

Boston University

EK301-A1

Professor Farny

Spring 2025

# Preliminary Design Lab Report

Jinyu Fang, Jessica Qiu, Riasat Audhy

Due Date: April 8, 2025

Submitted: April 8, 2025

# I. Introduction

---

For our preliminary design report, this project aims to use computational analysis to evaluate and optimize truss designs, helping ensure they meet specified load and cost requirements. By using a computer program, we can efficiently calculate internal member forces, identify which members are most at risk of buckling, and assess the overall cost-effectiveness of each design. This method allows us to explore multiple configurations, compare their strengths and weaknesses, and ultimately select a design that offers the best balance between performance and cost. In this report, we verify our computational model and present two preliminary truss designs for comparison.

A simple truss, which is the focus of this project, relies on members that experience only tension or compression. This design approach uses material efficiently while maintaining stability. The structure's strength comes from forming triangular units, with members connected only at their endpoints. A stable, statically determinate truss follows Equation (1):

$$M = 2J - 3 \quad (1)$$

where  $M$  is the number of members and  $J$  is the number of joints. There are exactly  $2J$  equations that can be obtained through equilibrium analysis of the horizontal and vertical forces at each joint, which means there must be exactly  $2J$  unknown forces for the system to be solvable. There are  $M$  unknown forces within the members, and 3 unknown reaction forces from the pin (horizontal and vertical) and rocker (vertical) supports. Setting these equal and rearranging provides Equation (1) to determine the number of members needed in the truss.

For this project, our truss must span 31 inches and support a 32 oz. load placed 8.5-12 inches from the pinned support. Additionally, each member must be between 8.5 and 14 inches long. Cost is another key constraint: the total virtual cost must remain under \$315, calculated using \$10 per joint and \$1 per inch of total member length. In developing our designs, we focused on the following key design principles:

1. Triangular geometry to maximize strength and stability.
2. Symmetry to ensure even load distribution and reduce stress imbalances.
3. Material efficiency by limiting the number of joints and total member length.
4. Shorter compression members to reduce the risk of buckling and increase load capacity.

These principles guided our design choices as we explored different truss layouts. In the sections that follow, we verify our analysis method and present a detailed comparison of our two preliminary designs.

## II. Method and Analysis

---

In the Appendix A.1, the MATLAB script was developed to analyze multiple truss designs efficiently. It accepts user input for the number of joints, member-to-joint connections, support force locations, joint coordinates, and applied loads. Using this data, the code constructs the connection matrix (C) and support matrices, then calculates matrix A, which contains the direction cosines of each truss member based on the geometry and connectivity of the structure. The system of equations representing static equilibrium is then solved using the formula  $T = \text{inv}(A) * L$ , where T is the vector of internal member and support forces, and L is the external load vector. Additionally, the code estimates the maximum theoretical load the truss can support before any member fails due to buckling, and evaluates the cost-efficiency of the design by computing the load-to-cost ratio. A negative value corresponds to compression force, and a positive value corresponds to tension force. As a result, our result by hand matches our result calculated by computer code, indicating the accuracy of our computational code.

Our approach was consistent with the procedure described in the lab manual. First, we analyzed the diagram and numbered each joint (j) and each member (m) of the truss. From there, we created six separate matrices:

**C Matrix-** This matrix has j rows for joints and m columns for members. If a joint is connected to a member, a '1' is placed in the intersecting cell; all other cells contain '0'. We verified the matrix by ensuring each column has exactly two '1's.

$$C_{j,m} = \begin{cases} 1, & \text{if member } m \text{ is connected to joint } j \\ 0, & \text{elsewhere} \end{cases}$$

**Sx Matrix-** A matrix with j rows and three columns, capturing only the reaction forces at the pin joint and roller joint at the base of the truss, specifically in the x-direction.

$$S_x = \begin{matrix} & S_{x1} & S_{y1} & S_{y2} \\ \begin{bmatrix} 1 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \end{matrix}$$

**Sy Matrix-** Similar in size and purpose to Sx, but focused on y-axis interactions. Column 1 is filled with 0s, while column 2 has entries for the pin joint and roller joint.

$$S_y = \begin{matrix} & S_{x1} & S_{y1} & S_{y2} \\ \begin{bmatrix} 0 & 1 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix} \end{matrix}$$

**X Matrix-** 1 x j matrix that stores the x-coordinates for each joint. The origin (0,0) was set at the bottom-left joint for this analysis.

$$\mathbf{X} = [x_1, x_2, x_3, x_4, \dots]$$

**Y Matrix-** 1 x j matrix that stores the y-coordinates for each joint, using the same origin as the 'X' matrix.

$$\mathbf{Y} = [y_1, y_2, y_3, y_4, \dots]$$

**L Matrix-** 2j x 1 matrix to store the location and magnitude of external forces on the truss. The first j rows represent forces in the x-direction (none for this project), while the second j rows contain y-direction forces, where we recorded the weight's magnitude (in oz.) at the appropriate joint + j row.

$$\mathbf{L} = \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ +mg \\ 0 \\ 0 \\ 0 \end{bmatrix} \begin{array}{l} \text{no horizontal load at J1} \\ \text{no horizontal load at J2} \\ \dots \\ \dots \\ \dots \\ \text{no vertical load at J1} \\ \text{load } mg \text{ at J2} \\ \text{no vertical load at J3} \\ \dots \\ \dots \end{array}$$

With our matrices ready, we built the 'A' matrix, a 2j x (m+3) matrix. The first 'j' rows represent interactions in the x direction, and the second 'j' rows cover interactions in the y direction. Each cell is filled with the force coefficients corresponding to the member tension at each joint, starting with forces along the x-axis in rows 1 to 'j' and ending with forces along the y-axis in rows 'j+1' to '2j'

$$\sum F_{x,1} : \left( \frac{x_2 - x_1}{r_{1,2}} \right) T_1 + \left( \frac{x_3 - x_1}{r_{1,3}} \right) T_2 + 0 \cdot T_3 + 0 \cdot T_4 \\ + 0 \cdot T_5 + 0 \cdot T_6 + 0 \cdot T_7 + 1 \cdot S_{x,1} + 0 \cdot S_{y,1} + 0 \cdot S_{y,2} = 0$$

$$\sum F_{y,1} : \left( \frac{y_2 - y_1}{r_{1,2}} \right) T_1 + \left( \frac{y_3 - y_1}{r_{1,3}} \right) T_2 + 0 \cdot T_3 + 0 \cdot T_4 \\ + 0 \cdot T_5 + 0 \cdot T_6 + 0 \cdot T_7 + 0 \cdot S_{x,1} + 1 \cdot S_{y,1} + 0 \cdot S_{y,2} = 0$$

Then using the values in A and L, we solved for the unknown forces of T:

$$\mathbf{T} = \mathbf{A}^{-1}(\mathbf{L}). \quad (2)$$

To validate the accuracy of the truss analysis program, a sample truss was analyzed manually, and its parameters (joint locations, member connections, and live load) were input into the program. The results generated by the program (Fig. 2) were compared with the manual

calculations (Fig. 1), and both methods yielded identical results. This confirmed that the program was capable of accurately calculating the forces acting on each truss member.

Following this validation, the main truss design process was initiated using an iterative approach made possible by automating the analysis. In line with the given specifications, two truss designs were created. Each design focused on minimizing member length to enhance buckling strength while ensuring that members followed the parameters of the given project and support a 32 oz load:

#### Summary of distance and cost specifications

Joint-to-joint span $L_{jj}$	$8.5 \text{ in.} \leq L_{jj} \leq 14 \text{ in.}$
Truss span	$31 \text{ in.}$
Load to pin support span	$8.5\text{-}12 \text{ in.}$
Total virtual cost	$< \$315$

$$\text{Cost} = C_1 J + C_2 L, \quad \text{where: } C_1 = \$10/\text{joint} \quad \& \quad C_2 = \$1/\text{in.}$$

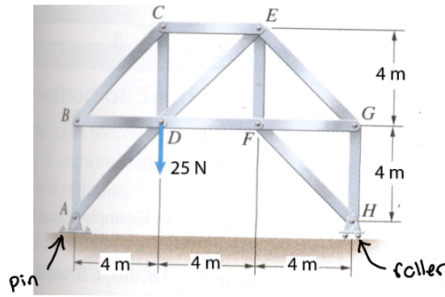
With this information we are able to run analysis on varying designs to determine, max force load, critical members, load to cot ratio, as well as calculate the overall cost of the trust.

## By hand:

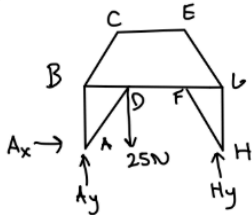
BOSTON UNIVERSITY  
College of Engineering  
EK 301 Engineering Mechanics I

### Truss project computational method validation problem

Determine the loads in each of the members and whether they are in tension or compression. Analyze the loads yourselves using standard equilibrium analysis, and MATLAB (results should match!).



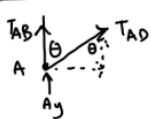
FBD



$$\begin{aligned}\sum F_x &= A_x = 0 \\ \sum F_y &= A_y + H_y - 25 = 0 \\ \sum M_A &= -25(4) + H_y \cdot 12 = 0\end{aligned}$$

$$\begin{aligned}H_y &= 8.33 \text{ N} \\ A_y &= 16.67 \text{ N} \\ A_x &= 0 \text{ N}\end{aligned}$$

FBD: A



$$\theta = \tan^{-1}\left(\frac{4}{4}\right) = 45^\circ$$

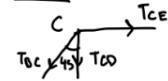
$$\begin{aligned}\sum F_x &= T_{AD} \sin \theta = 0 \\ \sum F_y &= T_{AD} \cos \theta + T_{AB} + A_y = 0 \\ T_{AD} &= 0, A_y = 16.67 \text{ N} \\ T_{AB} &= -16.67 \text{ N}\end{aligned}$$

FBD: B



$$\begin{aligned}\theta &= \tan^{-1}\left(\frac{4}{4}\right) = 45^\circ \\ \sum F_x &= T_{BC} \cos \theta + T_{BD} = 0 \\ \sum F_y &= T_{BC} \sin \theta - T_{AB} = 0 \\ -16.67 &= T_{BC} \sin 45^\circ \\ T_{BC} &= -23.57 \text{ N}, T_{BD} = 16.67 \text{ N}\end{aligned}$$

FBD: C



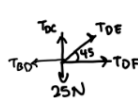
$$\begin{aligned}\sum F_x &= T_{CE} - T_{BC} \sin 45^\circ = 0 \\ \sum F_y &= -T_{BC} \cos 45^\circ - T_{CD} = 0 \\ T_{CE} &= -16.67 \text{ N} \\ T_{CD} &= 16.67 \text{ N}\end{aligned}$$

FBD: H



$$\begin{aligned}\sum F_x: -T_{HF} \sin 45^\circ &= 0 \\ T_{HF} &= 0 \text{ N} \\ \sum F_y: T_{HG} + H_y &= 0 \\ T_{HG} &= -H_y \\ T_{HG} &= -8.33 \text{ N}\end{aligned}$$

FBD: D

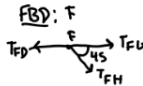


$$\begin{aligned}\sum F_x: T_{DF} + T_{DE} \cos 45^\circ - T_{DD} &= 0 \\ \sum F_y: T_{DC} + T_{DE} \sin 45^\circ - 25 &= 0 \\ T_{DE} &= 11.79 \text{ N} \\ T_{DF} &= 8.33 \text{ N}\end{aligned}$$

FBD: E



$$\begin{aligned}\sum F_x: T_{EL} \cos 45^\circ - T_{ED} \cos 45^\circ - T_{EC} &= 0 \\ T_{EL} &= -11.79 \text{ N} \\ \sum F_y: -T_{ED} \sin 45^\circ - T_{EL} \sin 45^\circ - T_{EF} &= 0 \\ T_{EF} &= 0 \text{ N}\end{aligned}$$



$$\begin{aligned}\sum F_x: -T_{FD} + T_{FL} + T_{FH} \cos 45^\circ &= 0 \\ \sum F_y: -T_{FH} \sin 45^\circ &= 0 \\ T_{FH} &= 0 \text{ N} \\ T_{FL} &= 8.33 \text{ N}\end{aligned}$$

## Results:

### Force in member:

1.  $AB = 16.7 \text{ N}$  in Compression
2.  $AD = 0 \text{ N}$
3.  $BC = 23.6 \text{ N}$  in Compression
4.  $BD = 16.7 \text{ N}$  in tension
5.  $CD = 16.7 \text{ N}$  in tension
6.  $CE = 16.7 \text{ N}$  in Compression
7.  $DE = 11.8 \text{ N}$  in tension
8.  $DF = 8.33 \text{ N}$  in tension
9.  $EF = 0 \text{ N}$
10.  $EG = 11.8 \text{ N}$  in Compression
11.  $FG = 8.33 \text{ N}$  in tension
12.  $FH = 0 \text{ N}$
13.  $GH = 8.33 \text{ N}$  in Compression

Figure 1: Calculation performed for "Example Problem"

### By computer code:

```
EK301, Section A1, Group6: Jessica Q. Jinyu F. Riasat A., 3/22/2025
Load: 25 oz
Member forces in oz:
m1: -16.667 (C)
m2: -0.000 (C)
m3: -23.570 (C)
m4: 16.667 (T)
m5: 16.667 (T)
m6: -16.667 (C)
m7: 11.785 (T)
m8: 8.333 (T)
m9: 0.000 (T)
m10: -11.785 (C)
m11: 8.333 (T)
m12: -0.000 (C)
m13: -8.333 (C)
Reaction forces in oz:
Sx1: 0.000
Sy1: 16.667
Sy2: 8.333
```

Figure 2: Example MATLAB Program Output

Figure 2 includes the final computer code output producing the information on the load applied, forces in each member, reaction forces, cost of the truss, and the theoretical max load/cost ratio. Given the 25 N load at joint D, the forces between each member are shown in Table 2 as a final result.

*Table 2: The forces of each member when experiencing 25N load*

<b>Member</b>	<b>Tension/Compression</b>	<b>Magnitude of the Force (N)</b>
AB	Compression	16.7
AD	-	0.0
BC	Compression	23,6
BD	Tension	16.7
CD	Tension	16.7
CE	Compression	16.7
DE	Tension	11.8
DF	Compression	8.33
EF	-	0
EG	Compression	11.8
FG	Tension	8.33
FH	-	0
GH	Compression	8.33



### III. Data & Design

#### Design #1:

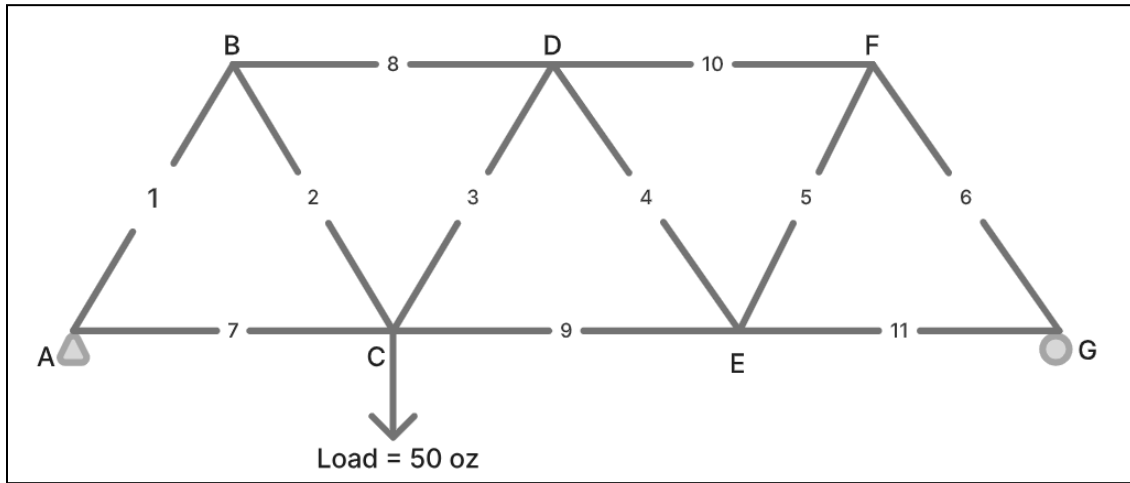


Figure 5: First Truss Design with A as a pin support, G as a roller, and a load of 50 oz on joint C.

From the Acrylic Data Fit provided, the fit error bar is a constant that results in  $U_{\text{fit}} = \pm 1.35$  oz (95%). Uncertainty was calculated based on this value, as a 5% of the buckling strength for each member. Using the code in Appendix A.2, we get the result in Table 3.

Table 3: Internal member forces, lengths, and buckling strengths for Design 1 under a 50 oz load.

Member Number	Member Length (inches)	Tension/Compression	Buckling Strength and Uncertainty ( $\pm$ oz)	Magnitude of the Force at Max Truss load (oz)
1	9.304	Compression	$42.09 \pm 1.35$	40.239
2	9.304	Tension	—	40.239
3	9.304	Tension	—	17.82
4	9.304	Compression	$42.09 \pm 1.35$	17.82
5	9.343	Tension	—	18.069
6	10.63	Compression	$33.06 \pm 1.35$	20.36
7	9.50	Tension	—	20.59

8	9.50	Compression	$40.53 \pm 1.35$	41.179
9	9.50	Tension	—	32.082
10	9.75	Compression	$38.66 \pm 1.35$	22.984
11	12.00	Tension	—	13.407

**Maximum load:** 50 oz  $\pm$  1.35 oz

**Member to buckle first:** Member 8

**Reaction forces in Oz:**

- Sx1 (Pin at A): 0.000 oz
- Sy1 (Pin at A): 34.677 oz
- Sy2 (Roller at G): 15.323 oz

**Cost of the truss:** \$177.53

**Theoretical Max Load/ Cost Ratio in Oz/\$:** 0.282 oz/\$

EK301, Section A1, Group6: Jessica Q. Jinyu F. Riasat A., 3/22/2025 ----- Load: 50 oz Member forces in oz: m1: -40.329 (C) m2: 40.329 (T) m3: 17.820 (T) m4: -17.820 (C) m5: 18.069 (T) m6: -20.360 (C) m7: 20.590 (T) m8: -41.179 (C) m9: 32.082 (T) m10: -22.984 (C) m11: 13.407 (T)  Reaction forces in oz: Sx1: 0.000 Sy1: 34.677 Sy2: 15.323  ----- The member to buckle first is member 8 Length of that member: 9.50 in Predicted buckling strength: 40.53 oz $\pm$ 1.35 oz The maximum load that the physical truss could support is 50 oz $\pm$ 1.35 oz  ----- Cost of the truss: \$177.53 Theoretical max load/cost ratio in oz/\$: 0.282 -----		Member Lengths (in inches): Member 1: 9.304 in Member 2: 9.304 in Member 3: 9.304 in Member 4: 9.304 in Member 5: 9.434 in Member 6: 10.630 in Member 7: 9.500 in Member 8: 9.500 in Member 9: 9.500 in Member 10: 9.750 in Member 11: 12.000 in  Buckling Strengths for Compression Members: -----   Member   Length (in)   Force (oz)   Buckling Strength $\pm$ Unc. -----   m1   9.304   -40.329   42.09 $\pm$ 1.35     m4   9.304   -17.820   42.09 $\pm$ 1.35     m6   10.630   -20.360   33.06 $\pm$ 1.35     m8   9.500   -41.179   40.53 $\pm$ 1.35     m10   9.750   -22.984   38.66 $\pm$ 1.35	
---	--	---	--

*Figure 6: Design 1 MATLAB Program Output*

## Design #2:

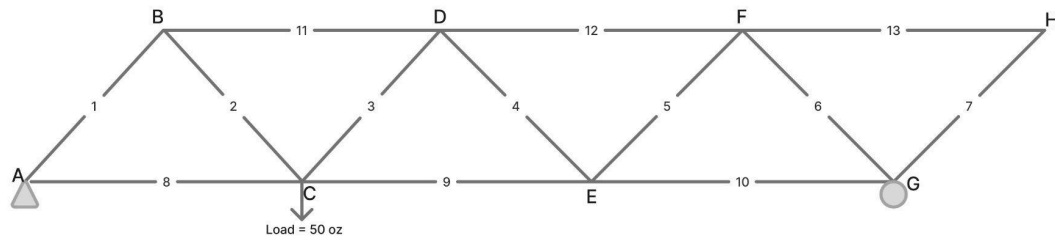


Figure 7: Second Truss Design with pin support at A, roller at G, and a load of 50 N on joint C.

From the Acrylic Data Fit provided, the fit error bar is a constant that results in  $U_{fit} = \pm 1.35$  oz (95% confidence). The uncertainty was calculated based on this value, as a 5% of the buckling strength for each member. Using the code provided in Appendix A.2, we get the result in Table 4.

Table 4: Internal member forces, lengths, and buckling strengths for Design 2 under a 50 oz load.

Member Number	Member Length (inches)	Tension/Compression	Buckling Strength and Uncertainty ( $\pm$ oz)	Magnitude of the Force at Max Truss load (oz)
1	11.835	Compression	$27.01 \pm 1.35$	27.551
2	11.835	Compression	$27.01 \pm 1.35$	27.551
3	11.835	Tension	—	0
4	11.835	Compression	$27.01 \pm 1.35$	0
5	11.835	Tension	—	0
6	11.835	Compression	$27.01 \pm 1.35$	0
7	11.705	Tension	—	0
8	9.000	Tension	—	10.432
9	9.000	Tension	—	0

10	9.000	Tension	—	0
11	9.000	Compression	—	0
12	9.000	Tension	—	0
13	8.5	Compression	$27.01 \pm 1.35$	0

**Maximum load:** 51 oz  $\pm$  1.35 oz

**Member to buckle first:** Member 2

**Reaction forces in Oz:**

- Sx1 (Pin at A): 0.000 oz
- Sy1 (Pin at A): 25.500 oz
- Sy2 (Roller at G): 25.500 oz

**Cost of the truss:** \$216.51

**Theoretical Max Load/ Cost Ratio in Oz/\$:** 0.236 oz/\$

EK301, Section A1, Group6: Jessica Q. Jinyu F. Riasat A., 4/5/2025 ----- Load: 51 oz Member forces in oz: m1: -27.551 (C) m2: -27.551 (C) m3: 0.000 (T) m4: -0.000 (C) m5: 0.000 (T) m6: -0.000 (C) m7: 0.000 (T) m8: 10.432 (T) m9: 0.000 (T) m10: 0.000 (T) m11: -0.000 (C) m12: 0.000 (T) m13: -0.000 (C)  Reaction forces in oz: Sx1: -0.000 Sy1: 25.500 Sy2: 25.500  ----- The member to buckle first is member 2 Length of that member: 11.88 in Predicted buckling strength: 27.01 oz $\pm$ 1.35 oz The maximum load that the physical truss could support is 51 oz $\pm$ 1.35 oz  ----- Cost of the truss: \$216.51 Theoretical max load/cost ratio in oz/\$: 0.236		Member Lengths (in inches): Member 1: 11.885 in Member 2: 11.885 in Member 3: 11.885 in Member 4: 11.885 in Member 5: 11.885 in Member 6: 11.885 in Member 7: 11.705 in Member 8: 9.000 in Member 9: 9.000 in Member 10: 9.000 in Member 11: 9.000 in Member 12: 9.000 in Member 13: 8.500 in  Buckling Strengths for Compression Members: ----- <table> <tr> <th>Member</th><th>Length (in)</th><th>Force (oz)</th><th>Buckling Strength <math>\pm</math> Unc.</th></tr> <tr> <td>m1</td><td>11.885</td><td>-27.551</td><td>27.01 <math>\pm</math> 1.35</td></tr> <tr> <td>m2</td><td>11.885</td><td>-27.551</td><td>27.01 <math>\pm</math> 1.35</td></tr> <tr> <td>m4</td><td>11.885</td><td>-0.000</td><td>27.01 <math>\pm</math> 1.35</td></tr> <tr> <td>m6</td><td>11.885</td><td>-0.000</td><td>27.01 <math>\pm</math> 1.35</td></tr> <tr> <td>m13</td><td>8.500</td><td>-0.000</td><td>49.57 <math>\pm</math> 1.35</td></tr> </table>		Member	Length (in)	Force (oz)	Buckling Strength $\pm$ Unc.	m1	11.885	-27.551	27.01 $\pm$ 1.35	m2	11.885	-27.551	27.01 $\pm$ 1.35	m4	11.885	-0.000	27.01 $\pm$ 1.35	m6	11.885	-0.000	27.01 $\pm$ 1.35	m13	8.500	-0.000	49.57 $\pm$ 1.35
Member	Length (in)	Force (oz)	Buckling Strength $\pm$ Unc.																								
m1	11.885	-27.551	27.01 $\pm$ 1.35																								
m2	11.885	-27.551	27.01 $\pm$ 1.35																								
m4	11.885	-0.000	27.01 $\pm$ 1.35																								
m6	11.885	-0.000	27.01 $\pm$ 1.35																								
m13	8.500	-0.000	49.57 $\pm$ 1.35																								

*Figure 8: Design 2 MATLAB Program Output*

## IV. Discussion and Conclusion

---

We used both hand calculations and computational analysis to evaluate the internal member forces, buckling strengths, and cost-efficiency of truss structures. Our MATLAB code allowed us to verify the accuracy of manual calculations while enabling efficient evaluation of different truss configurations under the given design constraints. Our analysis of two preliminary truss designs allowed us to evaluate their stability, load capacity, cost-effectiveness, and overall structural efficiency. Both designs aimed to support a 32 oz load with minimal material cost while adhering to stability requirements and Equation (1).

Design 1 followed key structural principles such as symmetry, efficient triangulation, and relatively short compression members. The analysis showed that the truss could support a maximum load of  $50 \text{ oz} \pm 1.35 \text{ oz}$  before member 8 buckled. This member had a buckling strength of 40.53 oz, which was just barely exceeded by its internal compressive force of 41.179 oz. All other members remained safely within their strength limits. The cost of this truss was \$177.53, leading to a theoretical load-to-cost ratio of 0.282 oz/\$. These results indicate that Design 1 is structurally sound and efficient, though there is limited safety margin in the buckling capacity of some compression members. This suggests that minor improvements in geometry or member length distribution, such as shortening member 8 or distributing compressive forces more evenly, could further enhance the performance.

Design 2, while capable of supporting a similar maximum load of  $51 \text{ oz} \pm 1.35 \text{ oz}$ , did so at a significantly higher cost of \$216.51, resulting in a lower load-to-cost ratio of 0.236 oz/\$. The first member predicted to buckle was member 2, with a buckling strength of 27.01 oz and an internal force of 27.551 oz, again showing minimal margin before failure. Additionally, several members in Design 2 experienced no force, suggesting inefficient material usage and potential overdesign. This inefficiency in force distribution and member utilization contributed to a higher cost without an important increase in performance.

In conclusion, Design 1 is clearly the more cost-effective and structurally efficient option. It maintained a higher load-to-cost ratio, utilized more of its members effectively, and better adhered to key structural design principles like triangulation and load path continuity. Though both designs demonstrated the ability to meet the loading requirement, Design 1 did so with lower material use and cost, making it the preferred choice.

To improve future designs, we could implement optimization routines into our MATLAB code to adjust member lengths, joint positions, and loading points to maximize the load-to-cost ratio. Additionally, we could include a more robust buckling prediction model that accounts for material imperfections or misalignment, providing more realistic safety margins. Further testing

could also assess the impact of small geometric changes on load distribution to avoid critical failures in individual members.

## V. Appendix

### A.1 — MATLAB Code Developed to Analyze Example Truss

#### Connection matrix C for joint

```
1 jointq = "How many joints? ";
2 numj = input(jointq);
3 numm = (2*numj) - 3;
4
5 C = zeros(numj, numm);
6
7 % Loop through each join and each member
8 for j = 1:numj;
9     for m = 1:numm
10         prompt = sprintf("Is member %d connected to joint %d? (1 for yes, 0 for no): ", m, j);
11         C(j,m) = input(prompt);
12     end
13 end
```

#### Connection matrix for support force Sx Sy

```
14 Sx1 = zeros(numj, 1);
15 Sy1 = zeros(numj, 1);
16 Sy2 = zeros(numj, 1);
17
18 Sx = zeros(numj, 3);
19 Sy = zeros(numj,3);
20
21 connection_matrixS_joint = zeros(numj,3);
22
23 % Loop through each joint and each force
24
25
26 for j = 1:numj
27     support = sprintf("Is the x reaction force on joint %d? (1 for yes, 0 for no): ", j);
28     Sx1 (j,1) = input(support);
29 end
30
31 for j = 1:numj
32     support = sprintf("Is the first Y reaction force on joint %d? (1 for yes, 0 for no): ", j);
33     Sy1 (j,1) = input(support);
34 end
35
36 for j = 1:numj
37     support = sprintf("Is the second Y reaction force on joint %d? (1 for yes, 0 for no): ", j);
38     Sy2 (j,1) = input(support);
39 end
40
41
42 Sx = [Sx1, zeros(numj,1), zeros(numj,1)];
43 Sy = [zeros(numj,1), Sy1, Sy2];
```

## Joint Location

```
44 X = zeros(numj,1);
45 Y = zeros(numj,1);
46
47 for j = 1:numj
48     x = input(sprintf("Location (x-axis) of joint %d from Origin",j));
49     X(j,1) = x;
50     y = input(sprintf("Location (y-axis) of joint %d from Origin",j));
51     Y(j,1) = y;
52 end
```

## Load

```
53 L = zeros(2*numj,1);
54
55 for j = 1:numj
56     loadh = input(sprintf("horizontal load at joint %d",j));
57     loadv = input(sprintf("vertical load at joint %d",j));
58     L(j,1) = loadh;
59     L(numj+j,1) = loadv;
60
61 end
```

## Member Lengths

```
62 R = zeros(numm, 1);
63 for i = 1:numm
64     joints = zeros(1,2);
65     for j = 1:numj
66         if C(j,i) == 1
67             if joints(1) == 0
68                 joints(1) = j;
69             else
70                 joints(2) = j;
71             end
72         end
73     end
74     if joints(2) == 0
75         error("Member %d only connected to one joint - check connection matrix C!", i);
76     end
77     X1 = X(joints(1));
78     Y1 = Y(joints(1));
79     X2 = X(joints(2));
80     Y2 = Y(joints(2));
81     R(i) = sqrt((X2 - X1)^2 + (Y2 - Y1)^2);
82 end
```

## Matrix A

```
83 A = zeros(2 * numj, numm);
84 for m = 1:numm
85     connected_joints = find(C(:, m) == 1);
86     joint1 = connected_joints(1);
87     joint2 = connected_joints(2);
88     dx = X(joint2) - X(joint1);
89     dy = Y(joint2) - Y(joint1);
90     length = sqrt(dx^2 + dy^2);
91     A(joint1, m) = dx / length;
92     A(joint2, m) = -dx / length;
93     A(joint1 + numj, m) = dy / length;
94     A(joint2 + numj, m) = -dy / length;
95 end
96 A = [A, S];
97
98 disp("Matrix A:");
99 disp(A);
```

## Solve for Forces

```
100 T = A \ L;
```

## Buckling Calculations

```
101 max_buckling = zeros(numm, 1);
102 for i = 1:numm
103     max_buckling(i) = 2390 * R(i)^(-1.811);
104 end
105
106 [maxload, ind] = max(L);
107 mass = 0;
108 Q = 1;
109 breakload = 0;
110
111 while Q == 1
112     mass = mass + 1;
113     L(ind) = mass;
114     T_crit = A \ L;
115     for mm = 1:numm
116         if abs(T_crit(mm)) > max_buckling(mm)
117             Q = 0;
118             breakload = mm;
119         else
120             max_th_load = max(L);
121         end
122     end
123 end
```

## Uncertainty

```
124 U_fit = 1.35; % From lab data
125 buckling_strength = max_buckling(breakload);
126 buckling_length = R(breakload);
127 uncertainty_max_load = U_fit;
```



	<h3>Printing Results</h3> <pre> 132 fprintf("\nEK301, Section A1, Group6: Jessica Q. Jinyu F. Riasat A., 3/22/2025\n"); 133 fprintf("Load: %d oz\n", 25); 134 fprintf("Member forces in oz:\n") 135 for i = 1:numm 136     if T(i) &gt; 0 137         tc = 'T'; 138     else 139         tc = 'C'; 140     end 141     fprintf("m%d: %.3f (%c)\n", i, T(i), tc); 142 end 143 144 fprintf("Reaction forces in oz:\n"); 145 fprintf("Sx1: %.3f\n", T(numm + 1)); 146 fprintf("Sy1: %.3f\n", T(numm + 2)); 147 fprintf("Sy2: %.3f\n", T(numm + 3)); </pre>
	<h3>Buckling + Uncertainty</h3> <pre> 148 fprintf("The member to buckle first is member %d\n", breakload); 149 fprintf("Length of that member: %.2f in\n", buckling_length); 150 fprintf("Predicted buckling strength: %.2f oz ± %.2f oz\n", buckling_strength, U_fit); 151 fprintf("The maximum load that the physical truss could support is %d oz ± %.2f oz\n", max_th_load, uncertainty_max_load); 152 153 fprintf("Cost of the truss: \$%.2f\n", Cost); 154 fprintf("Theoretical max load/cost ratio in oz/\$: %.3f\n", abs(max_th_load / Cost)); </pre>

## A.2 — MATLAB Code Developed to Analyze Design 1 & 2 Truss

	<h3>Connection matrix C for joint</h3> <pre> 1 jointq = "How many joints? "; 2 numj = input(jointq); 3 numm = (2*numj) - 3; 4 5 C = zeros(numj, numm); 6 7 % Loop through each join and each member 8 for j = 1:numj; 9     for m = 1:numm 10         prompt = sprintf("Is member %d connected to joint %d? (1 for yes, 0 for no): ", m, j); 11         C(j,m) = input(prompt); 12     end 13 end </pre>
--	--

### Connection matrix for support force Sx Sy

```
14 Sx1 = zeros(numj, 1);
15 Sy1 = zeros(numj, 1);
16 Sy2 = zeros(numj, 1);
17
18 Sx = zeros(numj, 3);
19 Sy = zeros(numj,3);
20
21 connection_matrixS_joint = zeros(numj,3);
22
23 % Loop through each joint and each force
24
25
26 for j = 1:numj
27     support = sprintf("Is the x reaction force on joint %d? (1 for yes, 0 for no): ", j);
28     Sx1 (j,1) = input(support);
29 end
30
31 for j = 1:numj
32     support = sprintf("Is the first Y reaction force on joint %d? (1 for yes, 0 for no): ", j);
33     Sy1 (j,1) = input(support);
34 end
35
36 for j = 1:numj
37     support = sprintf("Is the second Y reaction force on joint %d? (1 for yes, 0 for no): ", j);
38     Sy2 (j,1) = input(support);
39 end
40
41
42 Sx = [Sx1, zeros(numj,1), zeros(numj,1)];
43 Sy = [zeros(numj,1), Sy1, Sy2];
```

### Joint Location

```
44 X = zeros(numj,1);
45 Y = zeros(numj,1);
46
47 for j = 1:numj
48     x = input(sprintf("Location (x-axis) of joint %d from Origin",j));
49     X(j,1) = x;
50     y = input(sprintf("Location (y-axis) of joint %d from Origin",j));
51     Y(j,1) = y;
52 end
```

### Load

```
53 L = zeros(2*numj,1);
54
55 for j = 1:numj
56     loadh = input(sprintf("horizontal load at joint %d",j));
57     loadv = input(sprintf("vertical load at joint %d",j));
58     L(j,1) = loadh;
59     L(numj+j,1) = loadv;
60
61 end
```

## Member Lengths

```
62 R = zeros(numm, 1);
63 for i = 1:numm
64     joints = zeros(1,2);
65     for j = 1:numj
66         if C(j,i) == 1
67             if joints(1) == 0
68                 joints(1) = j;
69             else
70                 joints(2) = j;
71             end
72         end
73     end
74     if joints(2) == 0
75         error("Member %d only connected to one joint - check connection matrix C!", i);
76     end
77     X_joint1 = X(joints(1));
78     Y_joint1 = Y(joints(1));
79     X_joint2 = X(joints(2));
80     Y_joint2 = Y(joints(2));
81     R(i) = sqrt((X_joint2 - X_joint1)^2 + (Y_joint2 - Y_joint1)^2);
82 end
```

## Initialize matrix A

```
83 A = zeros(2 * numj, numm);
84 for m = 1:numm
85     connected_joints = find(C(:, m) == 1);
86     joint1 = connected_joints(1);
87     joint2 = connected_joints(2);
88     dx = X(joint2) - X(joint1);
89     dy = Y(joint2) - Y(joint1);
90     length = sqrt(dx^2 + dy^2);
91     R(m) = length;
92     A(joint1, m) = dx / length;
93     A(joint2, m) = -dx / length;
94     A(joint1 + numj, m) = dy / length;
95     A(joint2 + numj, m) = -dy / length;
96 end
97
98 A = [A, R];
99
100 % Display matrix A
101 disp('Matrix A:');
102 disp(A);
```

## Solve for Forces

```
103 T = A \ L;
```

## Buckling calculations

```
104 max_buckling = zeros(numm, 1);
105 for i = 1:numm
106     max_buckling(i) = 2390 * R(i)^(-1.811);
107 end
108
109 % Theoretical max load calculation
110 [maxload, ind] = max(L);
111 Q = 1;
112 mass = 0;
113 breakload = 0;
114
115 while Q == 1
116     mass = mass + 1;
117     L(ind) = mass;
118     T_crit = A \ L;
119     for mm = 1:numm
120         if abs(T_crit(mm)) > max_buckling(mm)
121             Q = 0;
122             breakload = mm;
123         else
124             max_th_load = max(L);
125         end
126     end
127 end
```

## Uncertainty (based on class average)

```
128 U_fit = 1.35; % oz
129 buckling_strength = max_buckling(breakload);
130 buckling_length = R(breakload);
131 uncertainty_max_load = U_fit;
```

## Failure analysis

```
132 load = max(L);
133 for i = 1:numm
134     rr(i) = load / T_crit(i);
135     P_load(i) = (2390) * R(i)^(-1.811);
136     W_fail(i) = -P_load(i) / rr(i);
137 end
138 Wfail = min(W_fail);
```

## Cost and efficiency

```
139 totallength = sum(R);
140 C1 = numj * 10;
141 C2 = totallength * 1;
142 Cost = C1 + C2;
```

## Print Results

```

143 fprintf("\nEK301, Section A1, Group6: Jessica Q. Jinyu F. Riasat A., 3/22/2025 \n");
144 fprintf("-----\n");
145 fprintf("Load: %d oz\n", load);
146 fprintf("Member forces in oz:\n");
147 for i = 1:numm
148     if T_crit(i) > 0
149         tc = 'T';
150     else
151         tc = 'C';
152     end
153     fprintf("m%d: %.3f (%c)\n", i, T_crit(i), tc);
154 end
155
156 fprintf("\nReaction forces in oz:\n");
157 fprintf("Sx1: %.3f\n", T_crit(numm+1));
158 fprintf("Sy1: %.3f\n", T_crit(numm+2));
159 fprintf("Sy2: %.3f\n", T_crit(numm+3));
160
161 fprintf("\n-----\n");
162 fprintf("The member to buckle first is member %d\n", breakload);
163 fprintf("Length of that member: %.2f in\n", buckling_length);
164 fprintf("Predicted buckling strength: %.2f oz ± %.2f oz\n", buckling_strength, U_fit);
165 fprintf("The maximum load that the physical truss could support is %d oz ± %.2f oz\n", max_th_load, uncertainty_max_load);
166
167 fprintf("\n-----\n");
168 fprintf("Cost of the truss: $%.2f\n", Cost);
169 fprintf("Theoretical max load/cost ratio in oz/$: %.3f\n", abs(max_th_load / Cost));
170 fprintf("-----\n");
171

```

```

173 % Member lengths in inches
174
175 fprintf('\nMember Lengths (in inches):\n');
176 for i = 1:numm
177     fprintf("Member %2d: %.3f in\n", i, R(i));
178 end
179
180
181 % Buckling Strengths for Compression Members
182
183 U_fit = 1.35; % from acrylic fit data
184 fprintf('\nBuckling Strengths for Compression Members:\n');
185 fprintf('-----\n');
186 fprintf('| Member | Length (in) | Force (oz) | Buckling Strength ± Unc. |\n');
187 fprintf('-----\n');
188
189 for i = 1:numm
190     force = T_crit(i);
191     if force < 0 % Only for compression members
192         length = R(i);
193         buckling = 2390 * length^(-1.811);
194         fprintf('| m%-2d | %7.3f | %8.3f | %8.2f ± %.2f |\n', ...
195             i, length, force, buckling, U_fit);
196     end
197 end
198 fprintf('-----\n');

```