RIYA Final Report (Summer 2021)

Dynamics of Vibration Absorber System with Focus on Nonlinear Paths

RIYA Scholar: Sibibalan Jeevanandam



<u>Institution:</u> Indian Institute of Technology Tirupati

Mentors: Prof. T. Sen and Prof. R. Singh

Acoustics & Dynamics Laboratory (adl.osu.edu)

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Introduction

<u>Motivation</u>: Vibration absorber is used to control vibrations of a primary structure and prevent damage to structural components. But a nonlinear path adds complexity and allows larger variations in the operating frequency regime [1].

Potential applications of absorbers and tuned dampers:

- Reduce response of structures subjected to seismic motions [2] or wind loads [3]
- ❖ Supress vibrations in the cutting tool during turning operations [4]

<u>Goal</u>: Investigate the effects of nonlinear elastic path(s) on the dynamic response of a vibration absorber system using the multi-term harmonic balance method (MHBM)

[1] R. E. Roberson, Synthesis of a nonlinear dynamic vibration absorber, Journal of the Franklin Institute, 254(3), 205-220, 1952.

[2] F. Sadek, B. Mohraz, A.W. Taylor, and R.M. Chung, A method of estimating the parameters of tuned mass dampers for seismic applications, Earthquake Engineering & Structural Dynamics, 26(6), 1997.

[3] K.C.S. Kwok and B. Samali, Performance of tuned mass dampers under wind loads, Engineering structures, 17(9), 655-667, 1995.

[4] E.C. Lee, C.Y. Nian, and Y.S. Tarng, Design of a dynamic vibration absorber against vibrations in turning operations, Journal of Materials Processing Technology, 108(3), 278-285, 2001.

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Tuned mass damper atop Taipei 101



Source: https://en.wikipedia.org/wiki/Tuned mass damper

Vibration absorbers installed on the Millenium Bridge, London



Source: https://en.wikipedia.org/wiki/Tuned mass damper

Literature Review

Author(s) (Journal, Year)	Topic	Comments
Roberson (JFI, 1952) [1]	Synthesis of a nonlinear dynamic vibration absorber	Undamped 2DOF model, Duffing iteration method
Sadek et al. (EESD, 1997) [2]	A method of estimating the parameters of tuned mass dampers for seismic applications	Multi-DOF structures, seismic loading, linear path with damping
Kwok & Samali (Engg. Struct., 1995) [3]	Performance of tuned mass dampers under wind loads	Linear undamped 2DOF model, passive and active vibration absorbers
Lee & Tarng (JMPT, 2001) [4]	Design of a dynamic vibration absorber against vibrations in turning operations	Linear damped 2DOF model, experimental verification
Sen & Singh (NOVEM Conf., 2018) [5]	Energy Exchange between Two Sub-systems Coupled with a Nonlinear Elastic Path	2DOF damped harmonic oscillator, nonlinear coupling, MHBM
Den Hartog (1985) [6]	Mechanical Vibrations	Dynamic vibration absorbers, forced vibrations with nonlinear springs

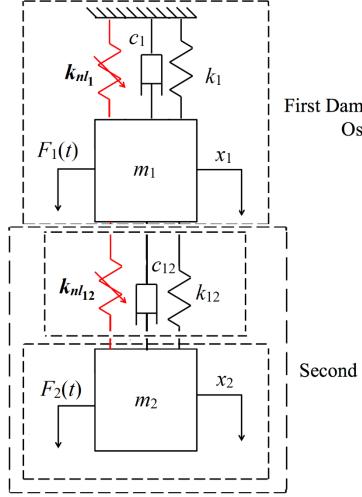


Problem Formulation

- ❖ Objective 1: Obtain nonlinear frequency responses of the example case using a semi-analytical multi-term harmonic balance method (MHBM), and verify results using Sen & Singh's approach [5]
- Objective 2: Investigate the dynamic effects of two nonlinear cubic springs on the nonlinear frequency responses
- Objective 3: Conduct parametric studies (relative linear stiffness, hardening/softening-type nonlinearity etc.) for the system

Scope and Assumptions

- 2DOF
- 2 springs with cubic nonlinearity
- Harmonic excitations on either/both masses
- Steady-state solutions and frequency-response curves
- Elastic coupling forces



First Damped Harmonic Oscillator

Second Damped Harmonic Oscillator

Example Case: 2DOF nonlinear vibration absorber system



Governing equations in the dimensionless form

Equations of motion

$$m_1\ddot{x_1} + (c_1 + c_{12})\dot{x_1} - c_{12}\dot{x_2} + (k_1 + k_{12})x_1 - k_{12}x_2 + k_{nl_1}x_1^3 + k_{nl_{12}}(x_1 - x_2)^3 = F_1^S + F_1^D\sin(\omega t + \phi_1)$$

$$m_2\ddot{x_2} + c_{12}\dot{x_2} - c_{12}\dot{x_1} + k_{12}x_2 - k_{12}x_1 + k_{nl_{12}}(x_2 - x_1)^3 = F_2^S + F_2^D\sin(\omega t + \phi_2)$$

Non-dimensional parameters

$$\mu = \frac{m_2}{m_1}, \gamma = \frac{c_{12}}{c_1}, \lambda = \frac{k_{12}}{k_1}, \omega_n = \sqrt{\frac{k_1}{m_1}}, \zeta = \frac{c_1}{2m_1\omega_n}, \alpha_1 = \frac{k_{nl_1}x_0^2}{k_1}, \alpha_{12} = \frac{k_{nl_{12}}x_0^2}{k_1}, \tau = \omega_n t$$

$$X_1 = \frac{x_1}{x_0}, X_2 = \frac{x_2}{x_0}, \epsilon_1 = \frac{F_1^D}{F_1^S}, \epsilon_2 = \frac{F_2^D}{F_1^S}, \sigma = \frac{F_2^S}{F_1^S}$$

where the operating point is defined as $x_0 = \frac{F_1^S}{k_1}$ (Assumption: $F_1^S \neq 0$).

Transforming the independent variable, $\theta = \omega \tau$, the <u>final equations in the dimensionless form</u> are:

$$\omega^{2} \frac{d^{2} X_{1}}{d\theta^{2}} + \left[2\zeta(1+\gamma)\right] \omega \frac{dX_{1}}{d\theta} - 2\zeta\gamma\omega \frac{dX_{2}}{d\theta} + (1+\lambda)X_{1} - \lambda X_{2} + \alpha_{1}X_{1}^{3} + \alpha_{12}(X_{1} - X_{2})^{3} = 1 + \epsilon_{1} \sin\left(\frac{\theta}{\omega_{n}} + \phi_{1}\right)$$

$$\mu\omega^{2} \frac{d^{2} X_{2}}{d\theta^{2}} + 2\zeta\gamma\omega \frac{dX_{2}}{d\theta} - 2\zeta\gamma\omega \frac{dX_{1}}{d\theta} + \lambda X_{2} - \lambda X_{1} + \alpha_{12}(X_{2} - X_{1})^{3} = \sigma + \epsilon_{2} \sin\left(\frac{\theta}{\omega_{n}} + \phi_{2}\right)$$



MHBM applied to 2DOF nonlinear system (Objective 1)

Assumed solution: Truncated Fourier series with N_h harmonics retained

$$X_{1}(\theta) = a_{0,1} + a_{1,1}\cos(\theta) + b_{1,1}\sin(\theta) + \dots + a_{N_{h},1}\cos(N_{h}\theta) + b_{N_{h},1}\sin(N_{h}\theta)$$

$$X_{2}(\theta) = a_{0,2} + a_{1,2}\cos(\theta) + b_{1,2}\sin(\theta) + \dots + a_{N_{h},2}\cos(N_{h}\theta) + b_{N_{h},2}\sin(N_{h}\theta)$$

Discretization of independent variable: $\theta = [\theta_1, \theta_2, ..., \theta_N]^T$, where $2N_h < N$ (Nyquist-Shannon sampling theorem)

$$\text{Let} \begin{bmatrix} 1 \cos(\theta_1) \sin(\theta_1) & \cdots & \cos(N_h \theta_1) \sin(N_h \theta_1) \\ \vdots & \ddots & \vdots \\ 1 \cos(\theta_N) \sin(\theta_N) & \cdots & \cos(N_h \theta_N) \sin(N_h \theta_N) \end{bmatrix} = \boldsymbol{F_r}, \text{ and } \begin{bmatrix} a_{0,k} \ a_{1,k} \ b_{1,k} \ \dots \ a_{N_{h,k}} \ b_{N_{h,k}} \end{bmatrix}^T = \boldsymbol{A_k} \ (k = 1,2)$$

$$R_1 = \omega^2 F_r D^2 A_1 + [2\zeta(1+\gamma)]\omega F_r D A_1 - 2\zeta\gamma\omega F_r D A_2 + (1+\lambda)F_r A_1 - \lambda F_r A_2 + \alpha_1 (F_r A_1)^3 + \alpha_{12} (F_r A_1 - F_r A_2)^3 - 1 - \epsilon_1 \sin\left(\frac{\theta}{\omega_n} + \phi_1\right)$$

$$R_2 = \mu\omega^2 F_r D^2 A_2 + 2\zeta\gamma\omega F_r D A_2 - 2\zeta\gamma\omega F_r D A_1 + \lambda F_r A_2 - \lambda F_r A_1 + \alpha_{12} (F_r A_2 - F_r A_1)^3 - \sigma - \epsilon_2 \sin\left(\frac{\theta}{\omega_n} + \phi_2\right)$$

Residue
$$\mathbf{R} = [\mathbf{R_1} \ \mathbf{R_2}]^T$$
, Jacobian $\mathbf{J} = \begin{bmatrix} \frac{\partial R_1}{\partial A_1} & \frac{\partial R_1}{\partial A_2} \\ \frac{\partial R_2}{\partial A_1} & \frac{\partial R_2}{\partial A_2} \end{bmatrix}$

The Fourier coefficients vector $\mathbf{A} = [\mathbf{A_1} \ \mathbf{A_2}]^T$ is computed using Newton Raphson method

At each iteration
$$i$$
, $\boldsymbol{A}_{i+1} = \boldsymbol{A}_i - (\boldsymbol{J}_i^T \boldsymbol{J}_i)^{-1} \boldsymbol{J}_i^T \boldsymbol{R}_i$



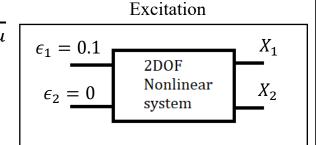
Verification of frequency responses using Sen & Singh [5]

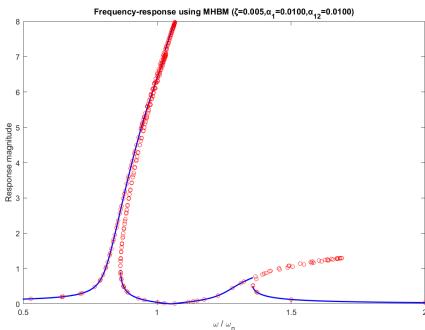
System parameters: $\mu=0.2, \gamma=0.2, \lambda=0.2, \zeta=0.005, \sigma=\frac{\mu}{1+\mu}$

 $\alpha_1 = 0.01, \alpha_{12} = 0.01$

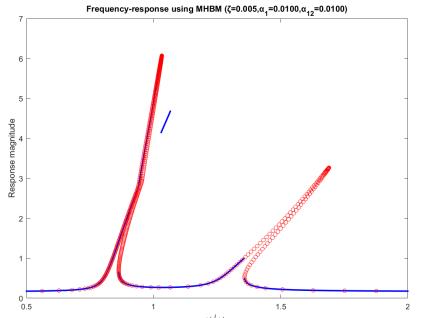
Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

Solution parameters: $N_h = 80$, N = 200





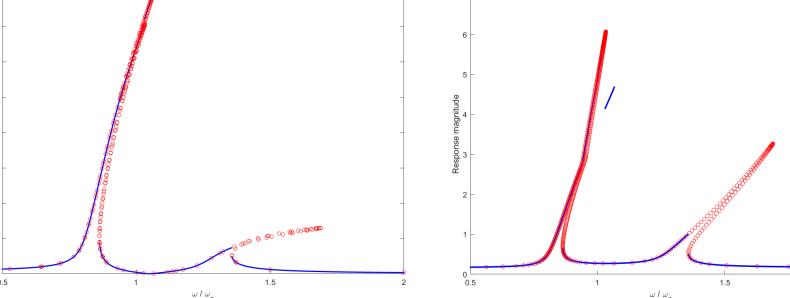
Frequency response curve: X_1



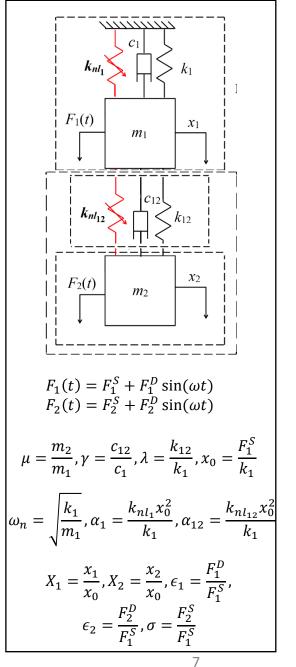
Elastic coupling force vs ω

Legend

MHBM without continuation method MHBM with continuation method [5]



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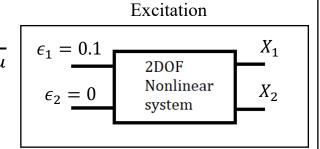
Jump phenomenon

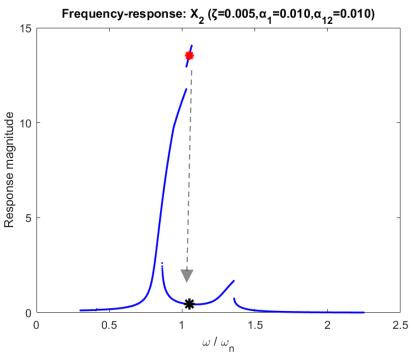
System parameters: $\mu = 0.2, \gamma = 0.2, \lambda = 0.2, \zeta = 0.005, \sigma = \frac{\mu}{1+\mu}$

 $\alpha_1 = 0.01, \alpha_{12} = 0.01$

Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

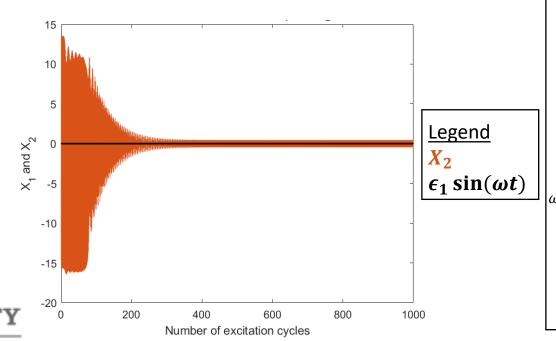
Solution parameters: $N_h = 80$, N = 200



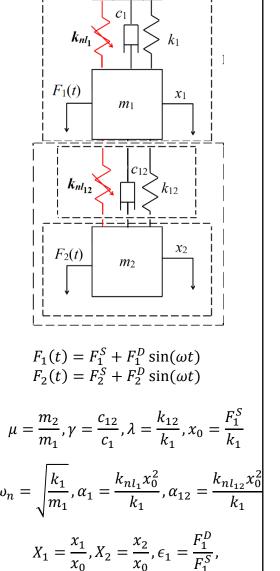


Frequency response curve: X_2

Numerical solution (using Runge-Kutta method) is computed at the **red** marker; the response attains a steady-state value represented by the **black** marker (on the stable branch)



Time-domain solution (Runge-Kutta method)



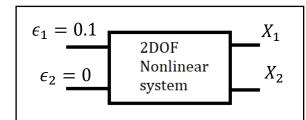
Effect of relative linear stiffness (λ)

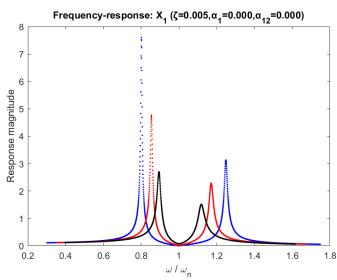
System parameters: $\gamma=0.2$, $\mu=\lambda$, $\zeta=0.005$, $\sigma=\frac{\mu}{1+\mu}$

 $\alpha_1 = 0.01, \alpha_{12} = 0.01$

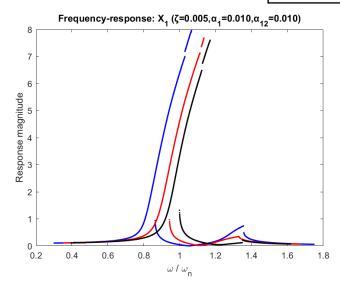
Solution parameters: $N_h = 80$, N = 200

Excitation





Frequency response curve: X_1 (Linear system)



Frequency response curve: X_1 (Nonlinear system)

Legend

$$\lambda = 0.2$$

$$\lambda = 0.1$$

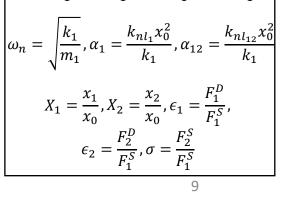
$$\lambda = 0.05$$

<u>Linear natural frequencies</u>

 $0.8\omega_n$, $1.25\omega_n$

 $0.85\omega_{n}, 1.17\omega_{n}$

 $0.9\omega_n$, $1.1\omega_n$



 $F_1(t)$

 $F_2(t)$

 m_1

 $F_1(t) = F_1^S + F_1^D \sin(\omega t)$ $F_2(t) = F_2^S + F_2^D \sin(\omega t)$

 $\mu = \frac{m_2}{m_1}, \gamma = \frac{c_{12}}{c_1}, \lambda = \frac{k_{12}}{k_1}, x_0 = \frac{F_1^S}{k_1}$

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The second natural frequency decreases (due to $\lambda \downarrow$), but the

hardening-type paths ($\alpha_1 > 0$, $\alpha_{12} > 0$) prevent the left-shift in

the response curves near the second natural frequencies

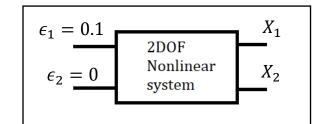
Vibration absorber for linear primary system

System parameters: $\mu = 0.2, \gamma = 0.2, \lambda = 0.2, \zeta = 0.005, \sigma = \frac{\mu}{1+\mu}$

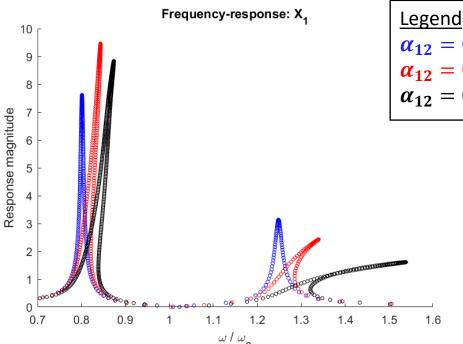
 $\alpha_1 = 0$

Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

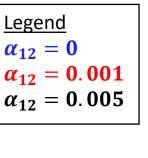
Solution parameters: $N_h = 80$, N = 200

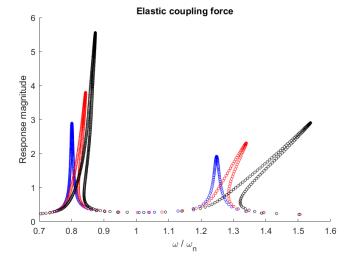


Excitation



Frequency response curve: X_1

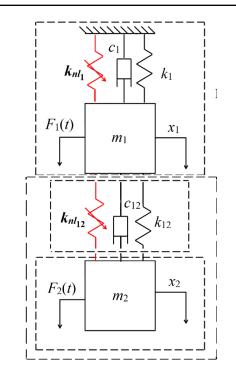




Elastic coupling force vs ω

Vibration amplitudes of the primary system reduce $\left|\omega_n = \sqrt{\frac{k_1}{m_1}}, \alpha_1 = \frac{k_{nl_1}x_0^2}{k_1}, \alpha_{12} = \frac{k_{nl_{12}}x_0^2}{k_1}\right|$ at a given frequency (ω) with increasing path nonlinearity levels

THE OHIO STATE UNIVERSITY Drawback: The maximum elastic force increases with increasing nonlinearity levels



$$F_1(t) = F_1^S + F_1^D \sin(\omega t)$$

 $F_2(t) = F_2^S + F_2^D \sin(\omega t)$

$$\mu = \frac{m_2}{m_1}, \gamma = \frac{c_{12}}{c_1}, \lambda = \frac{k_{12}}{k_1}, x_0 = \frac{F_1^S}{k_1}$$

$$lpha_n = \sqrt{rac{k_1}{m_1}}$$
 , $lpha_1 = rac{k_{nl_1}x_0^2}{k_1}$, $lpha_{12} = rac{k_{nl_{12}}x_0^2}{k_1}$

$$X_1 = \frac{x_1}{x_0}, X_2 = \frac{x_2}{x_0}, \epsilon_1 = \frac{F_1^D}{F_1^S},$$

 $\epsilon_2 = \frac{F_2^D}{F_1^S}, \sigma = \frac{F_2^S}{F_1^S}$

Effect of α on the elastic force

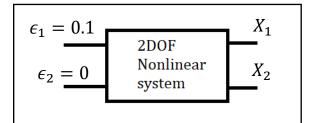
System parameters: $\mu = 0.2, \gamma = 0.2, \lambda = 0.2, \zeta = 0.005, \sigma = \frac{\mu}{1+\mu}$

Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

Solution parameters: $N_h = 80$, N = 200

Elastic force vs ω 12 Elastic Force Magnitude 2 0.5 1.5 2.5 $\omega / \omega_{\rm n}$

Excitation



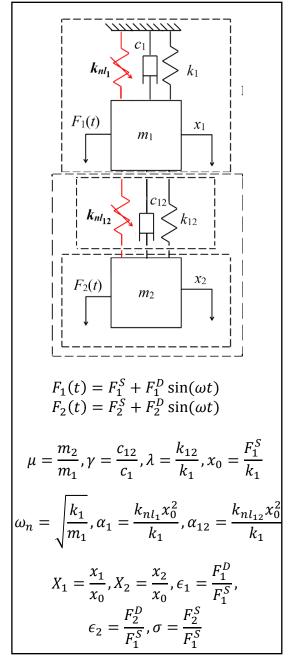
<u>Legend</u>

$$\alpha_1 = \alpha_{12} = 0.01$$

$$\alpha_1 = \alpha_{12} = 0.01$$
 $\alpha_1 = \alpha_{12} = 0.1$

The magnitude of elastic coupling force increases with an increase in path nonlinearity level

High elastic forces may cause mechanical fatigue in the springs



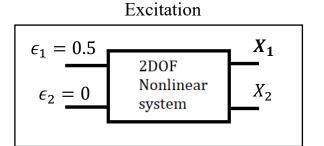
Mixed nonlinearities

System parameters: $\mu = 0.1$, $\gamma = 0.1$, $\lambda = 0.1$, $\zeta = 0.005$, $|\alpha_1| = |\alpha_{12}| = 0.00001$

Natural frequencies (for linear system): $0.85\omega_n$, $1.7\omega_n$

Solution parameters: $N_h = 80$, N = 200

Legend $\alpha_1 = 0, \alpha_{12} = 0$ $\alpha_1 > 0, \alpha_{12} > 0$ $\alpha_1 < 0, \alpha_{12} > 0$



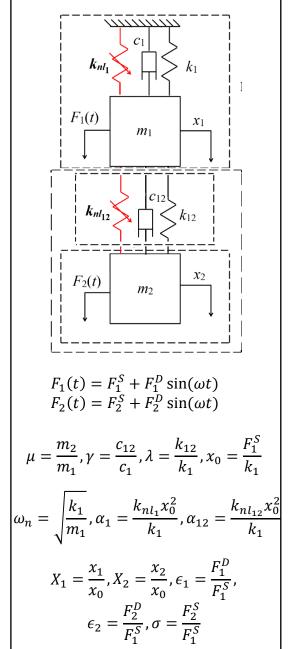
Frequency-response: X₄ 50 Response magnitude 0 0 0 10 0.8 1.2 0.4 0.6 1.4 0.2 $\omega / \omega_{\rm p}$

Frequency response curve: X_1

Overlap between black and red curves near the second natural frequency

The response near the second natural frequency is significantly affected only by the path nonlinearity $(k_{nl_{12}})$





Vibration absorber for nonlinear primary system

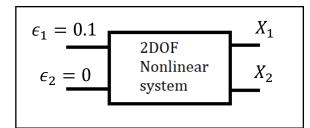
Excitation

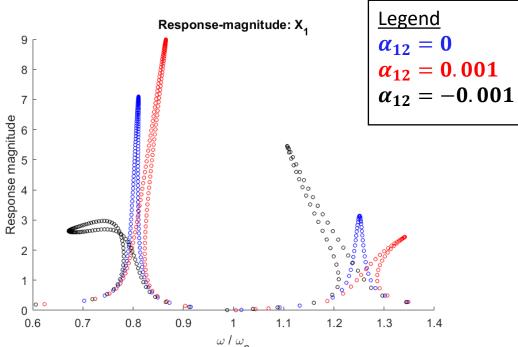
System parameters: $\mu = 0.2, \gamma = 0.2, \lambda = 0.2, \zeta = 0.005, \sigma = \frac{\mu}{1+\mu}$

 $\alpha_1 = 0.001$

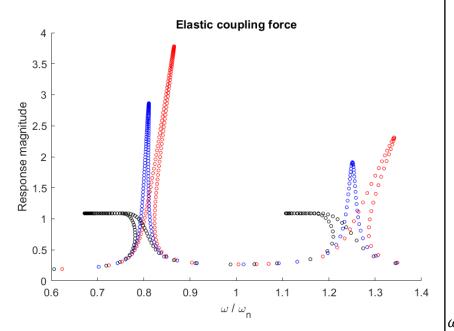
Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

Solution parameters: $N_h = 80$, N = 200







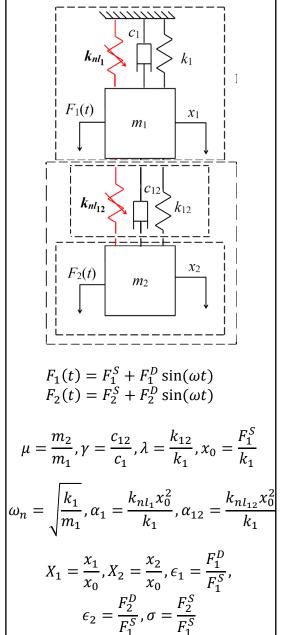


Elastic coupling force vs ω

Frequency response curve: X_1

Elastic coupling force corresponding to the softening-type path (black curve) appears to saturate near both natural frequencies (effect of $k_{12} > 0 \& \alpha_{12} < 0$)





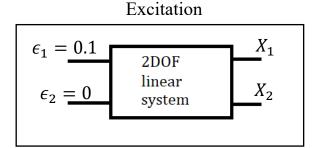
Softening-type path

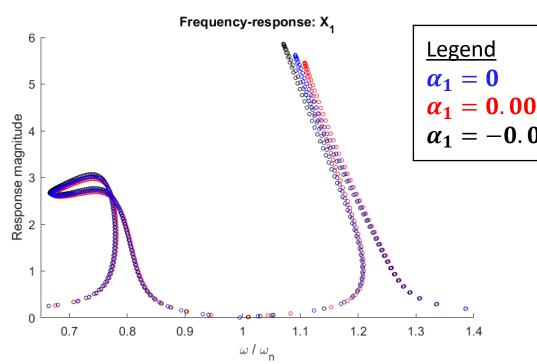
System parameters: $\mu = 0.2, \gamma = 0.2, \lambda = 0.2, \zeta = 0.005, \sigma = \frac{\mu}{1+\mu}$

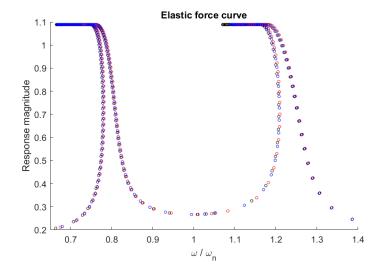
$$\alpha_{12} = -0.001$$

Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

Solution parameters: $N_h = 80$, N = 200





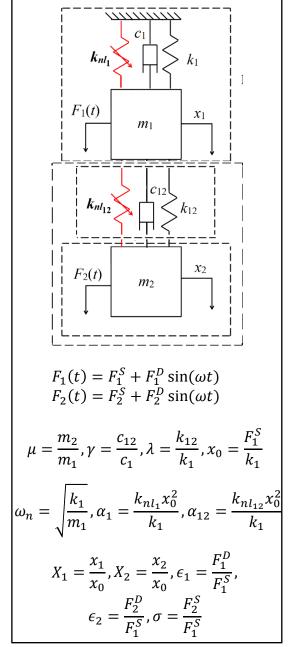


Elastic coupling force vs ω

Frequency response curve: X_1

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In this case, the type of nonlinearity (α_1) in the primary system does not significantly affect the nature of the response curves

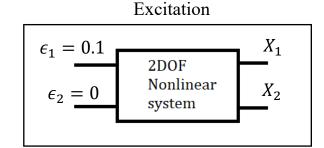


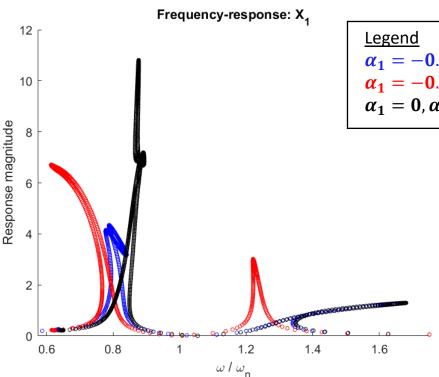
Effects of individual nonlinearity

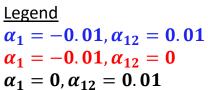
System parameters: $\mu = 0.2, \gamma = 0.2, \lambda = 0.2, \zeta = 0.005, \sigma = \frac{\mu}{1+\mu}$

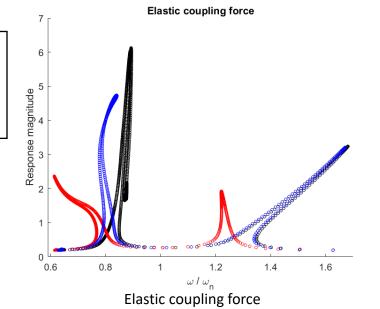
Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

Solution parameters: $N_h = 80$, N = 200

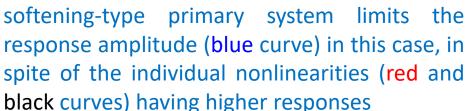






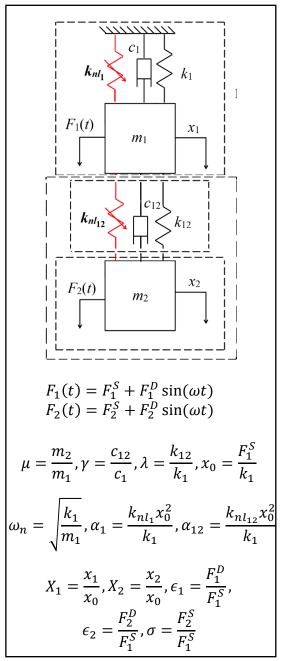


Addition of a hardening-type path to the





Frequency response curve: X_1



Conclusion

- MHBM is successfully applied to a 2DOF nonlinear system
- Nonlinear paths affect the dynamics of the system; frequency response curves have multiple branches
- The MHBM solutions using both methods match for the most stable branches; some set of solutions are mutually exclusive
- The response near the second natural frequency is governed primarily by the path nonlinearity $(k_{nl_{12}})$ for all cases
- Choice of nonlinear path (hardening/softening-type) depends on the primary system, and the response amplitude and elastic force considerations

Methods of verifying results

- ❖ Sen & Singh's approach [5]
- Numerical solutions (Runge-Kutta method)

Difficulties/Challenges

- Obtaining unstable branches in the frequency response curves using MHBM
- ❖ Numerical errors in MHBM solutions
- Physical interpretation of results in some cases of softening-type nonlinear path (Appendix IV)

Future scope

- Determining system parameters for a specific application
- Excitation applied to secondary system (e.g. excitation due to wind loads)
- Experimental validation
- Presentation of work in a conference proceeding



Appendix I: Lessons Learnt

- **❖** Basic characteristics of Duffing oscillators
- Vibration isolation
- Multi-term Harmonic Balance Method
- Numerical methods to solve nonlinear equations and the associated errors
- Basics of signal processing
- Importance and some techniques of non-dimensionalization
- Judgement of real-life parameters and approximations
- Practices in technical presentation



Appendix II: List of Selected References

- [1] R. E. Roberson, Synthesis of a nonlinear dynamic vibration absorber, Journal of the Franklin Institute, 254(3), 205-220, 1952.
- [2] F. Sadek, B. Mohraz, A.W. Taylor, and R.M. Chung, A method of estimating the parameters of tuned mass dampers for seismic applications, Earthquake Engineering & Structural Dynamics, 26(6), 1997.
- [3] K.C.S. Kwok and B. Samali, Performance of tuned mass dampers under wind loads, Engineering structures, 17(9), 655-667, 1995.
- [4] E.C. Lee, C.Y. Nian, and Y.S. Tarng, Design of a dynamic vibration absorber against vibrations in turning operations, Journal of Materials Processing Technology, 108(3), 278-285, 2001.
- [5] O.T. Sen and R. Singh, Energy exchange between two sub-systems coupled with a nonlinear elastic path, NOVEM 2018 Conference, Ibiza, May 2018. (Also, the matlab code used to obtain nonlinear frequency responses with both stable and unstable solutions.)
- [6] J.P. Den Hartog, Mechanical Vibrations, Dover, 79-105 and 370-377, 1985.



Appendix III: Pendulum – SDOF system with cubic nonlinearity

Governing equation of **SDOF** pendulum with

harmonic excitation: $\ddot{\phi} + \frac{g}{l}\sin(\phi) = \theta\sin(\omega t)$

Using series approximation, $\sin \phi \approx \phi - \frac{\phi^3}{3!}$,

$$\ddot{\phi} + \frac{g}{l}\phi - \frac{g}{(3!)l}\phi^3 = \theta\sin(\omega t)$$

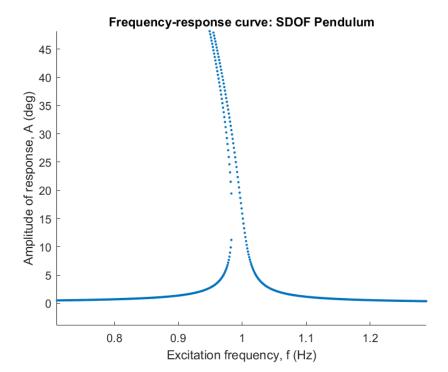
Harmonic Balance Method (HBM) for Duffing Oscillator

Assumed solution: $\phi(t) = A \sin(\omega t)$, $\omega = 2\pi f$

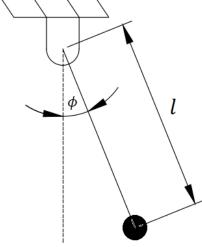
Substituting the assumed solution and balancing $\sin(\omega t)$ on both sides,

$$\left(\frac{g}{8l}\right)A^3 + \left(\omega^2 - \frac{g}{l}\right)A + \theta = 0$$

Real roots of the above equation are plotted against f to get the characteristic backbone curve



Backbone curve: $A vs f (l = 250mm, \theta = \frac{\pi}{18}N/kgm)$



Schematic: SDOF Pendulum

Pendulum is often used as a vibration absorber

Limitations of Single-term HBM:

- Cannot capture higher harmonics of the solution
- Cannot accommodate the effect of damping

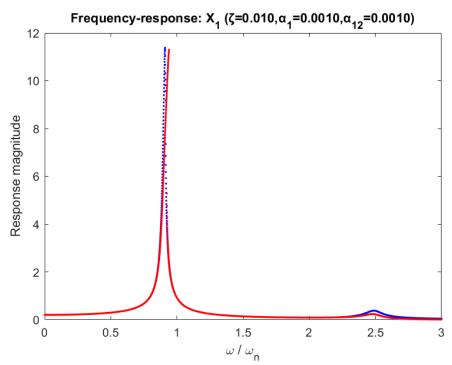


Appendix III: Principle of superposition

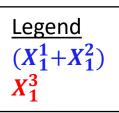
System parameters: $\mu = 0.2$, $\gamma = 1$, $\lambda = 1$, $\zeta = 0.01$, $\alpha_1 = \alpha_{12} = 0.001$

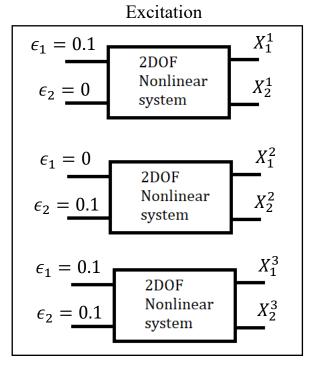
Natural frequencies (for linear system): $0.9\omega_n$, $2.49\omega_n$

Solution parameters: $N_h = 80$, N = 200



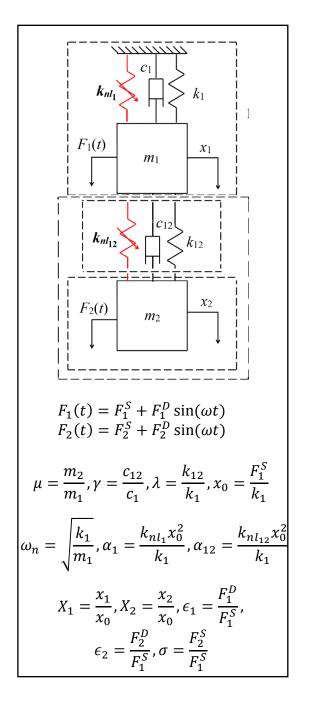
Frequency response curve, X_1





Superposition principle doesn't apply to the given nonlinear system



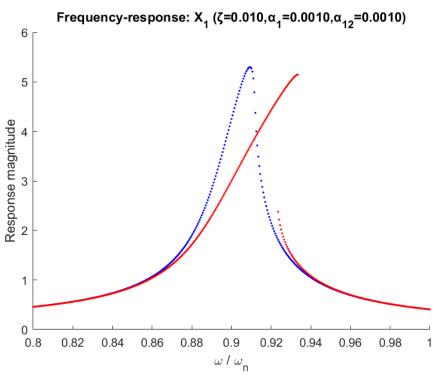


Appendix III: Amplitude effect

System parameters: $\mu = 0.2$, $\gamma = 1$, $\lambda = 1$, $\zeta = 0.01$, $\alpha_1 = \alpha_{12} = 0.001$

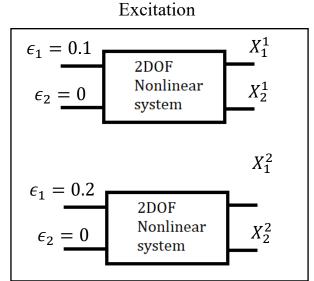
Natural frequencies (for linear system): $0.9\omega_n$, $2.49\omega_n$

Solution parameters: $N_h = 80$, N = 200

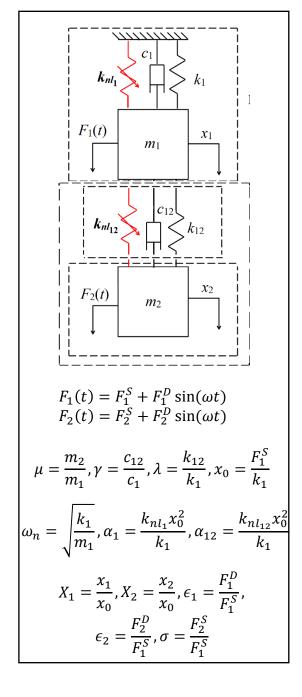


Frequency response curve: X_1 (near first natural frequency)

 $\begin{array}{c}
\underline{\text{Legend}} \\
X_1^1 \\
(0.5)X_1^2
\end{array}$



Unlike a linear system, the response of the given nonlinear system cannot be scaled

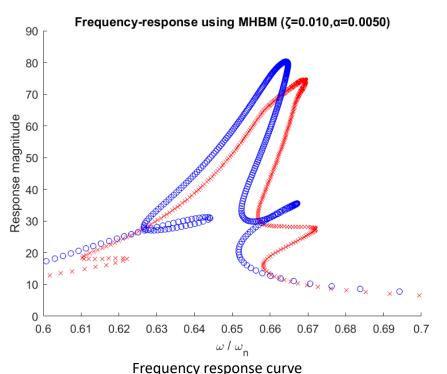


Appendix III: Reciprocity

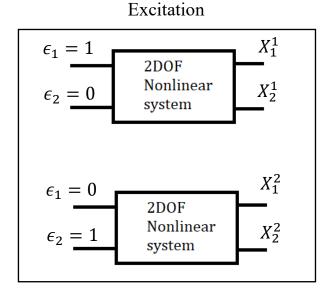
System parameters: $\mu = 1$, $\gamma = 1$, $\lambda = 1$, $\zeta = 0.01$, $\alpha_1 = 0$, $\alpha_{12} = 0.005$

Natural frequencies (for linear system): $0.62\omega_n$, $1.62\omega_n$

Solution parameters: $N_h = 80$, N = 200



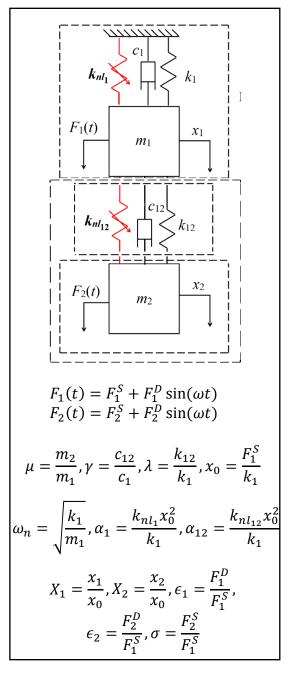
 $\frac{\text{Legend}}{X_1^2}$ $\frac{X_1^2}{X_2^1}$



Reciprocity doesn't apply to the given nonlinear system

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(near first natural frequency)



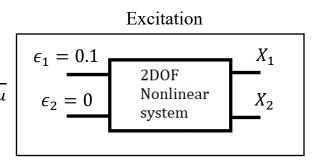
Appendix III: Numerical solution ($\alpha = 0.1$)

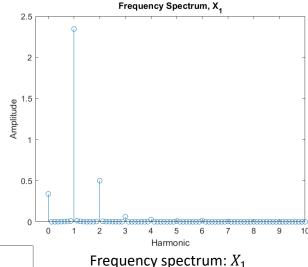
System parameters: $\mu = 0.2, \gamma = 0.2, \lambda = 0.2, \zeta = 0.005, \sigma = \frac{\mu}{1+\mu}$

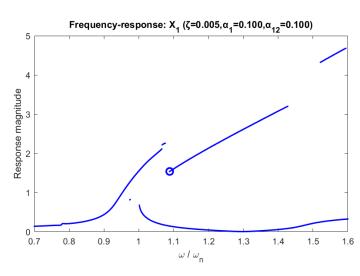
 $\alpha_1 = 0.1, \alpha_{12} = 0.1$

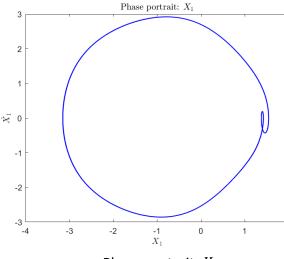
Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

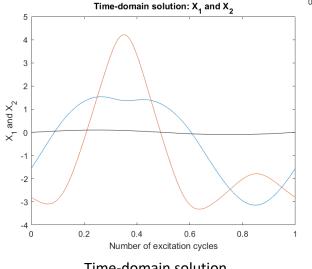
Solution parameters: $N_h = 80$, N = 200

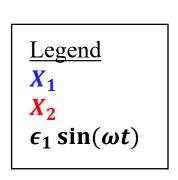












Frequency response curve: X_1 (near first natural frequency)

Phase portrait: X_1

Time-domain solution

Numerical solution (using Runge-Kutta method) is computed at $\omega = 1.09\omega_n$ (blue marker)



At higher nonlinearity levels, the response contains multiple harmonics

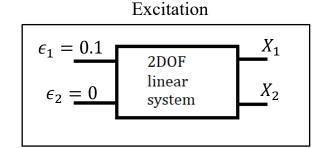
Appendix IV: Softening-type path (α =0.01)

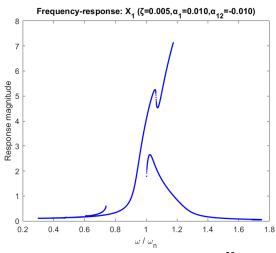
System parameters: $\mu = 0.2, \gamma = 0.2, \lambda = 0.2, \zeta = 0.005, \sigma = \frac{\mu}{1+\mu}$

$$\alpha_1 = 0.01$$
, $\alpha_{12} = -0.01$

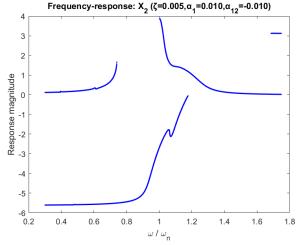
Natural frequencies (for linear system): $0.8\omega_n$, $1.25\omega_n$

Solution parameters: $N_h = 80$, N = 200

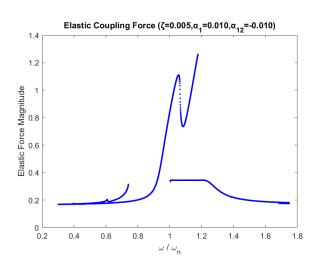








Frequency response curve: X_2

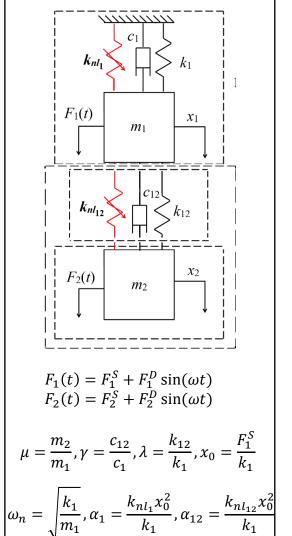


Elastic coupling force vs ω

The backbone curves (X_1) appear to collapse into each other

Challenge: Isolating nonlinear effects from numerical errors in the MHBM solution





$$X_1 = \frac{x_1}{x_0}, X_2 = \frac{x_2}{x_0}, \epsilon_1 = \frac{F_1^D}{F_1^S},$$