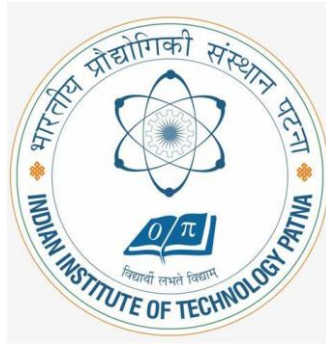


INDIAN INSTITUTE OF TECHNOLOGY

PATNA



GROUP :03

SESSION:2024-2025

STEEL STRUCTURE PROJECT REPORT



FINK TRUSS

Course: CE320 – Steel Structures

Department: Civil Engineering

PROJECT TITLE:

FINK TRUSS ANALYSIS

GROUP :03

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Beam

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Column

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OBJECTIVE

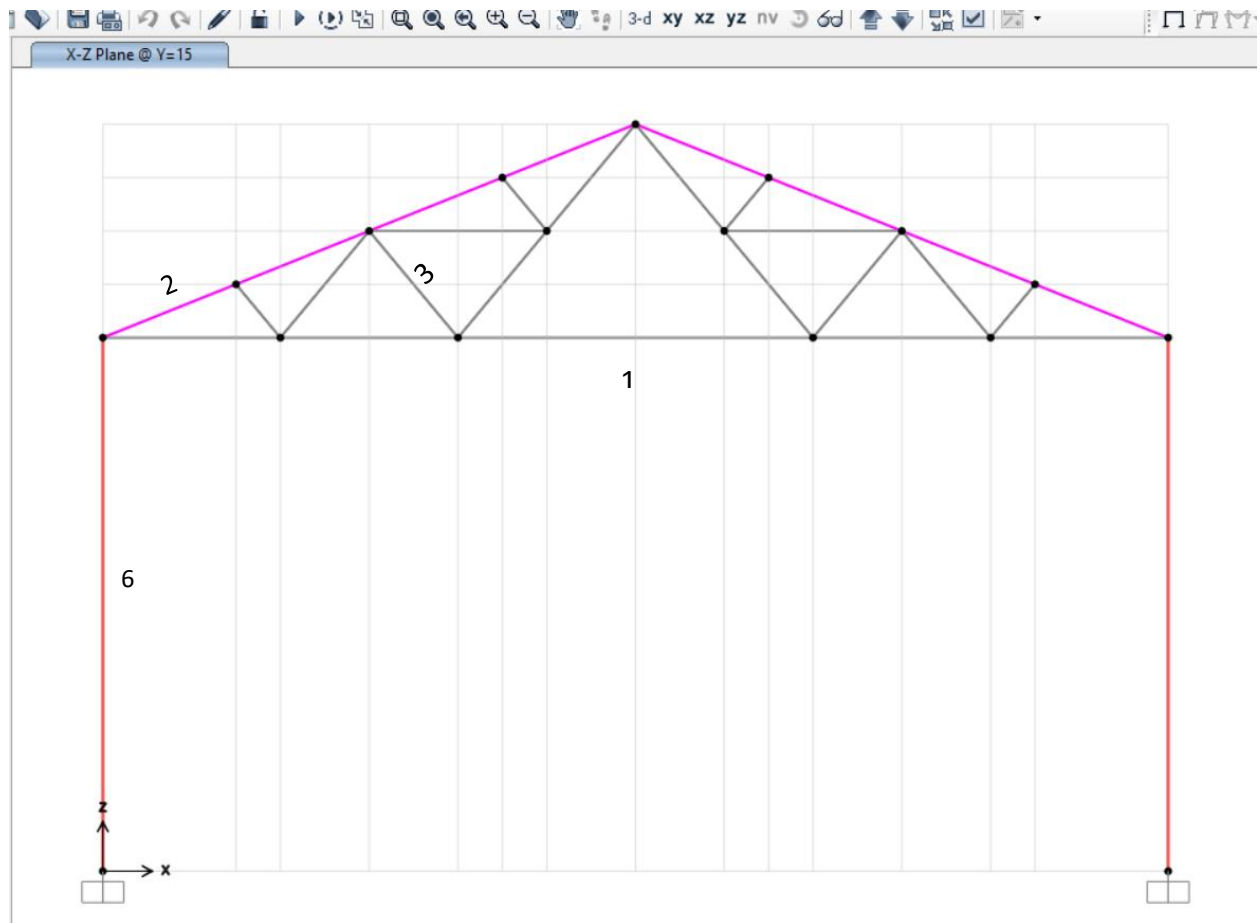
Designing an industrial building roof truss with the following features:

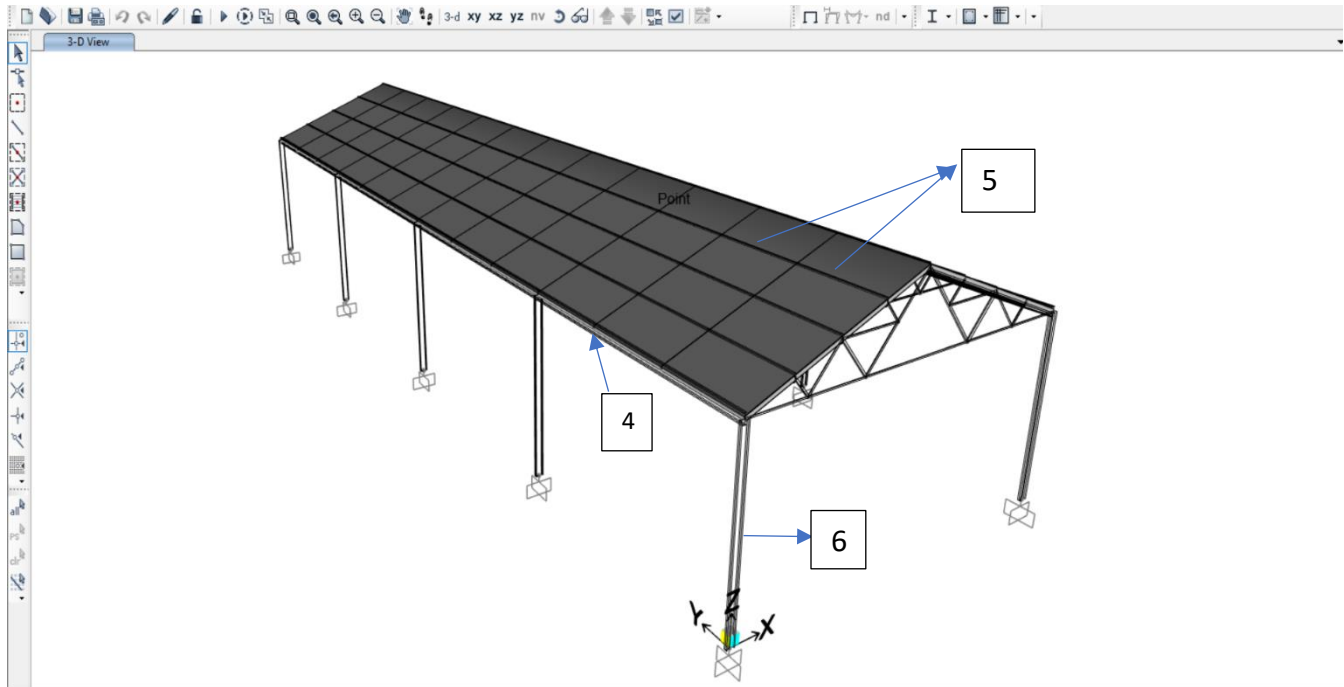
- Proposed location of the industrial building to be designed for 50 years of life – **Silchar**.
- Dimension of industrial shed – 60 m × 20 m
- Ceiling Height – 10 m (floor to roof truss bottom)
- Spacing of columns – 15 m (centre to centre)
- Spacing of trusses – not more than 5 m
- Rise of the roof truss – approx 4 m
- Truss type – Fink Truss
- Roof span – 20 m
- Yield strength of steel members = 250 MPa
- Wind speed – 55 m/s (Silchar falls in terrain category 1)
- Wind Pressure – 2.81 kN/m²

Load cases:

- **Dead load – 1.2 kN/m²**
- **Live load – 1 kN/m²**

- 2D & 3D View
- 2D View of Fink Truss Geometry





IS: 875(Part 3): Wind Loads on Buildings and Structures -Proposed Draft & Commentary

WIND LOAD CALCULATION

5.3 –Design Wind Speed (v_z) = $v_b k_1 k_2 k_3 k_4$,

Where,

Basic wind speed, v_b = **55m/s**

V_z = design wind speed at any height z in m/s,

$k_1 = 1$, probability factor (risk coefficient) (5.3.1) (Mean Probable design life of structure in years, 50 years)

$k_2 = 1.082$, terrain roughness and height factor, **Terrain category = 1** (5.3.2)

$k_3 = 1$ topography factor (5.3.3)

$k_4 = 1.15$ importance factor for the cyclonic region (5.3.4).

$$v_z = 1 \times 1.082 \times 1 \times 1.15 \times 55 = 68.4365 \text{ m/s}$$

5.4 – Design Wind Pressure

$$P_z = 0.6 v_z^2 = 0.6 \times (68.4365)^2$$

$$= 2.81013 \text{ KN/m}^2$$

where

P_z = wind pressure in N/m² at height z, and

v_z = design wind speed in m/s at height z.

The design wind pressure p_d can be obtained as,

$$P_d = K_d \cdot K_a \cdot K_c \cdot P_z = 0.9 \times 0.9 \times 1 \times 2.81013 = \underline{2.276 \text{ KN/m}^2} \text{ where}$$

$K_d = 0.9$ Wind directionality factor

$K_a = 0.9$ Area averaging factor

$K_c = 1$ Combination factor (6.2.3.13)

Wind force (F):

$$F = (C_{pe} - C_{pi}) \times A_e \times P_d \quad \alpha = 21.80^\circ$$

$$h/w \leq 0.5$$

FOR C_{pe} :

For 0°		For 90°	
EF	GH	EF	GH
-0.328	-0.4	-0.7	-0.6

For C_{pi} :

For 0°	
EF	GH
±0.764	±0.5

So, $(C_{pe} - C_{pi})$: -

For 0°	
EF	GH
-1.092	-0.9

Perpendicular Distance between two purlin = **2.6925m * Cos(21.80°) = 2.4999 m**

F_w = Force in the windward direction per unit length

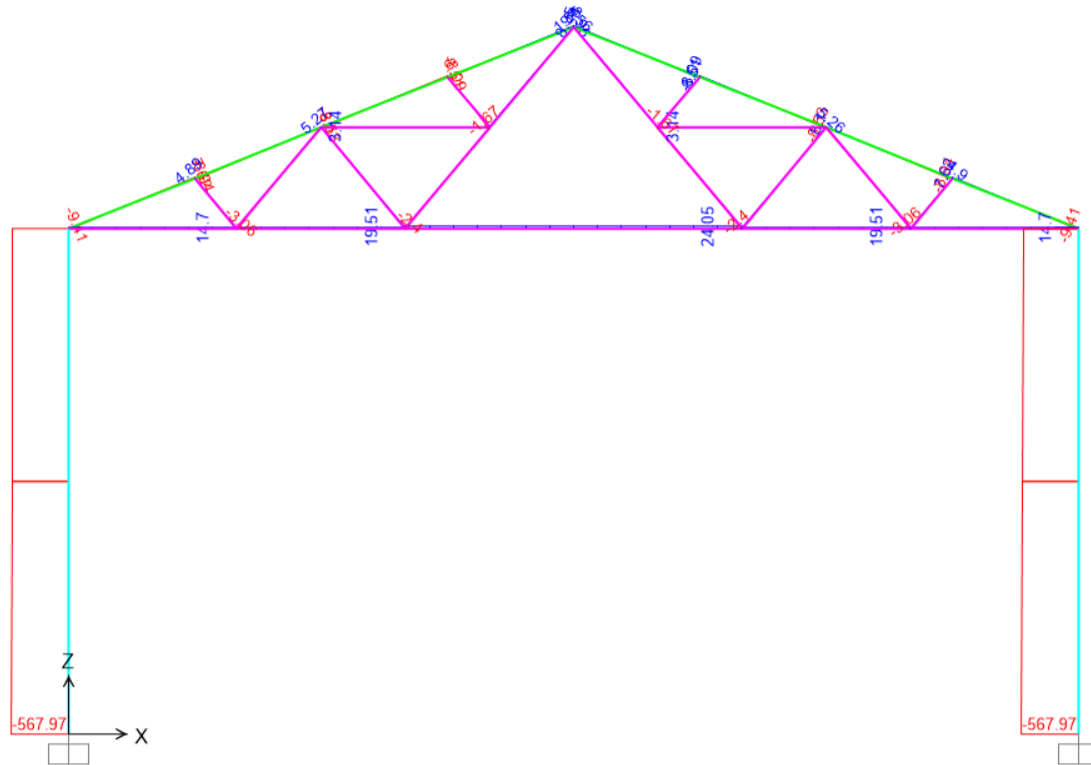
$$= -1.092 \times 2.4999 \times 2.276 = \mathbf{-6.2133 \text{ KN/m (governing)}}$$

F_w = Force in the Leeward direction per unit length

$$= -0.9 \times 2.4999 \times 2.276 = \mathbf{5.1208 \text{ KN/m}}$$

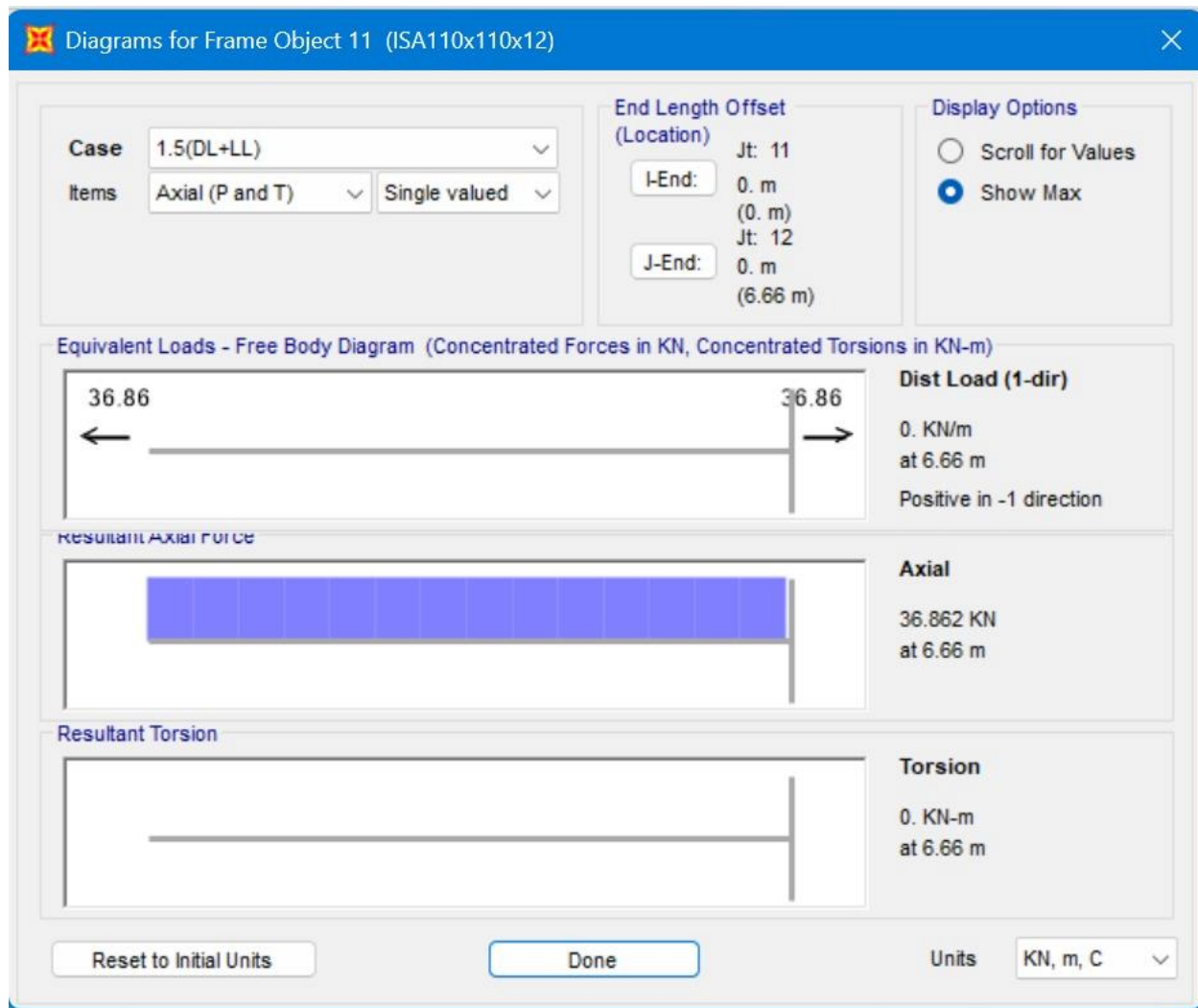
ANALYSIS

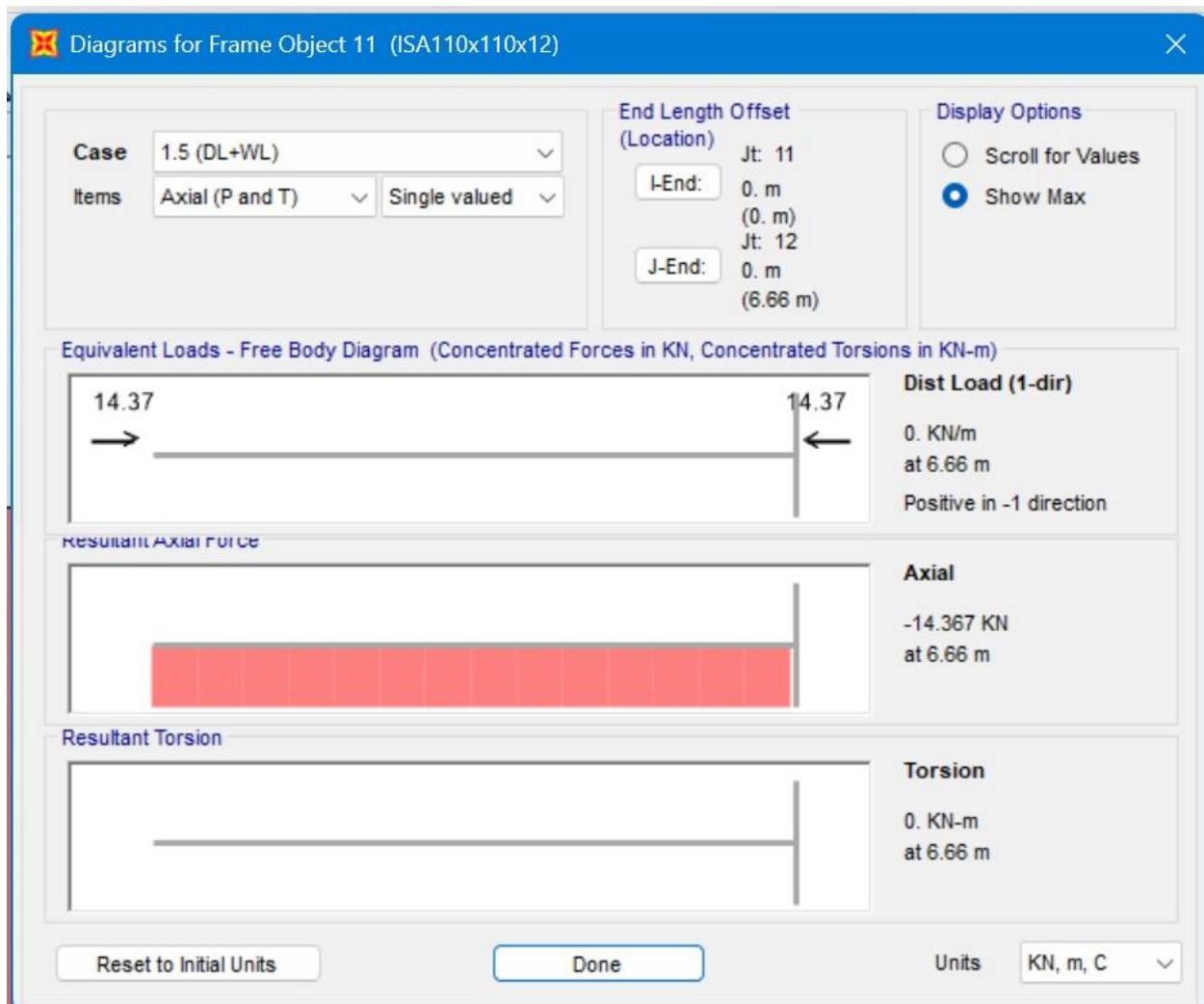
For Truss Member

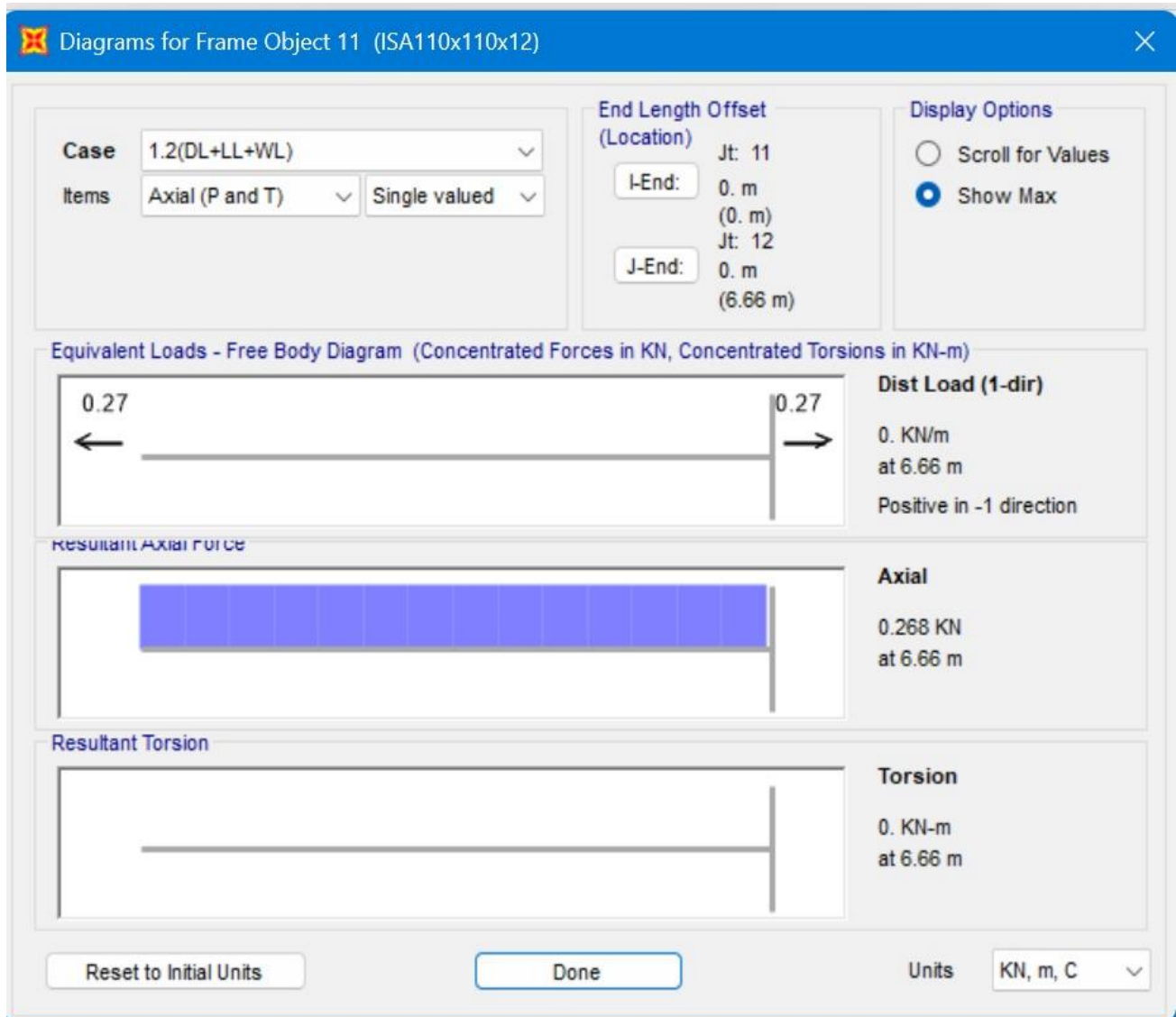


• Bottom chord (Member 1)

Maximum Axial Tension Load = 36.862 kN ,Load Combination = 1.5(DL+LL)

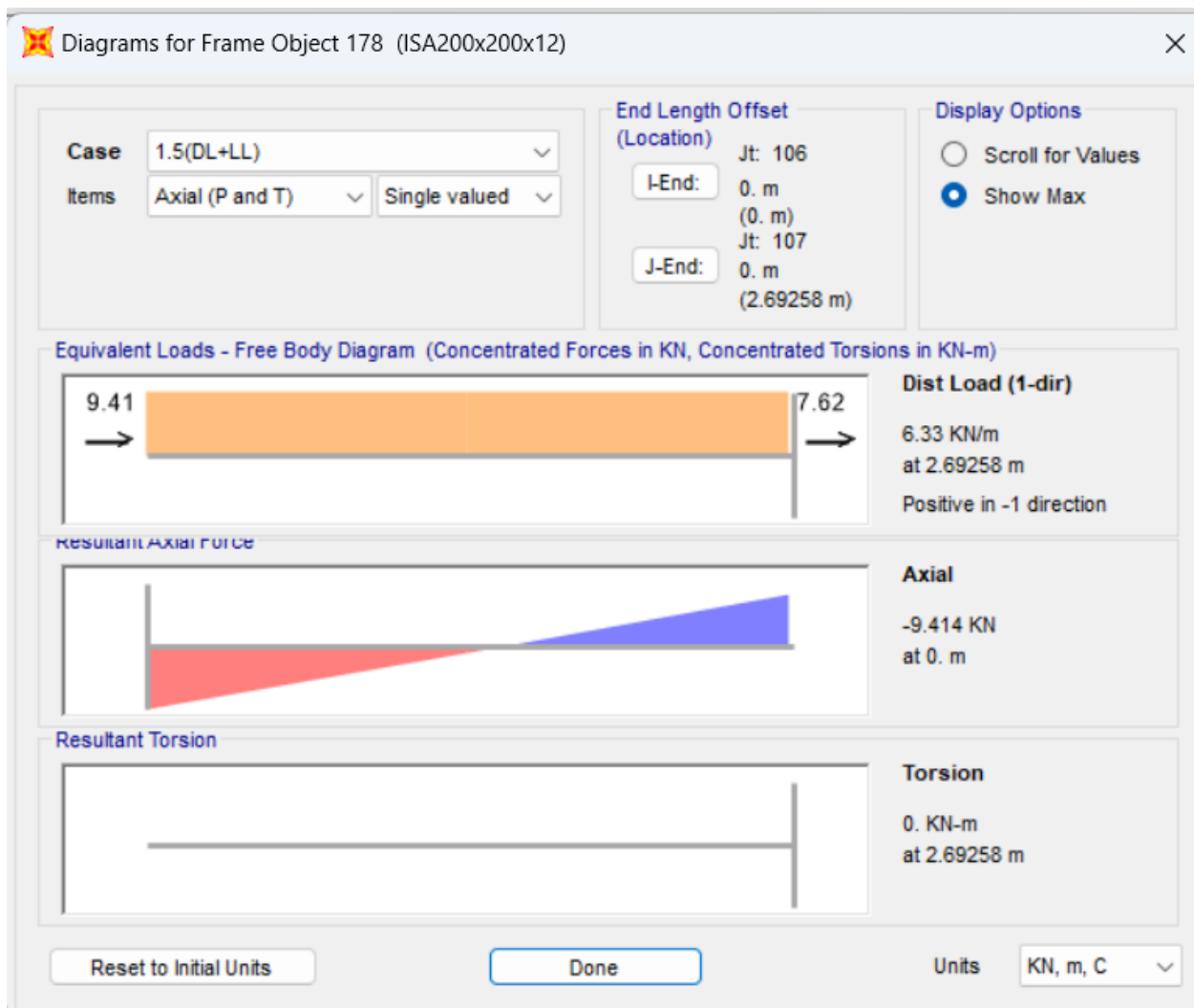


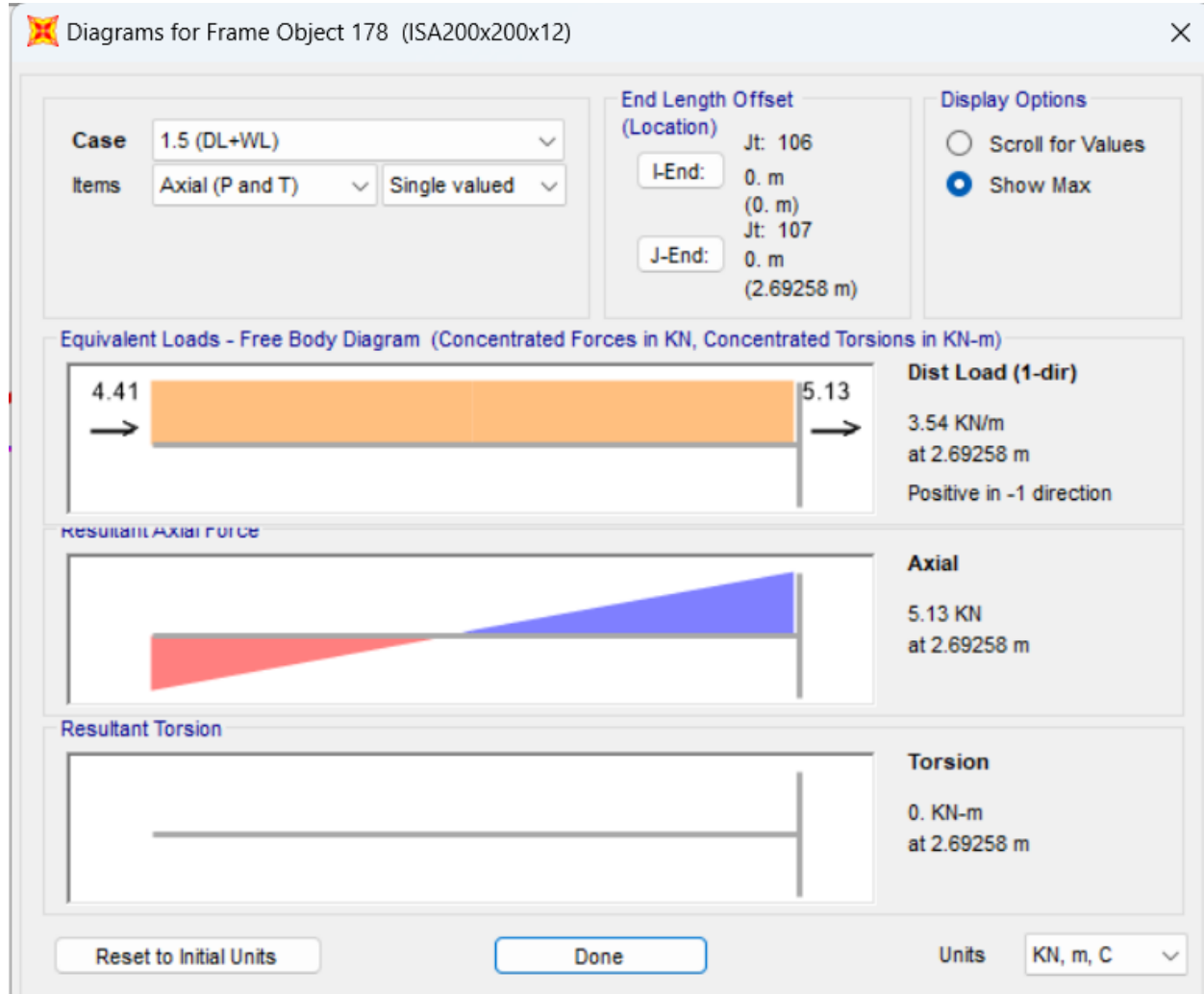




- Top chord (Member 2)

Maximum Axial Compression Load = -9.414 kN ,Load
Combination = 1.5(DL+LL)





Case 1.2(DL+LL+WL) Items Axial (P and T) Single valued	End Length Offset (Location) Jt: 106 I-End: 0. m (0. m) Jt: 107 J-End: 0. m (2.69258 m)	Display Options <input type="radio"/> Scroll for Values <input checked="" type="radio"/> Show Max
---	--	--

Equivalent Loads - Free Body Diagram (Concentrated Forces in KN, Concentrated Torsions in KN-m)

	Dist Load (1-dir) 5.06 KN/m at 2.69258 m Positive in -1 direction
--	---

Resultant Axial Force

	Axial 6.816 KN at 2.69258 m
--	--

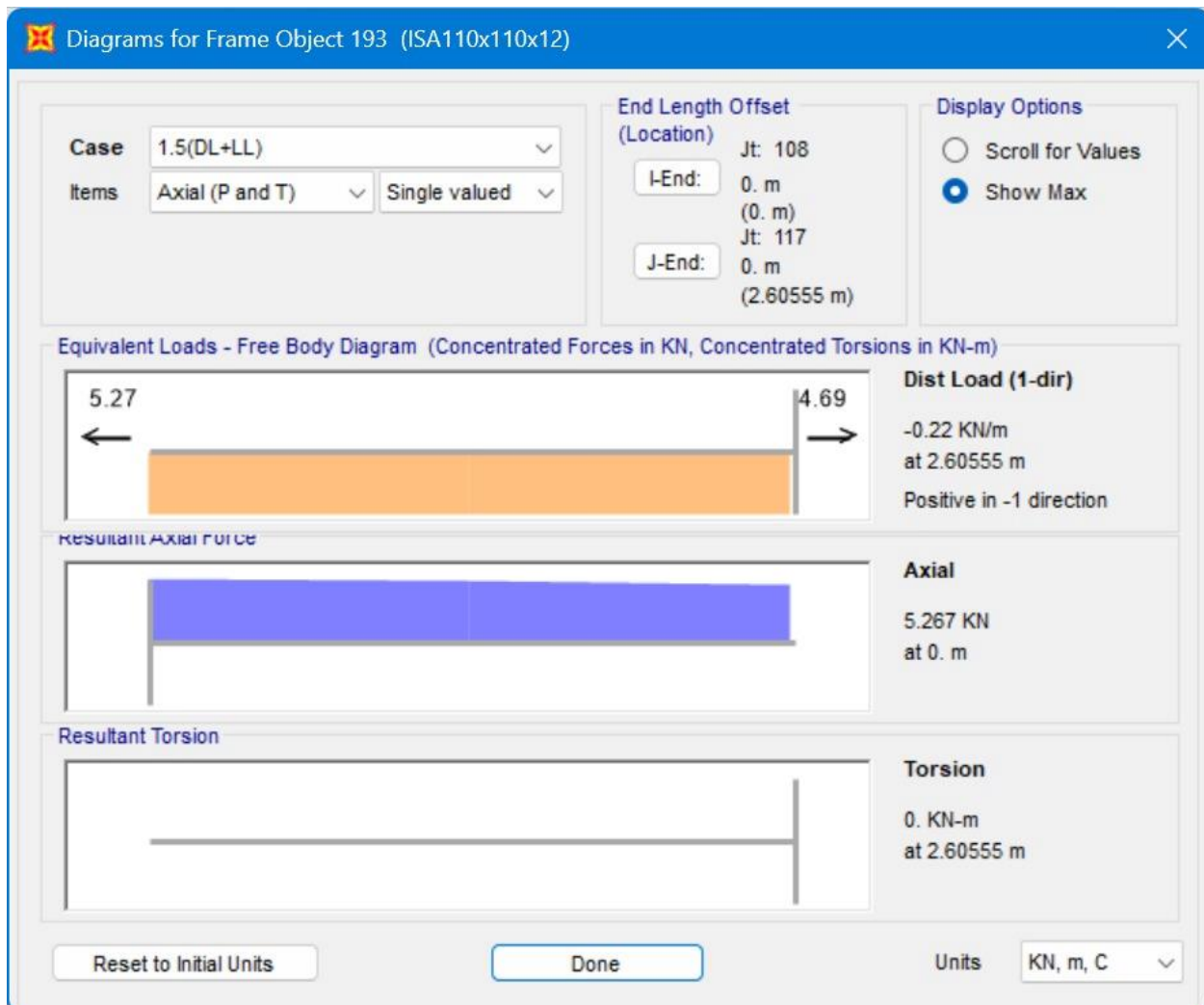
Resultant Torsion


	Torsion 0. KN-m at 2.69258 m
--	---

Reset to Initial Units	Done	Units KN, m, C
------------------------	------	----------------

- Diagonal Chord (Member 3)

Maximum Axial Tension Load = 5.267 kN ,Load Combination = 1.5(DL+LL)




Diagrams for Frame Object 193 (ISA110x110x12)

Case
1.5 (DL+WL)

Items
Axial (P and T)
Single valued

End Length Offset (Location)

Jt: 108
I-End: 0. m (0. m)
Jt: 117
J-End: 0. m (2.60555 m)

Display Options

☐ Scroll for Values
☒ Show Max

0.91
←

→
0.33

Dist Load (1-dir)
-0.22 KN/m at 2.60555 m
Positive in -1 direction

Resultant Axial Force

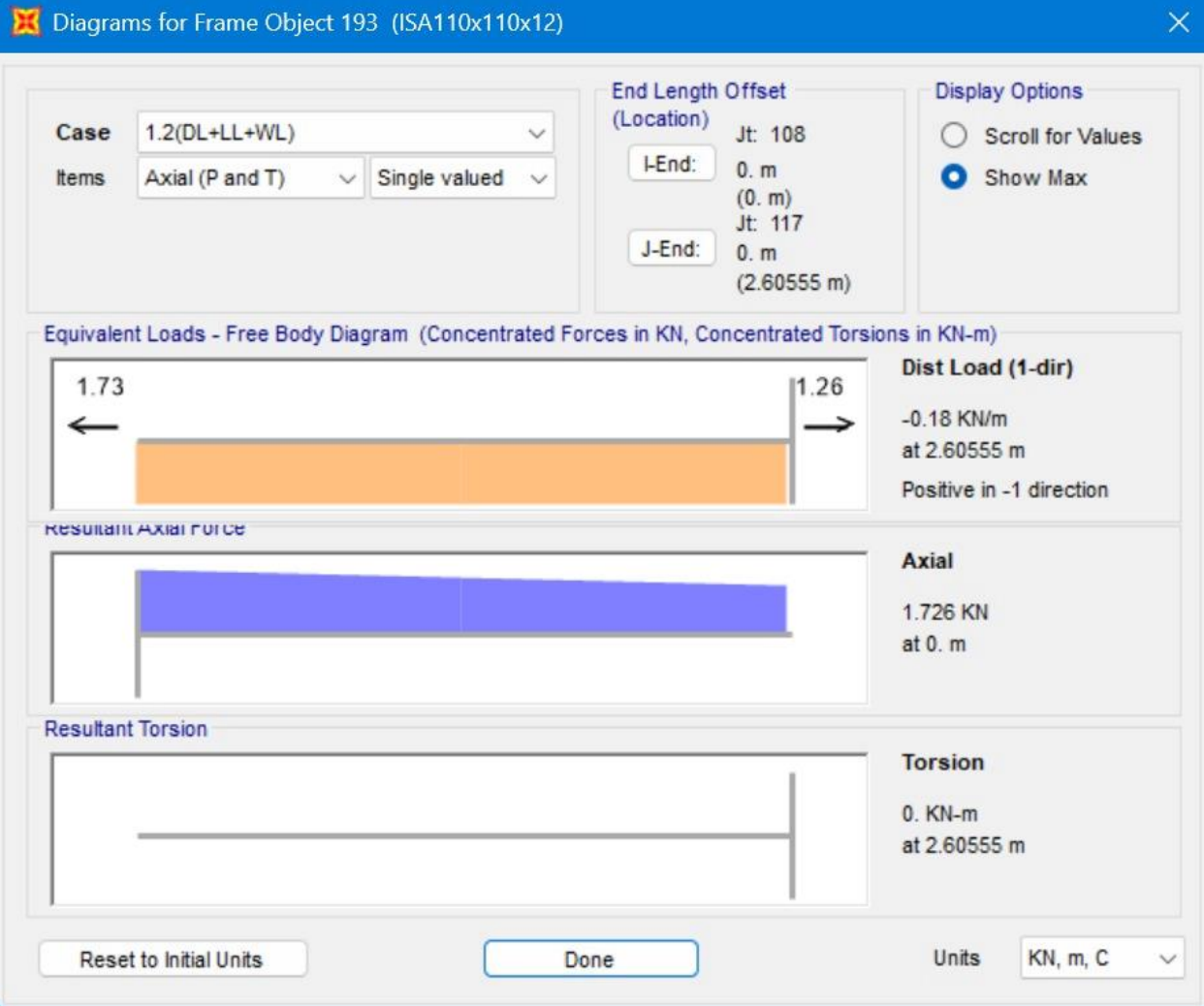
Axial
0.907 KN at 0. m

Resultant Torsion

Torsion
0. KN-m at 2.60555 m

Reset to Initial Units
Done

Units
KN, m, C

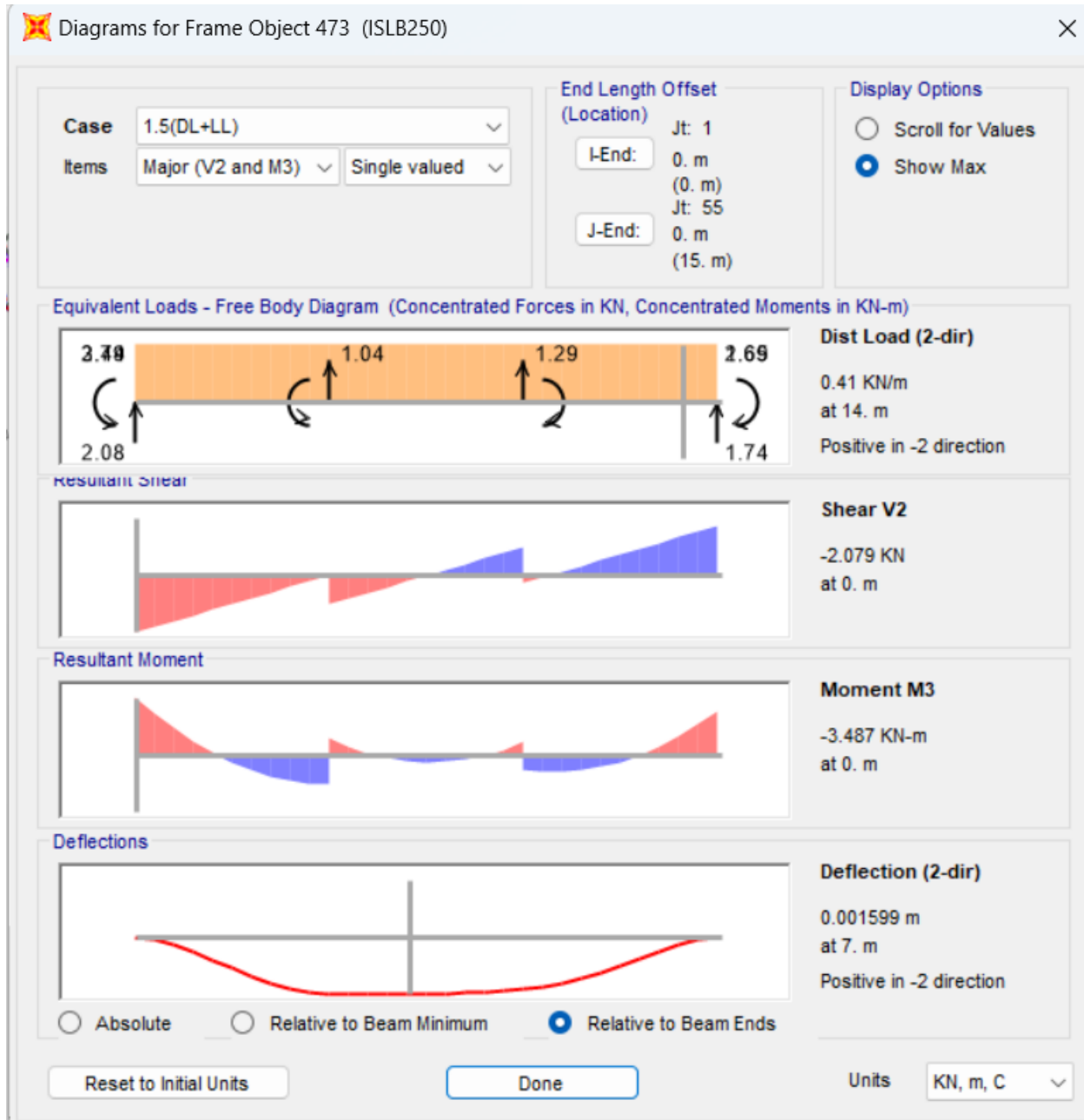


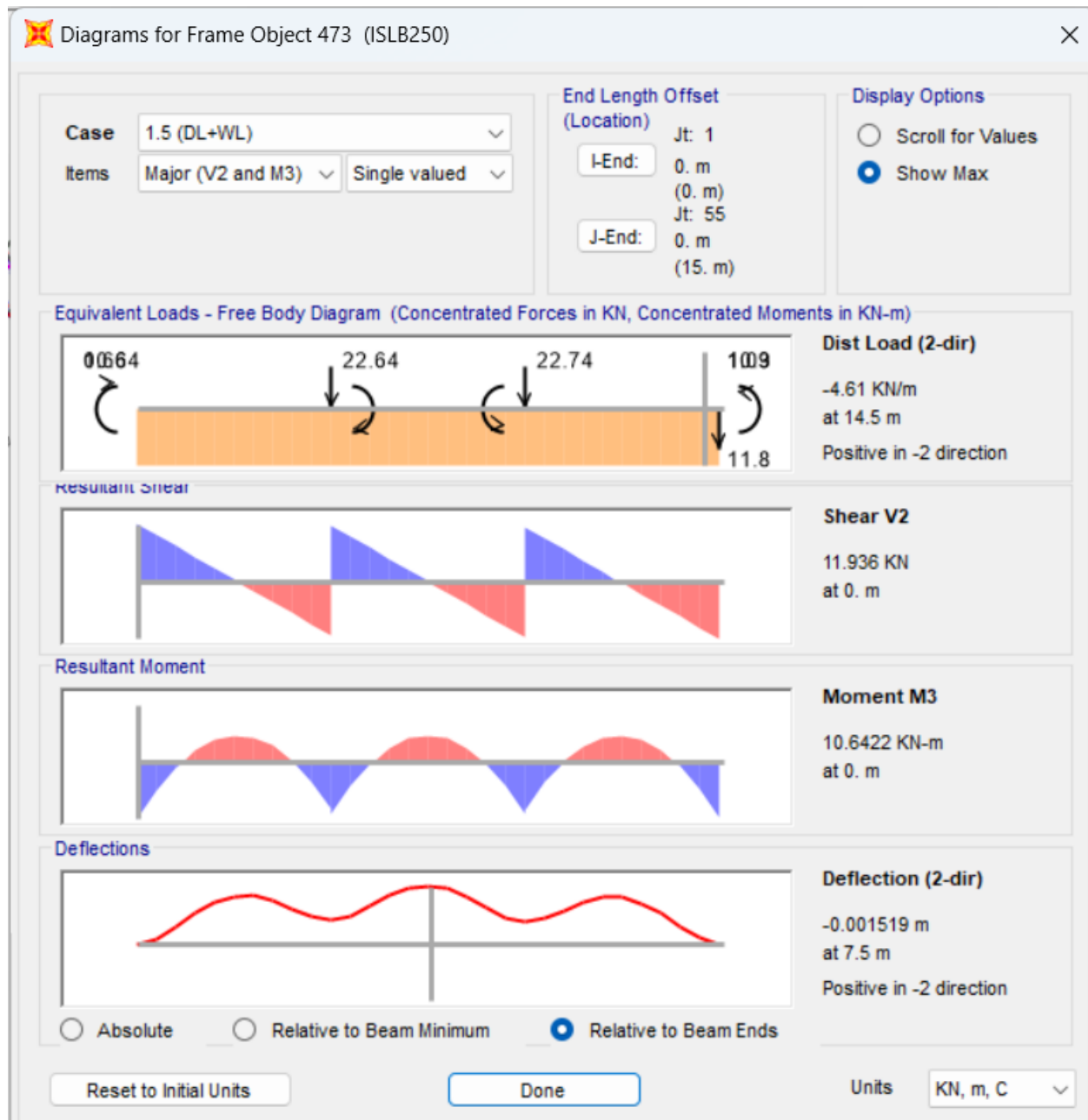
- For Beams(Member 4)

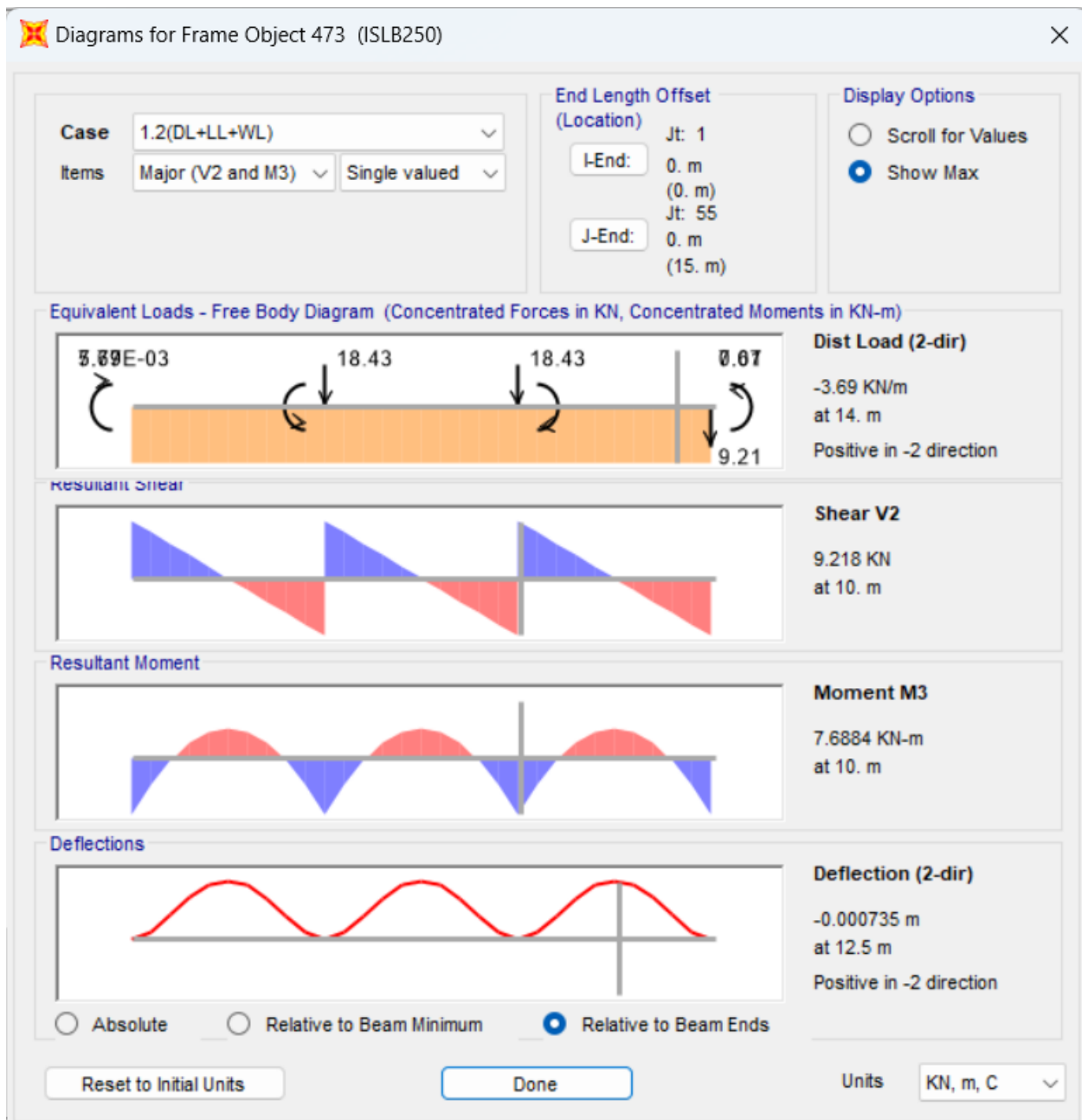
Maximum Bending Moment :10.6422 KNm

Maximum Shear Force :11.936 KN

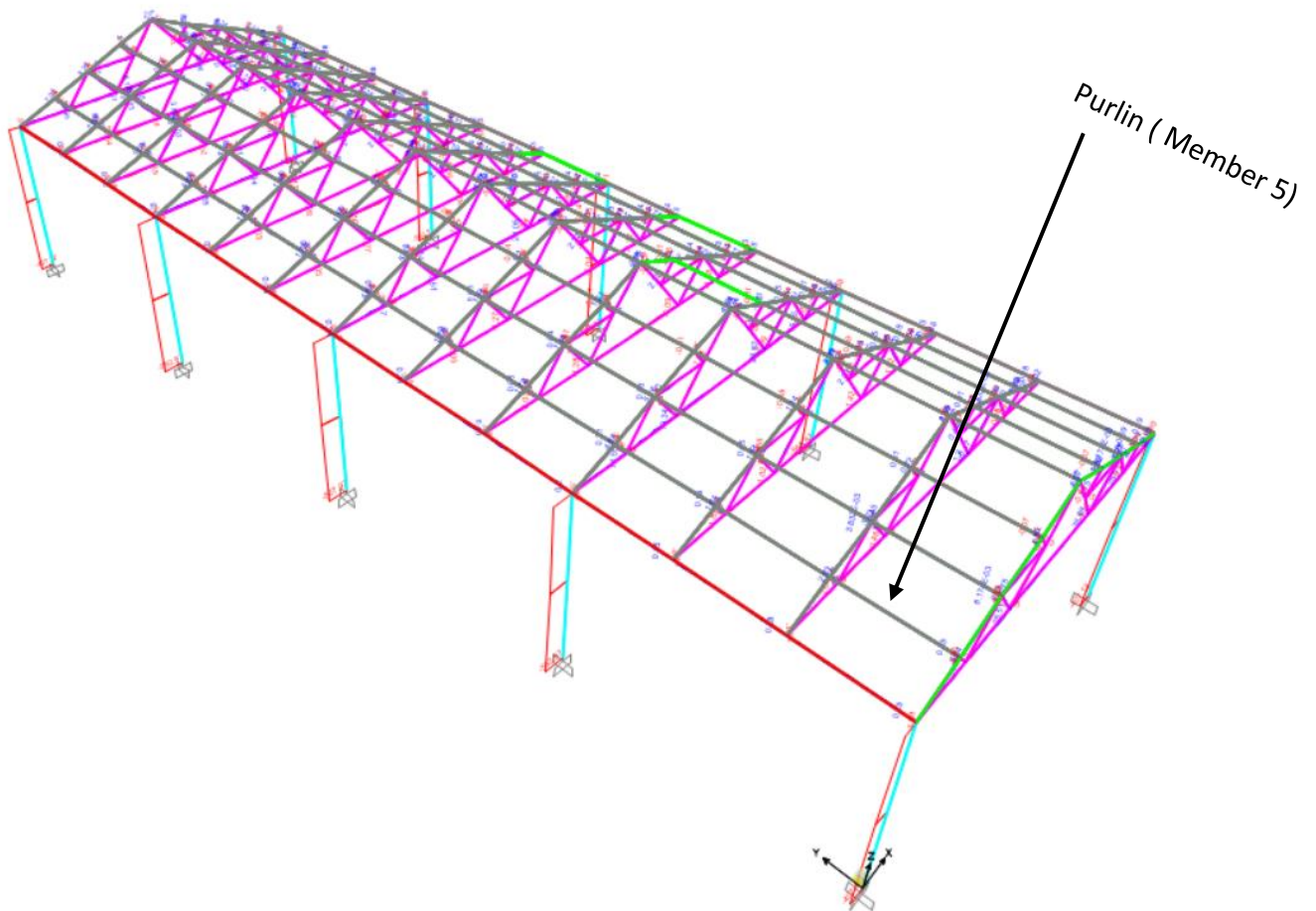
Load Combination :1.5(DL+WL)





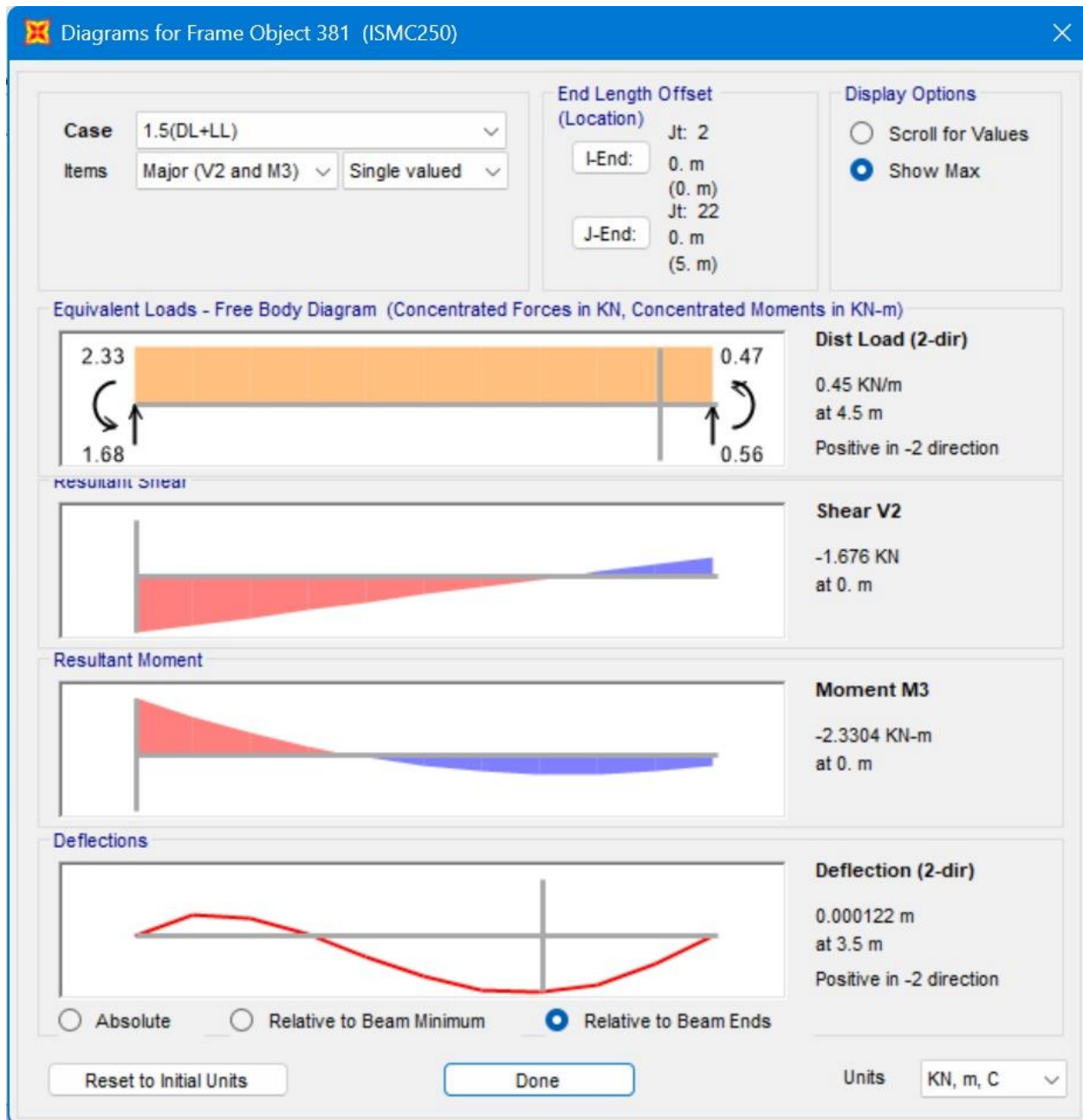


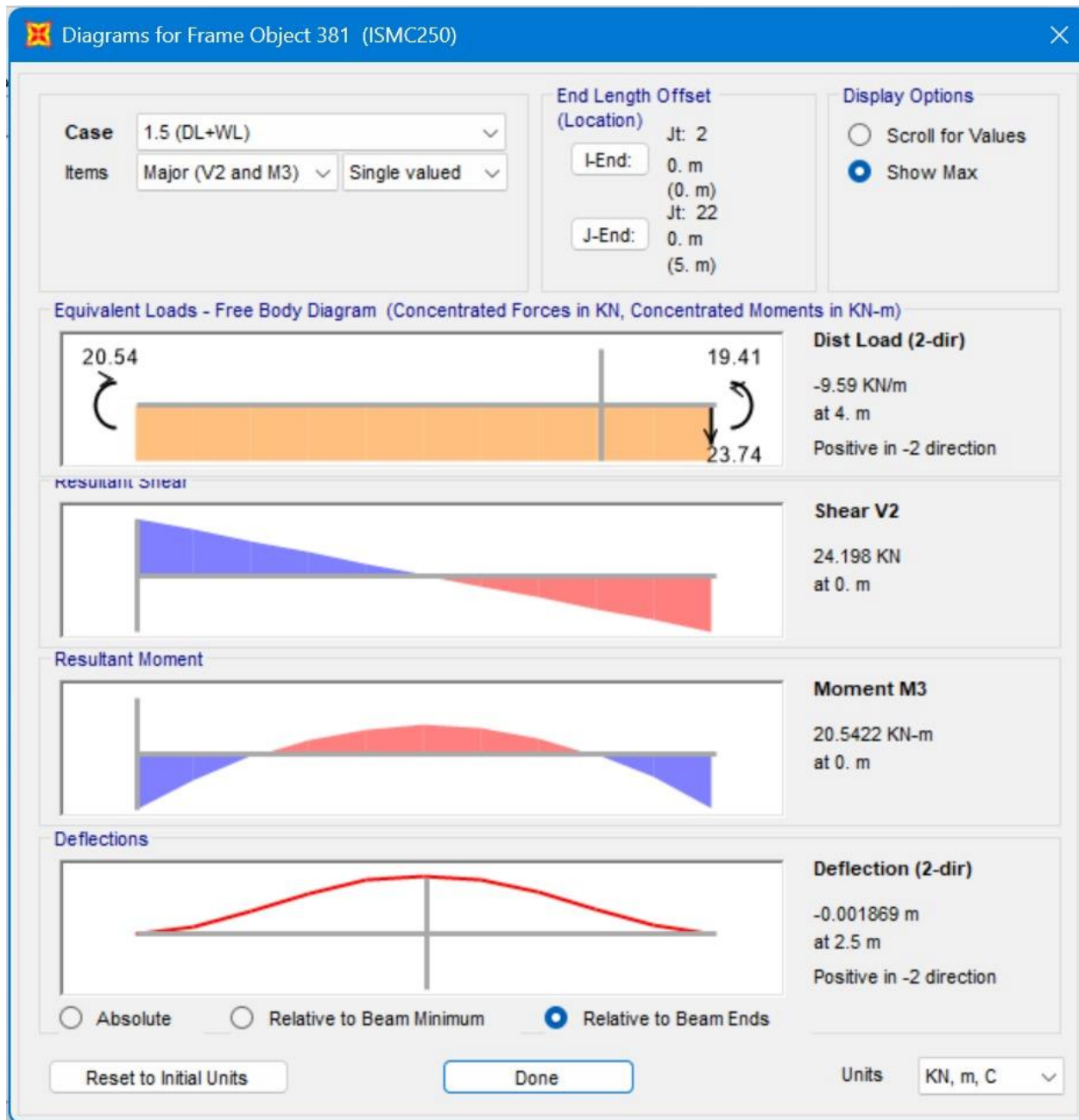
- For Purlin(Member 5)



Maximum Shear Force:24.198KN

Maximum Bending Moment :20.5422 KNm







Diagrams for Frame Object 381 (ISMC250)



Case 1.2(DL+LL+WL) ▾

Items Major (V2 and M3) ▾ Single valued ▾

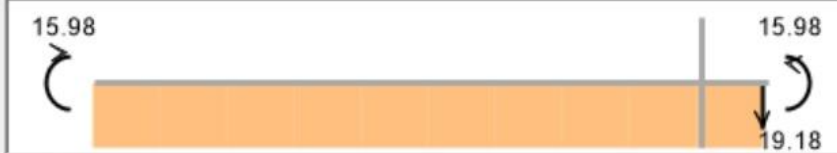
End Length Offset
(Location)

I-End: 0. m
(0. m)
Jt: 22
J-End: 0. m
(5. m)

Display Options

☐ Scroll for Values
☒ Show Max

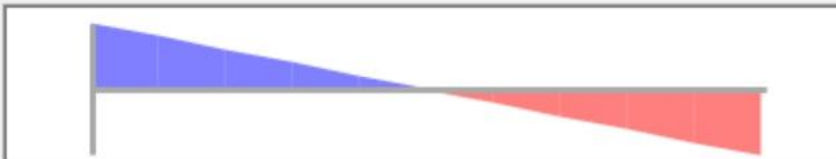
Equivalent Loads - Free Body Diagram (Concentrated Forces in KN, Concentrated Moments in KN-m)



Dist Load (2-dir)

-7.67 KN/m
at 4.5 m
Positive in -2 direction

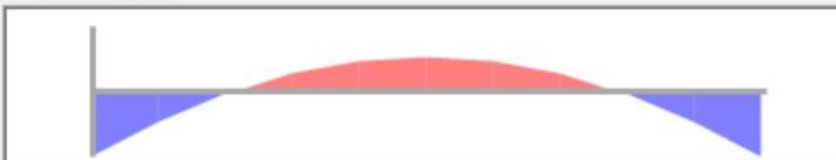
Resultant Shear



Shear V2

19.178 KN
at 0. m

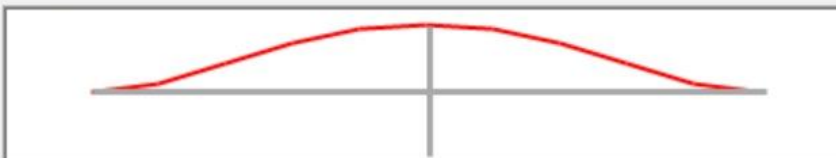
Resultant Moment



Moment M3

15.9823 KN-m
at 0. m

Deflections



Deflection (2-dir)

-0.001495 m
at 2.5 m
Positive in -2 direction

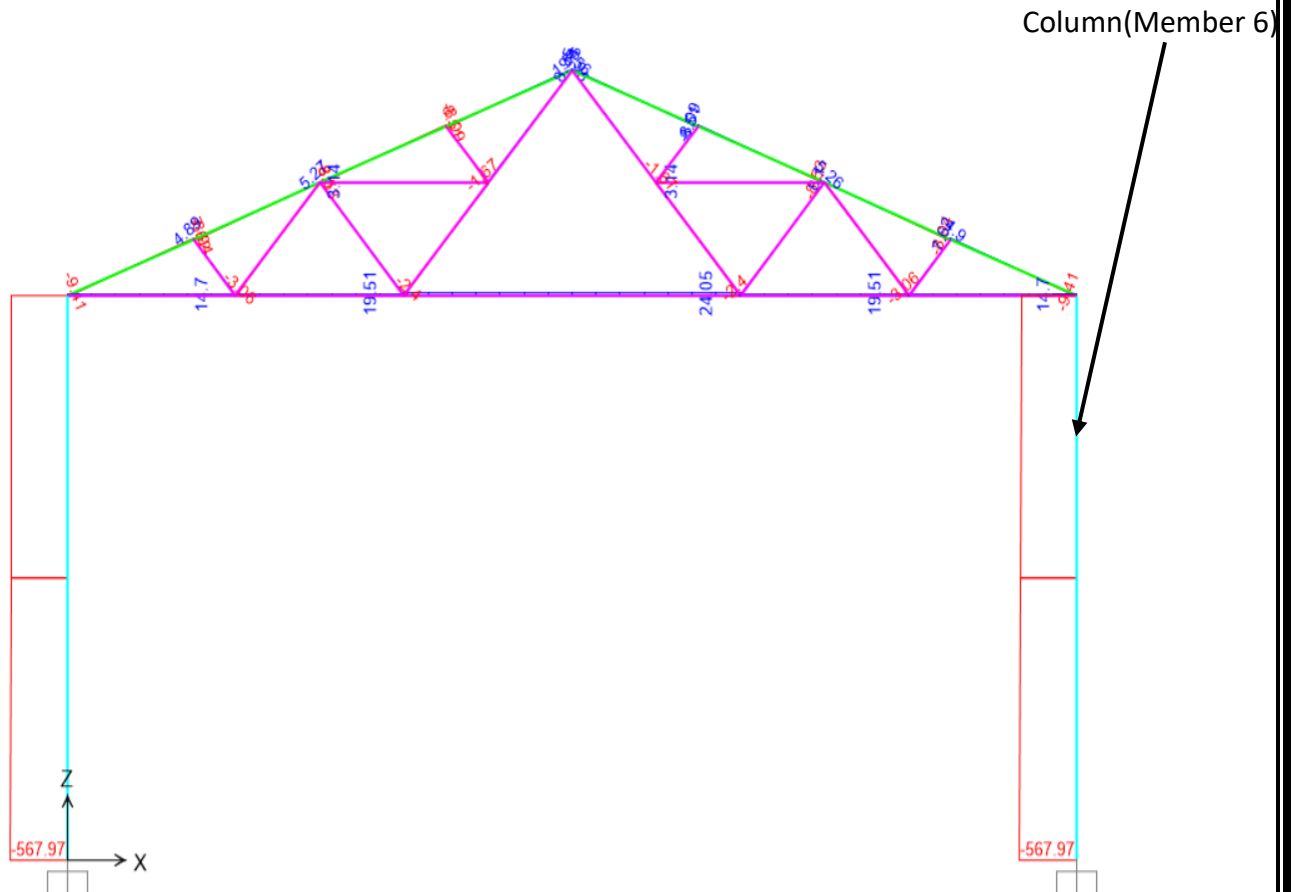
☐ Absolute ☐ Relative to Beam Minimum ☒ Relative to Beam Ends

Reset to Initial Units

Done

Units KN, m, C ▾

- For Column(Member 6)



Case

1.5(DL+LL)

▼

Items

Axial (P and T)

▼

Single valued

▼

End Length Offset (Location)

Jt: 106

I-End: 0. m (0. m)

Jt: 121

J-End: 0. m (10. m)

Display Options

☐ Scroll for Values

☒ Show Max

Equivalent Loads - Free Body Diagram (Concentrated Forces in KN, Concentrated Torsions in KN-m)

551.42

→

567.97

←

Dist Load (1-dir)

-1.65 KN/m at 10. m

Positive in -1 direction

Resultant Axial Force

Axial

-567.972 KN at 10. m

Resultant Torsion

Torsion

0. KN-m at 10. m

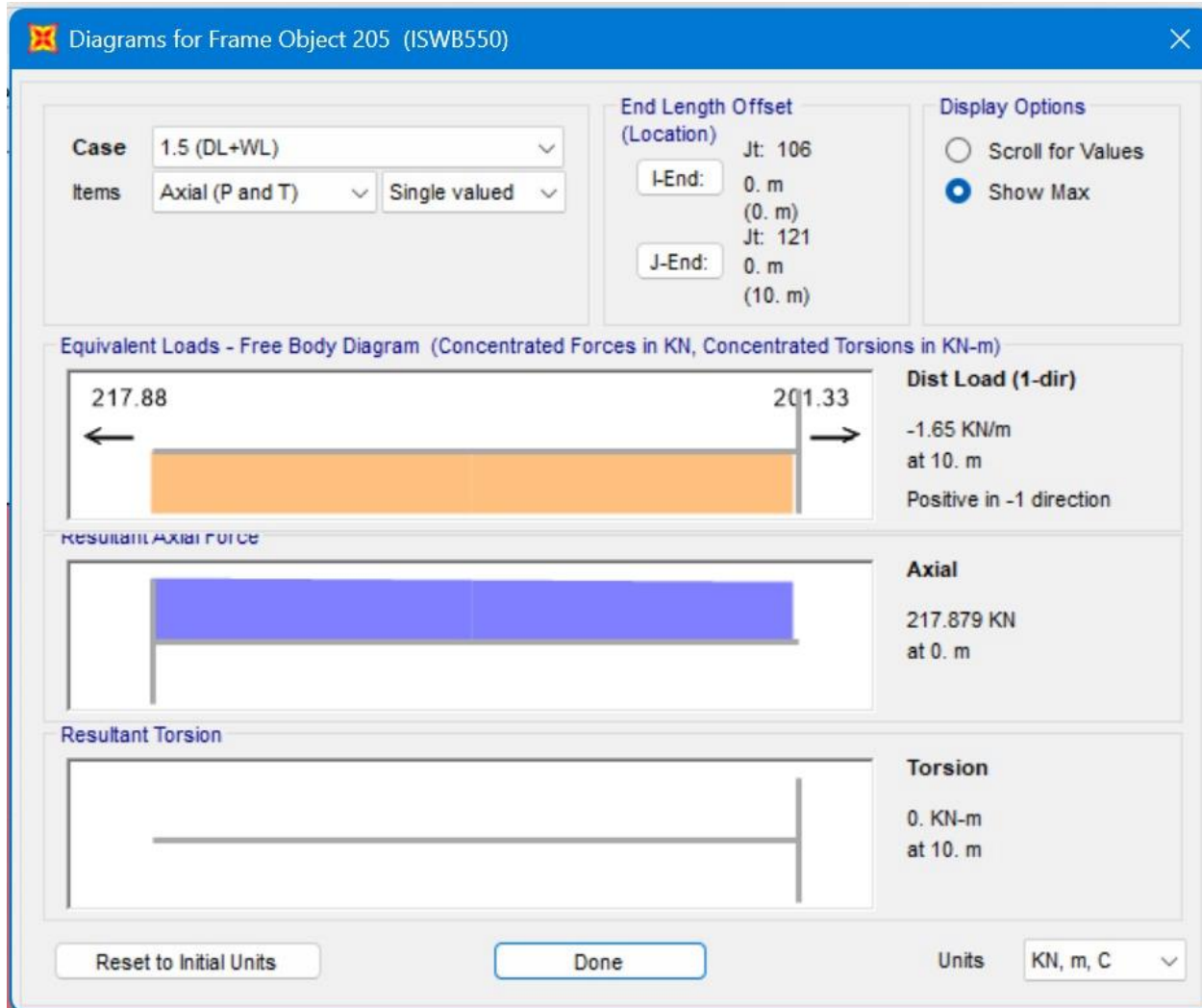
Reset to Initial Units

Done

Units

KN, m, C

▼



Diagrams for Frame Object 205 (ISWB550)

Case

1.2(DL+LL+WL)

Items

Axial (P and T)

Single valued

End Length Offset (Location)

I-End:

0. m (0. m)

J-End:

0. m (10. m)

Display Options

☐ Scroll for Values
 ☒ Show Max

Equivalent Loads - Free Body Diagram (Concentrated Forces in KN, Concentrated Torsions in KN-m)

2.31

→

15.55

←

Dist Load (1-dir)

-1.32 KN/m at 5. m

Positive in -1 direction

Resultant Axial Force

Axial

-15.554 KN at 10. m

Resultant Torsion

Torsion

0. KN-m at 10. m

Reset to Initial Units

Done

Units

KN, m, C

Table Forces in Critical Members

Member No.	Member	Load Type	Maximum Load	Load Case
1	Top Chord	Axial Force	-9.414 kN	1.5 (DL+LL)
			5.13 kN	1.5 (DL+WL)
			6.816 kN	1.2 (DL+LL+WL)
2	Bottom Chord	Axial Force	36.862 kN	1.5 (DL+LL)
			-14.367 kN	1.5 (DL+WL)
			0.268 kN	1.2 (DL+LL+WL)
3	Diagonal Chord	Axial Force	5.267 kN	1.5 (DL+LL)
			0.907 kN	1.5 (DL+WL)
			1.726 kN	1.2 (DL+LL+WL)
4	Beam	Shear Force	-2.079 kN	1.5 (DL+LL)
			11.936 kN	1.5 (DL+WL)
			9.218 kN	1.2 (DL+LL+WL)
		Bending Moment	-3.487 kN-m	1.5 (DL+LL)
			10.6422 kN-m	1.5 (DL+WL)
			7.6884 kN-m	1.2 (DL+LL+WL)
5	Purlin	Shear Force	-1.676 kN	1.5 (DL+LL)
			24.198 kN	1.5 (DL+WL)
			19.178 kN	1.2 (DL+LL+WL)
		Bending Moment	-2.3304 kN-m	1.5 (DL+LL)
			20.5422 kN-m	1.5 (DL+WL)
			15.9823 kN-m	1.2 (DL+LL+WL)
6	Column	Axial Force	-567.972 kN	1.5 (DL+LL)
			217.879 kN	1.5 (DL+WL)
			-15.554 kN	1.2 (DL+LL+WL)