

Tutorial 6: Uniform Open Channel Flow

Question 1

For the trapezoidal channel shown in Figure 1, calculate

- The uniform flow velocity and discharge if $h = 2\text{m}$, $S_0 = 1:2000$ and $k_s = 0.49\text{mm}$.
- The uniform flow depth, h , if $Q = 20\text{ m}^3/\text{s}$, $S_0 = 1:2000$ and $k_s = 0.49\text{mm}$.

Assume water at 10°C .

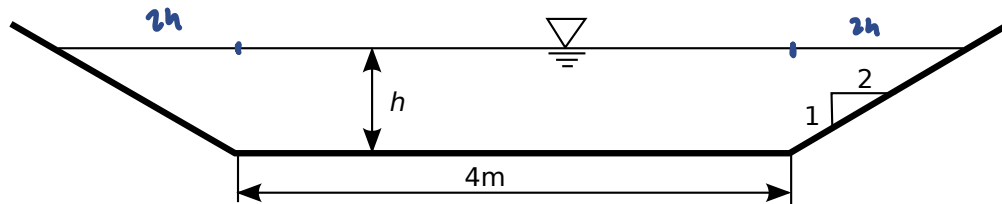


Figure 1: Open channel cross section

Numerical results: (i) $U = 2.02\text{m/s}$ and $Q = 32.32\text{m}^3/\text{s}$. (ii) Find h through iteration (not helpful to state result here).

(i) Using Darcy Weisbach Model:

$$U = \left(\frac{8g}{f} \right)^{1/2} R^{1/2} S^{1/2}, \text{ where } \frac{1}{f^{1/2}} \approx -2 \log_{10} \left(\frac{k_s/R}{14.84} \right)$$

$$\approx -2 \log_{10} \left(\frac{0.49 \times 10^{-3} / 1.2361}{14.84} \right)$$

$$R = \frac{A}{P} = \frac{\frac{1}{2}(4+4+4h)(h)}{4+2\sqrt{4h^2+h^2}} = 1.2361\text{m}$$

$$f \approx 0.012$$

$$\therefore U = \left(\frac{8 \times 9.81}{0.012} \right)^{1/2} \times 1.2361^{1/2} \times \frac{1}{2000}^{1/2}$$

$$= 2.02\text{ m/s}$$

doing till this step is enough, but remember that this assumption is based on Re is very big, so we shall check whether it is true or not

$$Re = \frac{UD}{\nu} = 7.65 \times 10^6, \text{ reading from Moody diagram } \sim \text{slightly increased but is ok.}$$

$k_s/4R = 1 \times 10^{-4}$

$$Q = UA = 2.02 \times \frac{1}{2} (4+4+4h)(h) = 32.32\text{ m}^3/\text{s}$$

(ii) remember to find h_n

if rectangular + wide

$$h_n = \left(\frac{f q^2}{8 g S_0} \right)^{1/2}$$

(no iteration needed)

if not rectangular & wide.

$$F(h_n) = \frac{Q}{A} - \left(\frac{8 g}{f} \right)^{1/2} \left(\frac{A}{P} \right)^{5/2} = 0$$

(iteration needed)

let $h_{n,0} = 1\text{m}$,

$$A = \frac{1}{2}(4+4+4(1))(1) = 6$$

$$P = 4 + 2\sqrt{4(1)^2 + 1^2} = 8.47$$

$$R = A/P = 0.708$$

$$f \approx 0.014$$

$$U = 1.41$$

$$F(h_n) = 1.9243$$

let $h_{n,1} = 1.5\text{m}$,

$$A = 10.5\text{m}^2$$

$$P = 10.7$$

$$R = A/P = 0.981$$

$$f \approx 0.013$$

$$U = 1.72$$

$$F(h_n) = 0.0184$$

let $h_{n,2} = 1.6\text{m}$

$$A = 11.52$$

$$P = 11.2$$

$$R = 1.03$$

$$f \approx 0.013$$

$$U = 1.76$$

$$F(h_n) = -0.027$$

$$\therefore h_n \approx \underline{\underline{1.55\text{m}}}$$

again, check Re whether it is big.

$$Re = \frac{UD}{\nu} = 5.5 \times 10^6 \text{ (ok)}$$

Question 2

The cross section of a combined sewer / storm water channel leading to a treatment plant is shown in Figure 2. To avoid overloading the plant during a storm, some of the flow is to spill into an interceptor when the depth y reaches 0.91m. The Manning coefficient is $n = 0.015 \text{ s/m}^{1/3}$ and the bed slope is $S_0 = 0.005$.

- Find the discharge Q in the sewer when spilling begins.
- What is the equivalent Darcy-Weisbach friction factor f for this channel?
- What is the equivalent Chezy coefficient C [$\text{m}^{1/2}/\text{s}$] for this channel?

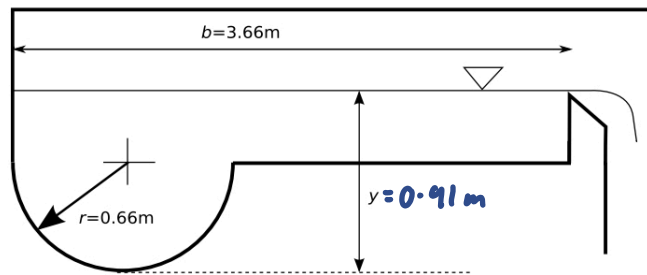


Figure 2: Sewer / storm water system at the point of spilling

Numerical results: (i) $Q = 3.57 \text{ m}^3/\text{s}$. (ii) $f = 0.026$. (iii) $C = 55.3 \text{ m}^{1/2}/\text{s}$.

(i) find Q when $h_n = y = 0.91 \text{ m}$,

$$V = \frac{1}{n} R^{2/3} S^{1/2} \quad R = \frac{A}{P} = \frac{0.5\pi r^2 + (y-r) \times b}{\pi r + 2(y-r) + (b-2r)} = \frac{1.5992}{4.9135} = 0.3255$$

$$= \frac{1}{0.015} (0.3255)^{2/3} (0.005)^{1/2}$$

$$= 2.2306 \text{ m/s}$$

$$Q = VA = 2.2306 \times 1.5992 = 3.567 \text{ m}^3/\text{s}$$

$$(ii) \frac{1}{f^{1/2}} \approx -2 \log \left(\frac{k_s/R}{14.84} \right)$$

the issue with this is we don't know what k_s is.

but from data sheet we can obtain the following formula:

$$f = \frac{8gn^2}{R^{1/3}} = 0.02567$$

(iii) from data sheets,

$$f = \frac{8g}{C^2} \quad C = \sqrt{\frac{8g}{f}} = 55.293$$

Question 3

The critical depth for the circular sewer shown in Figure 3 is $h_c = 0.69\text{m}$. The sewer diameter is $2r = 1.52\text{m}$. Find the discharge for $h = h_c$.

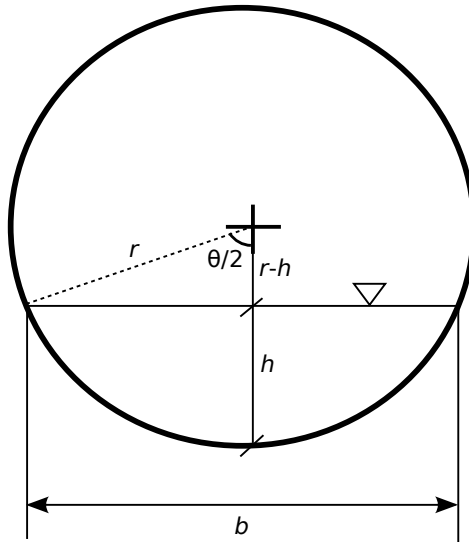


Figure 3: Circular sewer

Numerical results: $Q = 1.82\text{m}^3/\text{s}$.

aim: find Q when $h = h_c$

at first I think I should use:

$$V = \left(\frac{8g}{f}\right)^{1/2} R^{1/2} S^{1/2} \text{ but I realised we are missing too much inputs: } f \text{ and } S$$

(f requires k_s which is not obtainable)

$h = h_c$ means critical

at critical depth: $\frac{dE(h)}{dh} = 0$ and $Fr = 1$

$$\hookrightarrow E(h) = h + \frac{q^2}{2gh^3} \quad \hookrightarrow Fr = \frac{V}{\sqrt{gh_m}}, \quad Q = VA$$

\hookrightarrow we can definitely get q with this

\hookrightarrow we can definitely get q with this too.

\hookrightarrow This one seems easier.

$$1.0 = \frac{Q/A}{\sqrt{gh}} \quad \leftarrow \text{only missing } A!$$

this h is h_m !

$$h_m = A/w$$

$$= A/b$$

$$Q = A\sqrt{gh} = 1.825$$

$$b = \sqrt{r^2 - (r-h)^2} \times 2$$

$$A = \frac{1}{2}r^2\theta - \frac{1}{2}r^2\sin\theta$$

$$= 0.801$$

$$\cos\frac{\theta}{2} = \frac{r-h}{r}$$

$$= \frac{1.52}{2} - 0.69$$

$$\frac{1.52}{2}$$

$$\theta = 169.43^\circ$$