



PROBABILISTIC MODELS FOR PENSTOCK INTEGRITY ASSESSMENT

Philippe BRYLA (EDF Hydro)
Emmanuel ARDILLON (EDF R&D)
Antoine DUMAS (PHIMECA)
Anne DUTFOY (EDF R&D)



PENSTOCK DIAGNOSES AT EDF

- EDF operates more than 450 hydropower plants
- Cumulated length > 250 km
- **Average age > 60 years**
- Loss of thickness due to corrosion
- **Complete diagnoses with penstock assessment** are performed periodically
 - Visual inspections (internal & external) with thickness measurements
 - Evaluation of the residual Margin Factor



PENSTOCK DIAGNOSIS & ASSESSMENT

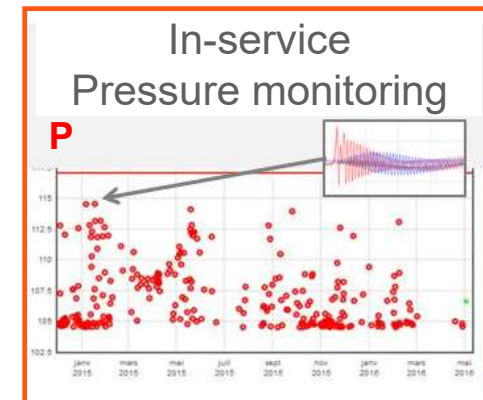
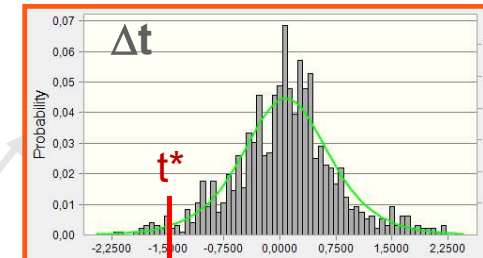
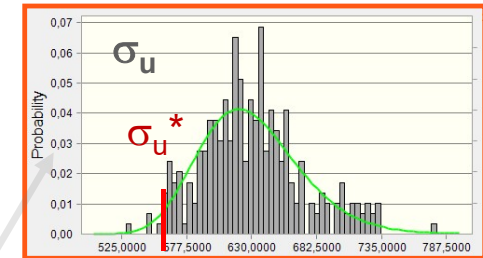
- Fitness for service of a penstock \Leftrightarrow **Margin Factor ≥ 1**

$$MF = \frac{f}{\sigma_c}$$

← Allowable stress
← Maximal in-service stress

- The Margin Factor depends on :

- Steel mechanical characteristics :
Yield Stress σ_y & Ultimate Tensile Strength : σ_u
- Residual Thickness : $t = t_{design} + \Delta t$
- Maximal in-service Pressure **P** (monitored)



PENSTOCK DIAGNOSIS & ASSESSMENT

- Diagnoses data show that σ_u and Δt scatter can be modelled by Normal or Log-Normal distributions
- The Margin Factor is calculated by taking a **calculation values** for Ultimate Tensile Strength σ_u^* and loss of thickness Δt^* at $\gamma=2$ **standard deviations** of their average values
- **Initial issue** : Search of minimal standard deviation multipliers γ such that a Margin Factor ≥ 1 guarantees the annual failure probability to be lower than a given target threshold P_{target}

$$P_{\text{target}} \sim 10^{-7} \text{ to } 10^{-6} \text{ pipe}^{-1}.\text{year}^{-1} \text{ (BS-7910, ISO-2394)}$$

MAIN STEPS OF THE STUDY

- 1st structural reliability model with **plastic collapse** failure criterion for corroded wall outside welded joints
- 2nd structural reliability model with generalized **Fracture Mechanics** failure criterion for welded joints with residual manufacturing flaws
- Extension of both models to pipes with a hydrostatic pressure test before commissioning : evaluation of **conditional failure probabilities**
- Evaluation of the upper bound of annual failure probabilities :
 - Large deterministic calculation grids (2 000 to 14 000) – Latin hypercubes
 - Probabilistic calculation grids
 - In-depth analysis for understanding the most influential factors

PLASTIC COLLAPSE MODEL (OUT OF WELDS) (1/2)

- Model with 4 random variables

Variable	Description	Distribution
σ_u	Ultimate tensile strength (MPa)	Lognormal
ε	Deviation to the general correlation $\sigma_y - \sigma_u$	Normal
Δt_{extra}	Manufacturing extra thickness (mm)	Normal
Δt_{corr}	Thinning due to corrosion (mm)	Normal

$$\sigma_y = A \cdot \sigma_u - B + \varepsilon$$

- Upper bound for annual corrosion rate :

$$\Delta t_{annual} = 100 \mu\text{m} \cdot \text{year}^{-1}$$

- Residual thickness – Year N :

$$t_N = t_{design} + \Delta t_{extra} - \Delta t_{corr}$$

- Residual thickness – Year N+1 :

$$t_{N+1} = t_N - \Delta t_{annual}$$

PLASTIC COLLAPSE MODEL (OUT OF WELDS) (2/2)

- **Failure criterion** : overcrossing of the flow stress σ_f by the hoop stress σ_c in the pipe wall

$$\sigma_c > \sigma_f$$

$$\sigma_f = \min \left(\frac{\sigma_y + \sigma_u}{2}; 0.85 \times \sigma_u \right)$$

- Annual failure probability :

$$P_{\text{annual}}(N) = P(G_N \geq 0 \mid G_{N+1} < 0)$$

$$\Rightarrow P_{\text{annual}}(N) = \frac{P(G_{N+1} < 0 \cap G_N \geq 0)}{1 - P(G_N < 0)}$$

- No failure before year N :

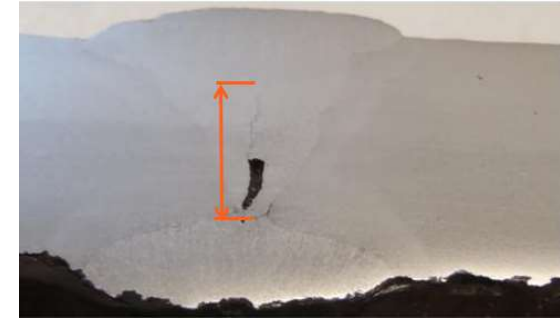
$$G_N = \sigma_f - \sigma_{c,N} > 0$$

- Failure before year N+1 :

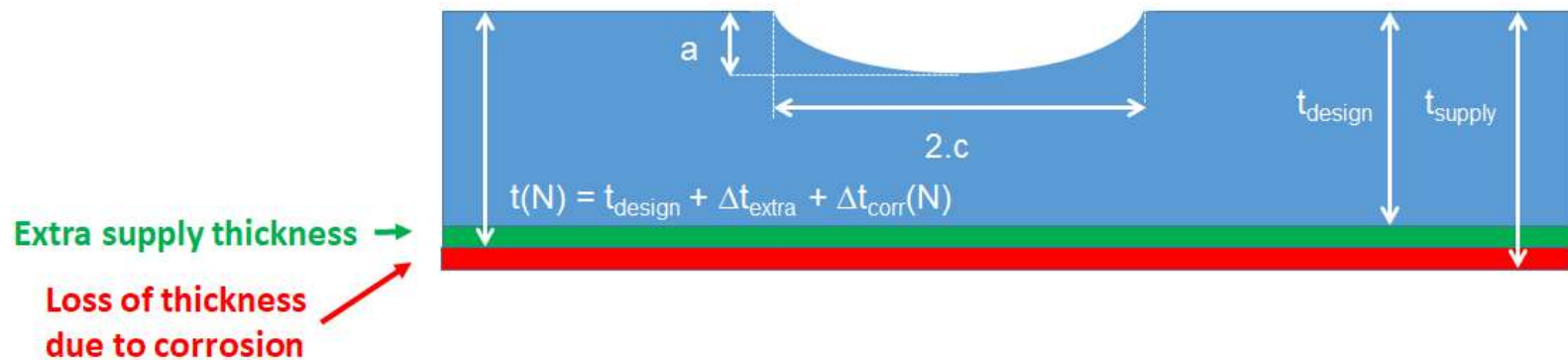
$$G_{N+1} = \sigma_f - \sigma_{c,N+1} < 0$$

FRACTURE MECHANICS MODEL FOR WELDS (1/4)

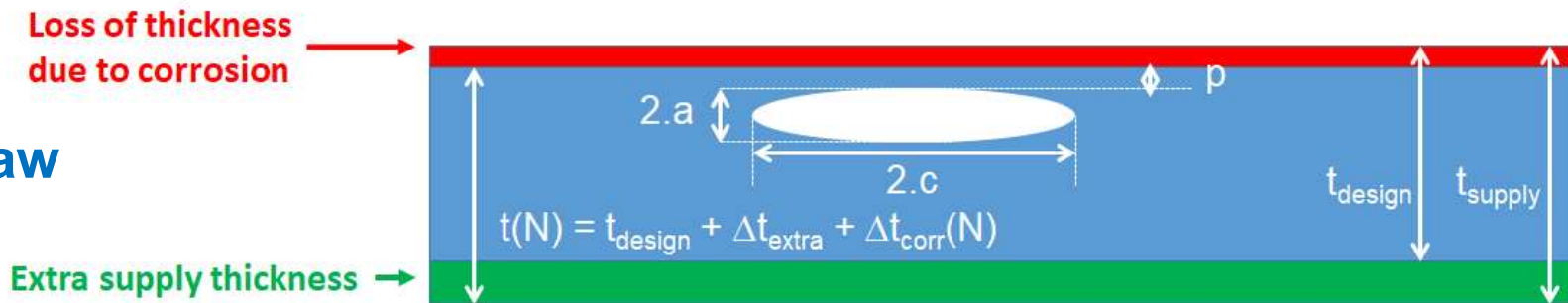
- Potential defects not detected by Non Destructive Testing
- Loss of thickness due to corrosion



Surface flaw



Embedded flaw



FRACTURE MECHANICS MODEL FOR WELDS (2/4)

- Model with 6 random variables

Variable	Description	Distribution
σ_u	Ultimate tensile strength (MPa)	Lognormal
ε	Deviation to the general correlation $\sigma_y - \sigma_u$	Normal
Δt_{extra}	Manufacturing extra thickness (mm)	Normal
Δt_{corr}	Thinning due to corrosion (mm)	Normal
a	Flaw maximum height (mm)	Uniform [0; a_{max}]
K_{IC}	Steel toughness (MPa.m ^{1/2})	Weibull

- + 1 new parameter : Residual stress σ_{res} in welds

FRACTURE MECHANICS MODEL FOR WELDS (3/4)

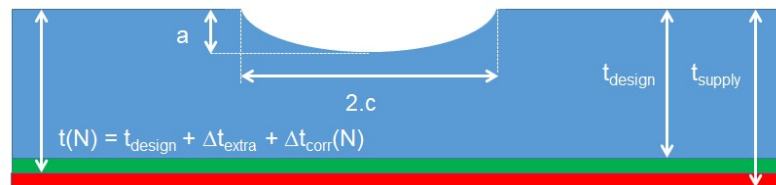
- New parameters depend on the manufacturing & NDT processes

- Residual stress σ_{res} depends on the relief process

- No stress relief (“as welded”) $\sigma_{res} = 0.8 \times \sigma_y$
- Partial mechanical relief $\sigma_{res} = 0.4 \times \sigma_y$
- Post welding Heat Treatment $\sigma_{res} = 0.2 \times \sigma_y$

- Height a_{max} of residual manufacturing flaws depends on the detectability performance of Non Destructive Test (NDT)

- S_1 : Magnetic Particle (MP) on machined welded joint $a_{max} = 1,0 \text{ mm}, 2.c_{max} = 5 \text{ mm}$
- S_2 : Liquid Penetrant (LP) on “as welded” joint $a_{max} = 1,5 \text{ mm}, 2.c_{max} = 10 \text{ mm}$
- S_3 : Poor quality test $a_{max} = 4,0 \text{ mm}, 2.c_{max} = 20 \text{ mm}$



FRACTURE MECHANICS MODEL FOR WELDS (4/4)

- Failure criterion (BS-7910)

$$G = K_R - f(L_R) < 0$$

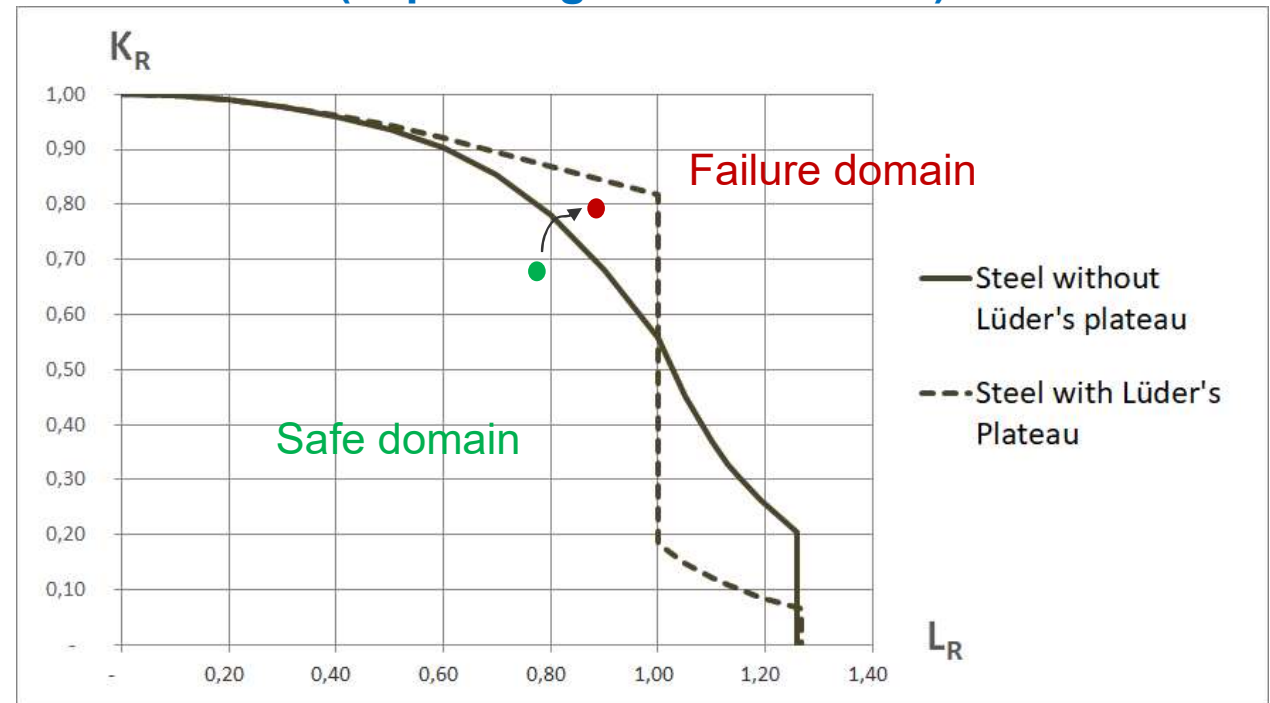
- with :

$$L_R = \frac{\sigma_C}{\sigma_y}$$

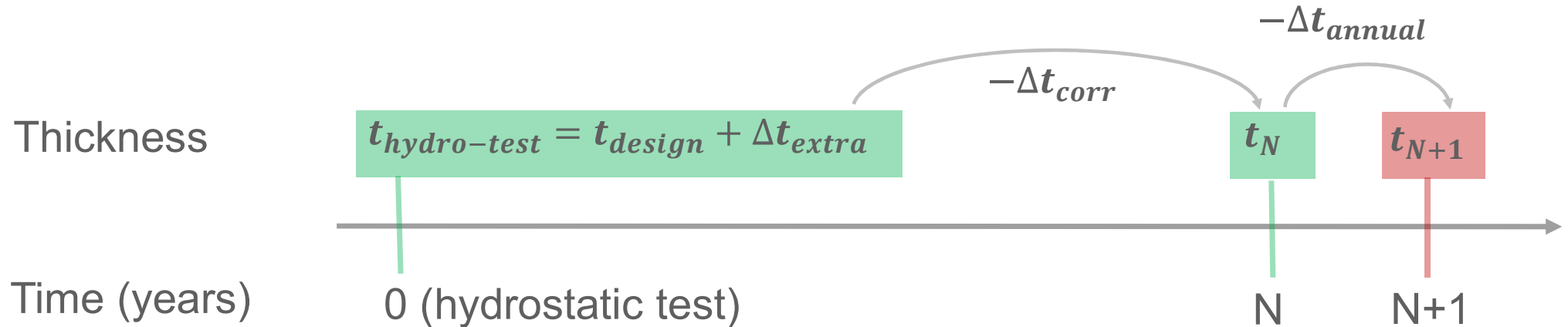
$$K_R = \frac{M \cdot (\sigma_C + \sigma_{rés}) \cdot \sqrt{\pi \cdot a}}{K_{IC}}$$

$$\sigma_C = \frac{f}{MF} \cdot \frac{t^*}{t}$$

Failure Assessment Diagrams
(depending on the material)



CONDITIONAL FAILURE PROBABILITIES KNOWING A SUCCESSFUL HYDROSTATIC TEST



- Conditional annual failure probability :

$$P_{annual-cond}(N) = P(G_{N+1} < 0 \mid G_N \geq 0 \cap G_{hydro-test} \geq 0)$$

$$\Rightarrow P_{annual-cond}(N) = \frac{P(G_{N+1} < 0 \cap G_N \geq 0 \cap G_{hydro-test} \geq 0)}{P(G_{hydro-test} \geq 0 \cap G_N \geq 0)}$$

PLASTIC COLLAPSE MODEL : DETERMINISTIC DESIGN OF EXPERIMENTS (DoE)

- > 2 000 penstock configurations (Latin hypercube)

Variable	Mean value	Variation Coefficient or standard-deviation
t_{design}	5 mm to 30 mm	-
σ_u	320 to 750 MPa	C.V. : 5% to 10%
ε	-50 to +50 MPa	C.V. : 2% to 5%
Δt_{appro}	0 to 1 mm	S.D. : 0.25 to 0.50 mm
Δt_{corr}	1 to 3 mm	S.D. : 0.25 to 1.00 mm

- In depth analysis of results :
 - Estimation of the upper bound of annual failure probability (for calculation values taken at $\gamma = 2$ or $Q_{2,5\%}$)
 - Identification of the major influent factors

MODEL IMPLEMENTATION (1/2)

- Very large number (thousands) of probability configurations to be performed
- System event probabilities calculated (System reliability)
 - Annual failure probability: **double intersection**
 - Conditional probability (given a successful initial Hydraulic Testing): **triple intersection**
 - For Fracture Mechanics, limit state function is locally non differentiable and can be discontinuous material with Lüder's plateau) \Rightarrow Convergence problems may occur
- This industrial need initiated an evolution in OT (V 1.14)
 - **System event** definition and reliability calculation
 - All the classical reliability methods available for single events have been adapted for **system events** :
 - FORM
 - FORM-IS
 - Subset Simulation
 - Directional Simulation / Adaptive directional Stratification
 - OpenTURNS is now ahead of other UQ tools, for System Events (UQLAB (ETH Zurich) uses only System-FORM)



MODEL IMPLEMENTATION (2/2)

- Implementation of the models in Persalys software
 - **Persalys-Penstock** becomes the 1st dedicated tool based on Persalys
- Adaptation of reliability methods developed in Persalys
 - Subset Simulation
 - Directional Simulation
 - Adaptive Stratified Sampling
- Development of specific methods, not yet included in OpenTURNS
 - Multi-constraint-FORM-IS : necessary for conditional probability calculation
 - FORM-IS-Best Algorithm : iterative selection of a satisfactory optimization algorithm (6 possibilities)
 - **Persalys-Penstock** includes enhanced reliability methods (not yet in OT or in Persalys)
- Comparative performance analysis of the methods is underway (R&D/PRISME)
- Treatment of large size DoEs ($>> 1\ 000$) is possible :
 - Optimized parallel computing
 - Large reduction of the computational time (CALIBRE: ratio 4,5)
- Probabilistic DoEs are also possible



EVENTS INTERSECTION PROBABILITY

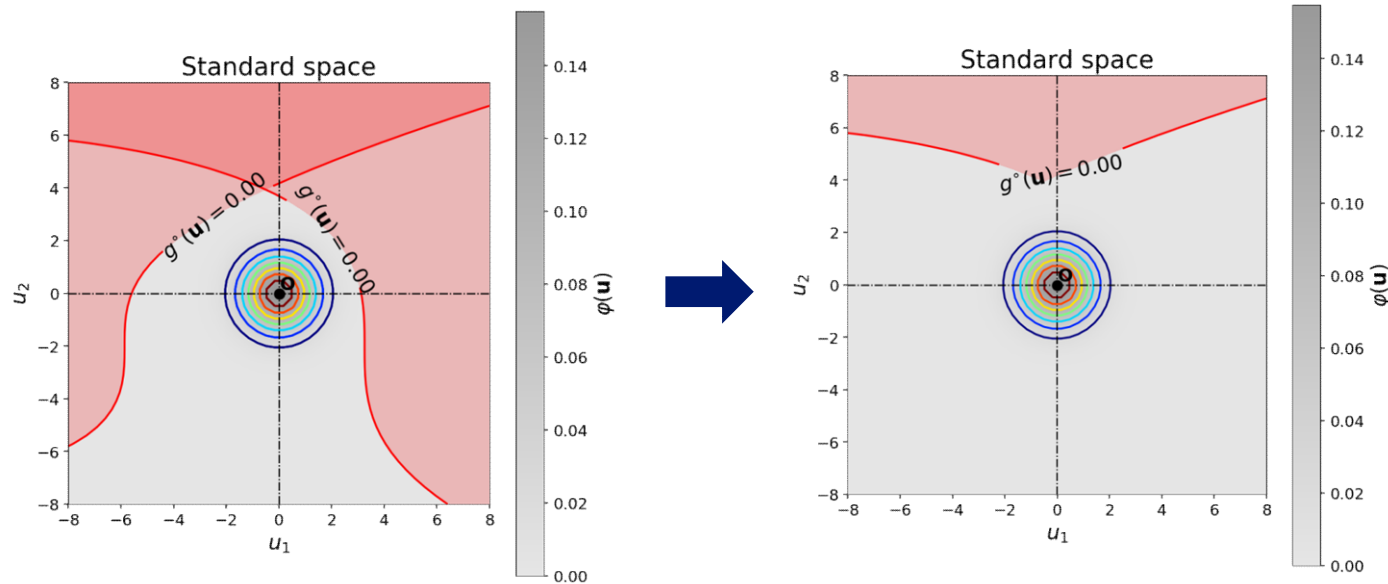
- Failure probability :

$$p_f = P \left(\bigcap_{i=0}^N g_i(\mathbf{X}) < 0 \right)$$

g_i = limit state functions

- Example in dimension with 2 limit state functions :

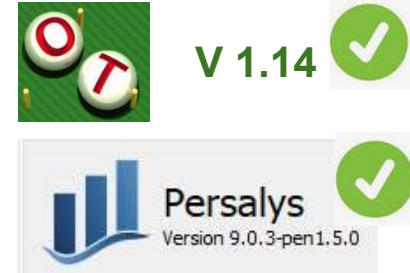
- $p_{f, Monte Carlo} = 1,24 \times 10^{-5}$ (818 000 samples, coefficient of variation = 10%)



INTERSECTION PROBABILITY EVALUATION (1/3)

■ General approach : FORM + Importance Sampling

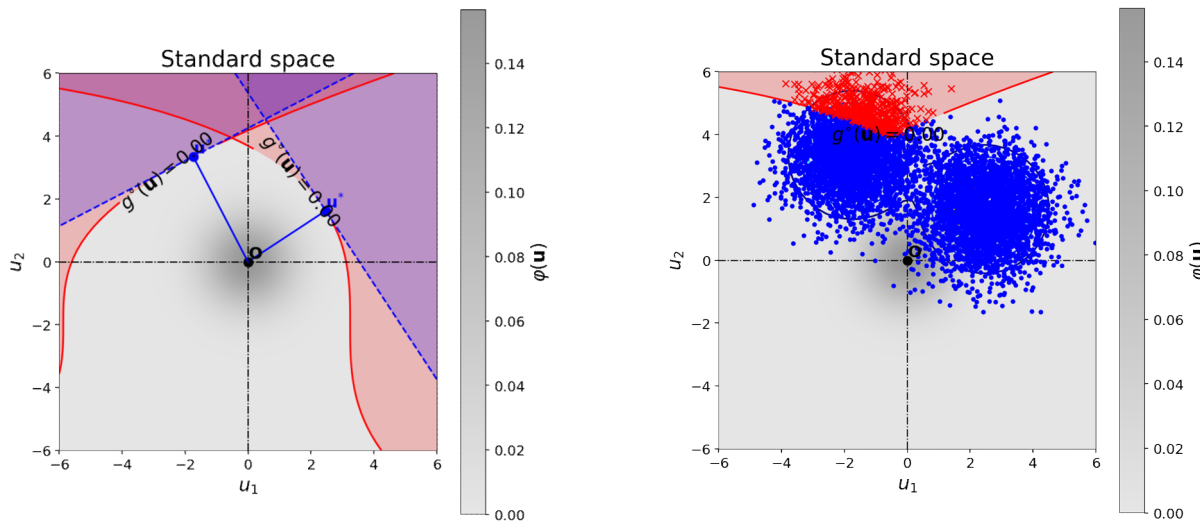
1. Classical method : System-FORM



- FORM on each limit state function + sampling around the N design points U^* (instrumental density = mixture of the N densities centered around the U^* points)

- **FORM** : $p_{f,FORM\ systeme} = P(\cap_{i=0}^N g_i(\mathbf{X}) < 0) = \Phi_N(-\boldsymbol{\beta}, [\rho]) = 4,21 \times 10^{-7}$

- **FORM-IS** : $P_{f,IS} = 1,42 \times 10^{-5}$ (6 530 samples, CoV = 10%)



Problem: depending on the relative position of the limit-state surfaces, the sampled values may not be centered around the right point

⇒ Poor computational efficiency

INTERSECTION PROBABILITY EVALUATION (2/3)

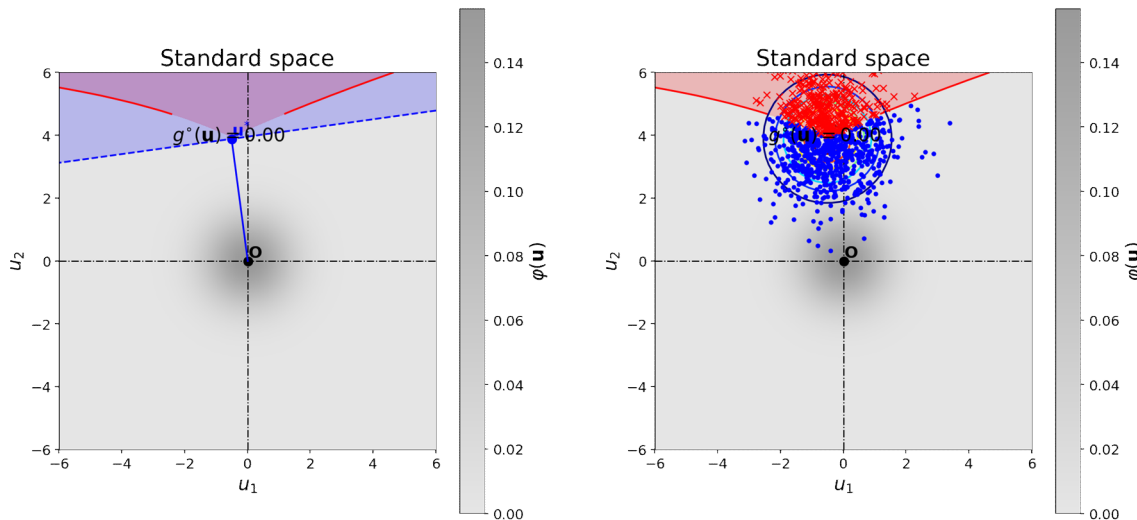
■ General approach : FORM + Importance Sampling

2. Multi-constraints FORM

- Multi-constraint optimization problem
- 4 optimization algorithms issued from NLOPT

$$\begin{aligned} & \text{Min} \sum_i u_i^2 \\ & \text{s. t. } \{g_i(U) = 0\}_{i=1,\dots,N} \end{aligned}$$

- **FORM-IS** : $P_{f,IS} = 1,42 \times 10^{-5}$ (970 samples, CoV = 10%)



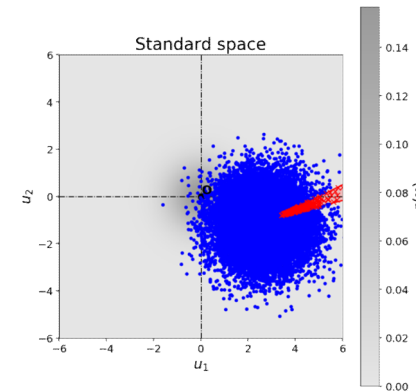
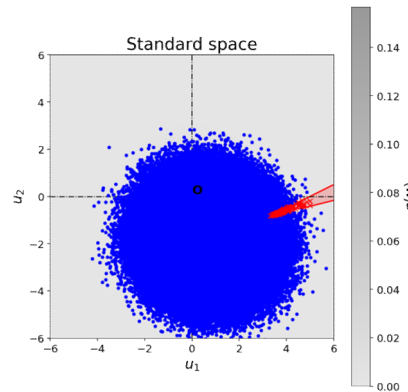
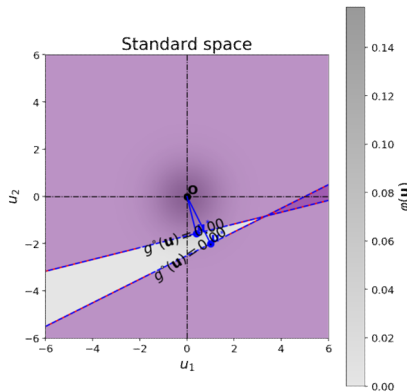
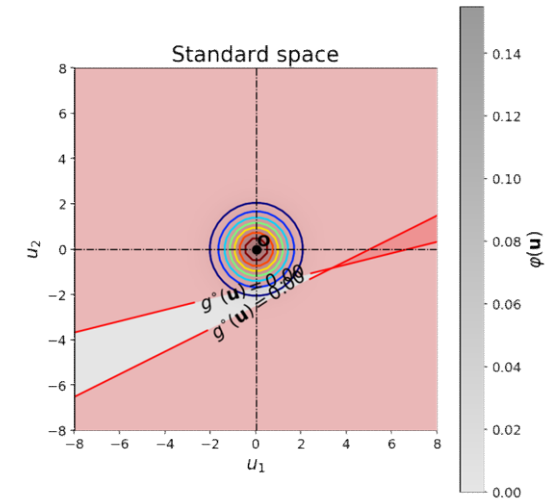
Problem: The search algorithm for the U^* point may not converge

But computational efficiency is generally increased

INTERSECTION PROBABILITY EVALUATION (3/3)

- For penstock conditional failure probability with successful hydro pressure test, the intersection can be located far from the U^* points

- Monte-Carlo (reference probability): $p_{f,reference} = 9 \times 10^{-6}$
- System-FORM (samples centered on the 2 U^* points):
 $P_{f,IS} = 1,1 \times 10^{-5}$ (1 074 000 samples, CoV = 10%)
- Multi-constraint FORM (samples centered on the intersection point)
 $P_{f,IS} = 9,1 \times 10^{-6}$ (17 580 samples, CoV = 10%)



PERSALYS-PENSTOCK INTERFACE (1/2)

Failure criterion

Critère de défaillance	Propriétés des matériaux	Épaisseur	Mécanique de la rupture	Méthode de calcul
Sortie				
Probabilité		Incrément annuel sachant l'épreuve hydrostatique		
Type de fonction de performance		Produit		
k épreuve		1		
k PMIS (pression maximale instantanée de service)		1		
Facteur de marge		1		
Contrainte admissible				
<input checked="" type="radio"/> $f_n = \min(R_{e_{calcul}} / CS_{Re} ; R_{m_{calcul}} / CS_{Rm})$				
<input type="radio"/> $f_x = 0.95 \times R_{e_{calcul}}$				
CS _{Rm} 2.4				
CS _{Re} 1.5				
Cas usuels : [2.4, 1.5] , [2.7, 1.6]				
Coefficients de sécurité				
<input type="checkbox"/> Utiliser les coefficients γ				
Niveau de probabilité pour Rm 0.977				
Niveau de probabilité pour la perte d'épaisseur 0.977				



Material properties

Critère de défaillance	Propriétés des matériaux	Épaisseur	Mécanique de la rupture	Méthode de calcul
Résistance à la rupture (Rm)				
R _{m_{calcul}} = 289				
Distribution		LogNormale	μ 320	
			cv 0.05	
Limite d'élasticité (Re)				
<input checked="" type="checkbox"/> Corrélé avec Rm				
R _{e_{calcul}} = 170				
A		0.834	Re = A * Rm - B + μ_ϵ	
B		64		
ϵ distribution		Normale	μ 0	
			ω 0.05	

Thickness

Critère de défaillance	Propriétés des matériaux	Épaisseur	Mécanique de la rupture	Méthode de calcul
Δe appro distribution				
Distribution		Normale	μ 0	
			σ 0.25	
Δe corr distribution				
Distribution		Normale	μ 1	
			σ 0.25	
Δe annuelle		0.1		
Épaisseur nominale e _{nom}		8		

PERSALYS-PENSTOCK INTERFACE (2/2)

Fracture Mechanics

Crère de défaillance Propriétés des matériaux Épaisseur Mécanique de la rupture Méthode de calcul

☒ Utiliser la mécanique de la rupture

Défaut

☐ Plateau de Lüder

Type de CND

Types ▶

Type de contrainte résiduelle

Type utilisateur

Déterministe

$\sigma_{res} =$ $\times Re$

Types ▶

Ténacité K_{IC}

Distribution

k_{0-25mm}

k

k_{min}

$k_0 = 93.070$

Module d'Young

E (MPa)

Rayon du tuyau

R (mm)

Numerical method

Crère de défaillance Propriétés des matériaux Épaisseur Mécanique de la rupture Méthode de calcul

Méthode

☒ FORM IS **2 main numerical methods applied**

☐ Subset

☐ FORM

☐ Monte-Carlo

☐ Simulation directionnelle

☐ Simulation directionnelle adaptative

Paramètres de méthode de simulation

Nombre maximal de simulations

Coefficient de variation

Paramètres FORM **Specific feature of Persalys-Penstock**

Nombre d'itérations

☒ Test automatique des algorithmes FORM

☒ Préférer une recherche rapide

Algorithme d'optimisation

Erreur absolue

Erreur relative

Erreur sur les résidus

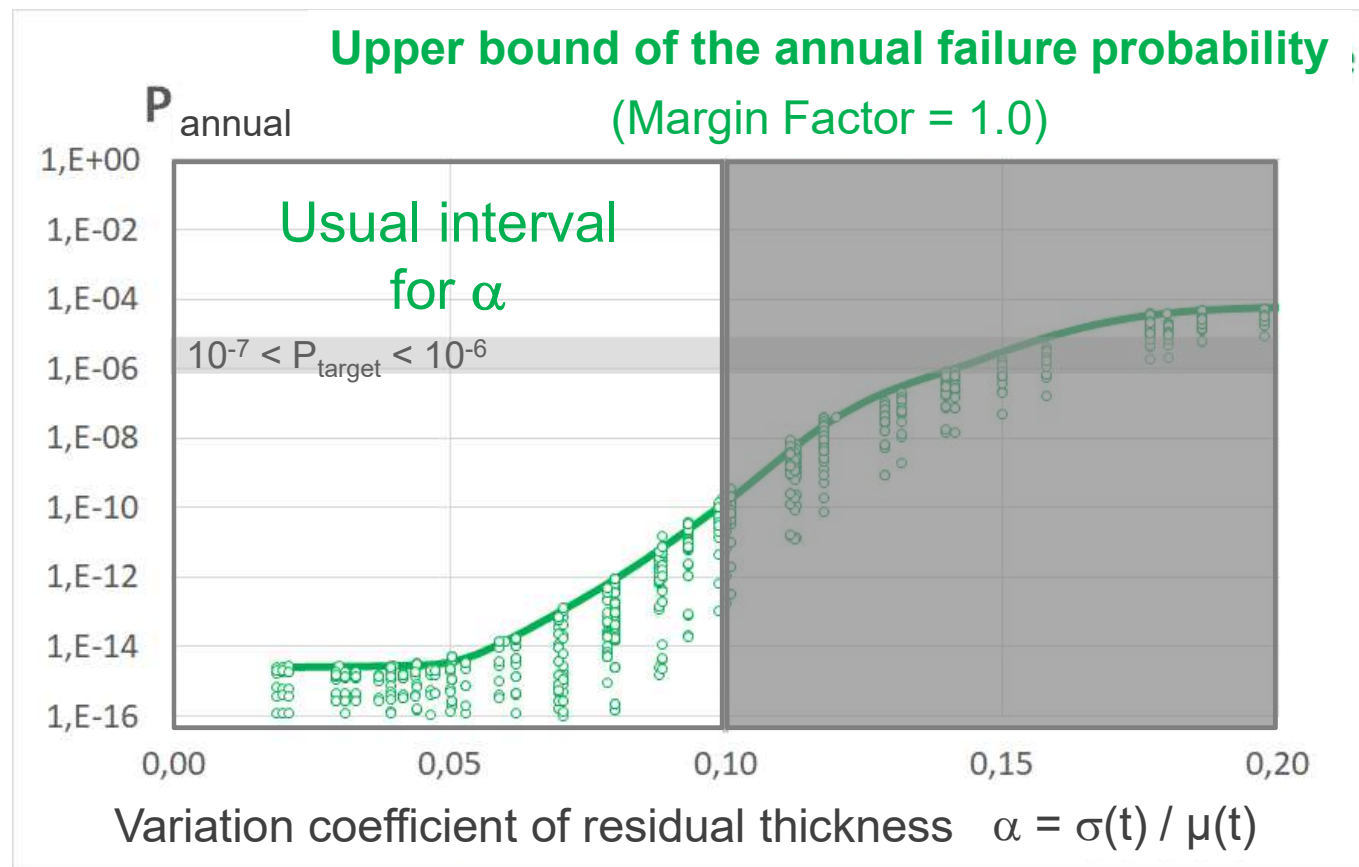
Erreur sur la contrainte



PLASTIC COLLAPSE MODEL : UPPER BOUND OF ANNUAL FAILURE PROBABILITY (DETERMINISTIC DoE)

- The upper bound of annual failure probability increases with the variation coefficient of residual thickness

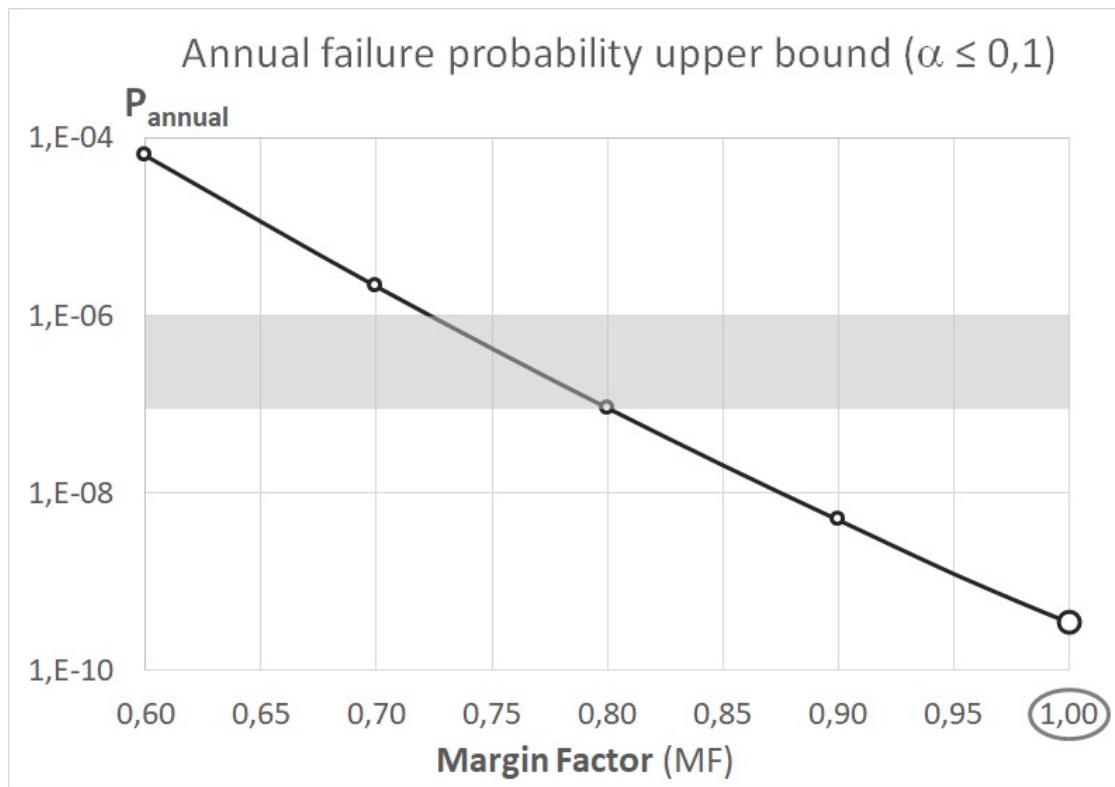
$$\alpha = \sigma(t) / \mu(t)$$



PLASTIC COLLAPSE MODEL :

$$P_{\text{ANNUAL-MAX}} = f(\text{MARGIN FACTOR})$$

(DETERMINISTIC DoE)

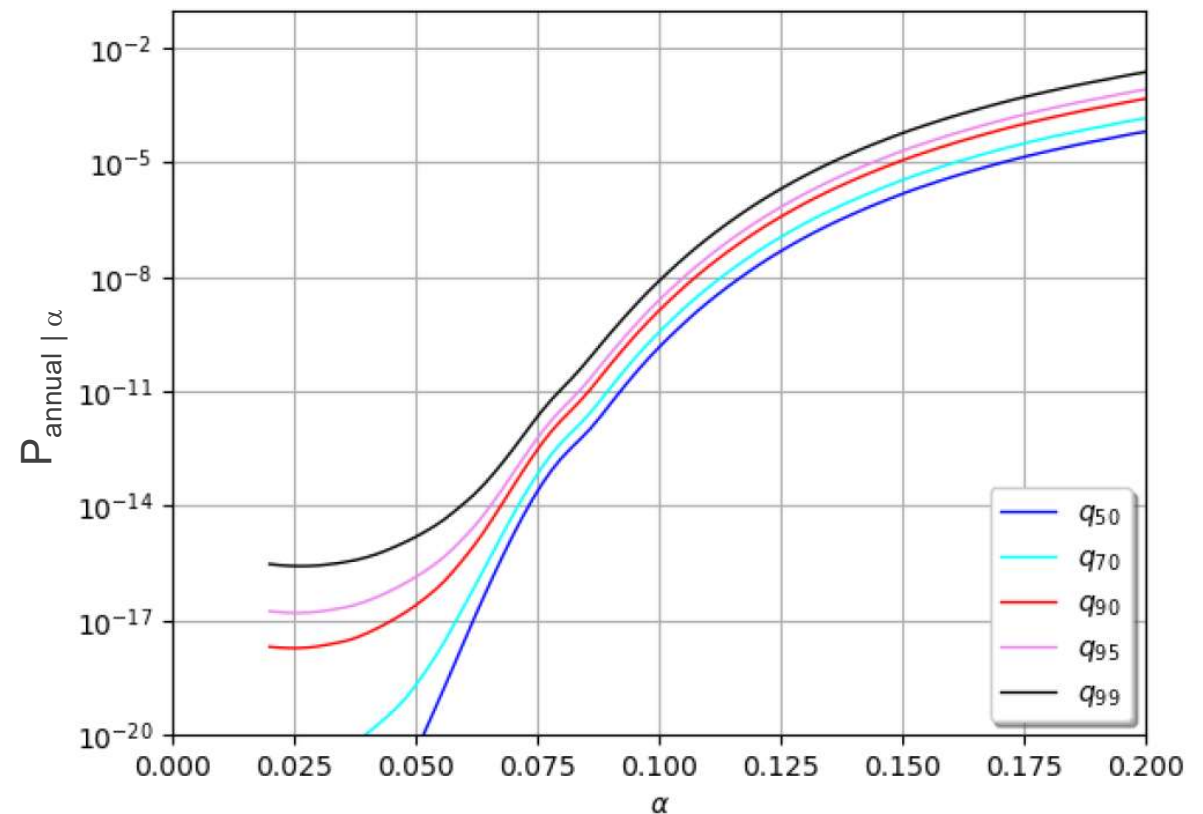


- Margin Factor = 1 (calculated with $\gamma = 2$)
- $\alpha \leq 0.1$

$$\Rightarrow P_{\text{annual-max}} < 10^{-9} \text{ pipe}^{-1} \cdot \text{year}^{-1}$$

PLASTIC COLLAPSE MODEL : EVALUATION OF ANNUAL FAILURE PROBABILITY (PROBABILISTIC DoE)

- Parameters of the random variables modelled by uniform distributions
- Application of a Monte-Carlo scheme to generate random parameters combinations for the calculation grid (instead of Latin hypercube)
- Estimation of the distribution of P_{annual} and of the marginal distribution $P_{\text{annual}} | \alpha$

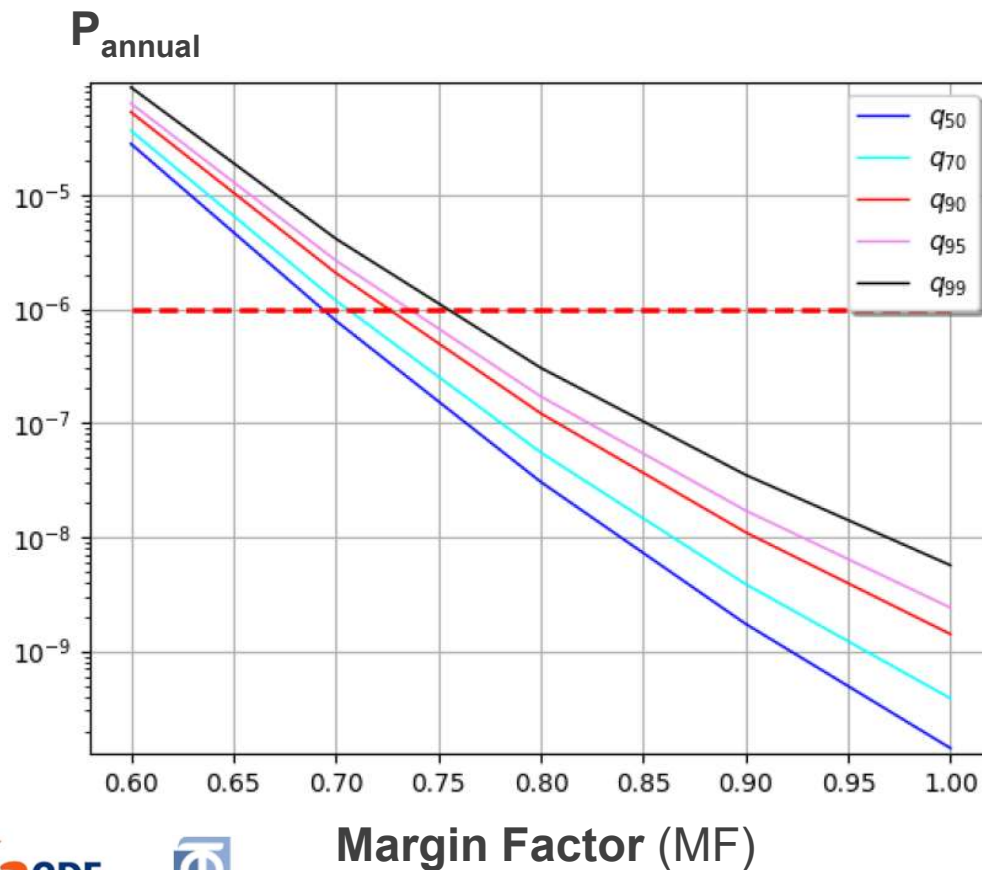


Conditional quantiles of $P_{\text{annual}} | \alpha$ as function of α

PLASTIC COLLAPSE MODEL :

$P_{\text{ANNUAL}} = f(\text{MARGIN FACTOR})$

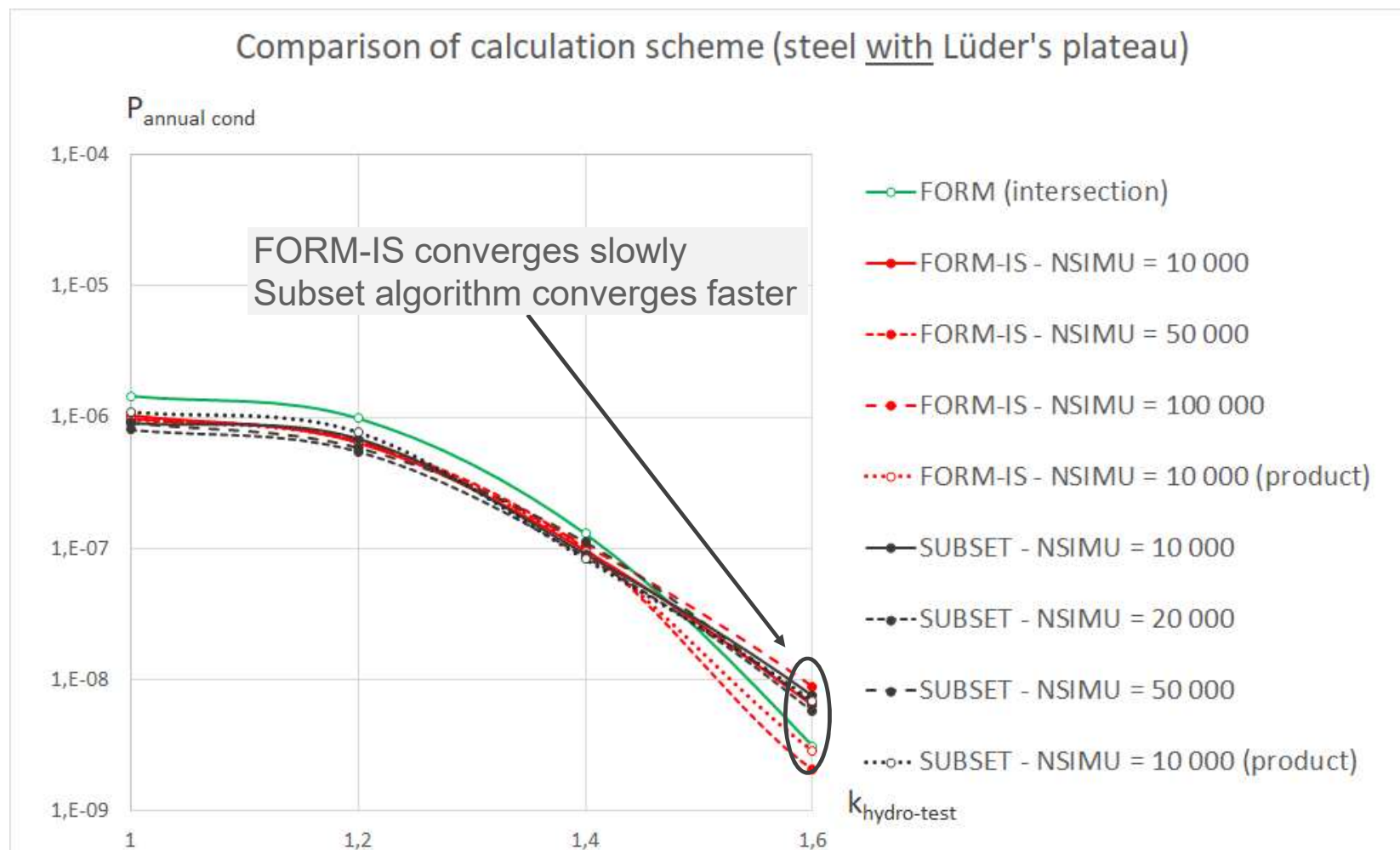
PROBABILISTIC DoE ($\sim 1.7 \times 10^6$ SAMPLES)



- Margin Factor = 1 (calculated with $\gamma = 2$)
- $\alpha \leq 0.1$

$\Rightarrow P_{\text{annual}} \ll 10^{-8} \text{ pipe}^{-1} \cdot \text{year}^{-1}$

COMPARISON OF NUMERICAL METHODS (1/2)



COMPARISON OF NUMERICAL METHODS (2/2)

Mean CPU time per elementary
probability calculation, with fracture mechanics option

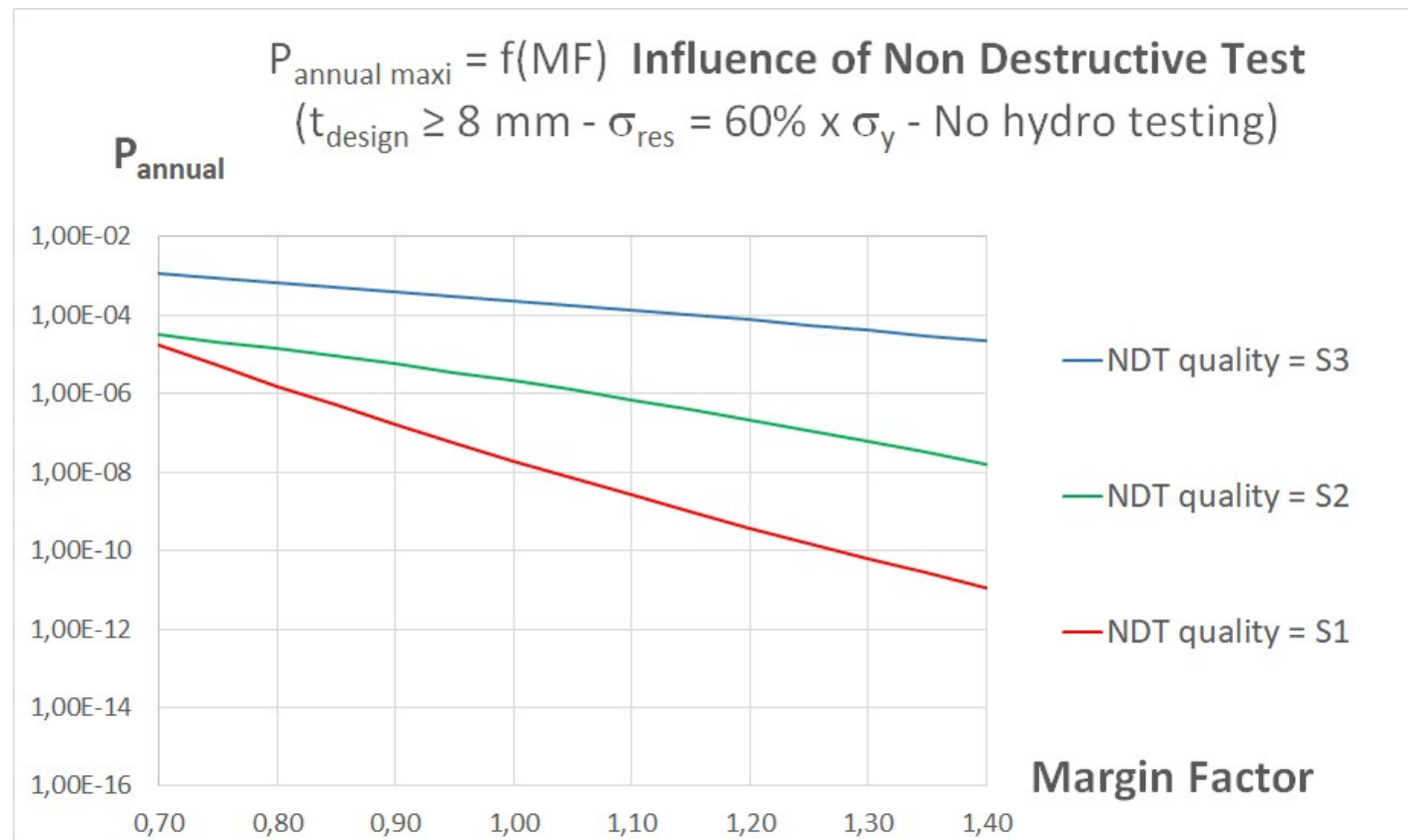
Numerical method	Intersection calculation	Max. simulations	Annual probability increment	Conditional annual probability (successful hydro-test)
FORM	Intersection	-	0,5 s	0,5 s
FORM-IS	Intersection	20 000	8,0 s	16,0 s
	Product	20 000	8,0 s	11,3 s
	Difference	20 000	1,2 s	-
SUBSET	Intersection	20 000	6,5 s	13,5 s
	Product	20 000	3,2 s	10,3 s
	Difference	20 000	5,4 s	-

Typical large calculation grid (fracture mechanics) :

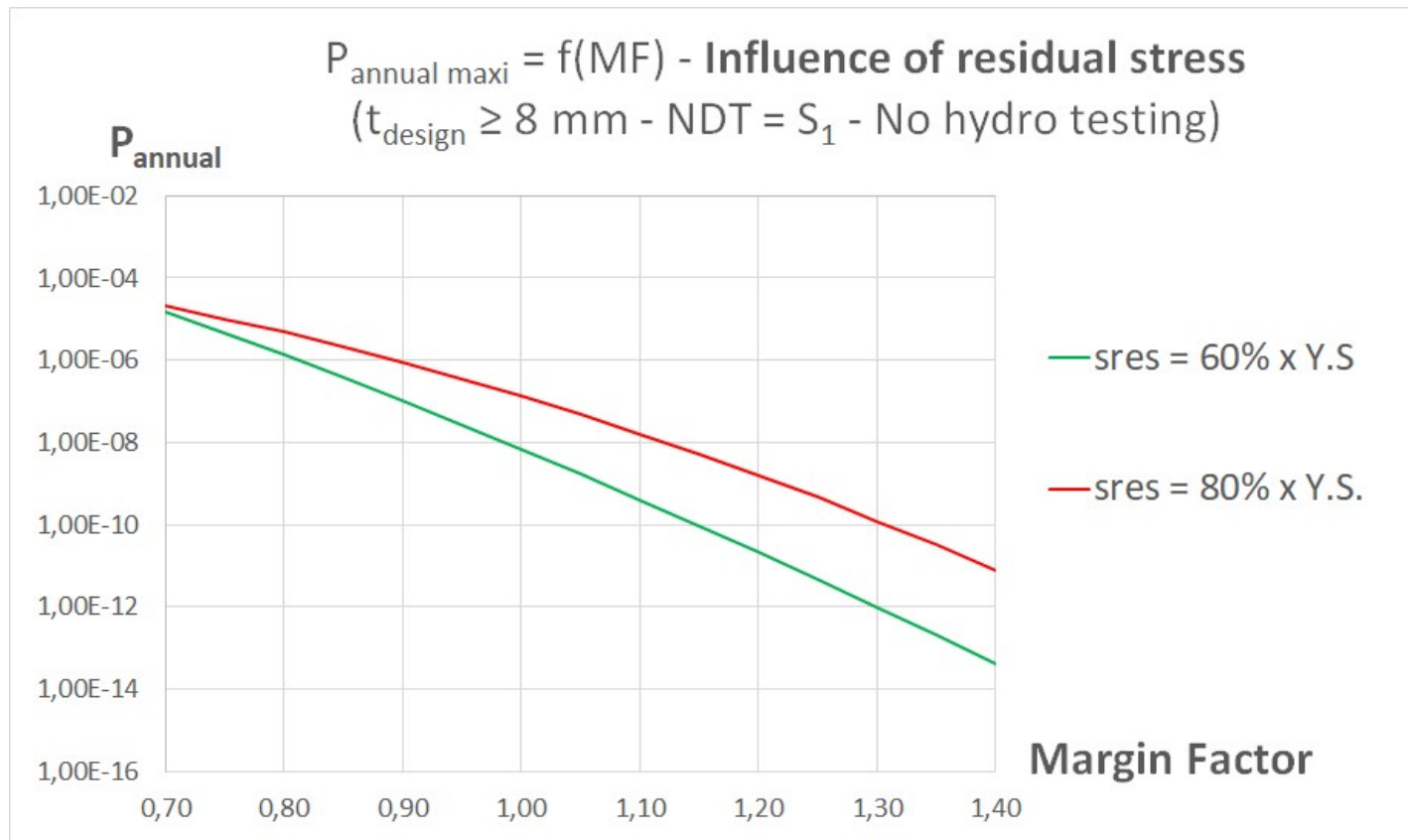
- 14 400 conditional annual probability
- Computation time ~ 100 000 s CPU

Intel(R) Core(TM) i5-8350U CPU @
1.70GHz 1.90 GHz
8,00 Go (7,84 Go utilisable)

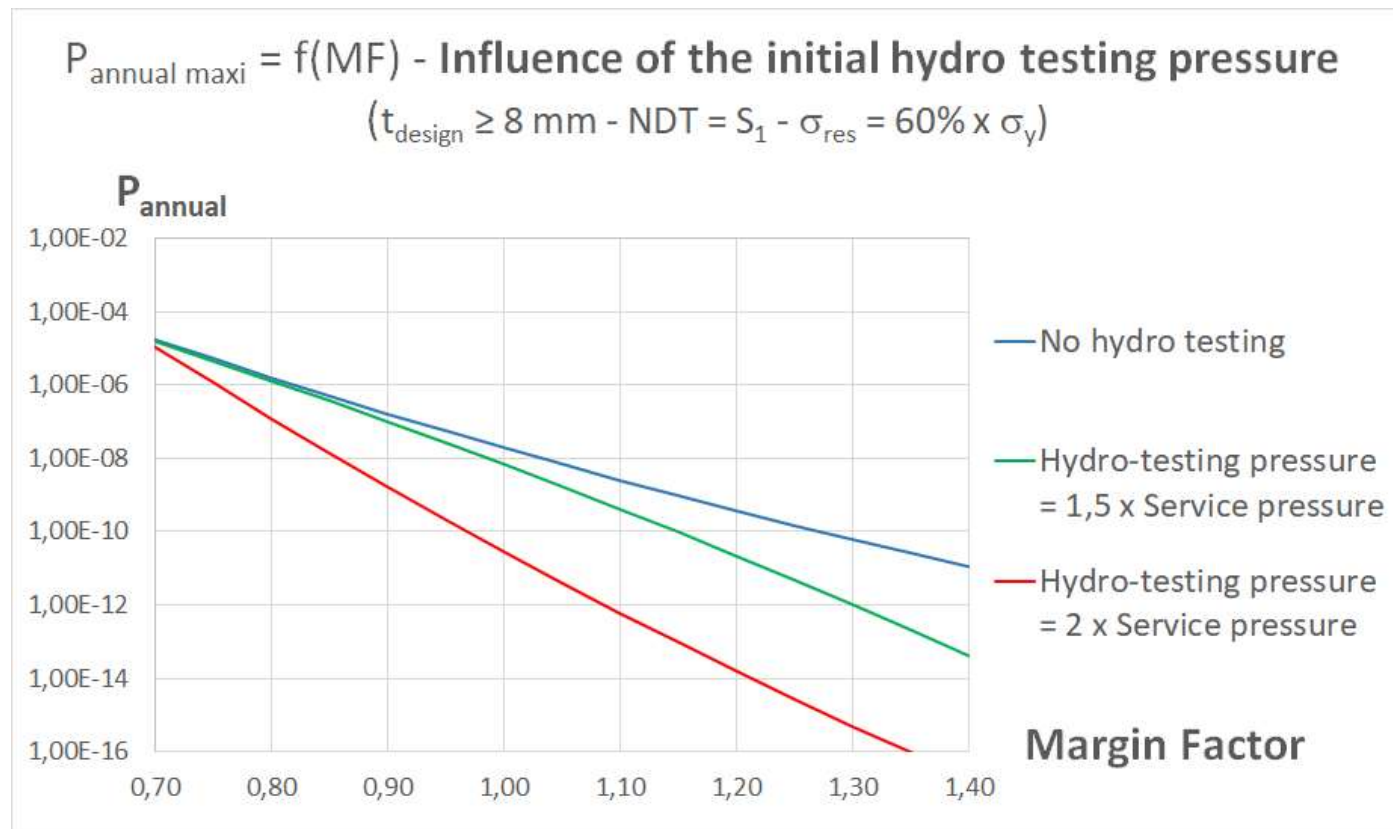
FRACTURE MECHANICS MODEL : INFLUENCE OF NON DESTRUCTIVE TESTING ON THE ANNUAL FAILURE PROBABILITY



FRACTURE MECHANICS MODEL : INFLUENCE OF MANUFACTURING PROCESS ON THE ANNUAL FAILURE PROBABILITY



FRACTURE MECHANICS MODEL : INFLUENCE OF INITIAL HYDRO TESTING ON THE CONDITIONAL ANNUAL FAILURE PROBABILITY



CONCLUSIONS

- The probabilistic models allowed to assess the sensitivity of the upper bound of annual failure probability with regard to the Margin Factor of a penstock
 - For plastic collapse criterion (out of welds, without planar flaws)
 - For fracture mechanic criterion (in welds with potential planar flaws)
- These models have been implemented in **Persalys-Penstock**
- Specific innovative numerical methods have been specifically developed and tested in order to calculate the probabilities corresponding to intersection of events
- **Persalys-Penstock** allows to solve – deterministic or probabilistic – large calculation grids
- Its application highlights that the annual failure probability depends on 4 major influencing factors :
 - The coefficient of variation of residual thickness
 - The relief of residual stress in welds
 - The detectability performance of Non destructive Tests
 - The initial hydrostatic pressure test leads to a significant reduction of the annual failure probability since the average cumulated corrosion thinning is moderate



TANK YOU !

QUESTIONS ?

