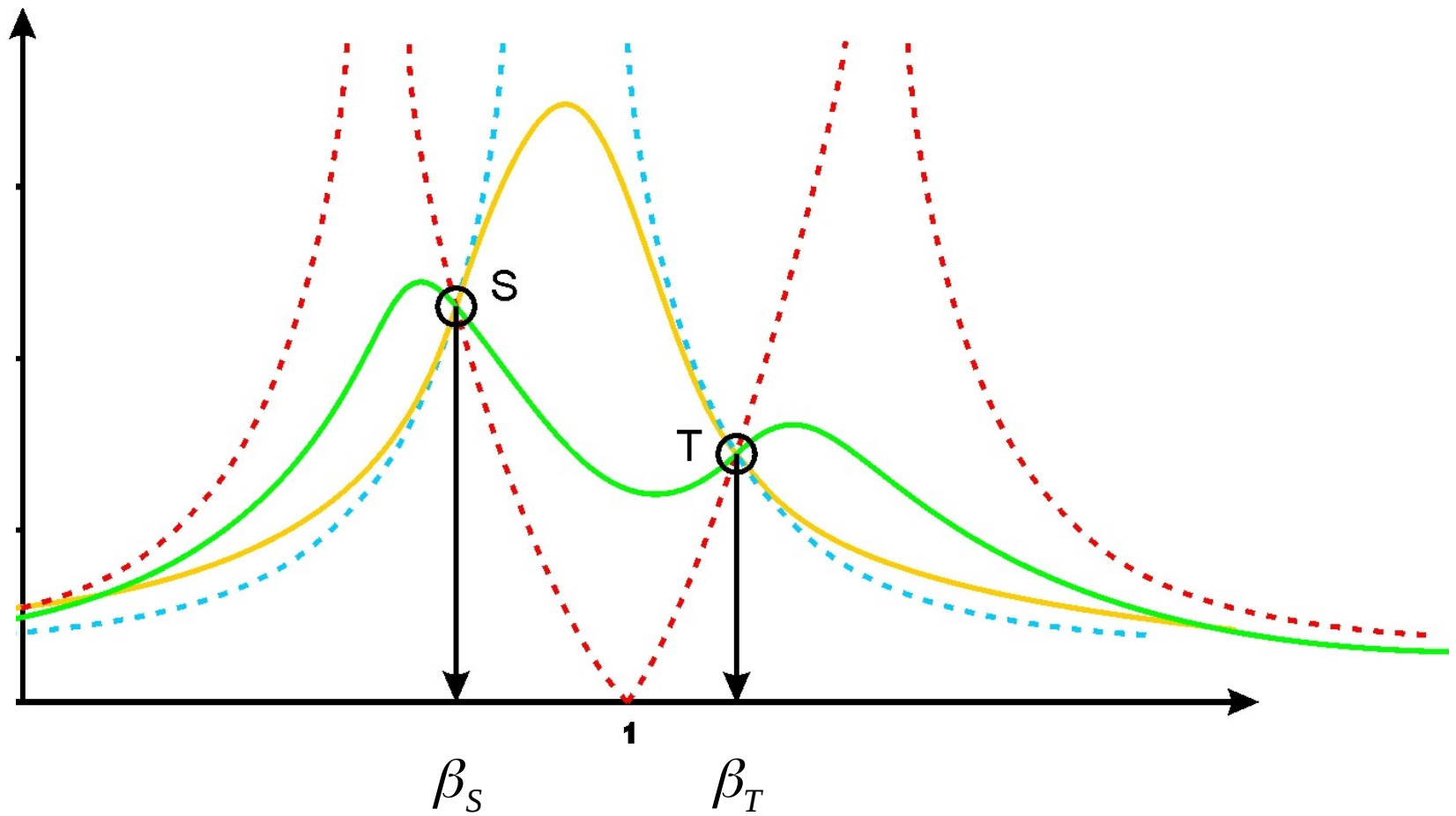


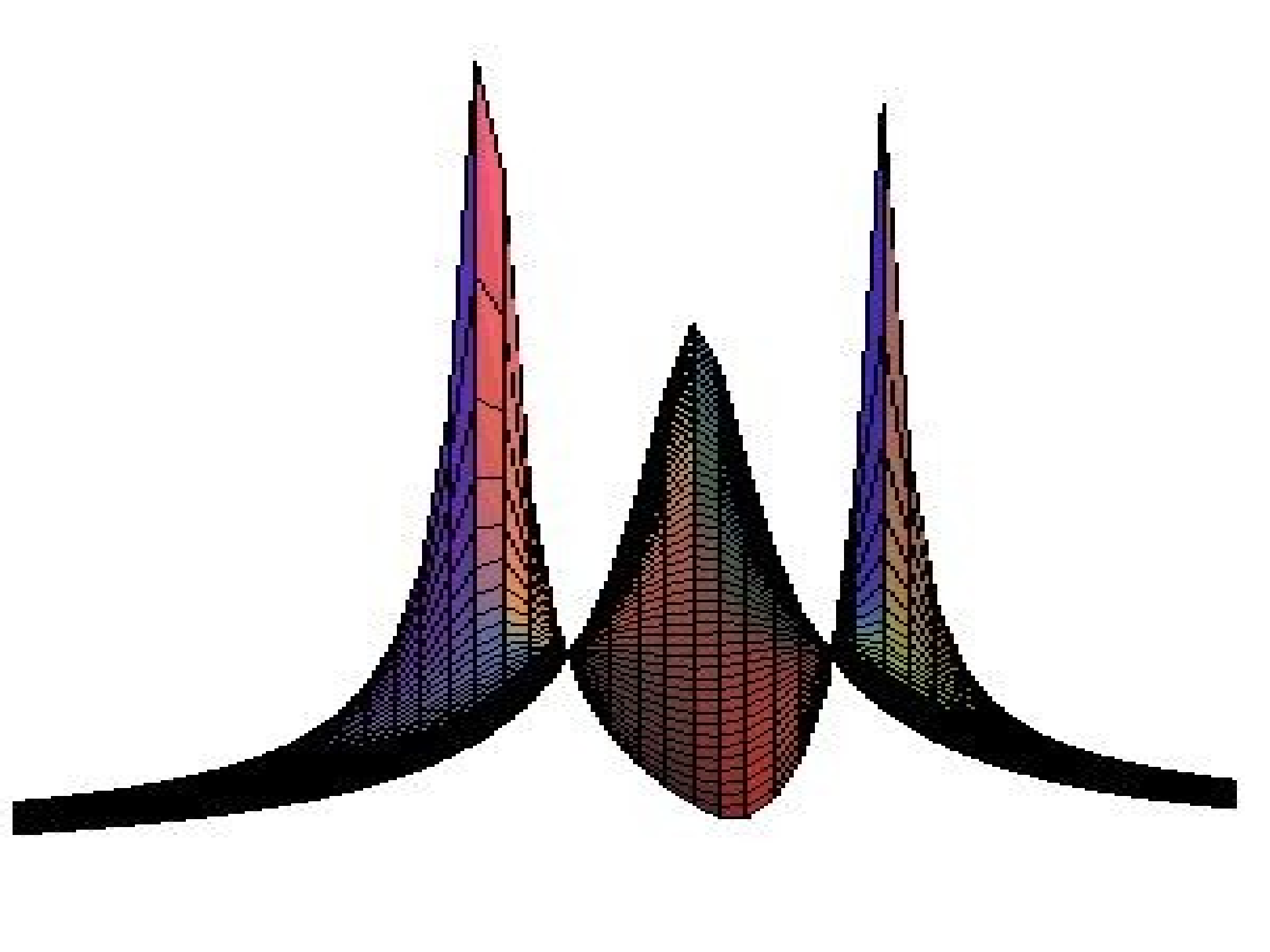
$$\frac{u_{1,\max}}{u_{1,\text{stat}}}$$

$$u_{1,\text{stat}}$$



All lines meet in the points S and T





$$\frac{u_1(\xi = 0)}{u_{1,stat}} = \frac{u_1(\xi = \infty)}{u_{1,stat}} \rightarrow \frac{\beta^2 - \alpha^2}{\mu \alpha^2 \beta^2 - (\beta^2 - 1)(\beta^2 - \alpha^2)} = \frac{1}{\beta^2(1 + \mu) - 1}$$

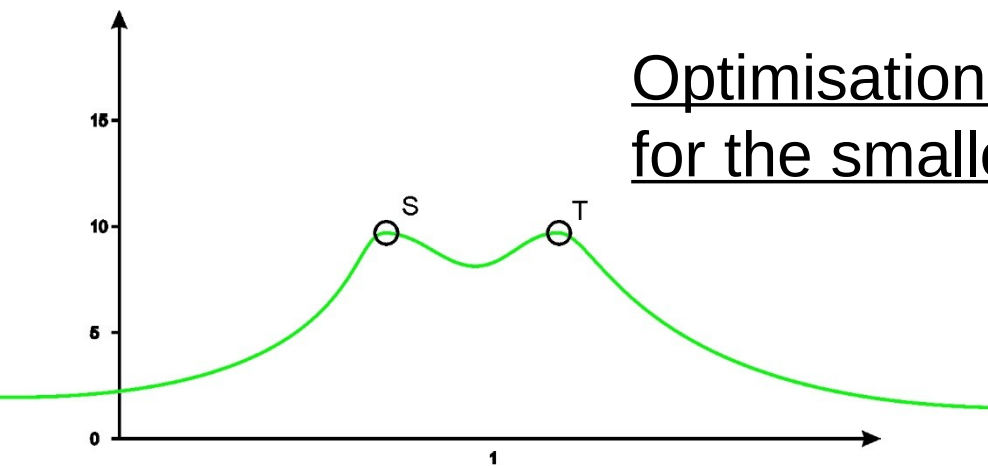
$$\rightarrow \beta^4 - 2\beta^2 \frac{1 + \alpha^2 + \mu \alpha^2}{2 + \mu} + \frac{2\alpha^2}{2 + \mu} = 0$$

$$\rightarrow \beta_{S,T}^2 = \frac{1 + \alpha^2 + \mu \cdot \alpha^2}{2 + \mu} \pm \sqrt{\left[\frac{1 + \alpha^2 \cdot (1 + \mu)}{2 + \mu} \right]^2 - \frac{2 \cdot \alpha^2}{2 + \mu}}$$

$$\text{in} \rightarrow \frac{u_1}{u_{1,stat}}(\xi = \infty) :$$

$$u_{1,S} = \frac{u_{1,stat}}{\beta_S^2 - 1 + \mu \cdot \beta_S^2}$$

$$u_{1,T} = \frac{u_{1,stat}}{\beta_T^2 - 1 + \mu \cdot \beta_T^2}$$



Optimisation of TMD

for the smallest deformation: $u_{1,S} = u_{1,T}$

$$\rightarrow \beta_S^2 + \beta_T^2 = \frac{2}{1 + \mu}$$

$$\beta_S^2 + \beta_T^2 = 2 \frac{1 + \alpha^2 + \mu \alpha^2}{2 + \mu}$$

Optimal ratio of
eigenfrequencies:

$$\alpha = \frac{1}{1 + \mu}$$

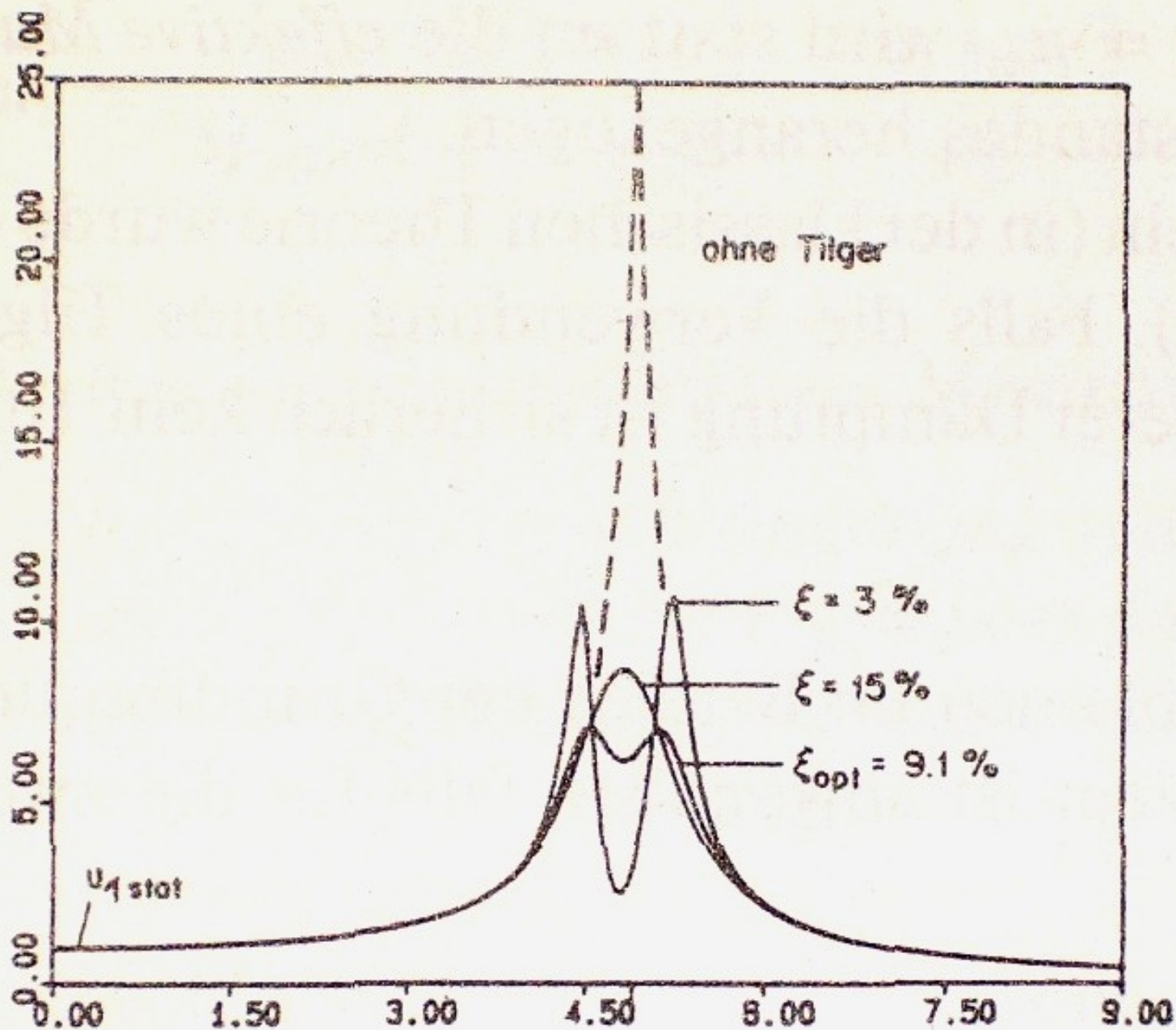
→ Optimal
spring constant k_2

Minimize $u_{1,S} = u_{1,T} = u_{1,stat} \cdot \sqrt{\frac{3 \cdot \mu}{8 \cdot (1 + \mu)^3}}$

→ Optimal damping constant:

$$\xi_{opt} = \sqrt{\frac{3 \cdot \mu}{8 \cdot (1 + \mu)^3}}$$

AMPLIFIKATION



Ratio of masses: μ

The higher the mass of the TMD is, the better is the damping.
Useful: from 0.02 (low effect) up to 0.1 (often constructive limit)

Ratio of frequencies: α

0.98 - 0.86

Damping Ratio of Lehr: ξ

0.08 - 0.20

Adjustment:

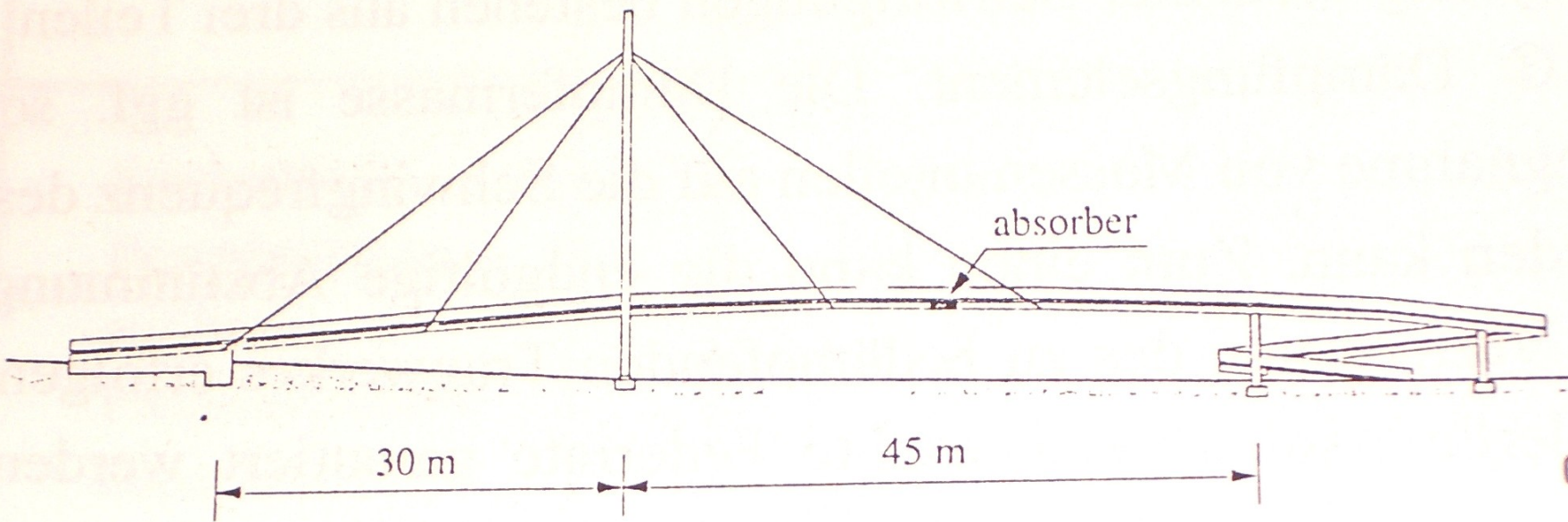
- Different Assumptions of Youngs Modulus and Weights
- Increased Main Mass caused by the load

Problem:

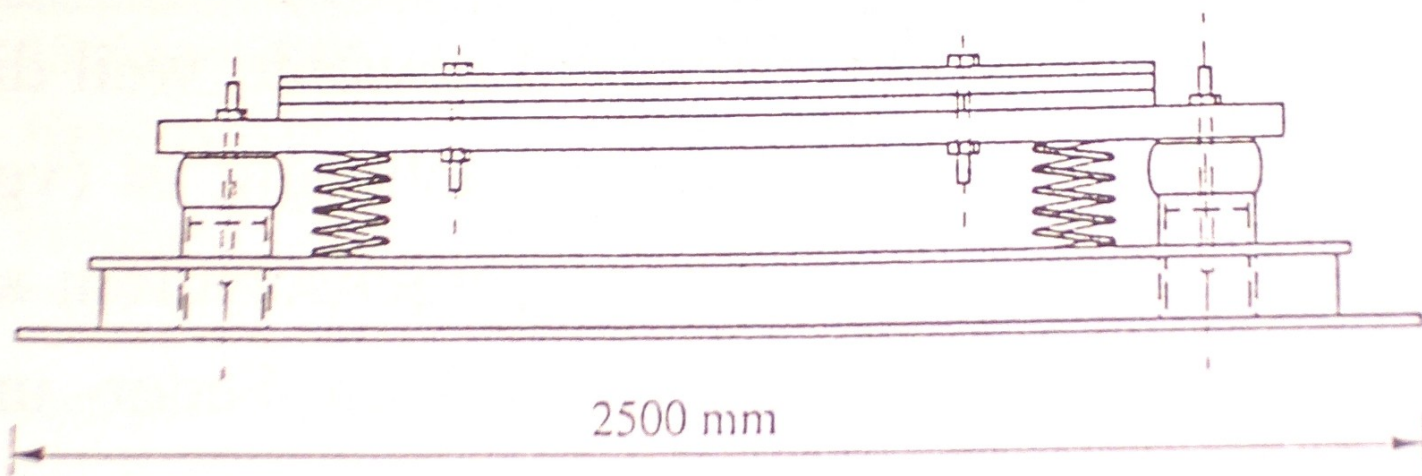
- **Large displacement of the damper mass**
 - Plastic deformation of the spring
 - Exceeding the limit of deformation

Realization

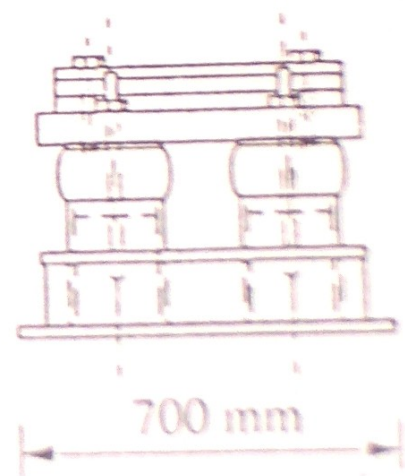




a)

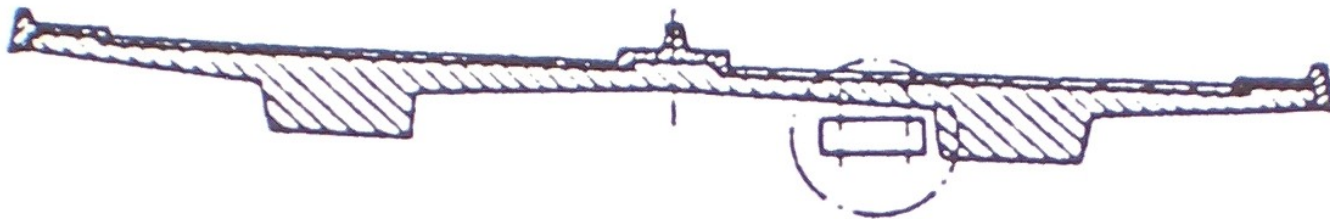


c)



d)

damping of torsional oscillation

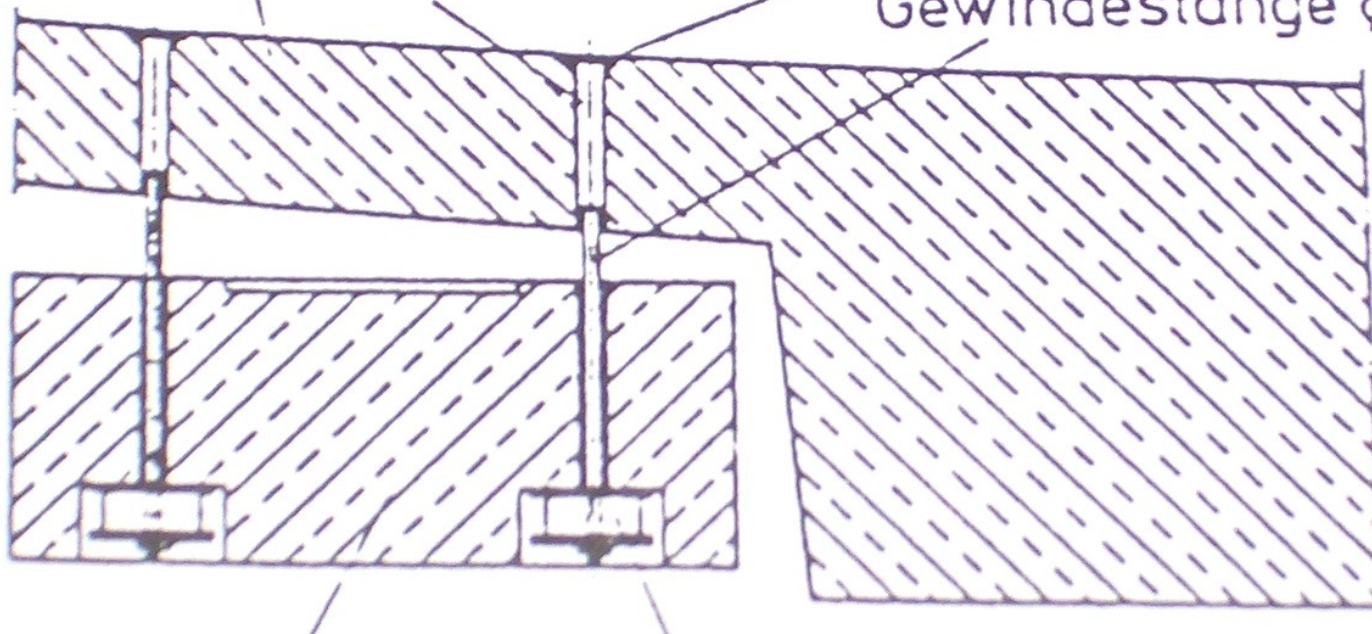


a)

Brücke (ohne Bewehrung) Stahlplatte 100/100/10

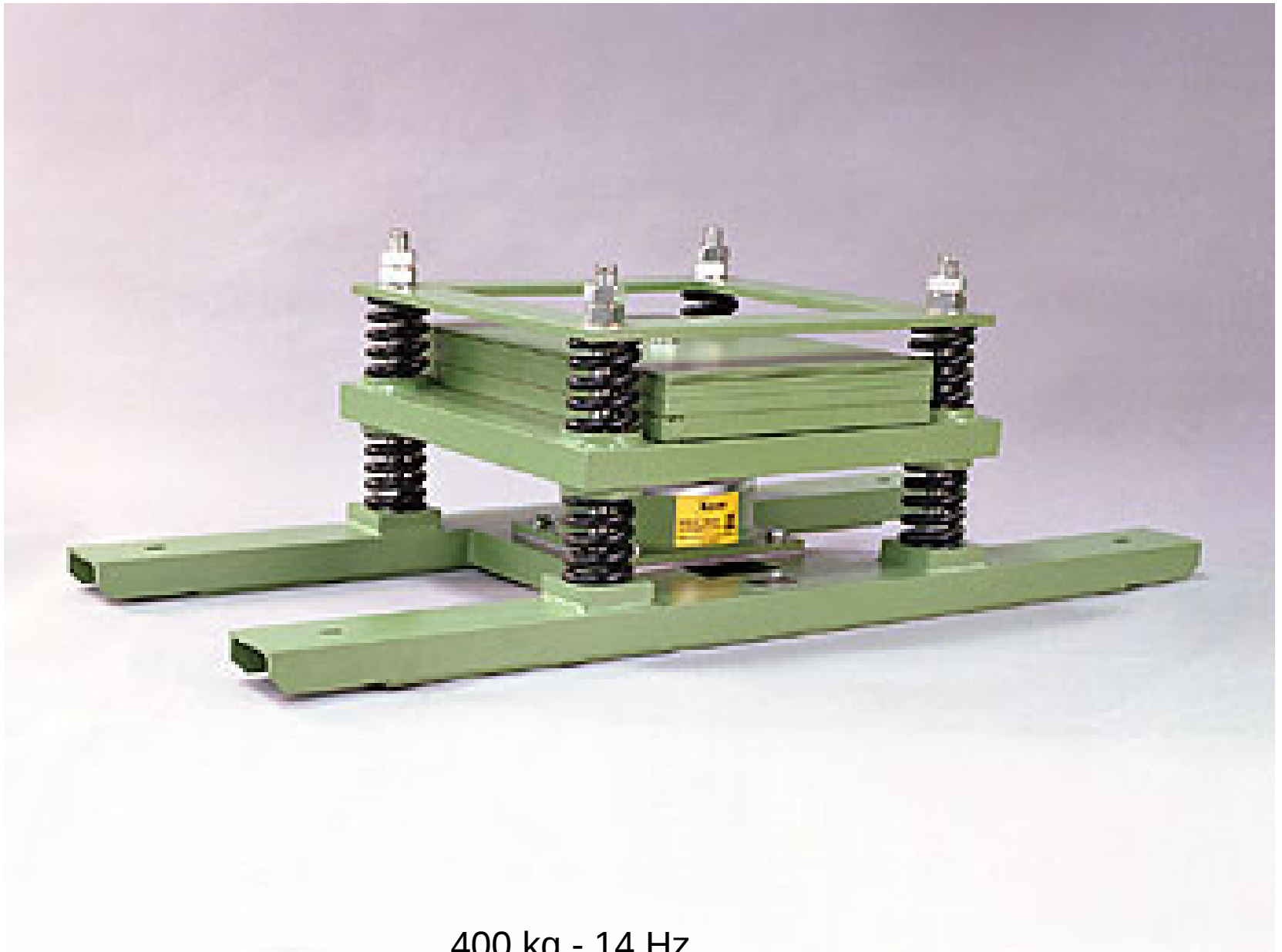
Hüllrohr $\varnothing 30$

Gewindestange $\varnothing 20$

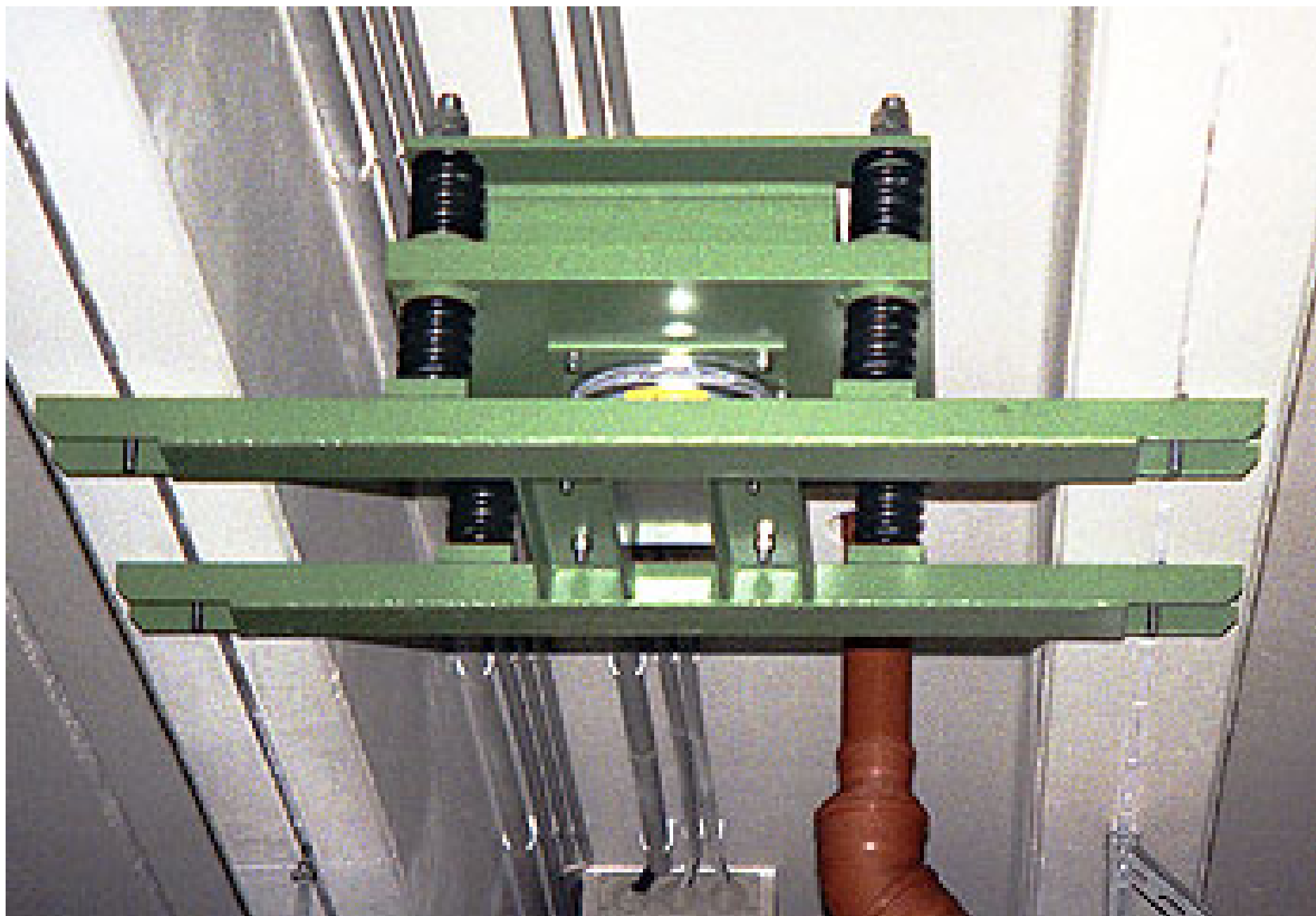


Tilgermasse Feder-Dämpferkörper

b)



400 kg - 14 Hz





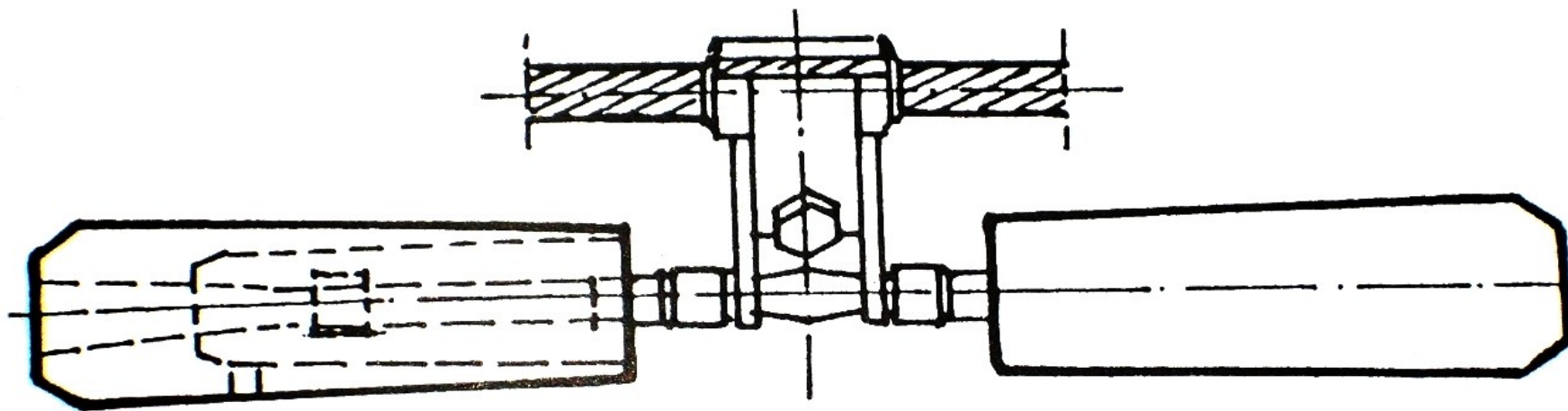
Millennium Bridge



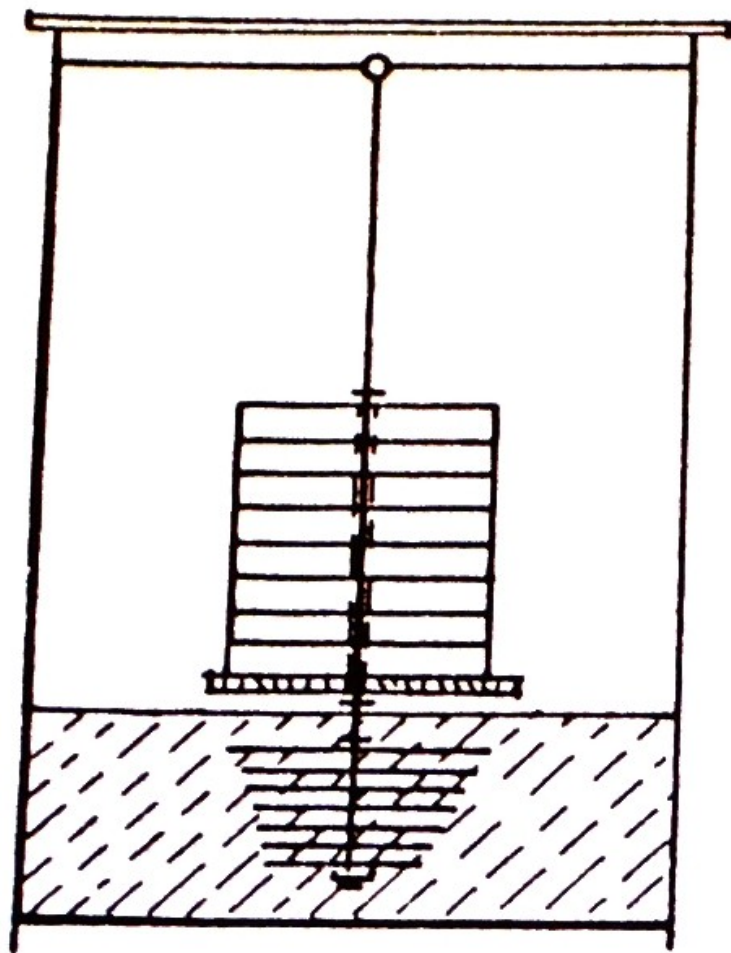




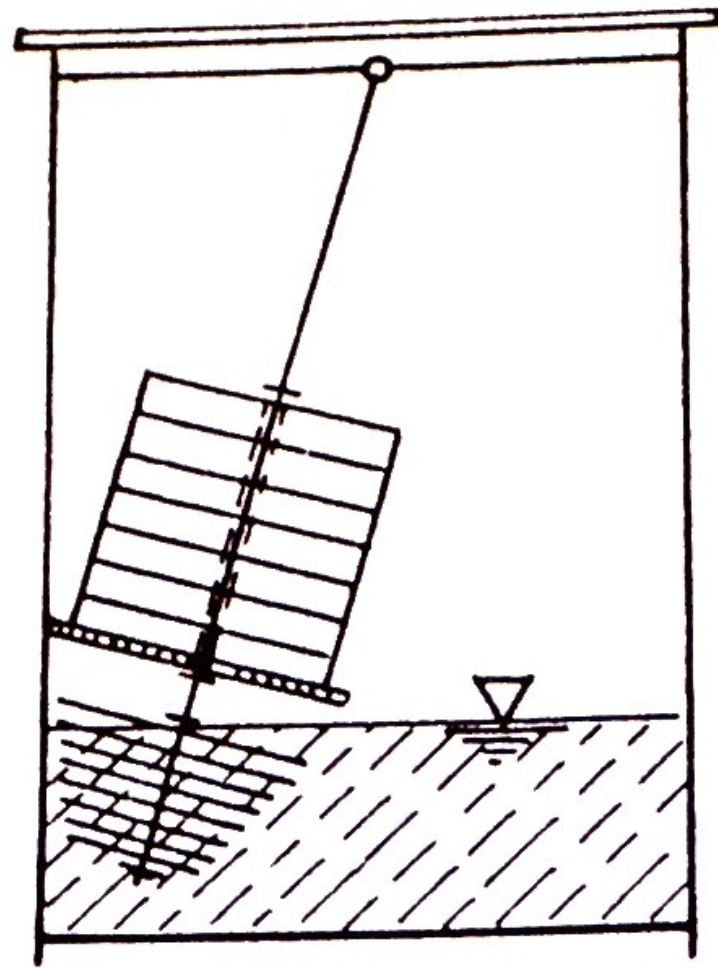
Mass damper on an electricity cable



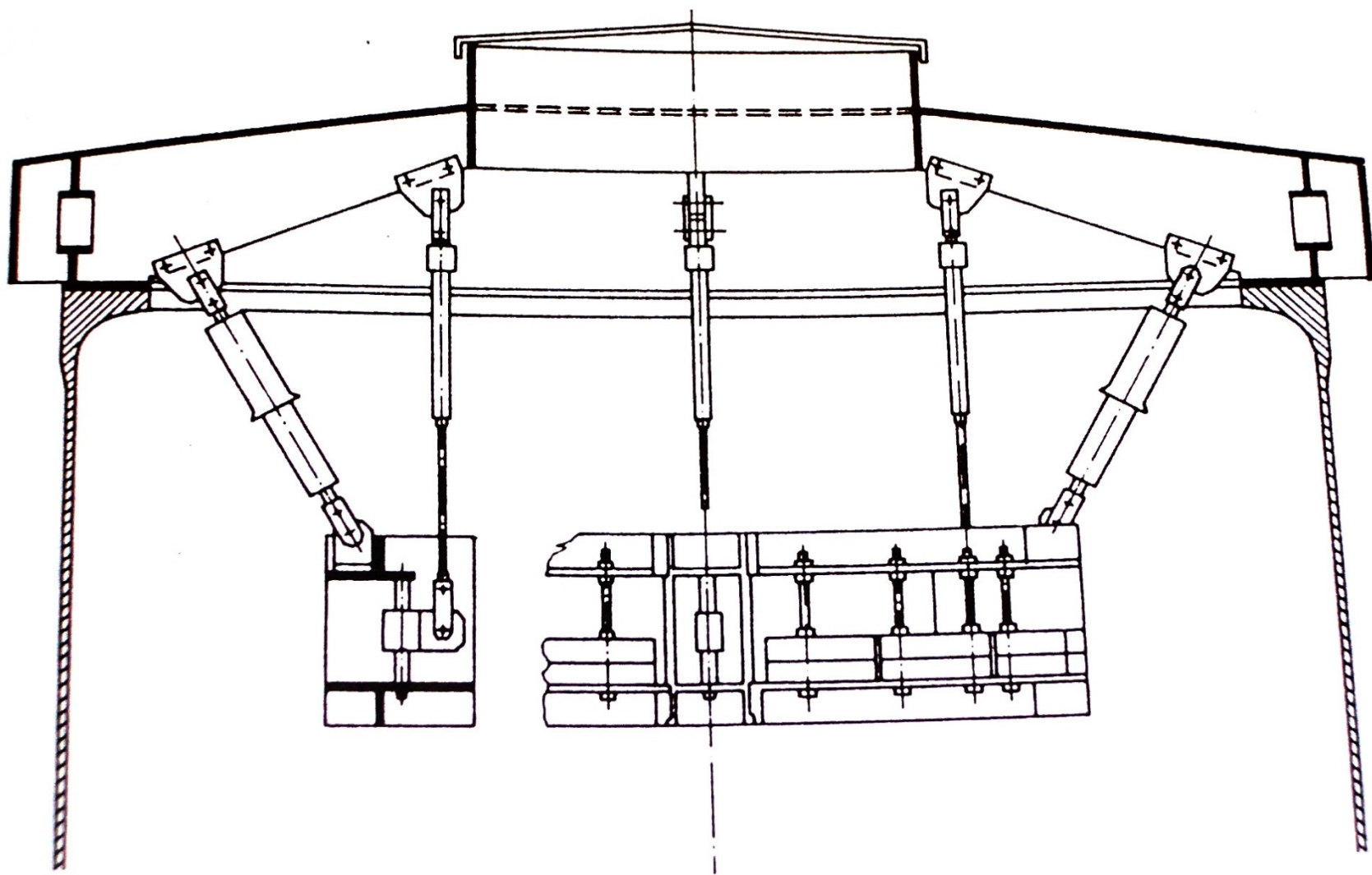
Pendular dampers

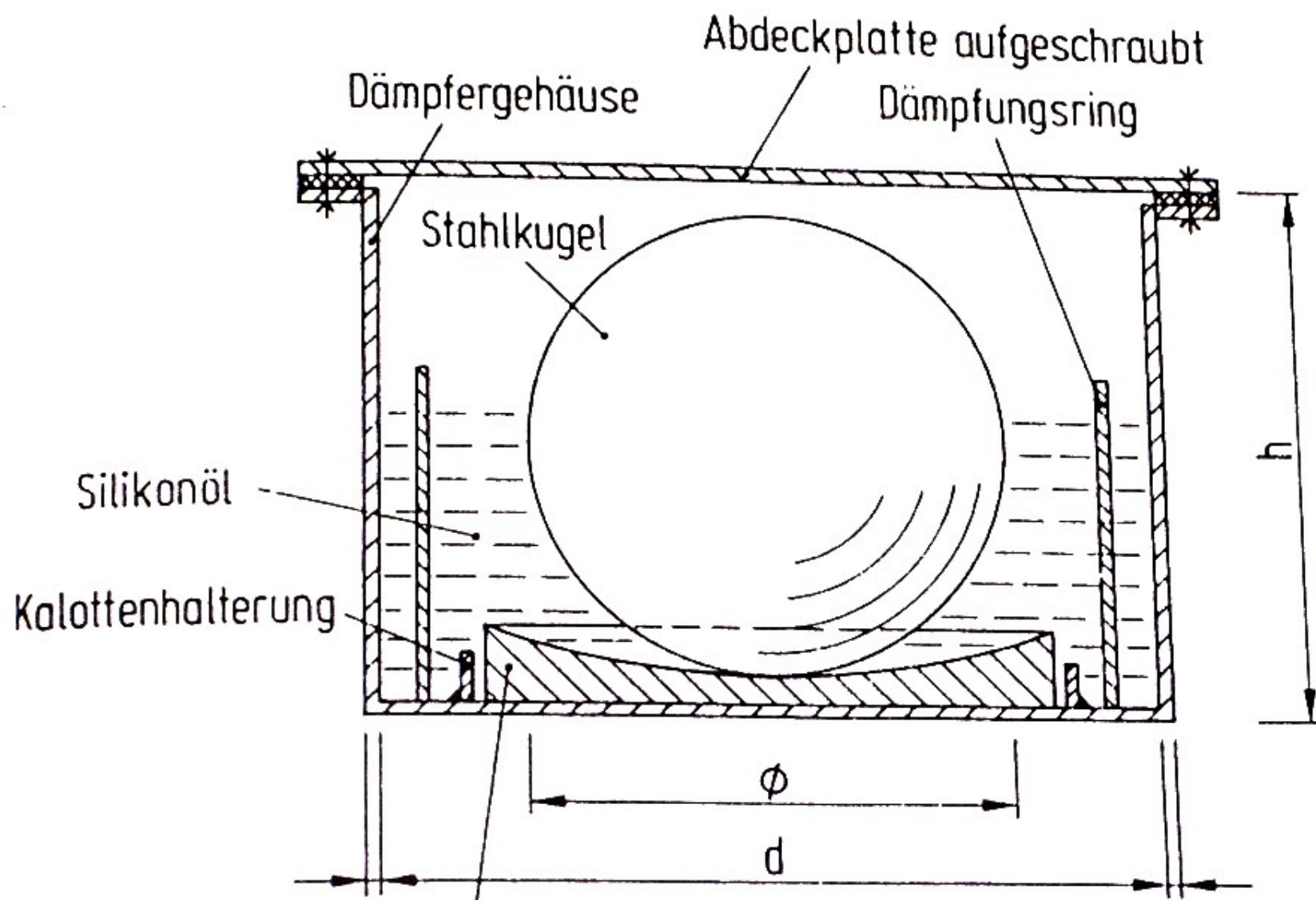


a)



b)





Kalotte als Rotationskörper eines
Kreisbogens oder einer Zykloide