

# Continuum Mechanics - Hand In 1

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March 2023

## 1 Theoretical Part

### 1.1

Calculate the area moment (I) of the shelf (assume the size in the x-direction is 1m)

$$I = \int dA y^2 = \int_{-1/2}^{1/2} dx \int_{-H/2}^{H/2} dy y^2 = 1 \cdot [H^3/8 - (H^3/8)] = H^3/12 \quad (1)$$

### 1.2

The ice shelf is influenced by gravity and water pressure. Write an expression for the transverse resultant ( $K_y$ ) of the distributed external forces acting on a small piece 'dz'. (Assume that the shelf is always partially submerged – i.e y is small.)

I am unsure what a transverse resultant is, so but I guess it is the " $K_y$ " I will be using the next subpart, ie. the total resulting force per "dz-slice".

I know that for every small piece of rod we have:

$$dzF = F_{water} - F_g \quad (2)$$

Where

$$F_{water} = [H_w + y(z)] \cdot \rho_{water} \cdot dz \quad (3)$$

$$F_g = 1 \cdot H\rho_{ice} \cdot g \cdot dz \quad (4)$$

### 1.3

Write an ordinary differential equation for the deflection ( $y(z)$ ).

I now use the following set of equations from the book and the  $K_y$  in the last sub-question

$$\mathcal{M}_x = -EI \frac{d^2 y}{dz^2} = -E \frac{H^3}{12} \frac{d^2 y}{dz^2}, \quad \mathcal{F}_y = \frac{d\mathcal{M}_x}{dz}, \quad K_y = -\frac{d\mathcal{F}_y}{dz}. \quad (5)$$

we get:

$$k - ay(z) = b \cdot \frac{\partial^4 y(z)}{\partial z^4} \quad (6)$$

where we have defined  $k = 1 \cdot H\rho g - H_w\rho_w g$ ,  $a = \rho_w g$  and  $b = EH^3/12$  for ease of reading.

## 1.4

This is solved by

$$y(z) = c_1 e^{\sqrt[4]{-1} \sqrt[4]{a/b} z} + c_2 e^{\sqrt[4]{-1} \sqrt[4]{a/b} z} + c_3 e^{\sqrt[4]{-1} \sqrt[4]{a/b} z} + c_4 e^{\sqrt[4]{-1} \sqrt[4]{a/b} z} + k/a \quad (7)$$

Clearly  $1/\sqrt[4]{a/b} = 1/\sqrt[4]{\frac{12\rho_w g}{EH^3}} \approx 637m$  must be the significant lengthscale (this is within the ice etc.)

## 1.5

Finding the moment means integrating the 'slice' of the rod, multiplying the pressure-forces by their respective lever (the same way that gives rise to the  $I = \int dy y^2$ ):

$$\mathcal{M}_x(L) = \int_{-H/2}^{H/2} dy y \cdot \rho_{ice} g y - \int_{-H/2}^{H_w + y(L) - H/2} dy y \cdot \rho_w y \cdot g(H_w + y(L) - H/2) \quad (8)$$

Where I am assuming no air pressure.

## 1.6

The boundary conditions must be no movement at the left boundary:

$$y(0) = 0 \quad (9)$$

no bending at the left boundary:

$$\frac{dy}{dz}(0) = 0 \quad (10)$$

from 1.3 and (8):

$$y''(L) = -\mathcal{M}_x/EI \quad (11)$$

Since the book kindly reminds us that  $y'''(z)$  corresponds to the resulting force and we want the rod to be in equilibrium, I get the final boundary condition:

$$y'''(L) = 0 \quad (12)$$

# 2 Practical Part

## 2.1

With the boundary conditions specified, solve the elastic problem by manually plugging in different sea-level heights,  $H_w$ , until  $H_w$  is found that gives an (approximately) minimal vertical shelf displacement (i.e. the shelf does not change shape). Notice that there will always be some displacement since the rectangle is not a proper equilibrium shape.

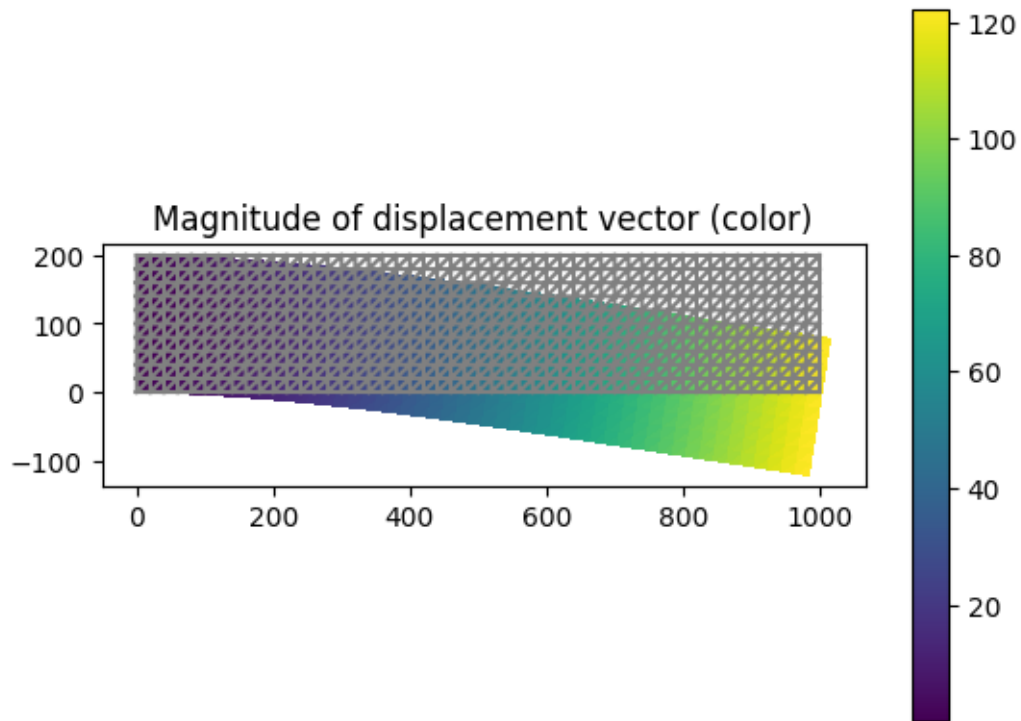


Figure 1: normal bend

Because overkill-solutions are more fun than not having Acute Stress, I wrote a simple gradient descent algorithm to find the water height that gives the minimal deflection.

I got 178.6

(this deflection can be noted to be just about  $height \cdot \rho_w / \rho_{ice}$  which makes sense, as the leftmost boundary in this case is negligible (Saint-Venant's principle maybe?))

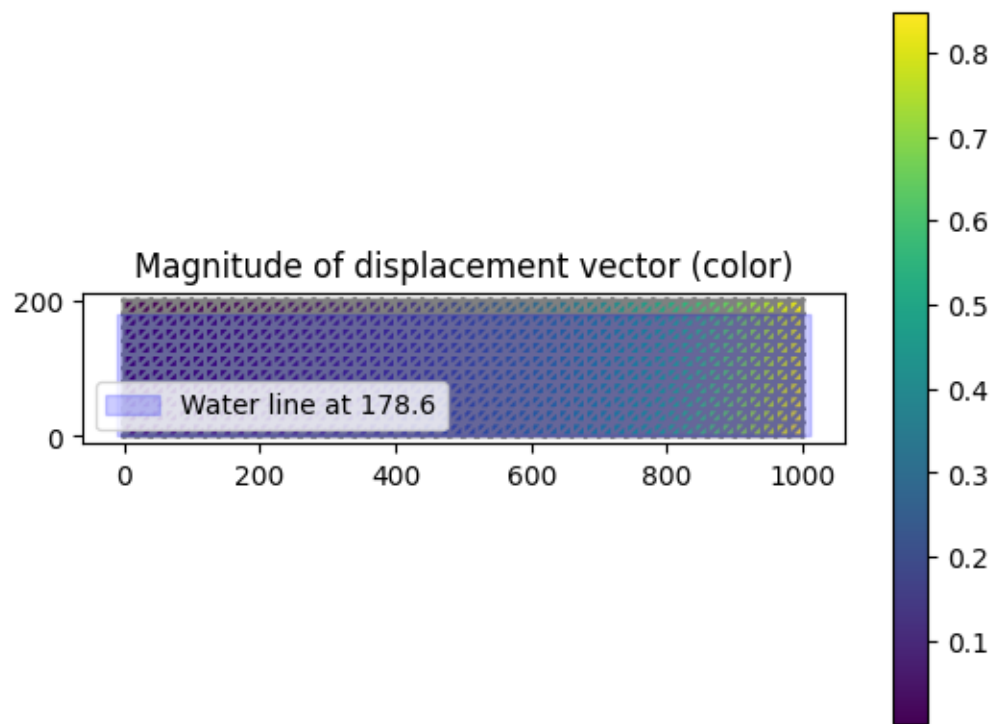


Figure 2: The minimal displacement

## 2.2

With the new shelf geometry, re-determine  $H_w$  such that the vertical displacement is minimal.

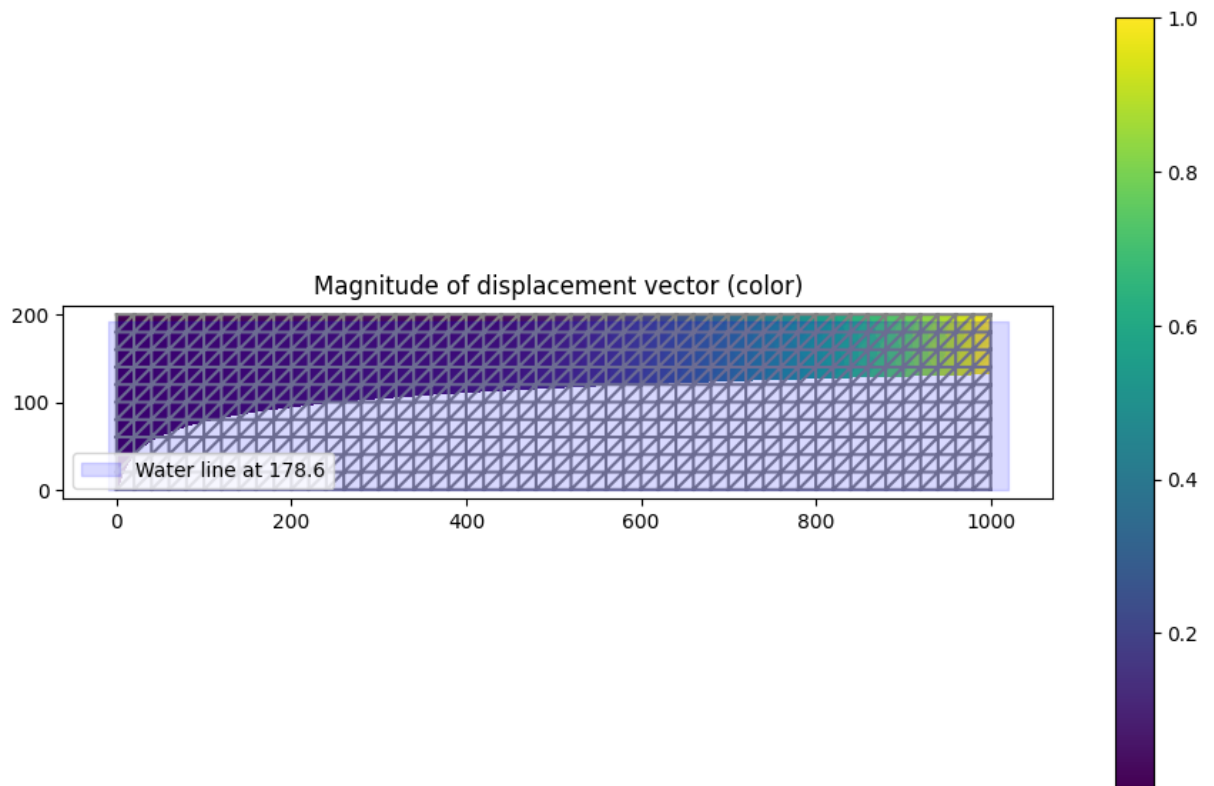


Figure 3: The minimal displacement for the full ice-shelve

## 2.3

How does the shelf displacement respond to a +1 m tide (i.e.  $H_w \rightarrow H_w + 1$  m) compared to the rectangular shelf?

max displacement for normal goes from 0.0232 to 0.102

max displacement for full goes from 0.0272 to 3.48

So the 'full' responds much worse!

## 2.4

Where does the shelf experience the greatest internal stresses? Plot the scalar stress measure "the von Mises stress"

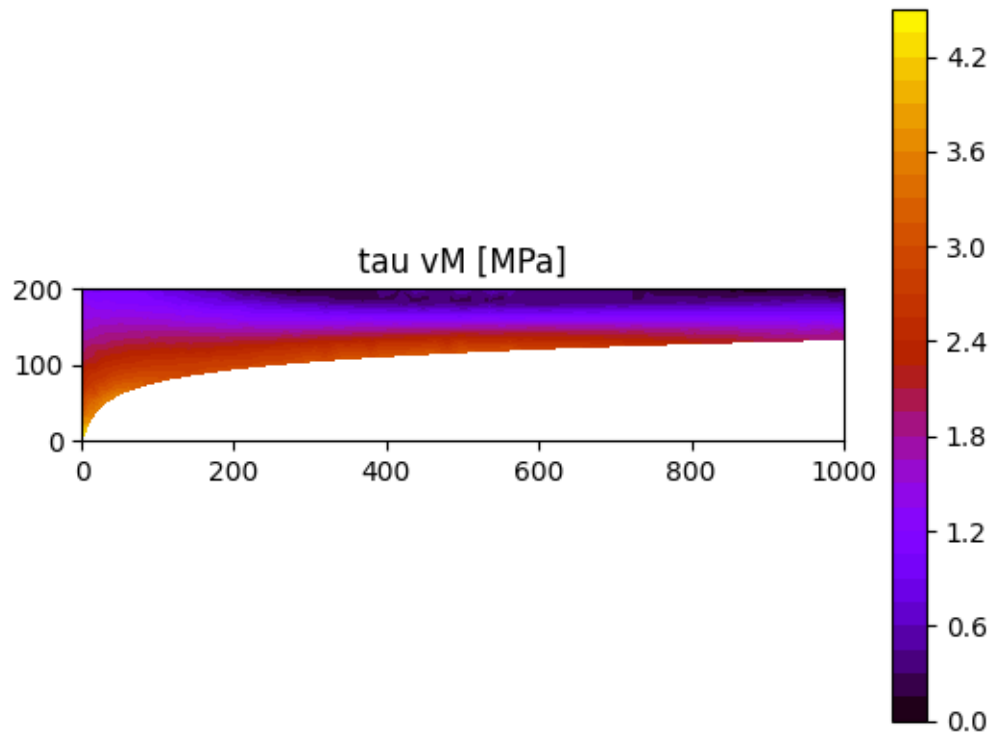


Figure 4: The von mises stress plottet on the original figure

## 2.5

For very large tides, is the displacement solution the new stable position? Hint: is the pressure at the ice–sea interface constant as the shelf lifts?

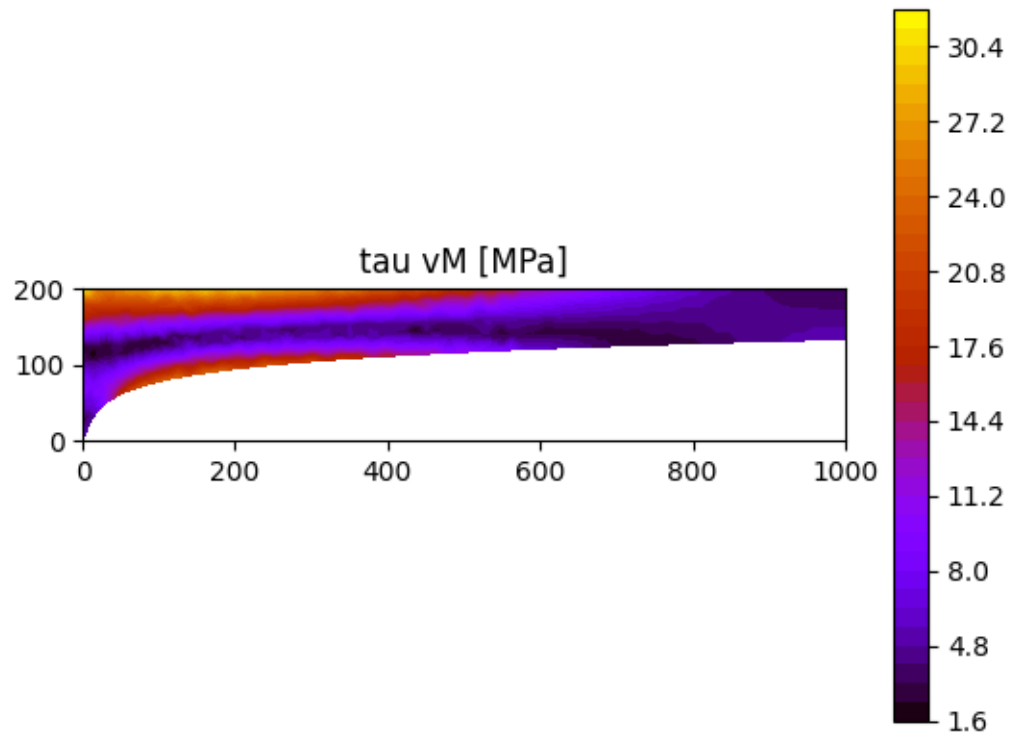


Figure 5: hw + 100 m

## 2.6

Re-determine Hw such that the displacement is minimal. For a 1 m tide, where is the shelf most likely to eventually break of (calve)?

hw = 191.9

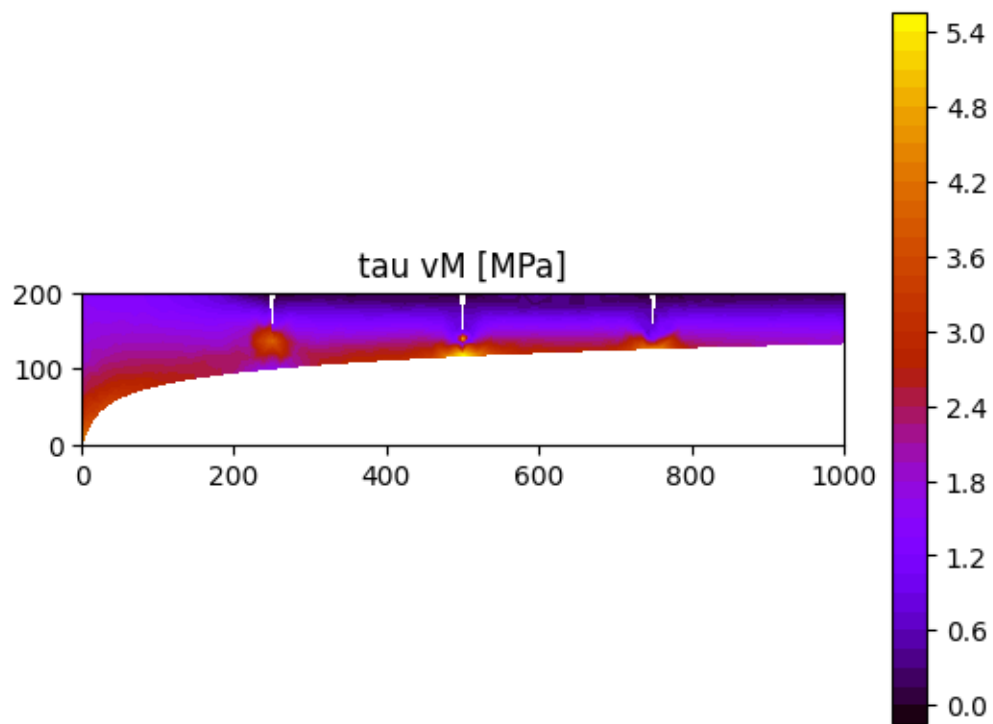


Figure 6: At water = hw = 192



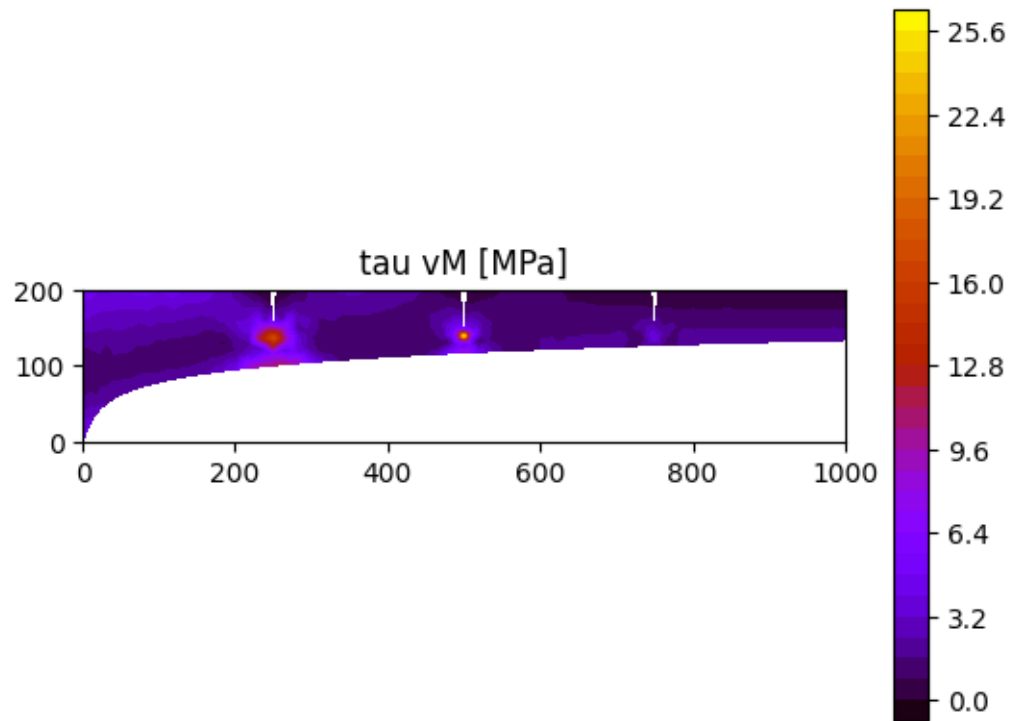


Figure 7: After 1 tide

Most likely will break at middle breaking point since it experiences highest stress there.

2.7

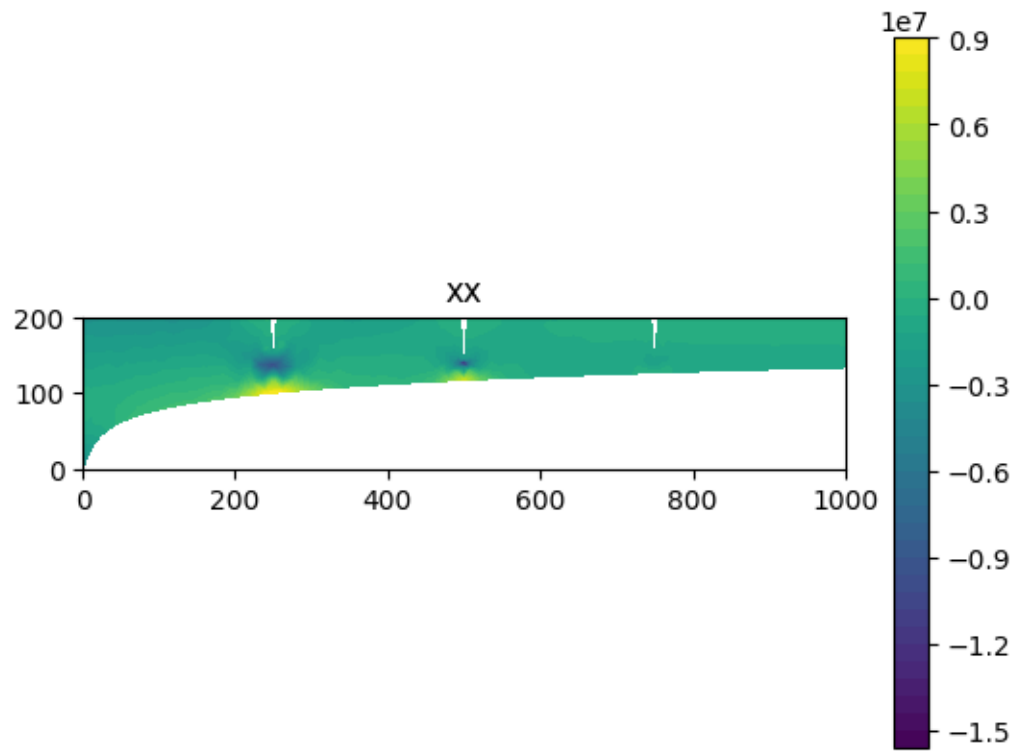


Figure 8: Plot for  $\tau_{xx}$

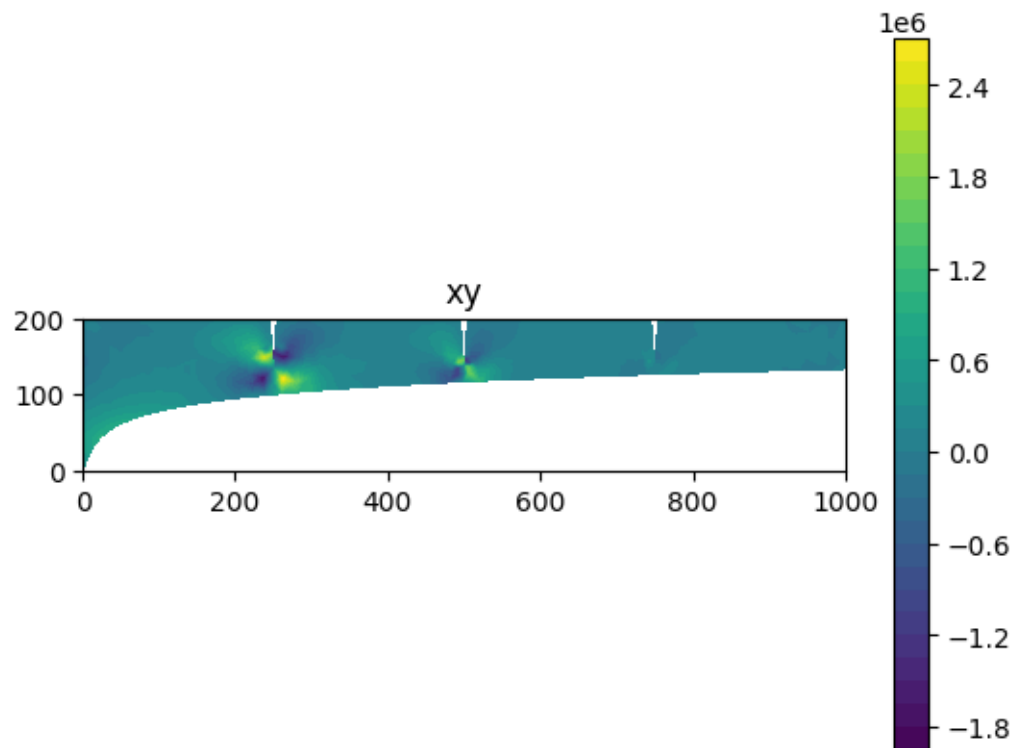


Figure 9: Plot for  $\tau_{xy}$  (where we know that  $xy = yx$ )

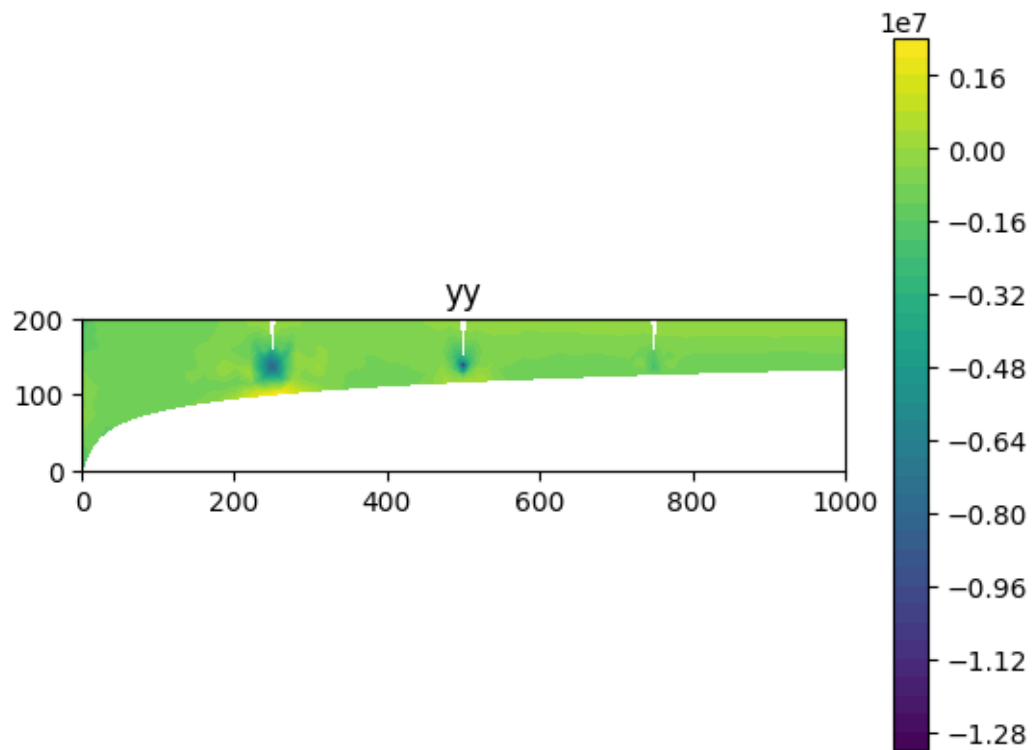


Figure 10: Plot for  $\tau_{yy}$

## 2.8

How does the reduced shelf respond to tides compared to the longer shelf?

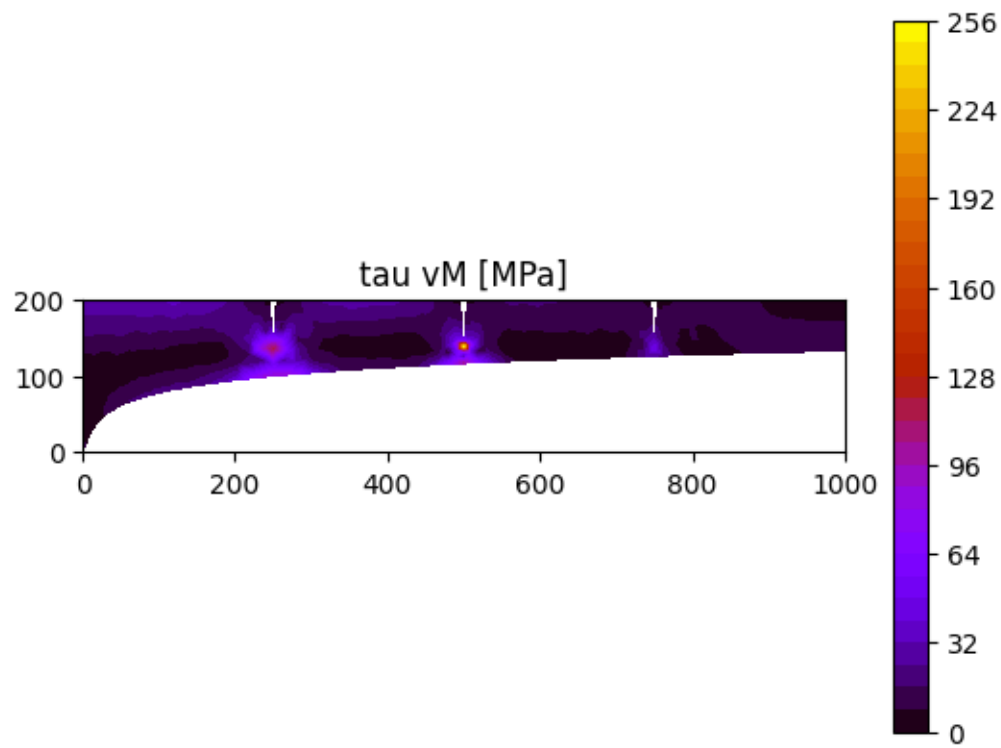


Figure 11: Caption

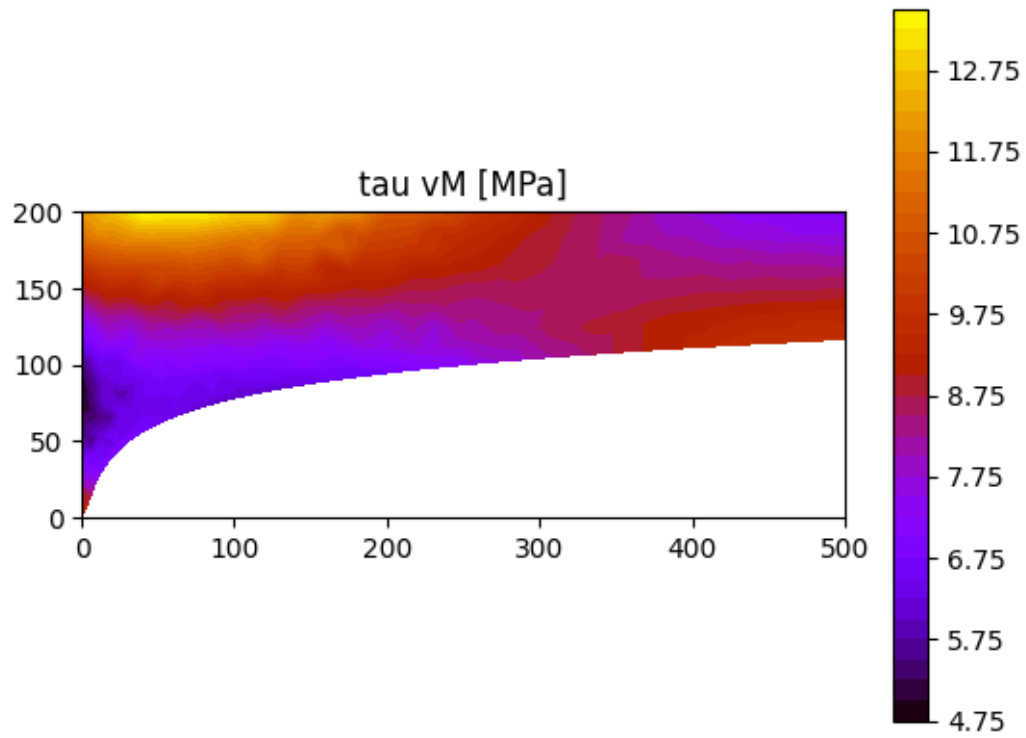


Figure 12: Caption

Von Mises stress is 20 times higher in the broken but uncalved.