

## Modeling and Simulation

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## Exercise Sheet Nr. 12

## Topic: Numerical Methods for Solving Partial Differential Eqs.: Finite-Element-Method

Starting point: Variation principle

$$\delta\Pi = 0$$

Discretization of displacement field:

$$u_i^{approx}(x,y,z) = \mathbf{N}_i(x,y,z)\mathbf{u}_i$$
  
 $\mathbf{N}(x,y,z) := \text{Interpolation matrix}$   
 $\mathbf{u} := \text{Node displacements}$ 

Discretization, distortion and stress fields (small distortions, linear elasticity):

$$\varepsilon_i = \frac{1}{2} \left( \nabla u_i^{approx} + (\nabla u_i^{approx})^T \right) = \frac{1}{2} \left( \nabla \mathbf{N}_i \mathbf{u}_i + (\nabla \mathbf{N}_i \mathbf{u}_i)^T \right) := \mathbf{B}_i \mathbf{u}_i$$
$$\sigma_i = \mathbf{C}_i \varepsilon_i = \mathbf{C}_i \mathbf{B}_i \mathbf{u}_i$$

Calculation of the element stiffness

$$\mathbf{K}_i = \int\limits_{V} \mathbf{B}_i^T \mathbf{C}_i \mathbf{B}_i \mathrm{d}V$$

Transferring the element stiffness  $K_i$  (local KOS!) in global stiffness matrix  $K_{Ges}$  (global KOS!). System equation for the special case of static:

$$\mathbf{f} = \begin{pmatrix} \mathbf{f_a} \\ \mathbf{f_b} \end{pmatrix} = \begin{pmatrix} \mathbf{K_{Ges}^{11}} & \mathbf{K_{Ges}^{12}} \\ \mathbf{K_{Ges}^{21}} & \mathbf{K_{Ges}^{22}} \end{pmatrix} \begin{pmatrix} \mathbf{u_a} \\ \mathbf{u_b} \end{pmatrix} = \mathbf{K_{Ges}} \mathbf{u}$$

 $\mathbf{u_a} := \text{known displacement boundary conditions}$ 

 $f_a := unknown reaction forces$ 

 $\mathbf{u_b} := \text{unknown displacements}$ 

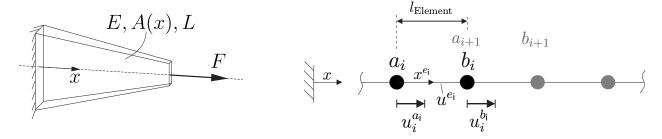
 $\mathbf{f_b} := \text{known external forces acting}$ 

Solving the system equation

$$\begin{split} \mathbf{K_{Ges}^{22}} \mathbf{u_b} &= \left(\mathbf{f_b} - \mathbf{K_{Ges}^{21}} \mathbf{u_a}\right) \\ \mathbf{f_a} &= \mathbf{K_{Ges}^{11}} \mathbf{u_a} + \mathbf{K_{Ges}^{12}} \mathbf{u_b} \end{split}$$

- homework -Exercise 1

Matlabfile: fem Bearbeitungsfile.m



A cantilever bar with conic rectangular cross-section (left figure) is given. The following data are given:

$$F = 20 \text{ [kN]}$$
  
 $E = 70 \text{ [GPa]}$   
 $L = 1 \text{ [m]}$   
 $A(x) = 0.001 - 0.0009x \text{ [m}^2\text{]}$ 

- 1. Write a system of differential equation for u(x) as a function of the given parameter using Hooke's law  $\sigma(x) = E\varepsilon(x)$  for small strains. Determine all the required boundary conditions.
- 2. Solve the differential equation using an appropriate Matlab-solver.
- 3. The structure is discretized by the first-order linear elements with the nodes  $a_i$  and  $b_i$  (right figure). The node  $a_i$  and  $b_i$  is shifted by  $u_i^{a_i}$  and  $u_i^{b_i}$ . Set up an appropriate approach for the displacement  $u^{e_i}(x^{e_i})$  for the ith element. Define the interpolation matrix  $\mathbf{N}(x^{e_i})$ , such that  $u^{e_i}(x^{e_i}) = \mathbf{N}(x^{e_i})\mathbf{u}$ . The total number of elements M can be chosen freely. Furthermore, the function of area A(x) must be discretized. Calculate an averaged area for each element using the end points  $A_i = \frac{A_{i,a_i} + A_{i,b_i}}{2}$ .
- 4. Determine the matrices  $\mathbf{B}(x^{e_i})$  and  $\mathbf{C}$  which derived from the discretized strain  $\varepsilon_i(x^{e_i}) =$  $\mathbf{B}(x^{e_i})\mathbf{u}$  and the stress  $\sigma_i(x^{e_i}) = \mathbf{C}\mathbf{B}(x^{e_i})\mathbf{u}$  for the *i*th element.
- 5. Find the stiffness matrix  $\mathbf{K}_i = \int_0^{V_i} \mathbf{B}^T(x^{e_i}) \mathbf{C} \mathbf{B}(x^{e_i}) dV$  for the *i*th element.
- 6. Find the total stiffness matrix

Find the total stiffness matrix 
$$\mathbf{K}_{Ges} = \begin{pmatrix} \mathbf{K}_{1}^{\mathbf{a_1}\mathbf{a_1}} & \mathbf{K}_{1}^{\mathbf{a_1}\mathbf{b_1}} & 0 & \dots & 0 \\ \mathbf{K}_{1}^{\mathbf{b_1}\mathbf{a_1}} & \mathbf{K}_{1}^{\mathbf{b_1}\mathbf{b_1}} + \mathbf{K}_{2}^{\mathbf{a_2}\mathbf{a_2}} & \mathbf{K}_{2}^{\mathbf{a_2}\mathbf{b_2}} & \dots & 0 \\ 0 & \mathbf{K}_{2}^{\mathbf{b_2}\mathbf{a_2}} & \mathbf{K}_{2}^{\mathbf{b_2}\mathbf{b_2}} + \mathbf{K}_{3}^{\mathbf{a_3}\mathbf{a_3}} & & & \\ \vdots & & & \ddots & & \\ 0 & & & \dots & & \mathbf{K}_{M}^{\mathbf{b_M}\mathbf{b_M}} \end{pmatrix}$$
 with  $\mathbf{K}_{\mathbf{i}} = \begin{pmatrix} \mathbf{K}_{\mathbf{i}}^{\mathbf{a_i}\mathbf{a_i}} & \mathbf{K}_{\mathbf{i}}^{\mathbf{a_i}\mathbf{b_i}} \\ \mathbf{K}_{\mathbf{i}}^{\mathbf{b_i}\mathbf{a_i}} & \mathbf{K}_{\mathbf{i}}^{\mathbf{b_i}\mathbf{b_i}} \end{pmatrix}$  for  $i = 1 \dots M$  elements.

with 
$$\mathbf{K_i} = \begin{pmatrix} \mathbf{K_i^{a_i a_i}} & \mathbf{K_i^{a_i b_i}} \\ \mathbf{K_i^{b_i a_i}} & \mathbf{K_i^{b_i b_i}} \end{pmatrix}$$
 for  $i = 1 \dots M$  elements.

- 7. Determine the displacement vector  $\mathbf{u} = \begin{pmatrix} \mathbf{u_a} & \mathbf{u_b} \end{pmatrix}^T$  and force vector  $\mathbf{f} = \begin{pmatrix} \mathbf{f_a} & \mathbf{f_b} \end{pmatrix}^T$  the vector components  $\mathbf{u_a}$  and  $\mathbf{f_b}$ , where  $\mathbf{u_a}$  the known displacement boundary conditions,  $\mathbf{f_a}$ the corresponding unknown reaction forces,  $\mathbf{u_b}$  the unknown displacements and  $\mathbf{f_b}$  die known acting external forces.
- 8. Calculate the unknown vectors  $\mathbf{u_b}$  and  $\mathbf{f_a}$ . What is the displacement u(x=L)? Finally, describe the displacement and the stress diagram for the exact solution (subtask 2) and for

the FEM solution over the length of the bar graphically. Adapt the number of element M for the FEM so that your FEM solution agrees well with the exact solution.

<u>Hint:</u> Use the *Matlab* command *stairs()* instead of *plot()* in order to represent the stress.