Homework problems 26-31 Due in class, Friday, 6 November 2020

26. Draw the shear and moment diagrams for the beam.

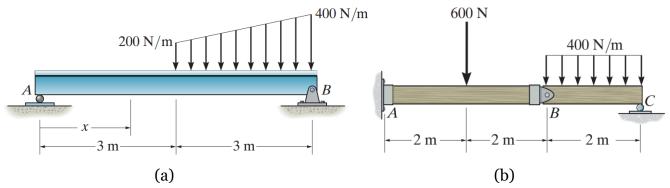


Figure 27

SOLUTION

Support Reactions: As shown on FBD.

Shear and Moment Functions:

For $0 \le x < 3$ m:

$$+\uparrow \Sigma F_y = 0;$$
 200 - V = 0 V = 200 N

$$\zeta + \Sigma M_{NA} = 0; \qquad M - 200 x = 0$$

$$M = \{200 \, x\} \, \mathbf{N} \cdot \mathbf{m}$$

200(3)=600H 200(3)=600H 45m / Im

Ans.

For 3 m < $x \le 6$ m:

$$+\uparrow \Sigma F_y = 0;$$
 $200 - 200(x - 3) - \frac{1}{2} \left[\frac{200}{3} (x - 3) \right] (x - 3) - V = 0$
$$V = \left\{ -\frac{100}{3} x^2 + 500 \right\} N$$

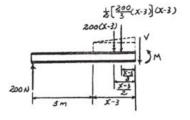


Ans.

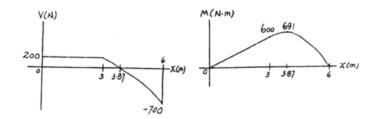
Set $V = 0, x = 3.873 \,\mathrm{m}$

$$\zeta + \Sigma M_{NA} = 0; \qquad M + \frac{1}{2} \left[\frac{200}{3} (x - 3) \right] (x - 3) \left(\frac{x - 3}{3} \right)$$
$$+ 200(x - 3) \left(\frac{x - 3}{2} \right) - 200x = 0$$
$$M = \left\{ -\frac{100}{9} x^3 + 500x - 600 \right\} \text{N} \cdot \text{m}$$

Ans.

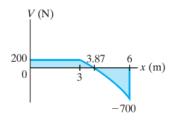


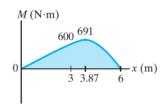
Substitute $x = 3.87 \text{ m}, M = 691 \text{ N} \cdot \text{m}$





For
$$0 \le x < 3$$
 m: $V = 200$ N, $M = \{200x\}$ N·m,
For 3 m $< x \le 6$ m: $V = \left\{-\frac{100}{3}x^2 + 500\right\}$ N,
 $M = \left\{-\frac{100}{9}x^3 + 500x - 600\right\}$ N·m





Support Reactions: Referring to the free-body diagram of segment BC shown in Fig. a,

$$\zeta + \Sigma M_B = 0;$$

$$C_y(2) - 400(2)(1) = 0$$

$$C_y = 400 \text{ N}$$

$$+\uparrow\Sigma F_{v}=0;$$

$$B_{y} + 400 - 400(2) = 0$$

$$B_{\rm v} = 400 \, {\rm N}$$

Using the result of \mathbf{B}_{v} and referring to the free-body diagram of segment AB, Fig. b,

$$+\uparrow\Sigma F_{v}=0;$$

$$A_{\rm y}-600-400=0$$

$$A_{\rm v} = 1000 \, {\rm N}$$

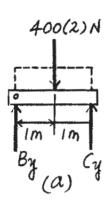
$$\zeta + \Sigma M_A = 0;$$

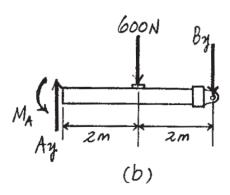
$$M_A - 600(2) - 400(4) = 0$$

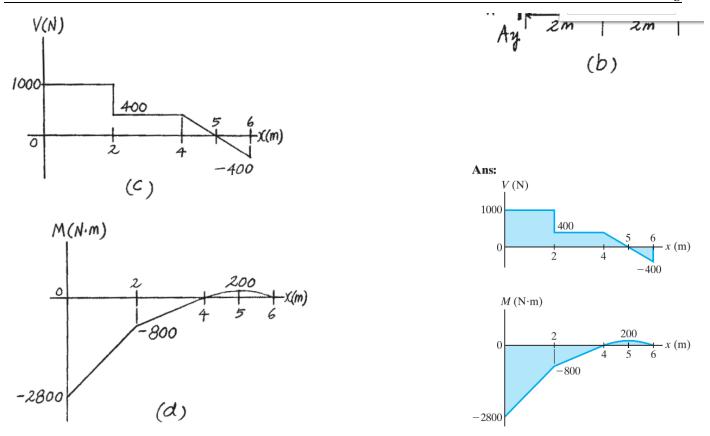
$$M_A = 2800 \text{ N}$$

Shear and Moment Diagrams: As shown in Figs. c and d.

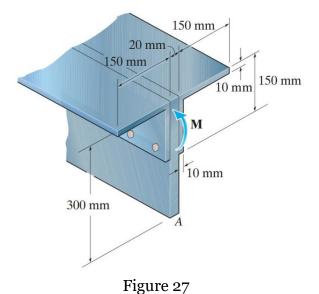








27. If the built-up beam is subjected to an internal moment of M = 75 kN·m, determine (a) the maximum tensile and compressive stress acting in the beam; (b) the amount of this internal moment resisted by plate A.



$$\overline{y} = \frac{\Sigma \widetilde{y} A}{\Sigma A} = \frac{0.15(0.3)(0.02) + 2[0.225(0.15)(0.01)] + 2[0.295(0.01)(0.14)]}{0.3(0.02) + 2(0.15)(0.01) + 2(0.01)(0.14)} = 0.2035 \text{ m}$$

Thus, the moment of inertia of the cross section about the neutral axis is

$$I = \Sigma \overline{I} + Ad^{2}$$

$$= \frac{1}{12}(0.02)(0.3^{3}) + 0.02(0.3)(0.2035 - 0.15)^{2}$$

$$+ 2\left[\frac{1}{12}(0.01)(0.15^{3}) + 0.01(0.15)(0.225 - 0.2035)^{2}\right]$$

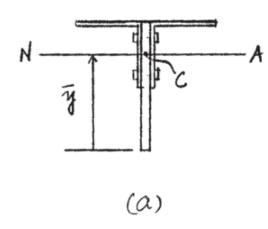
$$+ 2\left[\frac{1}{12}(0.14)(0.01^{3}) + 0.14(0.01)(0.295 - 0.2035)^{2}\right]$$

$$= 92.6509(10^{-6}) \text{ m}^{4}$$

Maximum Bending Stress: The maximum compressive and tensile stress occurs at the top and bottom-most fiber of the cross section.

$$(\sigma_{\text{max}})_c = \frac{My}{I} = \frac{75(10^3)(0.3 - 0.2035)}{92.6509(10^{-6})} = 78.1 \text{ MPa}$$

$$(\sigma_{\text{max}})_t = \frac{Mc}{I} = \frac{75(10^3)(0.2035)}{92.6509(10^{-6})} = 165 \text{ MPa}$$
Ans.



$$\overline{y} = \frac{\Sigma \widetilde{y} A}{\Sigma A} = \frac{0.15(0.3)(0.02) + 2[\ 0.225(0.15)(0.01)] + 2[\ 0.295(0.01)(0.14)]}{0.3(0.02) + 2(0.15)(0.01) + 2(0.01)(0.14)} = 0.2035 \ \mathrm{m}$$

Thus, the moment of inertia of the cross section about the neutral axis is

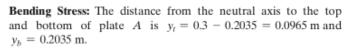
$$I = \overline{I} + Ad^{2}$$

$$= \frac{1}{12}(0.02)(0.3^{3}) + 0.02(0.3)(0.2035 - 0.15)^{2}$$

$$+ 2\left[\frac{1}{12}(0.01)(0.15^{3}) + 0.01(0.15)(0.225 - 0.2035)^{2}\right]$$

$$+ 2\left[\frac{1}{12}(0.14)(0.01^{3}) + 0.14(0.01)(0.295 - 0.2035)^{2}\right]$$

$$= 92.6509(10^{-6}) \text{ m}^{4}$$



$$\sigma_t = \frac{My_t}{I} = \frac{75(10^3)(0.0965)}{92.6509(10^{-6})} = 78.14 \text{ MPa (C)}$$

$$\sigma_b = \frac{My_b}{I} = \frac{75(10^3)(0.2035)}{92.6509(10^{-6})} = 164.71 \text{ MPa (T)}$$

The bending stress distribution across the cross section of plate A is shown in Fig. b. The resultant forces of the tensile and compressive triangular stress blocks are

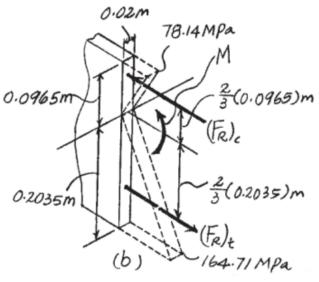
$$(F_R)_t = \frac{1}{2} (164.71)(10^6)(0.2035)(0.02) = 335 144.46 \text{ N}$$

 $(F_R)_c = \frac{1}{2} (78.14)(10^6)(0.0965)(0.02) = 75 421.50 \text{ N}$

Thus, the amount of internal moment resisted by plate A is

$$M = 335144.46 \left[\frac{2}{3} (0.2035) \right] + 75421.50 \left[\frac{2}{3} (0.0965) \right]$$
$$= 50315.65 \text{ N} \cdot \text{m} = 50.3 \text{ kN} \cdot \text{m}$$

35 m A (a)



28. A shaft is made of a polymer having an elliptical cross section. If it resists an internal moment of $M = 50 \text{ N} \cdot \text{m}$, determine the maximum bending stress in the material (a) using the flexure formula, where $I_z = 1/4 \times \pi(0.08 \text{ m})(0.04 \text{ m})^3$, (b) using integration. Sketch a three-dimensional view of the stress distribution acting over the cross-sectional area.

Ans.

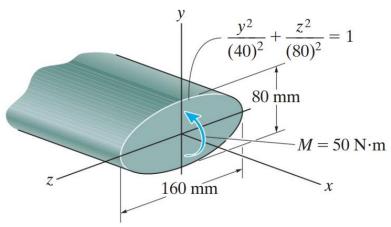


Figure 28

(a)

$$I = \frac{1}{4}\pi \ ab^3 = \frac{1}{4}\pi (0.08)(0.04)^3 = 4.021238(10^{-6}) \text{ m}^4$$

$$\sigma_{\text{max}} = \frac{Mc}{I} = \frac{50(0.04)}{4.021238(10^{-6})} = 497 \text{ kPa}$$

Ans.

(b)
$$M = \frac{\sigma_{\text{max}}}{c} \int_{A} y^{2} dA$$

$$= \frac{\sigma_{\text{max}}}{c} \int y^{2} 2z dy$$

$$z = \sqrt{0.0064 - 4y^{2}} = 2\sqrt{(0.04)^{2} - y^{2}}$$

$$2 \int_{-0.04}^{0.04} y^{2} z dy = 4 \int_{-0.04}^{0.04} y^{2} \sqrt{(0.04)^{2} - y^{2}} dy$$

$$= 4 \left[\frac{(0.04)^{4}}{8} \sin^{-1} \left(\frac{y}{0.04} \right) - \frac{1}{8} y \sqrt{(0.04)^{2} - y^{2}} (0.04^{2} - 2y^{2}) \right]_{-0.04}^{0.04}$$



$$\sigma_{\text{max}} = \frac{50(0.04)}{4.021238(10^{-6})} = 497 \text{ kPa}$$

 $= \frac{(0.04)^4}{2} \sin^{-1} \left(\frac{y}{0.04} \right) \Big|_{-0.04}^{0.04}$

 $= 4.021238(10^{-6}) \text{ m}^4$

Ans.

29. The member has a square cross section and is subjected to the moment $M = 850 \text{ N} \cdot \text{m}$ as shown. Determine the stress at each corner and sketch the stress distribution. Set $\theta = 30^{\circ}$.

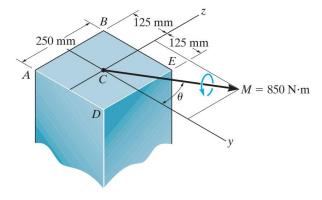


Figure 29

$$M_{y} = 850 \cos 45^{\circ} = 601.04 \text{ N} \cdot \text{m}$$

$$M_{z} = 850 \sin 45^{\circ} = 601.04 \text{ N} \cdot \text{m}$$

$$I_{z} = I_{y} = \frac{1}{12}(0.25)(0.25)^{3} = 0.3255208(10^{-3}) \text{ m}^{4}$$

$$\sigma = -\frac{M_{z}y}{I_{z}} + \frac{M_{y}z}{I_{y}}$$

$$\sigma_{A} = -\frac{601.04(-0.125)}{0.3255208(10^{-3})} + \frac{601.04(-0.125)}{0.3255208(10^{-3})} = 0$$

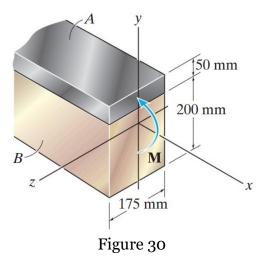
$$\sigma_{B} = -\frac{601.04(-0.125)}{0.3255208(10^{-3})} + \frac{601.04(0.125)}{0.3255208(10^{-3})} = 462 \text{ kPa}$$

$$\sigma_{D} = -\frac{601.04(0.125)}{0.3255208(10^{-3})} + \frac{601.04(-0.125)}{0.3255208(10^{-3})} = -462 \text{ kPa}$$
Ans.

The negative sign indicates compressive stress.

30. The composite beam is made of steel (A) bonded to brass (B) and has the cross section shown. If it is subjected to a moment of $M = 6.5 \text{ kN} \cdot \text{m}$, determine the maximum bending stress in the brass and steel. Also, what is the stress in each material at the seam where they are bonded together? $E_{br} = 100 \text{ GPa}$, $E_{st} = 200 \text{ GPa}$.

Ans.



$$n = \frac{E_{\rm st}}{E_{\rm br}} = \frac{200(10^9)}{100(10^9)} = 2$$

$$\overline{y} = \frac{(350)(50)(25) + (175)(200)(150)}{350(50) + 175(200)} = 108.33 \,\text{mm}$$

$$I = \frac{1}{12}(0.35)(0.05^3) + (0.35)(0.05)(0.08333^2) + \frac{1}{12}(0.175)(0.2^3) +$$

$$(0.175)(0.2)(0.04167^2) = 0.3026042(10^{-3}) \text{ m}^4$$

Maximum stress in brass:

$$(\sigma_{\rm br})_{\rm max} = \frac{Mc_1}{I} = \frac{6.5(10^3)(0.14167)}{0.3026042(10^{-3})} = 3.04 \text{ MPa}$$
 Ans.

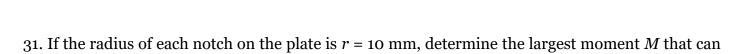
Maximum stress in steel:

$$(\sigma_{\rm st})_{\rm max} = \frac{nMc_2}{I} = \frac{(2)(6.5)(10^3)(0.10833)}{0.3026042(10^{-3})} = 4.65 \,{\rm MPa}$$
 Ans.

Stress at the junction:

$$\sigma_{\rm br} = \frac{M\rho}{I} = \frac{6.5(10^3)(0.05833)}{0.3026042(10^{-3})} = 1.25 \text{ MPa}$$
 Ans.

$$\sigma_{\rm st} = n\sigma_{\rm br} = 2(1.25) = 2.51 \,{\rm MPa}$$
 Ans.



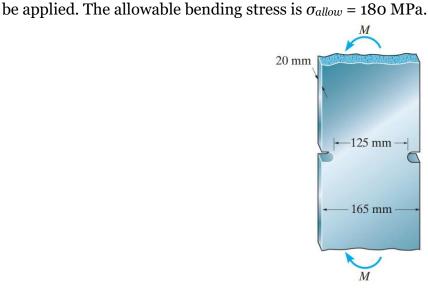


Figure 31

Stress Concentration Factor: From the graph in the text with $\frac{b}{r} = \frac{20}{10} = 2$ and $\frac{r}{h} = \frac{10}{125} = 0.08$, then K = 2.1.

Allowable Bending Stress:

$$\sigma_{\text{max}} = \sigma_{\text{allow}} = K \frac{Mc}{I}$$

$$180(10^6) = 2.1 \left[\frac{M(0.0625)}{\frac{1}{12}(0.02)(0.125^3)} \right]$$

$$M = 4464 \text{ N} \cdot \text{m} = 4.46 \text{ kN} \cdot \text{m}$$
Ans.