Numerical Aspects of a Phase Field Model for Fracture Mechanics

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- Fracture Mechanics
- Phase Field Model
- Numerical Examples
- 4 Conclusion
- Future Scope



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Fracture Mechanics



 Griffith employed an energy-balance approach to find the total strain energy (U) in a linear material:

$$U = -\frac{\sigma^2}{2E} . \pi a^2 \tag{1}$$

 σ : Cauchy's stress; E: Young's modulus; a: length of crack; $\beta=\pi$ (plane stress loading)

• Surface energy (S) absorbed to create new surfaces (γ : surface energy):

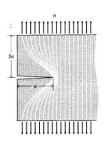


Figure 1: Idealization of unloaded region

$$S = 2\gamma a \tag{2}$$

Value of critical crack length:

$$\frac{\partial(S+U)}{\partial a} = 2\gamma - \frac{\sigma^2}{E}\pi a = 0 \tag{3}$$

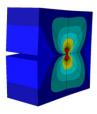
Fracture Mechanics



Solving:

$$\sigma = \sqrt{\frac{2E\gamma}{\pi a}} \tag{4}$$

 Including sinks like energy dissipation due to plastic flow near the crack tip:



$$\sigma = \sqrt{\frac{EG_c}{\pi a}}$$

Figure 2: A fractured module

 G_c : critical strain energy release rate

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 Most of the approaches in fracture models represent cracks as discrete discontinuities, except phase field model.



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- In phase field model, defect e.g. crack is approximated by a phase-field, which smoothens the boundary of a crack over a small region.
- Major advantage is that evolution of the crack follows from solution of a coupled system of Partial Differential Equations.

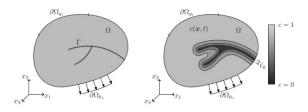


Figure 3: A phase field model



• Elastic energy (ψ) is defined as:

$$\psi(\epsilon) = [(1 - \phi)^2 + k]\psi^+ + \psi^-$$
 (6)

$$\psi^{\pm}(\epsilon) = \frac{1}{2} \lambda \langle tr(\epsilon) \rangle_{\pm}^{2} + \mu tr(\epsilon_{\pm}^{2})$$
 (7)

with

$$\langle x \rangle_+ = \begin{cases} x & x > 0 \\ 0 & x \leqslant 0 \end{cases}$$

 ϕ : phase-field parameter; k: viscous parameter; λ,μ : Lame parameter; $\epsilon_{\pm} = \sum_{I=1}^{3} \langle \epsilon_I \rangle_{\pm} \mathbf{n}_I \otimes \mathbf{n}_I$, ϵ_I : principle strains, \mathbf{n}_I : principle strain directions.

- Phase field variable (ϕ) is controlled by the tensile elastic energy ψ^+ , contributing to crack propagation.
- Compressive elastic energy ψ^- doesn't contribute to crack propagation.



• The boundary value problem in the absence of body force in a elastic body Ω is: find $(\boldsymbol{u},\phi):\Omega \to \mathbb{R}^d$.

$$[(1-\phi)^2 + k]\nabla \cdot \boldsymbol{\sigma} = 0 \tag{8}$$

$$-G_c I_0 \nabla^2 \phi + \left[\frac{G_c}{I_0} + 2H^+ \right] \phi = 2H^+ \tag{9}$$

 I_0 : width of failure zone; $\sigma = \frac{\partial \psi(\epsilon)}{\partial \epsilon}$;

$$H^+ = \begin{cases} \psi^+(\epsilon) & \text{if } \psi^+(\epsilon) < H_n^+ \\ H_n^+ & \text{otherwise, } (H_n^+ \text{ :tensile strain energy at load step n}) \end{cases}$$

• Boundary conditions: $[(1-\phi)^2+k]\nabla\cdot\boldsymbol{\sigma}=\bar{\boldsymbol{t}}$ on Γ_N , $\boldsymbol{u}=\bar{\boldsymbol{u}}$ on Γ_D , $\nabla\phi\cdot\boldsymbol{n}=0$ on Γ_N .

^{*}Hirshikesh, A FEniCS implementation of the phase field method for quasi-static brittle fracture,2018



- Staggered algorithm is used to solve weak forms:(spatial discretization of coupled equations)
 - Initialization: Displacement (\boldsymbol{u}), phase field (ϕ) and history (H) are known at initial time step.
 - 2 Compute History: It is obtained from it's definition at that particular time step.
 - **3** Compute Phase field: Phase field (ϕ) is solved using given displacement field (\boldsymbol{u}) .
 - **3** Compute Displacement field: Updated phase field (ϕ) is used to solve for displacement field (\boldsymbol{u}) .
- The numerical implementation of this algorithm has been done in FEniCS.



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Problem Statement (Shear Loading):

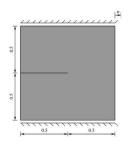


Figure 4: Plate with edge crack

- Given: Quasi-static shear loading in 2D plate with plane strain and existing crack.
- Unknowns : Displacement field (u) and phase field (ϕ) .
- Output: Crack propagation inside the plane.



Problem Statement (Shear Loading): Other given conditions-

- Griffith critical energy (G_c) = 2700 J/ m^2
- Poisson ratio $(\nu) = 0.3$
- Young's modulus (E) = 210 GPa
- Plane strain and existing crack.
- Width of failure zone $(l_0) = 0.011 \text{ mm}$
- Viscous damping parameter $(k) = 10^{-6}$.
- Staggered algorithm for displacement and phase field.

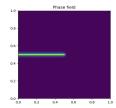


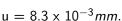
Case 1:

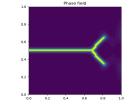
- Mesh is refined.
- History function used :

$$H(\boldsymbol{u}) = \frac{1}{2}k_n[tr(\epsilon)]^2 + \mu tr((\epsilon^{dev})^2)$$

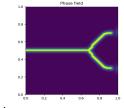
$$k_n = \lambda + \mu$$
; $\epsilon^{dev} = Deviatoric strain tensor.$







$$u = 15.5 \times 10^{-3} mm$$
.



$$u = 30 \times 10^{-3} mm$$
.

Figure 5: Crack progression at various loads increments

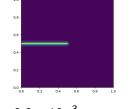


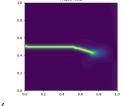
Case 2:

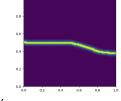
- Mesh is refined.
- History function used :

$$H(\boldsymbol{u}) = \frac{1}{2}k_n\Big[\frac{1}{2}\big[tr(\boldsymbol{\epsilon}) + abs(tr(\boldsymbol{\epsilon}))\big]\Big]^2 + \mu tr((\boldsymbol{\epsilon^{dev}})^2)$$

$$k_n = \lambda + \mu$$
; $\epsilon^{dev} = \text{Deviatoric strain tensor}$.







 $u = 8.3 \times 10^{-3} mm$.

$$\mu = 12.7 \times 10^{-3} mm$$
.

 $u = 15.5 \times 10^{-3} mm$.

Figure 6: Crack progression at various loads increments



Case 3:

- $I_0 = 7.5 \times 10^{-3} mm$.
- Mesh is refined.
- l_0 is a model parameter that controls the width of the smooth approximation of the crack.

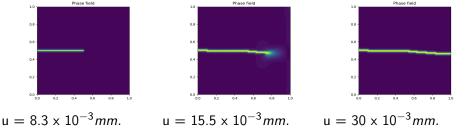


Figure 7: Crack progression at various loads increments





Problem Statement (Tension Loading):

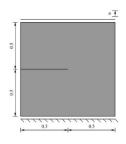


Figure 8: Plate with edge crack

- Given: Quasi-static Tension loading in 2D plate with plane strain and existing crack.
- Unknowns : Displacement field (\boldsymbol{u}) and phase field (ϕ) .
- Output: Crack propagation inside the plane.



Problem Statement (Tension Loading): Other given conditions-

- Griffith critical energy $(G_c) = 2700 \text{ J/}m^2$
- Poisson ratio $(\nu) = 0.3$
- Young's modulus (E) = 210 GPa
- Width of failure zone $(I_0) = 0.011 \text{ mm}$
- Plane strain and existing crack.
- Viscous damping parameter $(k) = 10^{-6}$.
- Staggered algorithm for displacement and phase field.

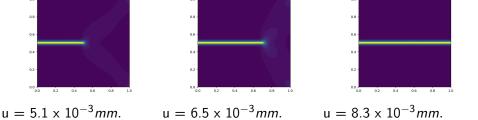


Case 1:

Mesh is refined.

Phase field

• History function used is same as earlier one.



Phase field

Figure 9: Crack progression at various loads increments



Python Code:

```
#### Spaces #####
W = VectorFunctionSpace(mesh, 'CG',1)
u , v = TrialFunction(W).TestFunction(W)
V = FunctionSpace(mesh, 'CG',1)
p , q = TrialFunction(V), TestFunction(V)
ESSES SESSES SESSES
#Coordinate system with bottom as reference axis:
class top(SubDomain):
    def inside(self,x,on_boundary):
        tol = 1e-10
        return abs(x[1]-1.0) < tol and on_boundary
class bottom(SubDomain):
    def inside(self,x,on boundary):
        tol = 1e-10
        return abs(x[1]) < tol and on boundary
class middle(SubDomain):
    def inside(self.x.on boundary):
        tol = 1e-3
        return abs(x[1]-0.5) < tol and x[0] <= 0.5
def epsilon(u):
    return 0.5*(grad(u) + grad(u).T)
def sigma(u):
    return lmbda*nabla div(u)*Identitv(2) + 2*mu*epsilon(u)
kn = lmbda + mu
def hist(u):
```

return 0.5*kn*(0.5*(tr(epsilon(u)) + abs(tr(epsilon(u)))))**2 + mu*tr(dev(epsilon(u))*dev(epsilon(u)))



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Conclusion



- Phase-field model provides a smooth representation of a crack and removes requirement to numerically track discontinuities in the displacement field.
- Results obtained are in good agreement with the standard results from the literature.



- 5 Future Scope

Future Scope



- Artificial viscous regularization can be used leading to more robust overall performance.
- An adaptive local refinement strategy that allows for efficient simulation of complex crack patterns can be used taking advantage of the character of the phase-field evolution to determine when refinement is needed.
- Phase field model can be extended and studied to the dynamic case as well.