

Dynamo for Engineers, Design for All

Processes that work for us



Ian Wise | Senior Engineer | Arup

Steven Brown | Engineer | Arup

Hugh Groves | Engineer | Arup

Some context

Life on planet Arup



Article
July 2017

Improving construction productivity

By Filipe Barbosa, Jan Mischke, and Matthew Parsons



McKinsey research finds seven levers can fix construction's productivity problem, but they require a new approach from all players. We heard from industry leaders about which barriers to change are most likely to fall first.

The McKinsey Global Institute (MGI's) *Reinventing construction: A route to higher productivity* report, released in February 2017, found that the construction industry has an intractable productivity problem. While sectors such as retail and manufacturing have reinvented themselves, construction seems stuck in a time warp. Global labor-productivity growth in construction has averaged only 1 percent a year over the past two decades, compared with growth of 2.8 percent for the total world economy and 3.6 percent in manufacturing (exhibit).

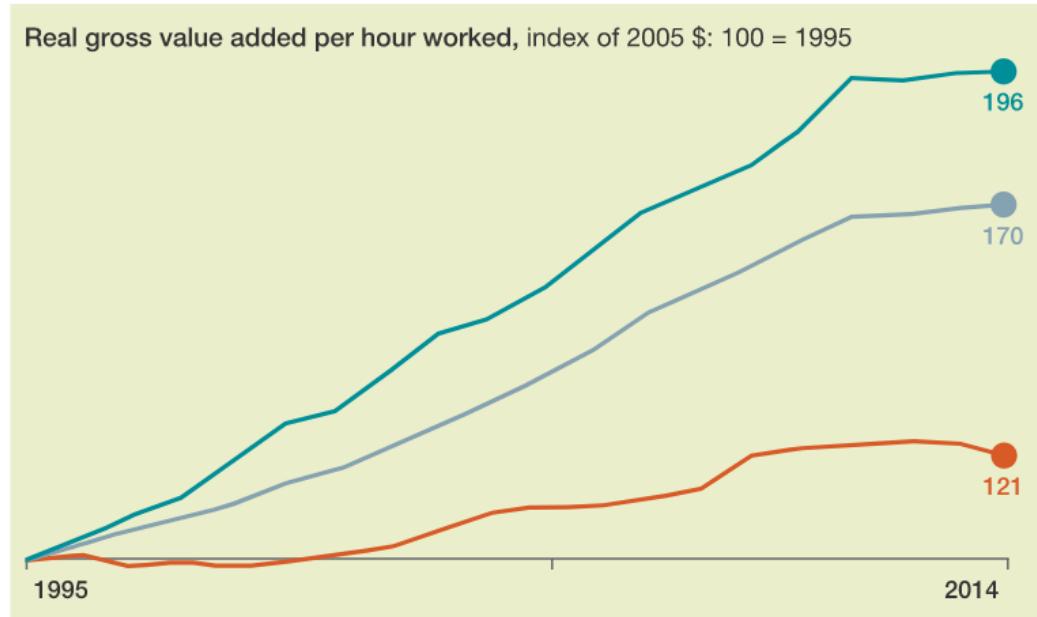
RELATED



Global Infrastructure Initiative

Convening global leaders in infrastructure and capital projects in pursuit of new solutions

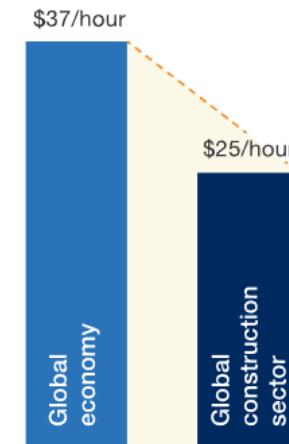
[Learn more](#)



Source: GGCD-10; national statistical agencies of Turkey, Malaysia, and Singapore; OECD, Rosstat; US Bureau of Economic Affairs; US Bureau of Labor Statistics; WIOD; World Bank; McKinsey Global Institute analysis

McKinsey&Company

Productivity gap =
\$1.63 trillion



Average value added by
employees per hour worked¹

¹2015 data in real 2005 dollars.

²Assumes construction productivity catches up with total economy productivity and current workers are reemployed at the total economy productivity rate.

McKinsey&Company

Economic value lost as a result of the gap,²
by region, \$ trillion





Article
June 2016

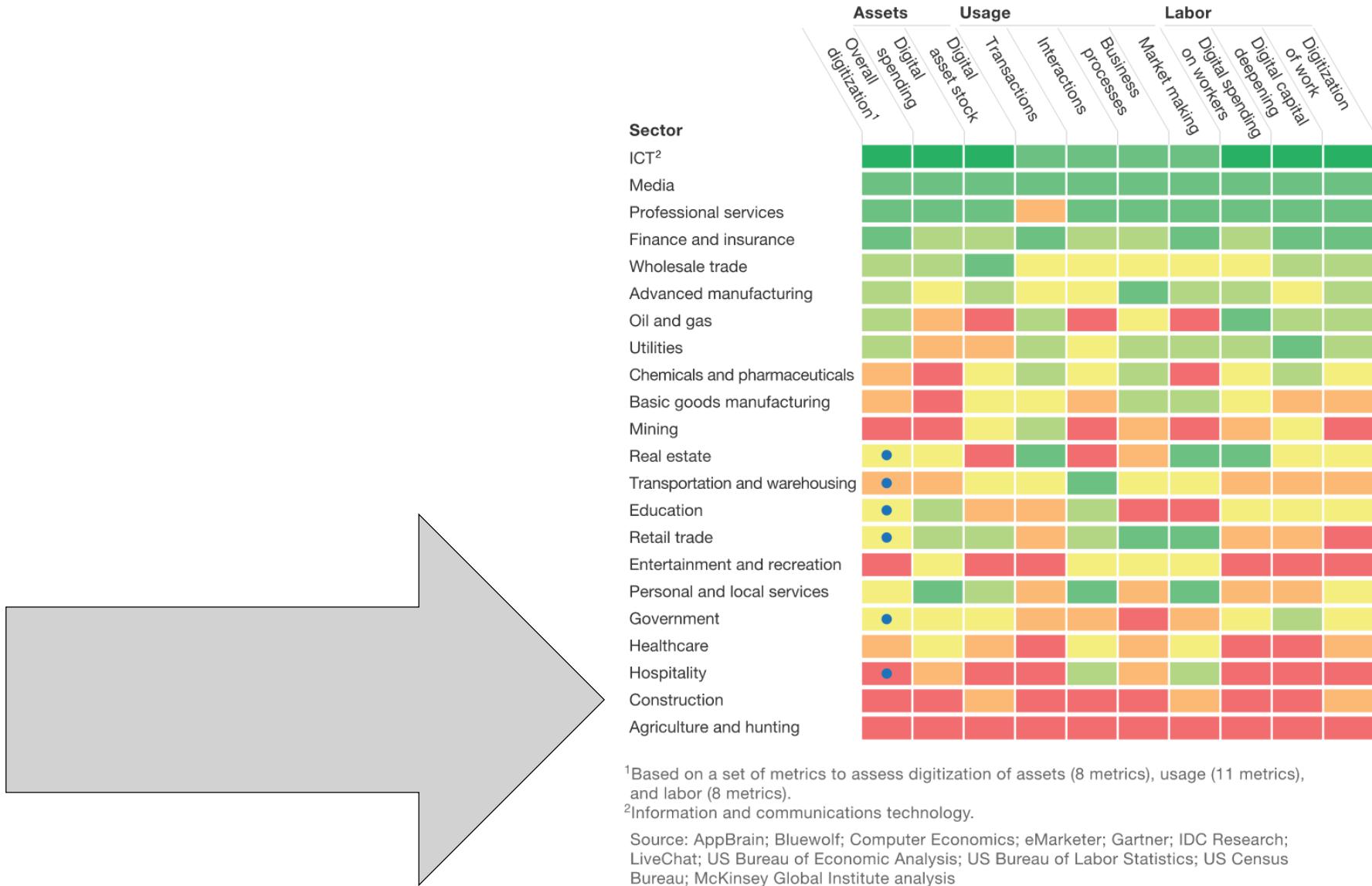
Imagining construction's digital future

By Rajat Agarwal, Shankar Chandrasekaran, and Mukund Sridhar



The industry needs to change; here's how to manage it.

The construction industry is ripe for disruption. Large projects across asset classes typically take 20 percent longer to finish than scheduled and are up to 80 percent over budget (Exhibit 1). Construction productivity has actually declined in some markets since the 1990s (Exhibit 2); financial returns for contractors are often relatively low—and volatile.



McKinsey&Company

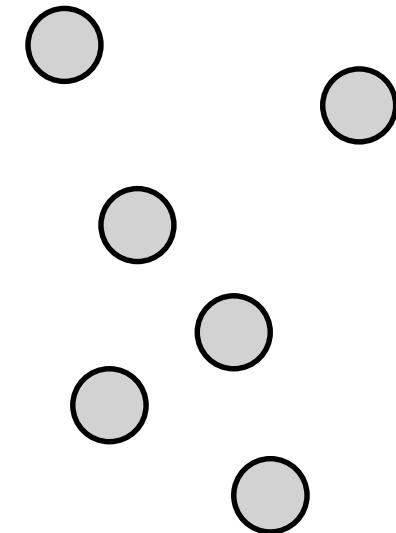
ARUP

Some things are very
hard and demand new
approaches

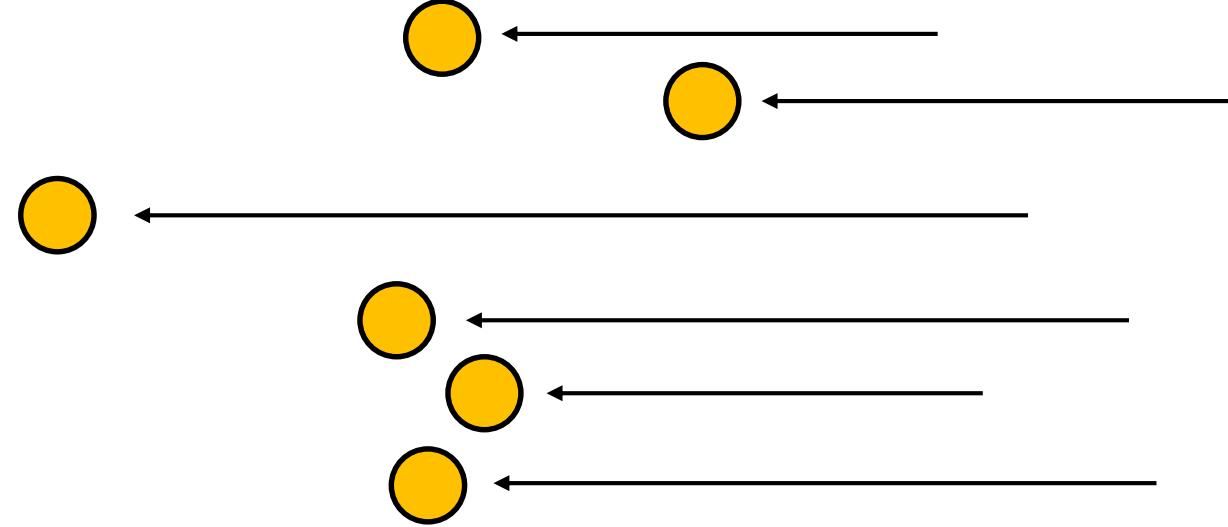
Child's play

Manageable

Impossible



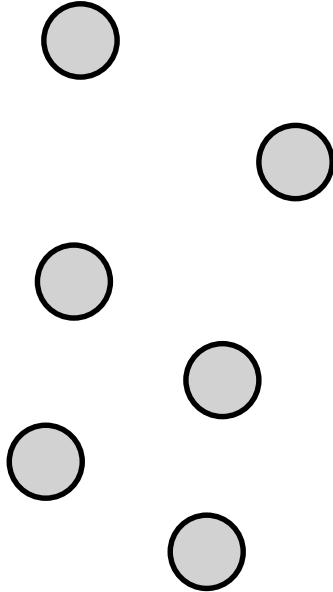
We can make
them practical
and achievable



Child's play

Manageable

Impossible

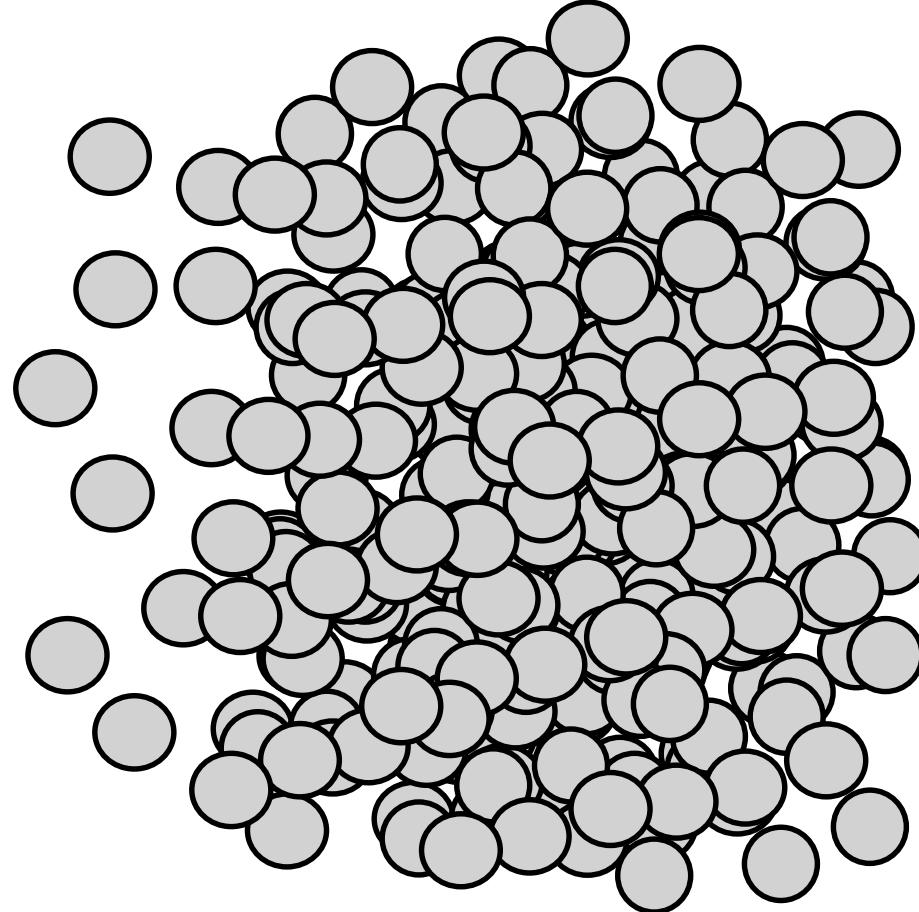


Child's play

Manageable

But what about the
things that we can
already do?

Impossible

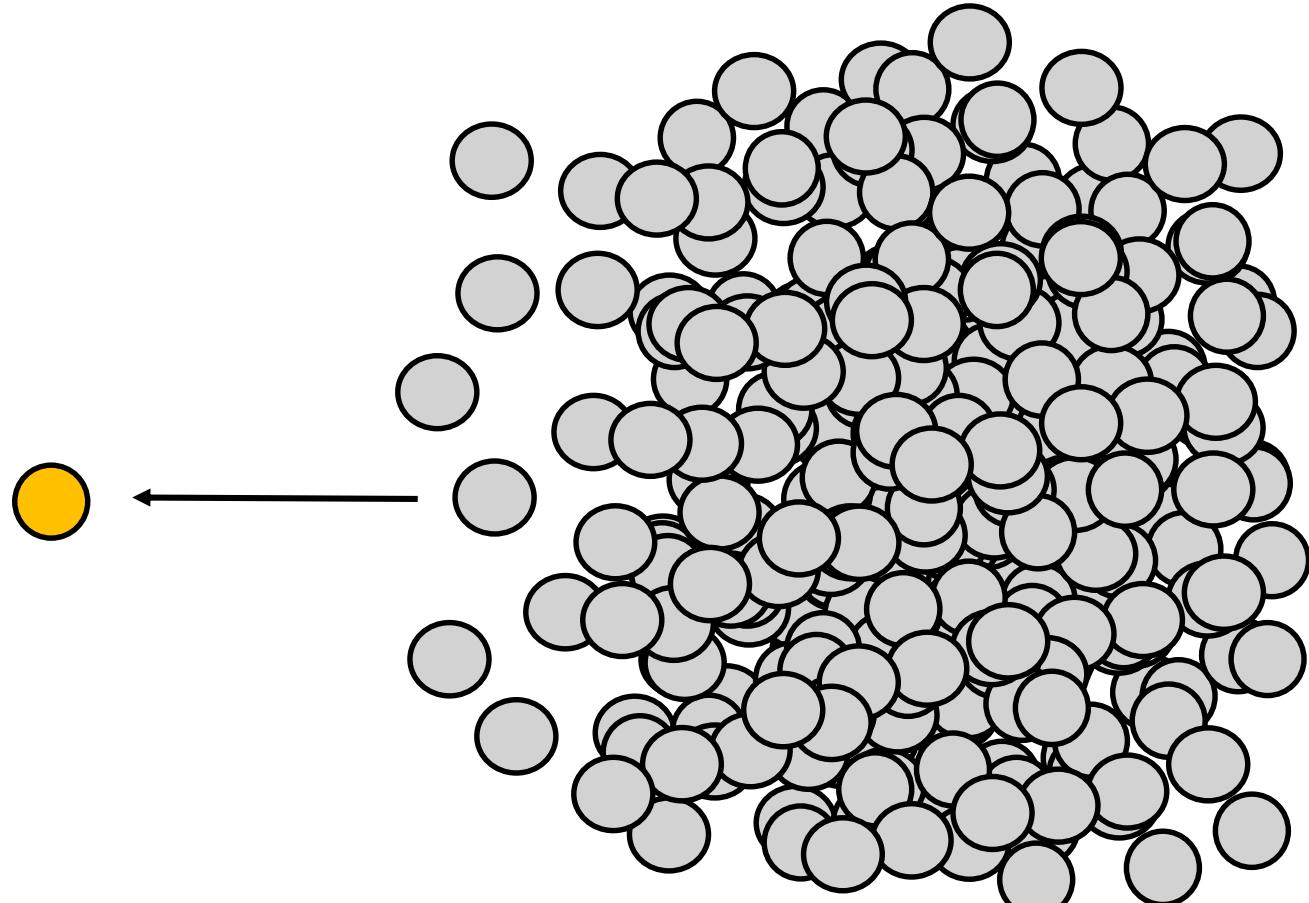


Child's play

Manageable

Because there are a
lot of those?

Impossible



Child's play

Manageable

Impossible

How *easy* can we
make them?

Current practice

And why we want to change it...



Spreadsheets

Data. Output.

- Calculations
- Specifications
- Drawings

So much data...

So many places!

Spreadsheets

Data. Output.



Where does
the data go
after this





Nowhere

Spreadsheets

Messy data that goes nowhere.



Is this
really what
we want



Not.

Not. Even.

Not. Even. Close.

Technology advances
So should we...

*"It is not the strongest of the species
that survives, nor the most intelligent
that survives. It is the one that is **most**
adaptable to change."*

- Charles Darwin.

*“So why do we insist on
doing things the ‘old’ way?”*

- Us.



LANXESS arena

RheinEnergie

Start

RheinEnergie



RheinEnergie

Start

RheinEnergie

Calculating...

Calculating...

Calculating...

Calculating...









WHAT IF...
ooo

Creating
A toolmakers mentality...



*"The point is that I don't design stuff for **myself**. I'm a toolmaker. I design things that **other people** want to use."*

- Robert Moog.



Spreadsheets

Messy data that goes nowhere.



Is this
really what
we want

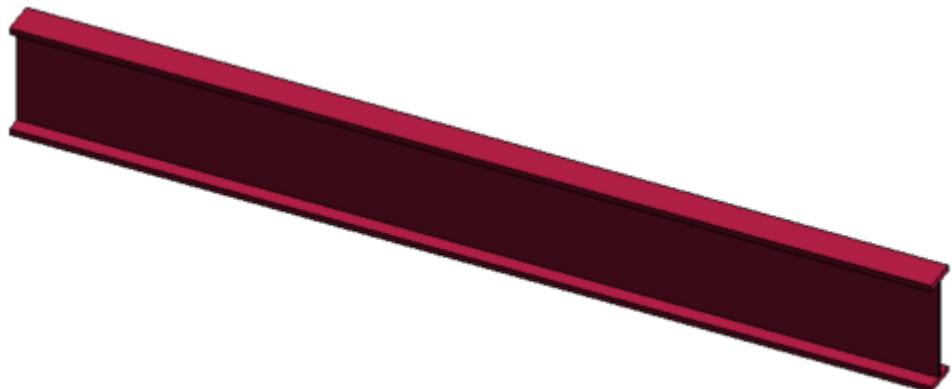
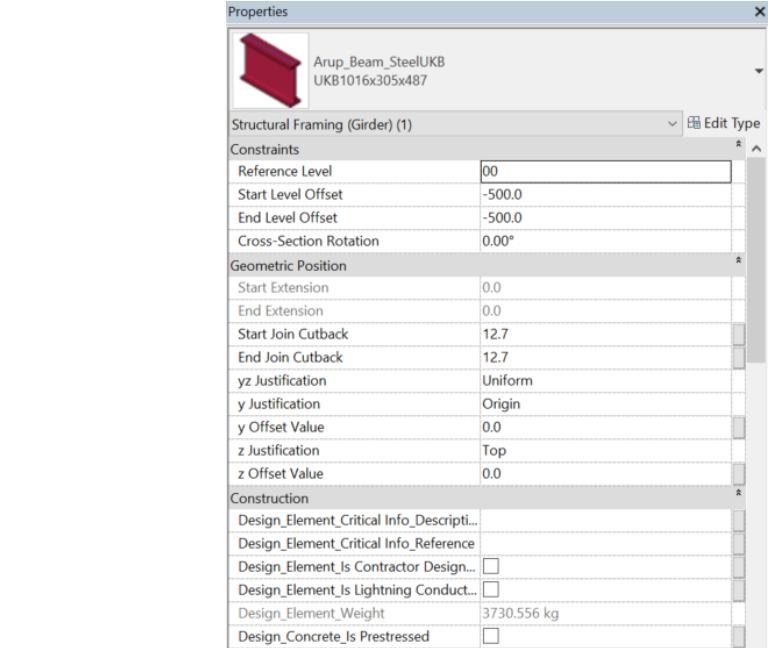


*"Why don't we use the data
where it is hosted?"*

- Us.



A single data
environment



Embedded
parameters for
intelligent
delivery

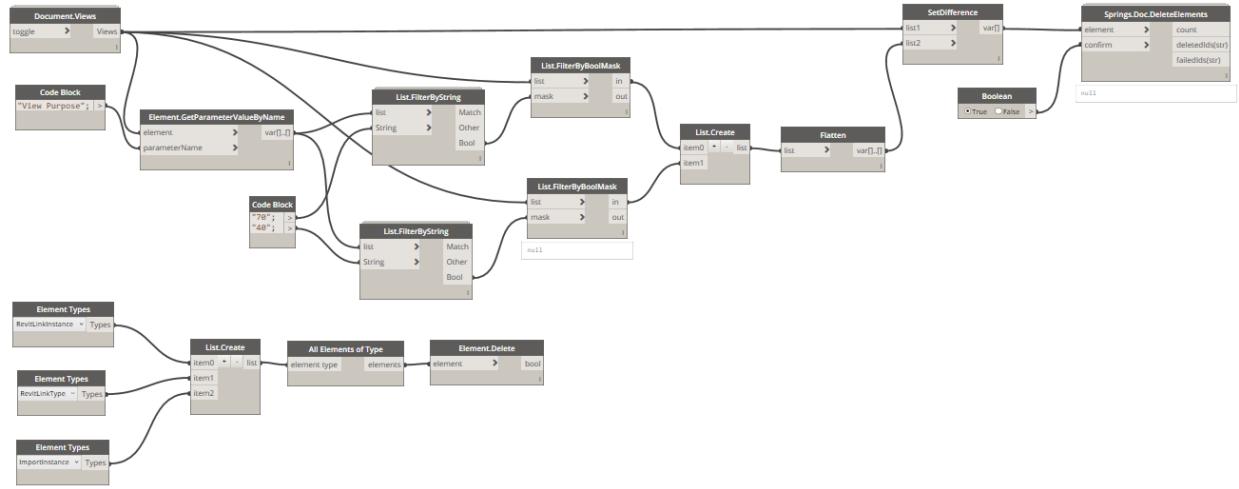


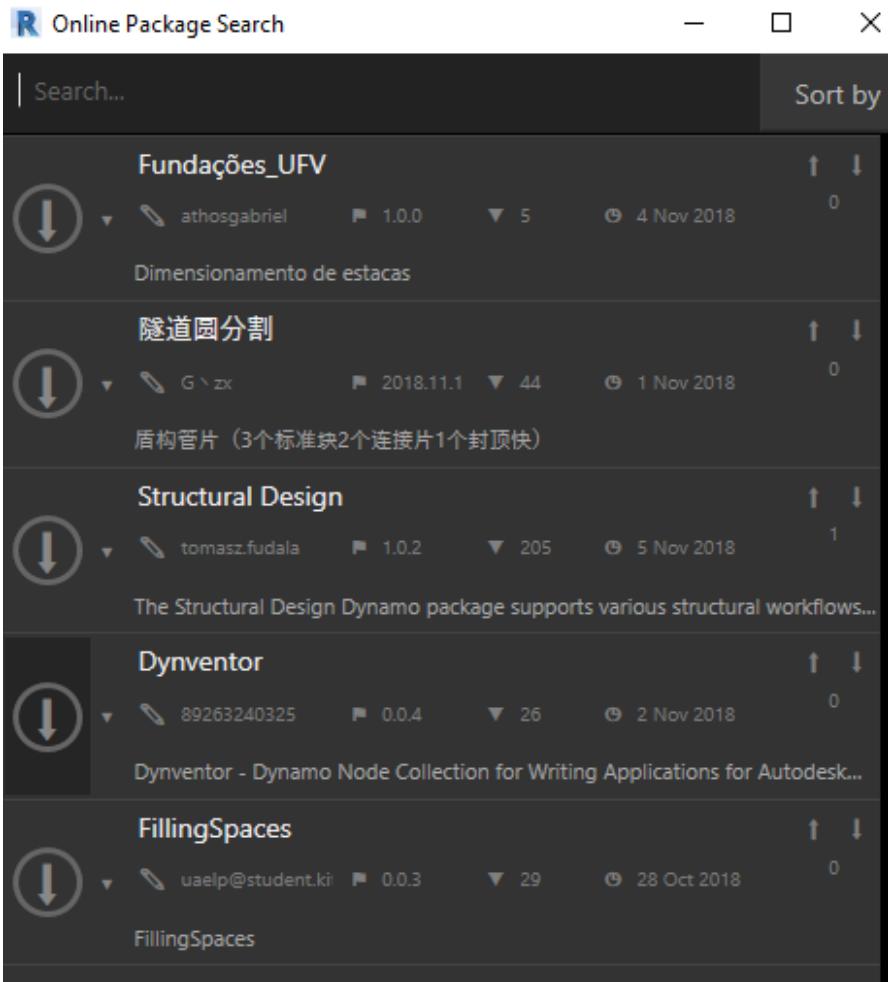
A single digital
tool



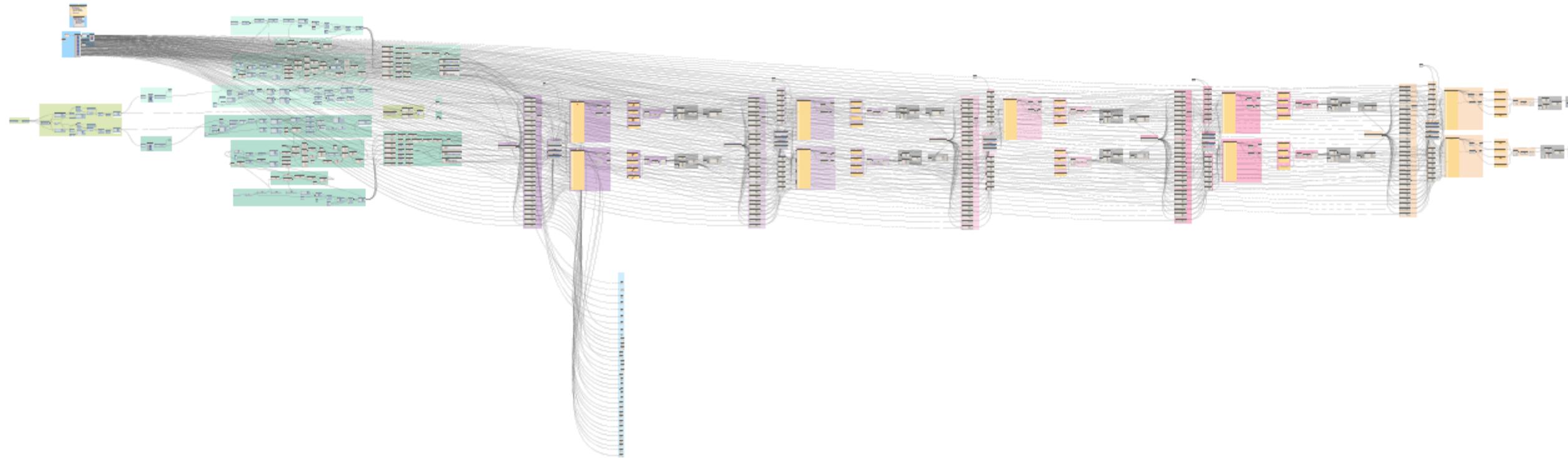
ARUP

Read and write parameters for integrated calculation





Libraries written
by other people



Parametric spaghetti

ARUP

Useful tools

We made some...



???

| B | C | D | E | F | G | H | I | J |
|---------------------|---|------|---|---|---|---|---|---|
| Right | | 0 | | | | | | |
| Min | | -1 | | | | | | |
| Sides | | 0 | | | | | | |
| Sum | | 0 | | | | | | |
| Bending Coefficient | | | | | | | | |
| U | | 0.43 | | | | | | |
| KLR | | 2 | | | | | | |
| KBT | | 1.7 | | | | | | |
| KL | | 1.7 | | | | | | |
| KB | | 3.0 | | | | | | |
| A | | 0.6 | | | | | | |

| A | B | C | D | E | F | G | I | J | K | L | M | N |
|--|------------------------------------|------------------|--------------------|---|---|---|---|---|---|---|---|---|
| Characteristic Properties from BS EN338:2005, BS EN1154:1999 and Karto VTT Catt.No.184/03) | | | | | | | | | | | | |
| Type | Grade | Profile | | | | | | | | | | |
| Solid | C24 | Straight | | | | | | | | | | |
| Load Duration (2.3.12 & Nat.Annes) | Service Class (2.3.13 & Nat.Annes) | | | | | | | | | | | |
| Medium Term | Class 1 | | | | | | | | | | | |
| Modification Factor (3.1.3) | | | | | | | | | | | | |
| Depth Factor (3.2, 3.3, 3.4) | yDirection | $k_{mod} = 0.80$ | | | | | | | | | | |
| | zDirection | $k_{h,z} = 1.00$ | $k_{h,m,z} = 1.00$ | | | | | | | | | |
| | | | $k_{h,m,z} = 1.00$ | | | | | | | | | |
| Length Factor (for LVL only) | | | | | | | | | | | | |
| Bending Factor (6.1.6) | $k_t = 1.00$ | | | | | | | | | | | |
| | $k_m = 0.70$ | | | | | | | | | | | |
| | | | | | | | | | | | | |

Sub-Contractor

Anup Riyad Metro Limited
13 Fitzroy Street
London W1T 4BQ
England

ARUP

Riyadh Metro Project Package 1

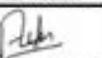
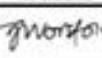
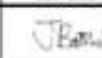
WP12 Deep Stations

Spreadsheet Verification Report - Structures

Document No. M-BAR-000DS0-CB00-ECA-000008

Document Status: For Review

Document Submittal History:

| Rev.: | Date: | Doc Status: | Prepared (Sub-Contractor) | Checked (Sub-Contractor) | Approved (Sub-Contractor) |
|-------|-----------|-----------------|---|---|---|
| 00A | 02-JUN-17 | For Information | Anton Thowles | Tim Worfold | John Batchelor |
| | | Signature: |  |  |  |

Note: All changes from the previous revision have a line to the left of the text.

| Document Review Status | Date: | Name: |
|---|-------|-------|
| A - Reviewed; Work may proceed | | |
| B - Reviewed with comments; Revise and resubmit; Work may proceed subject to incorporation of comments | | |
| C - Objection - Revise and resubmit; Work may not proceed | | |
| D - Rejected | | |
| E - Review not required; Work may proceed | | |

Document No. M-BAR-000DS0-CB00-ECA-000008
Spreadsheet Verification Report - Structures

Printed: 1 Aug 17

© 2013 High Commission for the Development of AlRiyadhi

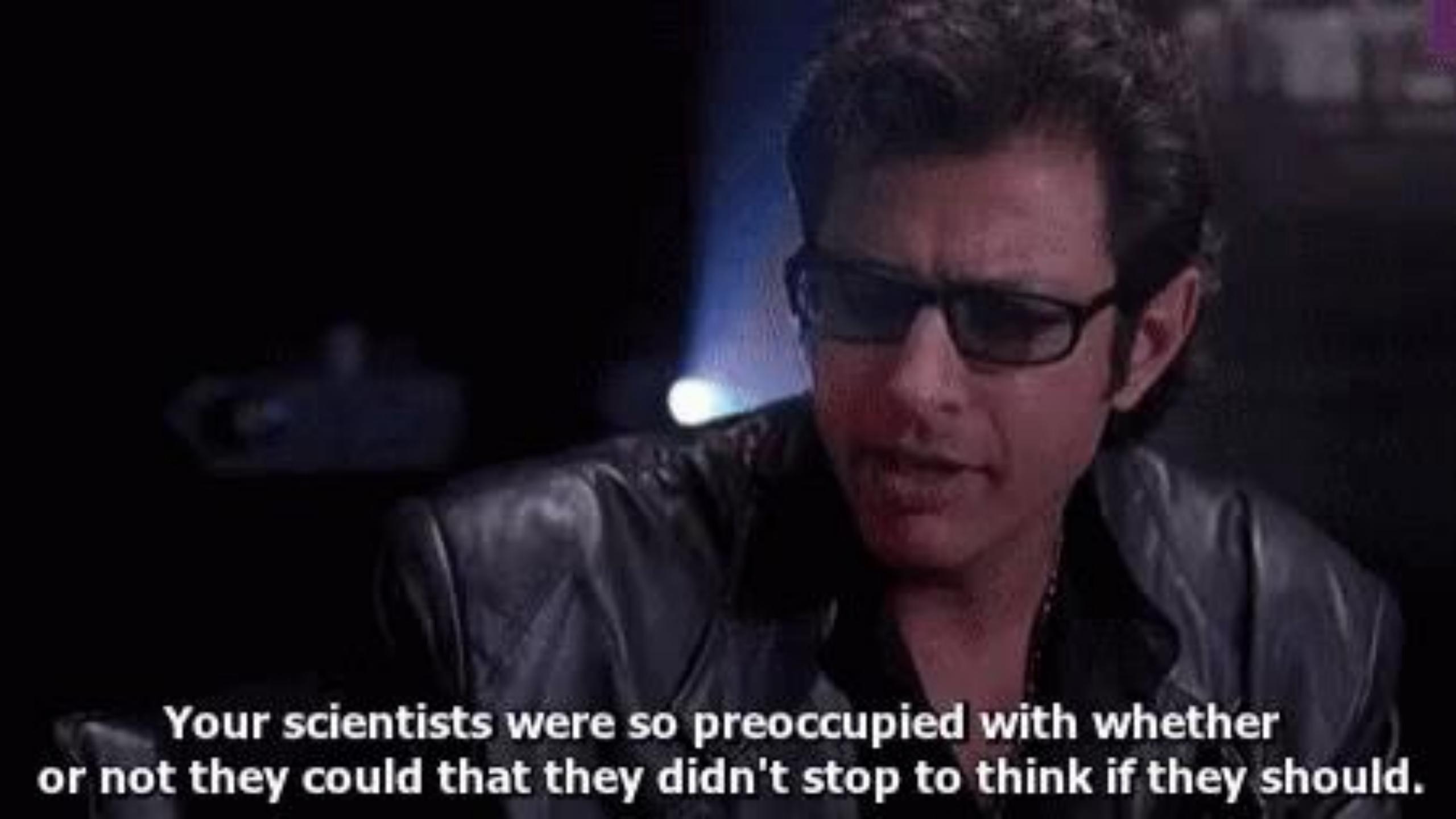
Electronic documents once printed, are uncontrolled and may become out-dated. Refer to Annex for current revision.



Error prone

Difficult to review

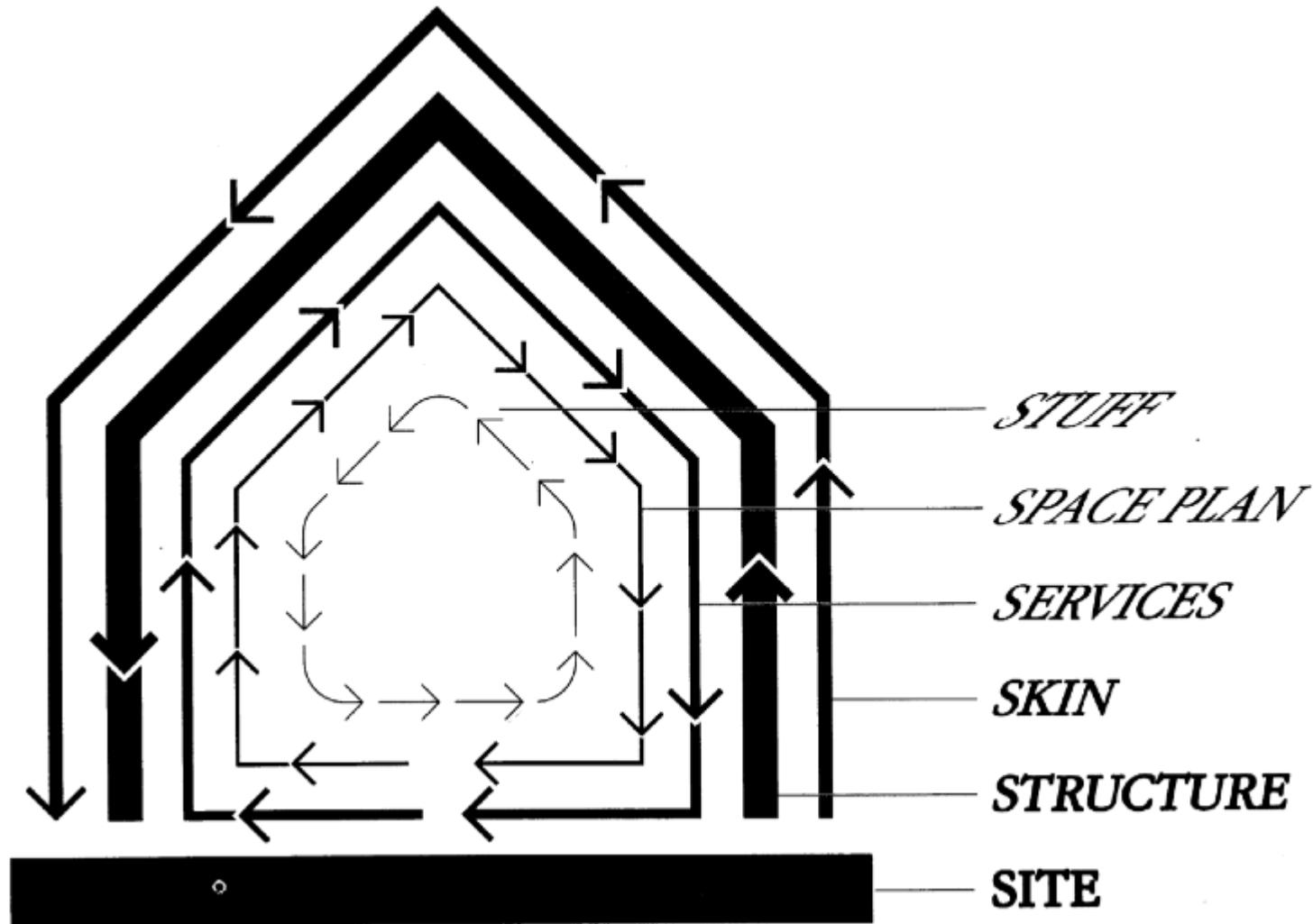
Never re-used



Your scientists were so preoccupied with whether
or not they could that they didn't stop to think if they should.



Simone Giertz



Change can
be an issue

Fast learns,
slow remembers.

Tooling

Patterns

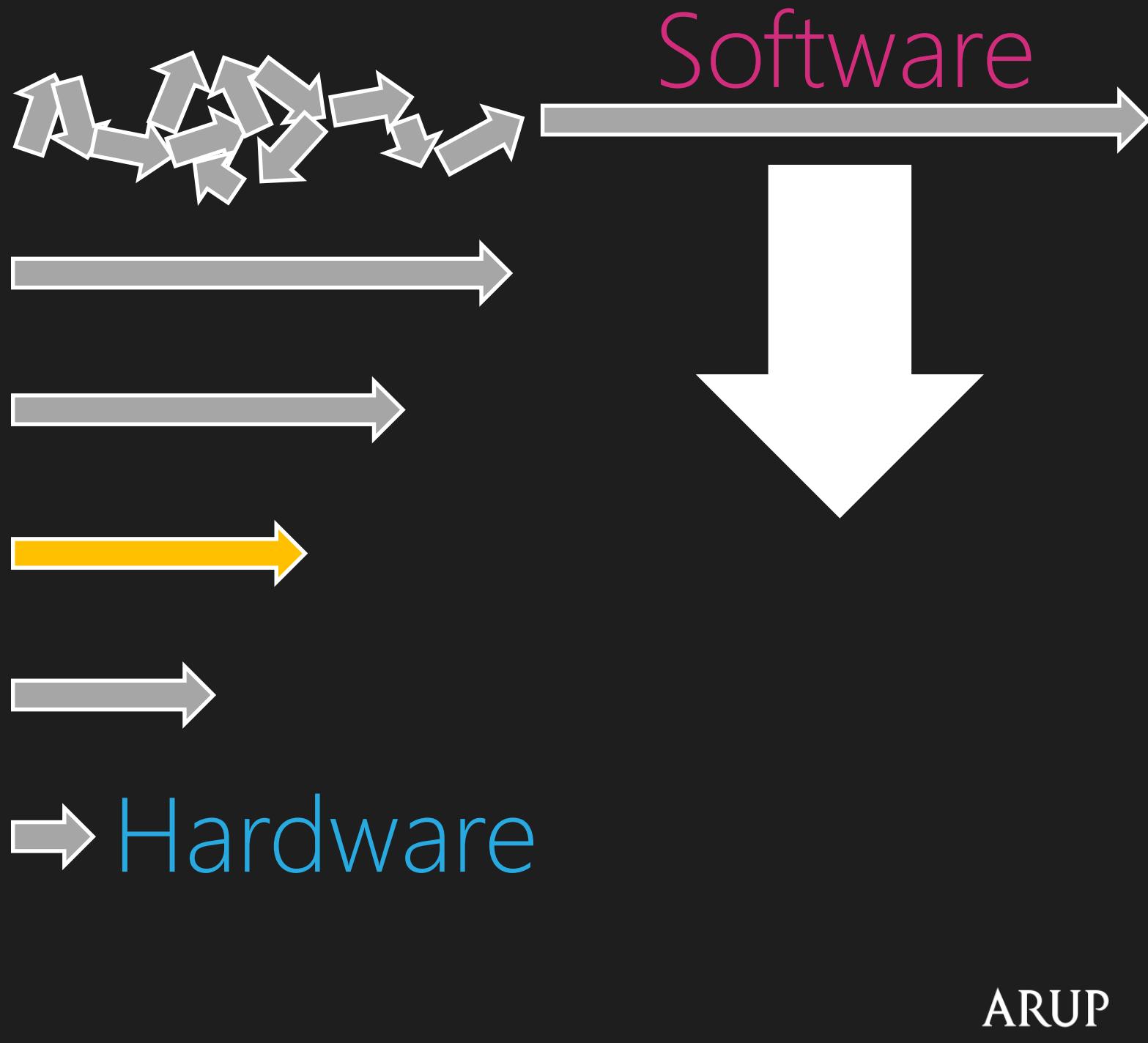
People

Documentation

Governance

Foundations

Rate of change



```

AddSubHeader("Inputs", false);
DeclareInput("d_w", d_w, "mm", "***EC3.GB.d_w***");
DeclareInput("e_c", e_c, "mm", "***EC3.GB.e_c***");
DeclareInput("e_p", e_p, "mm", "***EC3.GB.e_p***");
DeclareInput("m", m, "mm", "***EC3.GB.m***");
DeclareInput("t_f", t_f, "mm", "***EC3.GB.t_f***");
DeclareInput("Sigma_leff", Sigma_leff, "mm", "***EC3.GB.Sigma_l_eff***");
DeclareInput("f_y", f_y, "MPa", "***EC3.f_y***");
DeclareInput("gamma_M0", gamma_M0, "", "***EC3.gamma_M0***");

AddSubHeader("Calculation", false);
CalcVariable("n", "min(e_c, e_p, 1.25 * m)", "mm");
CalcVariable("e_w", "d_w / 4", "mm");

TStubPlasticMomentResistance calc_TStubPlasticMomentResistance = new TStubPlasticMomentResistance(t_f, Sigma_leff, f_y, gamma_M0, 1);
if (SubCalculationValid("calc_M_pl_1_Rd", calc_TStubPlasticMomentResistance))
    AddVariable("M_pl_1_Rd", calc_TStubPlasticMomentResistance.Result, "Nmm", "***EC3.GB.TStubPlasticMomentResistance***");
else
    return;

CalcVariable("F_T_1_Rd", "((8 * n - 2 * e_w) * M_pl_1_Rd) / ((2 * m * n) - (e_w * (m + n)))", "N", "***EC3.GB.TStubModel1***");
ConvertVariable("F_T_1_Rd", "kN", 1.0 / 1000.0);

AddResult(V("F_T_1_Rd"));

```

Code the calculations

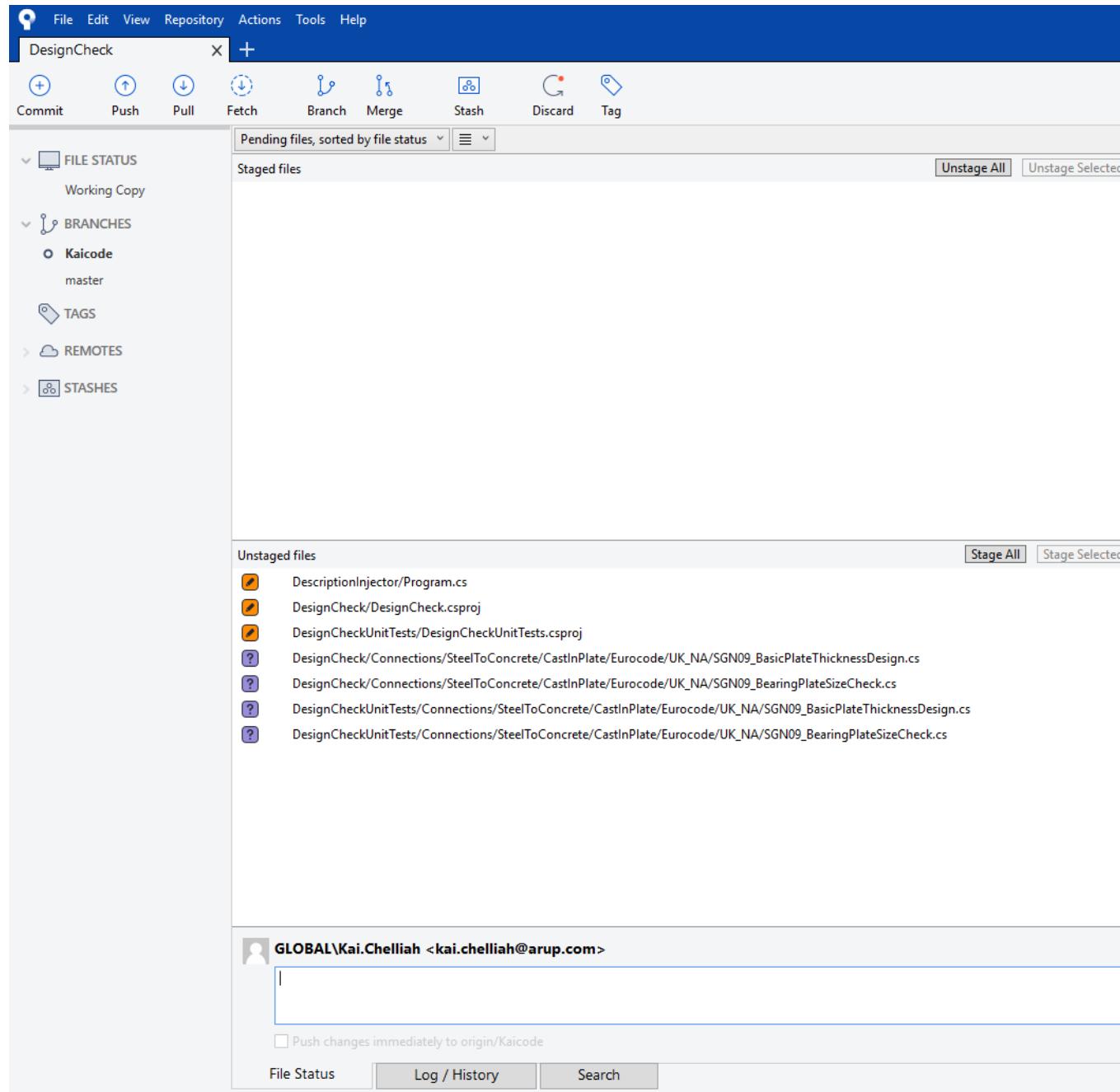
Easy to read...

Honestly

| DesignCheckUnitTests (75) | | |
|---------------------------|---|--|
| ! | DesignCheckUnitTests.NHBC.Part4.Section2 (11) | |
| ! | FoundationsInShrinkableSoils (11) | |
| ! | FoundationsInShrinkableSoils_1 | |
| ! | FoundationsInShrinkableSoils_10 | |
| ! | FoundationsInShrinkableSoils_11 | |
| ! | FoundationsInShrinkableSoils_2 | |
| ! | FoundationsInShrinkableSoils_3 | |
| ! | FoundationsInShrinkableSoils_4 | |
| ! | FoundationsInShrinkableSoils_5 | |
| ! | FoundationsInShrinkableSoils_6 | |
| ! | FoundationsInShrinkableSoils_7 | |
| ! | FoundationsInShrinkableSoils_8 | |
| ! | FoundationsInShrinkableSoils_9 | |

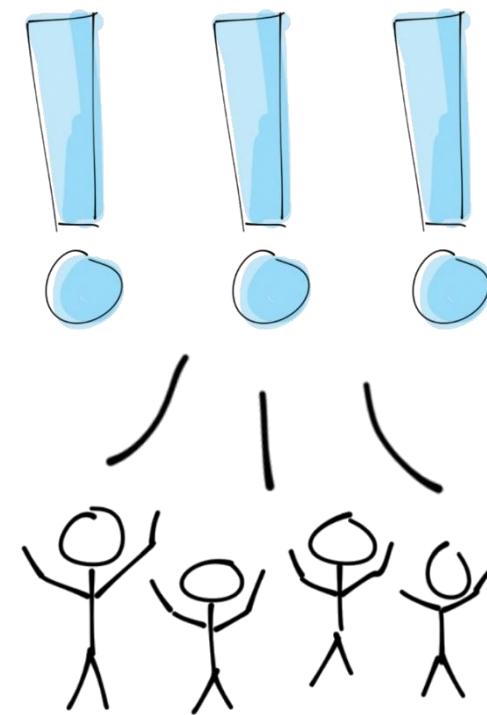
| Passed Tests (26) | | |
|-------------------|---|--------|
| ✓ | Concrete5pcFractileTensileStrength_C40 | < 1 ms |
| ✓ | Concrete95pcFractileTensileStrength_C40 | < 1 ms |
| ✓ | ConcreteMeanAxialTensileStrength_C40 | < 1 ms |
| ✓ | ConcreteMeanCompressiveStrength_C40 | < 1 ms |
| ✓ | ConcreteSecantModulusOfElasticity_C40 | < 1 ms |
| ✓ | ConcreteStrengthReductionFactorCrackedInShear_1 | < 1 ms |
| ✓ | DesignCompressiveStrength_1 | < 1 ms |
| ✓ | PunchingResistanceWithoutReinforcement_1 | < 1 ms |
| ✓ | PunchingResistanceWithoutReinforcement_2 | < 1 ms |
| ✓ | PunchingResistanceWithoutReinforcement_UKNA1 | 6 ms |
| ✓ | PunchingResistanceWithoutReinforcement_UKNA2 | < 1 ms |
| ✓ | PunchingResistanceWithoutReinforcement_UKNA3 | < 1 ms |
| ✓ | RectangularThreeSidesSupported_Example3E | 7 ms |
| ✓ | RectangularThreeSidesSupported_Example3F | < 1 ms |
| ✓ | RectangularThreeSidesSupportedOrthotropic_1 | 1 ms |
| ✓ | RectangularThreeSidesSupportedOrthotropic_2 | < 1 ms |
| ✓ | RectangularThreeSidesSupportedOrthotropic_3 | < 1 ms |
| ✓ | RectangularThreeSidesSupportedOrthotropic_4 | < 1 ms |
| ✓ | RectangularThreeSidesSupportedOrthotropic_5 | < 1 ms |
| ✓ | RectangularThreeSidesSupportedOrthotropic_6 | < 1 ms |
| ✓ | ShearStudDesignResistance_1 | 1 ms |
| ✓ | StrutStrength_NoTransverse_CalcsCorrect | < 1 ms |
| ✓ | TStudMode1_SumsCorrect_1 | 2 ms |
| ✓ | TStudMode2_SumsCorrect_1 | 1 ms |
| ✓ | UnreinforcedShearLimit_Defaults_ArgumentException | < 1 ms |
| ✓ | UnreinforcedShearLimit_Defaults_SumsCorrect | 3 ms |

Write unit tests



Centralise your
versioning and
quality control

No fear of losing your work



But how do you feed it information?

Useful tools

We made some...

Arup Cardiff



ARUP

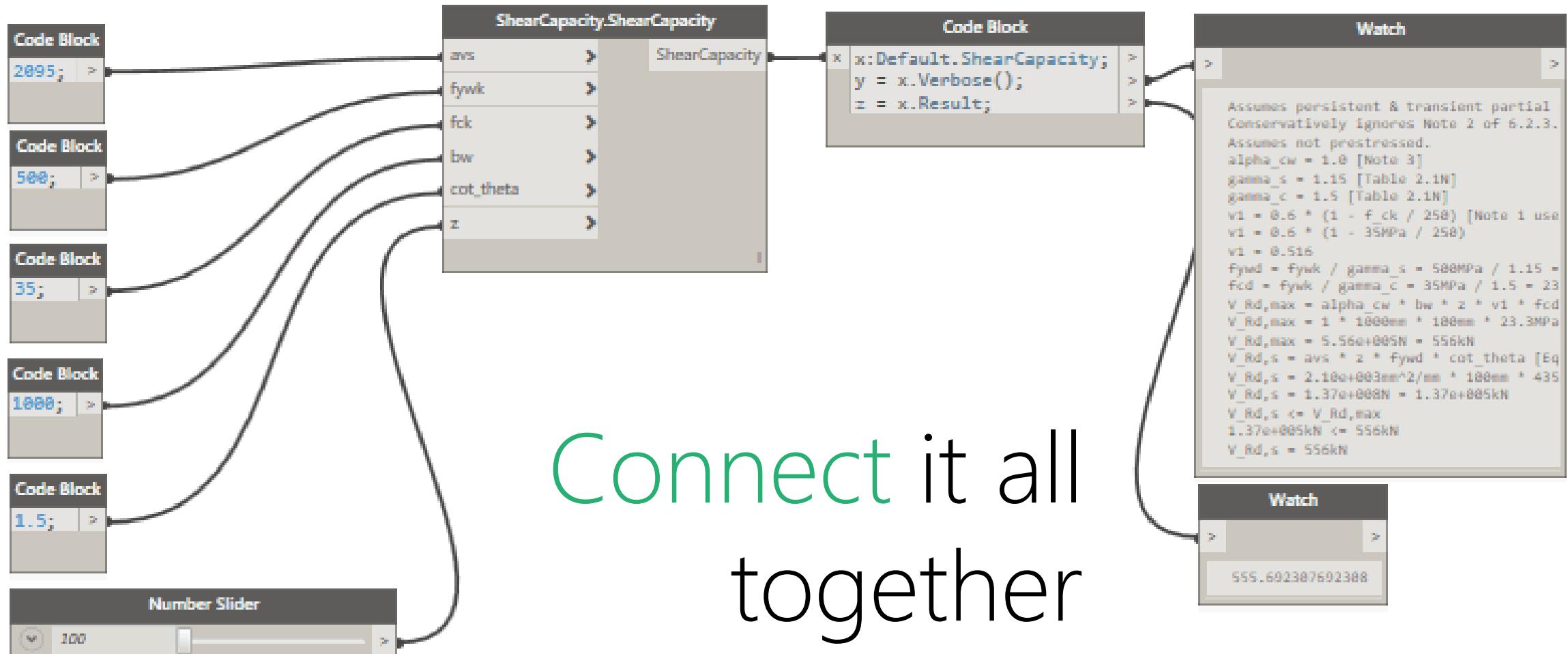


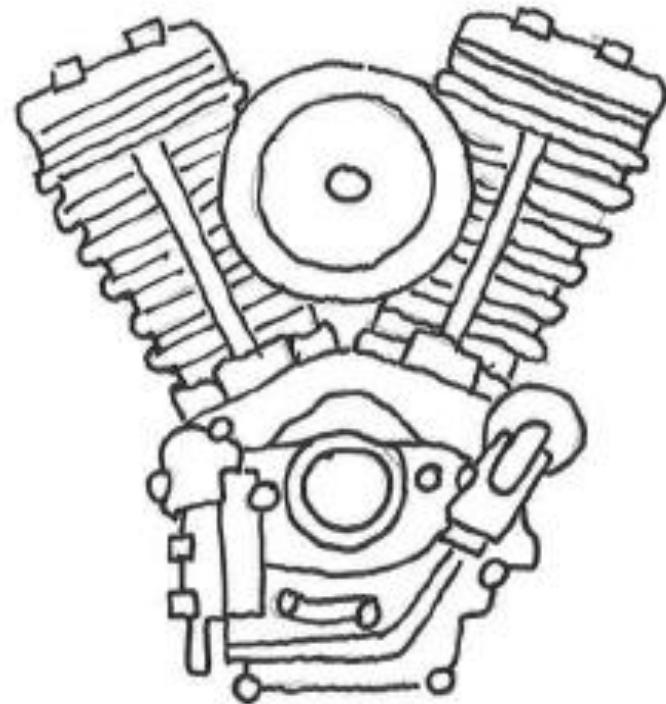
"Hello!"



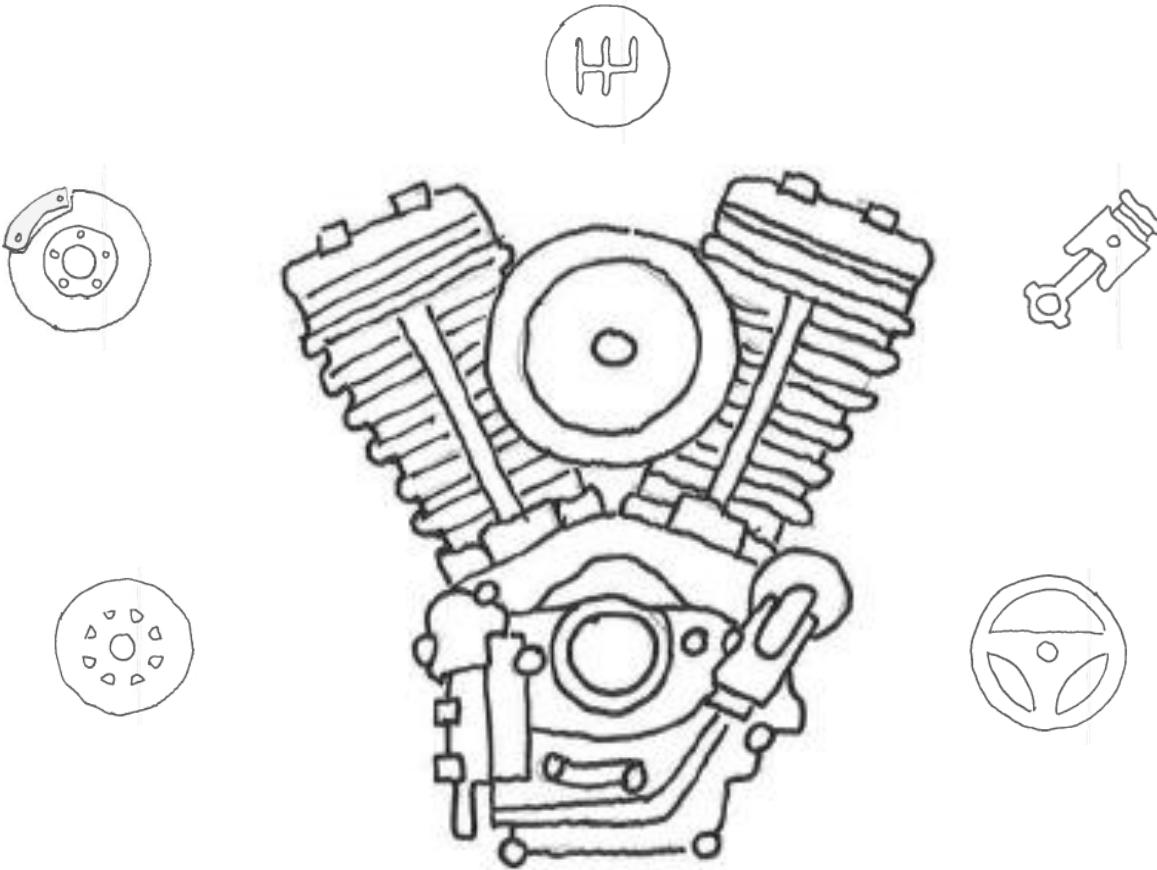
"We've
got a data
problem"

"Ah-ha!"





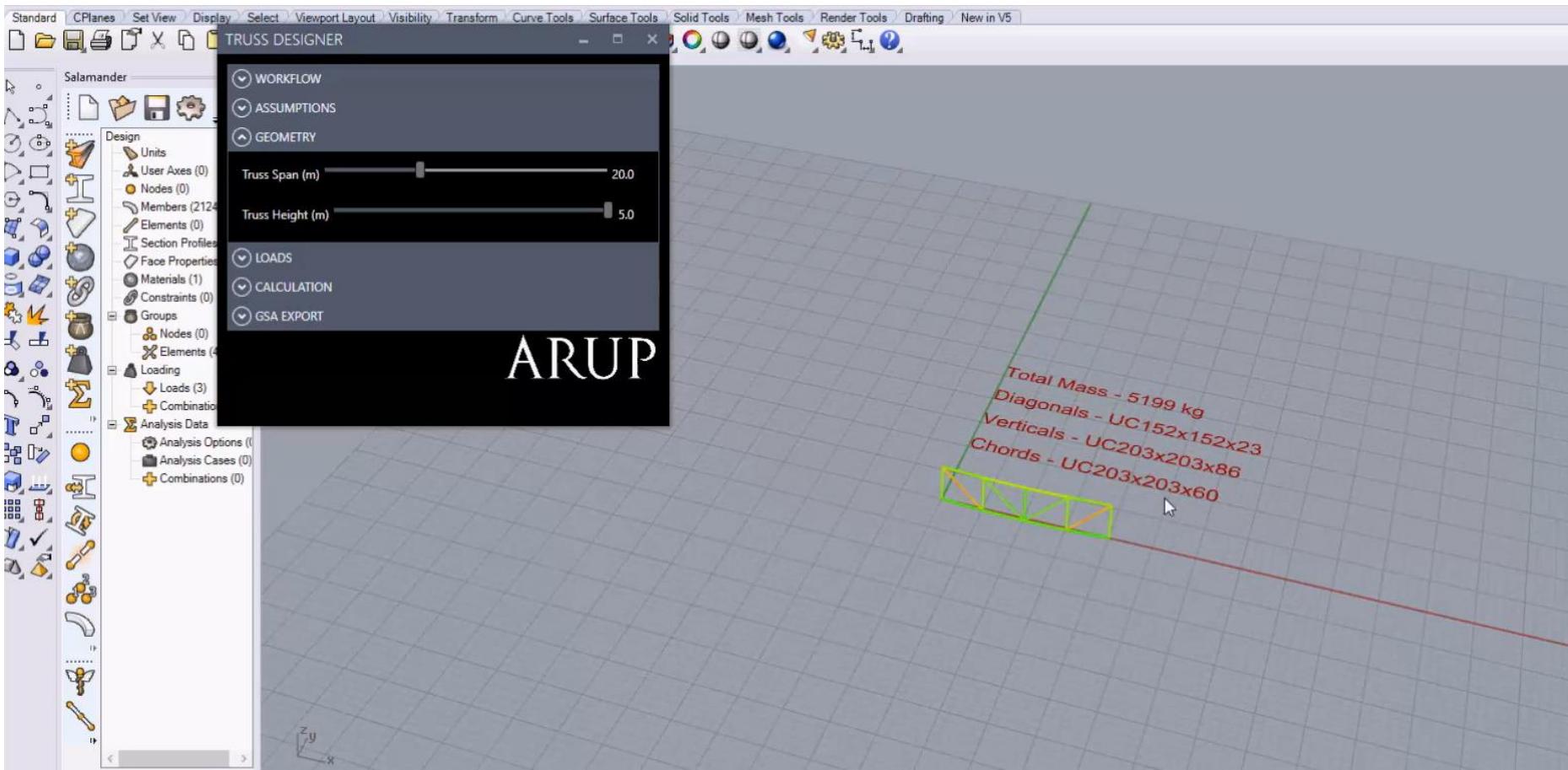
We built an engine



That could be
built around

Lets be disruptive

So we tried it...



My first
parametric truss

ARUP

*"Yeah, yeah – but what
about the real stuff?"*

- Everyone we work with.



Ok, ok, we'll take on something "real" then...

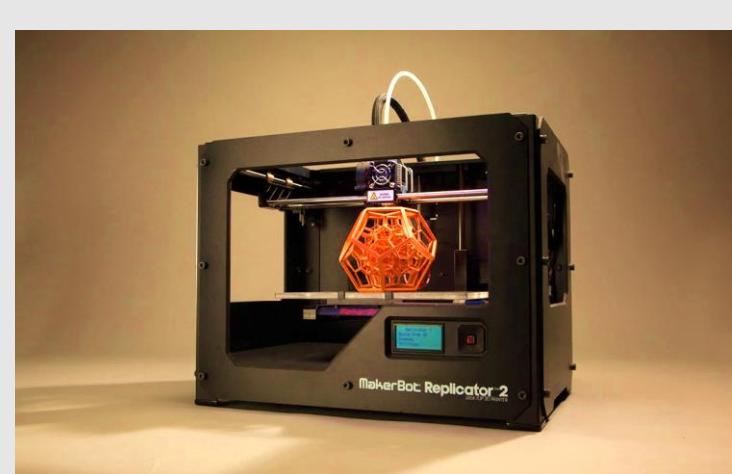
ARUP

Leaving our comfort zone...



Hand crafted

*Engineering the products one
at a time*



Mass bespoke

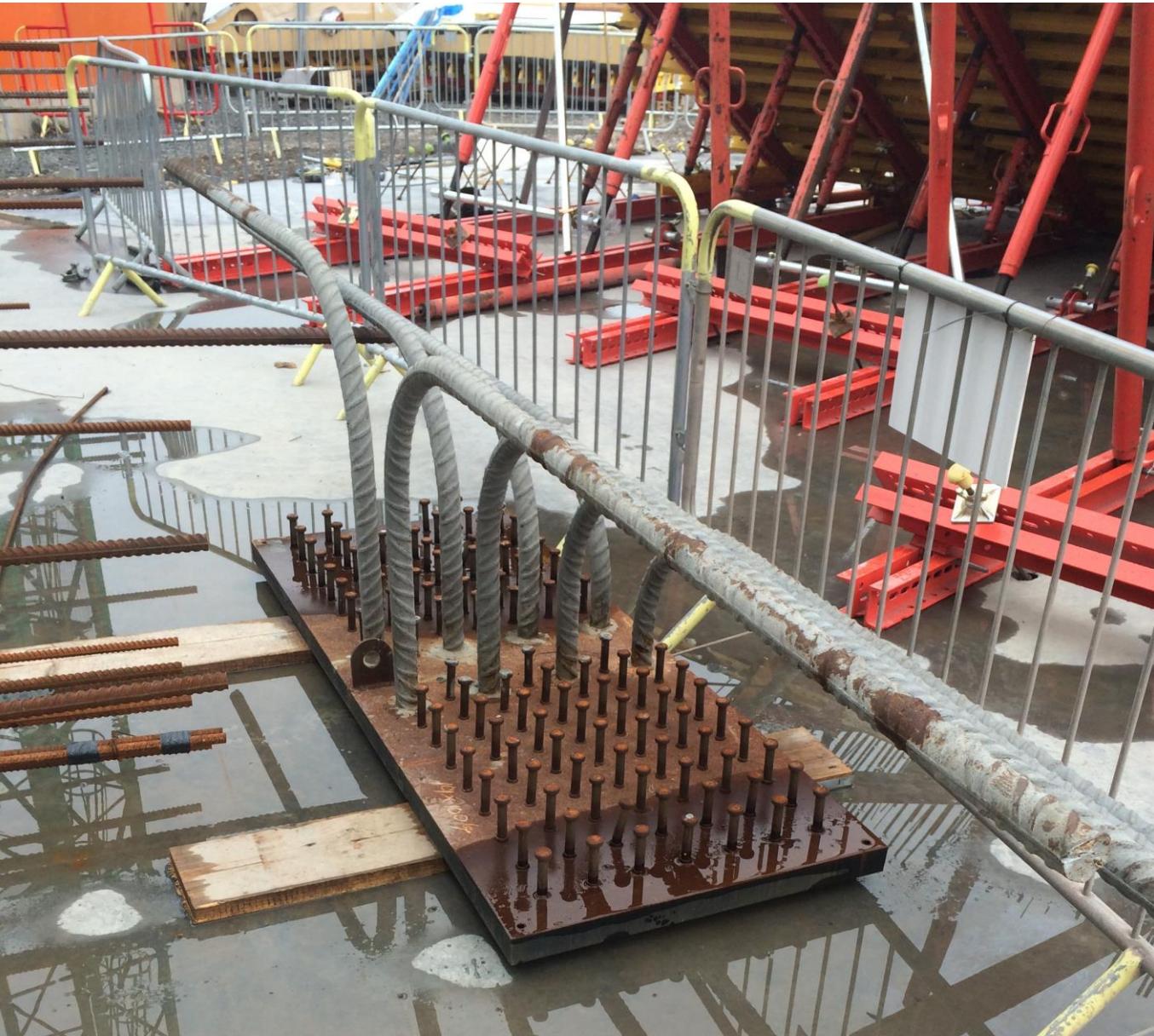
The model for how we need
to be consistently working

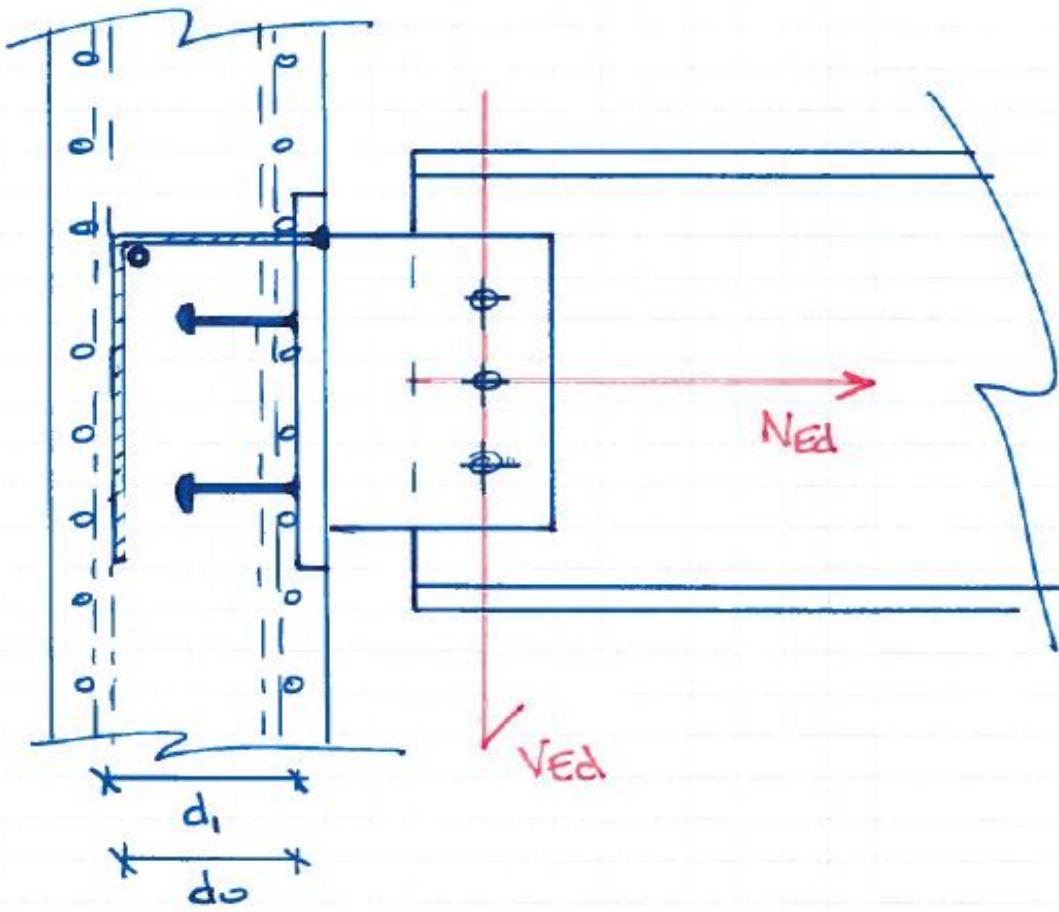


Mass production

*Engineering the process that
creates the products*

Start with the hard problems

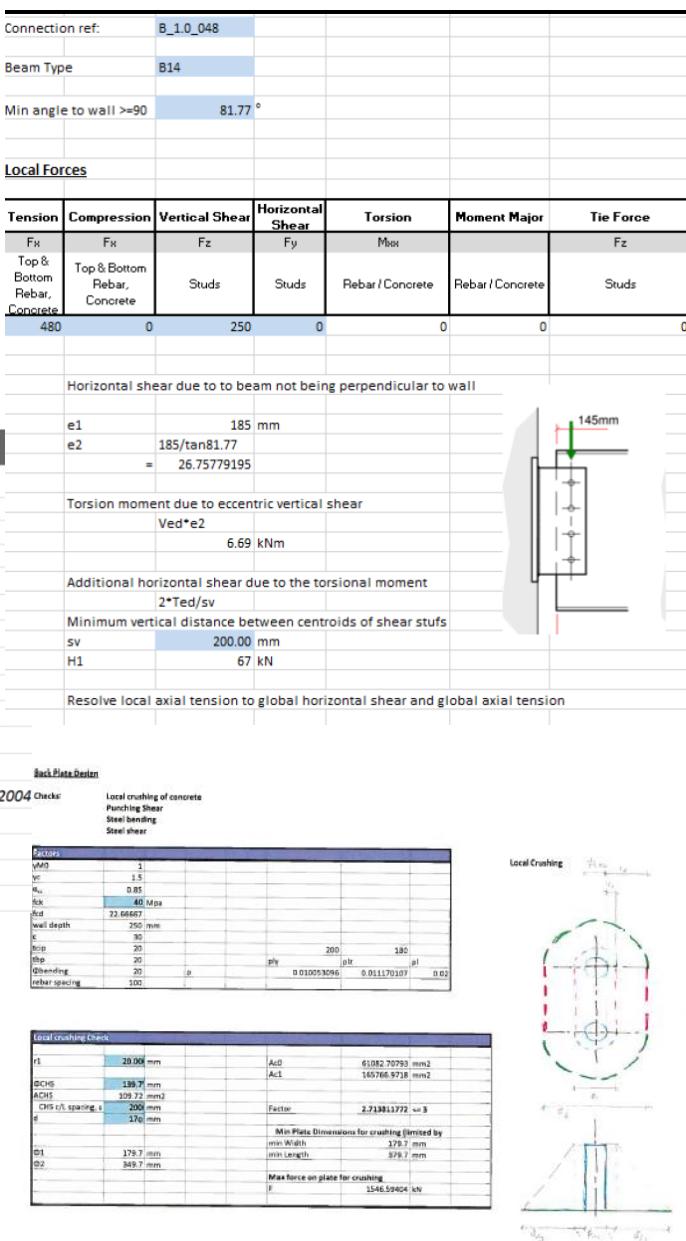




Why are they
so complex?

Standard design?

| A | B | C | D | E | F | G | H |
|----|---|-------------------------|-------------------------|--|---|---|---|
| 20 | Design Checks | | | | | | |
| 21 | Rebar Anchor Design | | | | | | |
| 22 | Cast-In Plate Thickness | | | | | | |
| 23 | Bending Steel | | | | | | |
| 24 | x | 87.5 mm | | | | | |
| 25 | P | 312.5 kN | | | | | |
| 26 | Design Shear Force, V_{Ed} | 100 kN | Minimum Plate Thickness | 27.9 mm | | | |
| 27 | Lever Arm, z | 160 mm | Chosen Plate Thickness | 35 mm | | | |
| 28 | Tension = Compression = | 312.5 kN | | | | | |
| 29 | Shear Stud Design | | | | | | |
| 30 | Robustness | | | | | | |
| 31 | Tie Force, T_{Ed} | 50 kN | h_0/d | 5 | | | |
| 32 | Axial Tension, T_A | 0 kN | α | 1 | | | |
| 33 | $P_{Rd,1}$ | 50 kN | $P_{Rd,1}$ | 81.7 kN | | | |
| 34 | $P_{Rd,2}$ | 50 kN | $P_{Rd,2}$ | 104.3 kN | | | |
| 35 | Anchor Design | | | | | | |
| 36 | Design Stud Resistance, P_{Rd} | 81.7 kN | | | | | |
| 37 | $V_{Ed,z}$ | 100 kN | | | | | |
| 38 | Max. Tensile Force | 312.5 kN | No. Studs Required | 1.22399 | | | |
| 39 | Axial Tension Present? | No, top row of a | I | J | K | L | |
| 40 | $T_{max,top}$ | 312.5 kN | | | | | |
| 41 | $T_{max,bottom}$ | 0 kN | | | | | |
| 42 | Top Anchors | | | | | | |
| 43 | No. of Top Anchors | 2 | | | | | |
| 44 | RC Wall | | | | | | |
| 45 | Wall Depth, t_{wall} | 300 mm | | Additional horizontal shear due to the torsional moment $2 * Ted / sv$ | | | |
| | Cover, c | 40 mm | | Minimum vertical distance between centroids of shear studs $sv = 200.00 \text{ mm}$ | | | |
| | Concrete Grade | C45/55 | | H1 67 kN | | | |
| | Characteristic Cylinder Strength, f_{ck} | 45 N/mm ² | | Resolve local axial tension to global horizontal shear and global axial tension | | | |
| | γ_c | 1.5 | | | | | |
| | α_{cc} | 0.85 | | | | | |
| | Design Compressive Strength, f_{cd} | 25.5 N/mm ² | | | | | |
| | $f_{ctk,0.05}$ | 2.7 N/mm ² | | | | | |
| | E_{cm} | 34400 N/mm ² | | | | | |
| | Horizontal Bar Diameter, Φ_h | 20 mm | | | | | |
| | Vertical Bar Diameter, Φ_v | 12 mm | | | | | |
| | Tension Bar Clearance | 20 mm | | | | | |
| | Transverse Bar at Tension Bend, Φ_{hb} | 25 mm | | | | | |
| | Local Effects | | | | | | |
| | Punching Shear | | | | | | |
| | Q_{Ed} | 6.75 kN | h_c | | | | |
| | h_c | 110 mm | a_m | | | | |
| | h_f | 20 mm | b_m | 280 mm | | | |
| | c_p | 1156.4 mm | b_{em} | 2180 mm | | | |
| | bars | 7 mm | m_{Ed} | 2.94 kNm/m | | | |
| | spacing | 200 mm | M_{Ed} | 1.60 kNm | | | |
| | p_x | 0.002539 | m_{Ed} | 5.73 kNm/m | | | |
| | p_y | 0.002325 | N_s | 83.91 kN | | | |
| | p | 0.00243 | x_s | 5.9 mm | | | |
| | V_{Rd} | 0.31 N/mm ² | z_s | 73 mm | | | |
| | V_{Rd} | 28.29 kN | m_{Rd} | 6.13 kNm/m | | | |



No.

ARUP

"Lets change that!"

- Us.

ARUP

We obtained funds...



...and built a team



The Investors
A wealth of experience
And very open minds

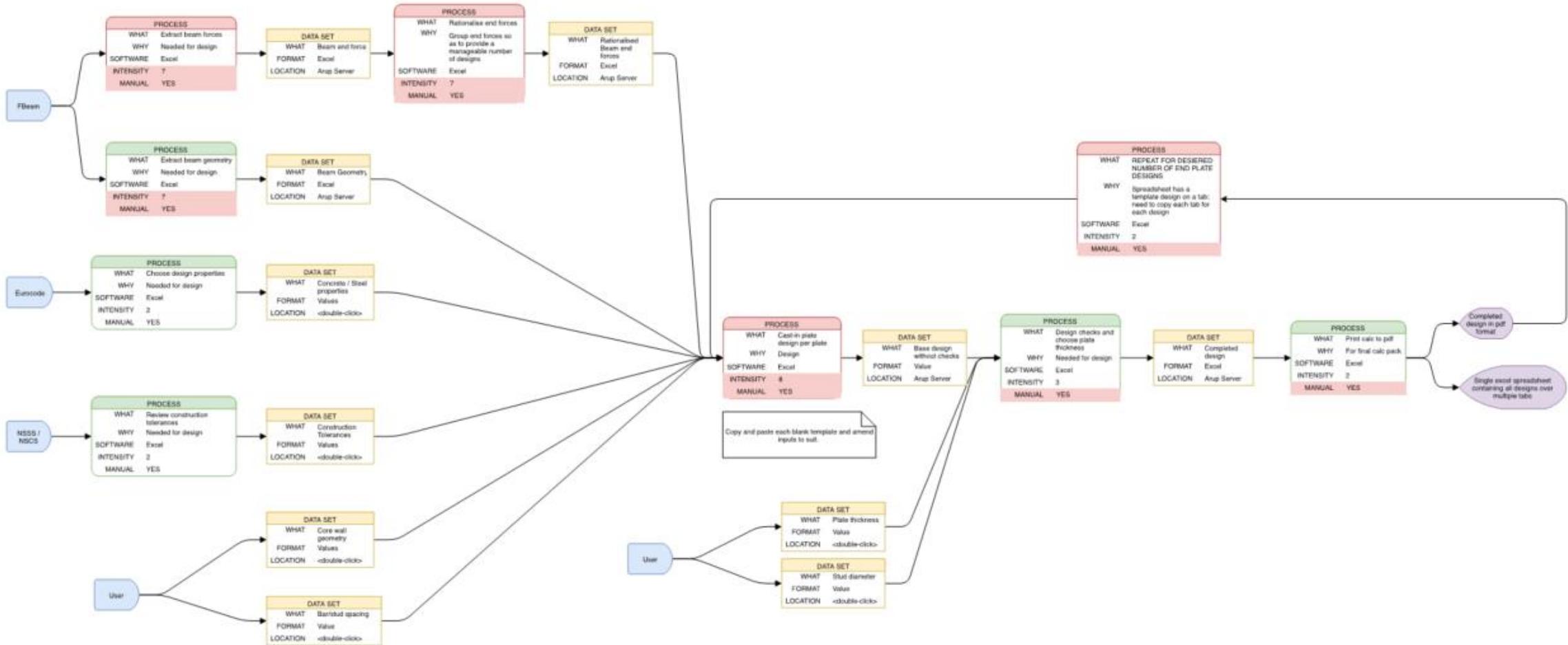


The Innovators
Two people with a big idea
And a lot of coffee

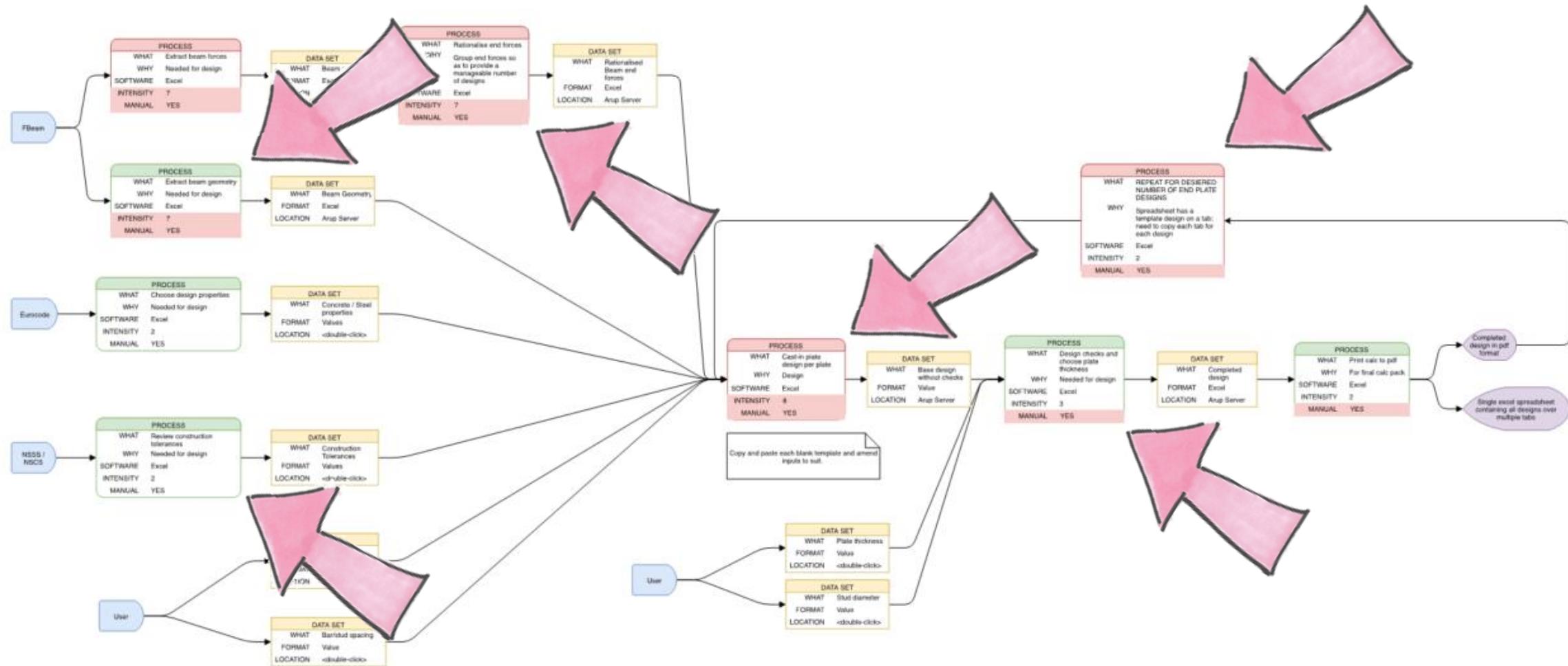


The Integrators
Digital natives
Impossible is not in their vocabulary

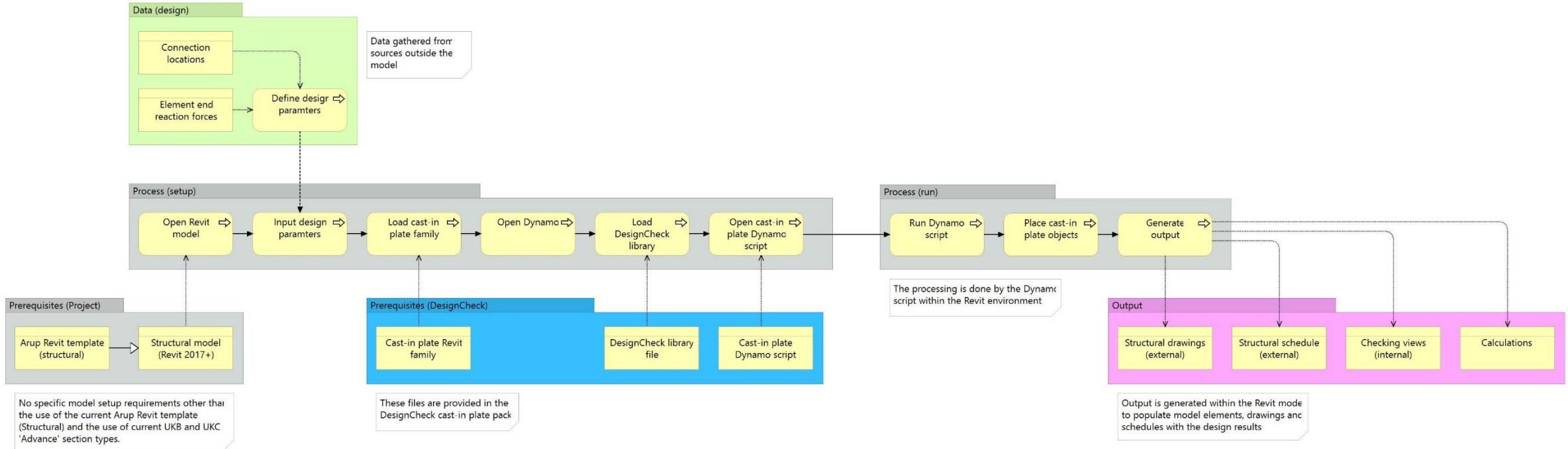
We mapped current methods...



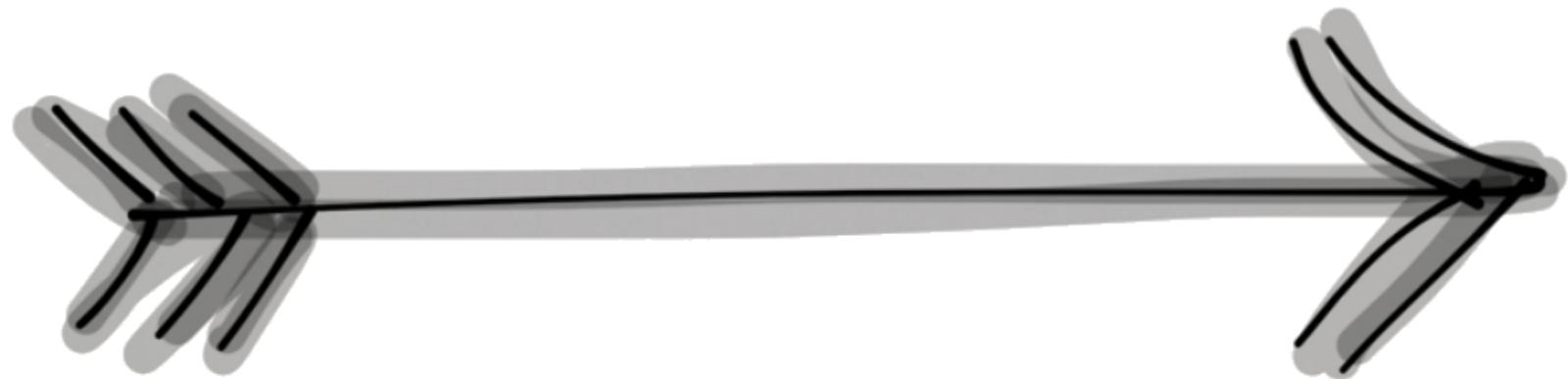
..highlighted the pain points...

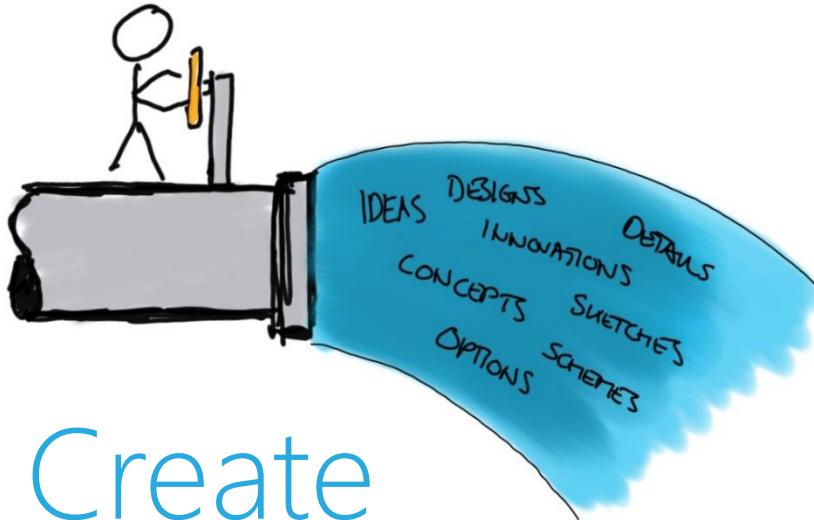


and made improvements...



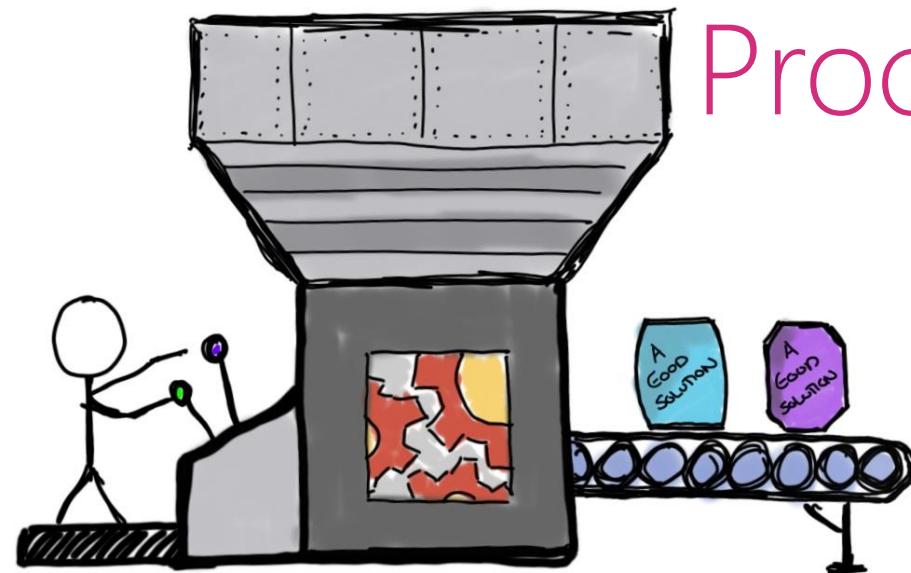
Creating flow





Create

Follow the value
Eliminate the waste



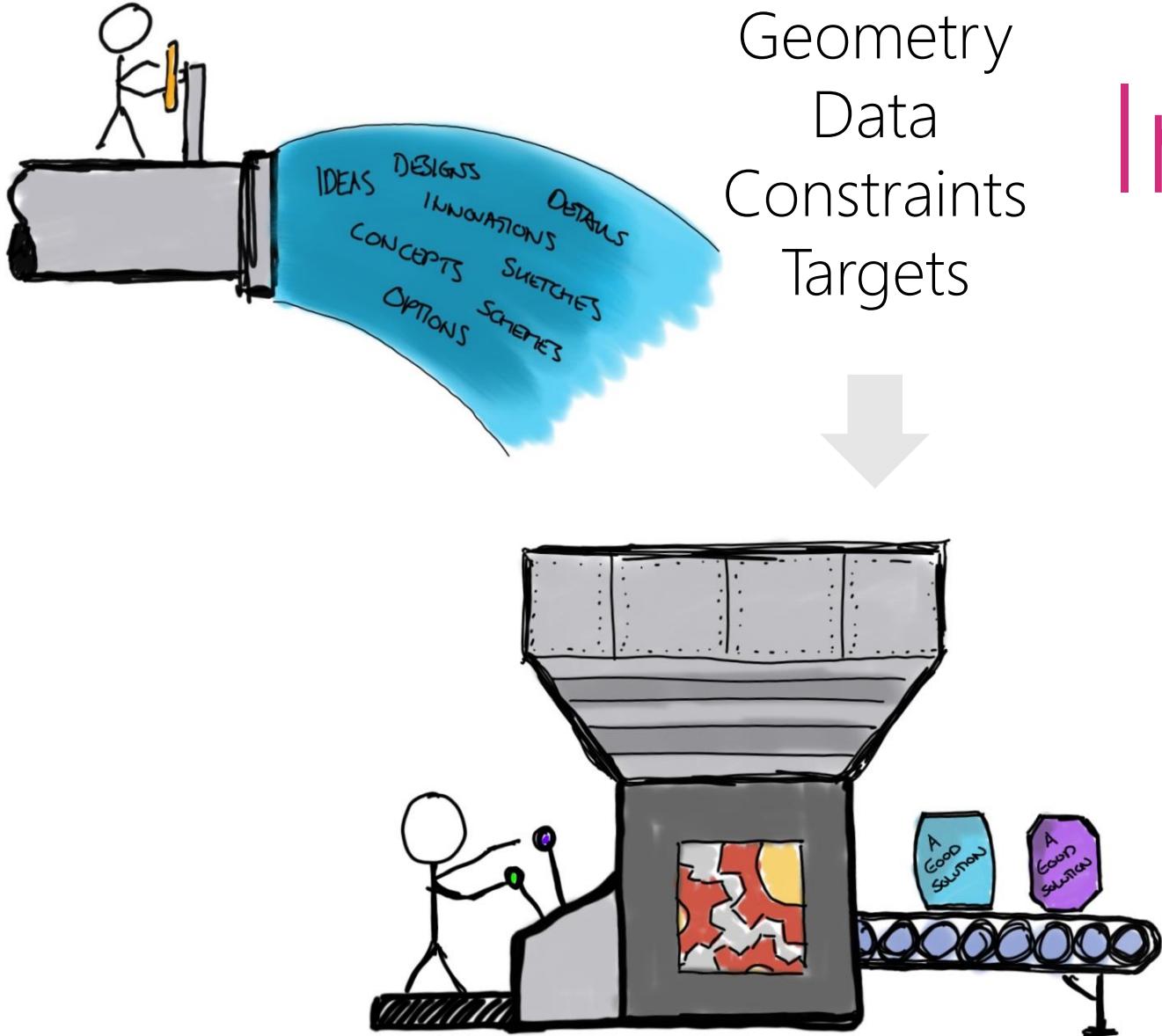
Process



Deliver

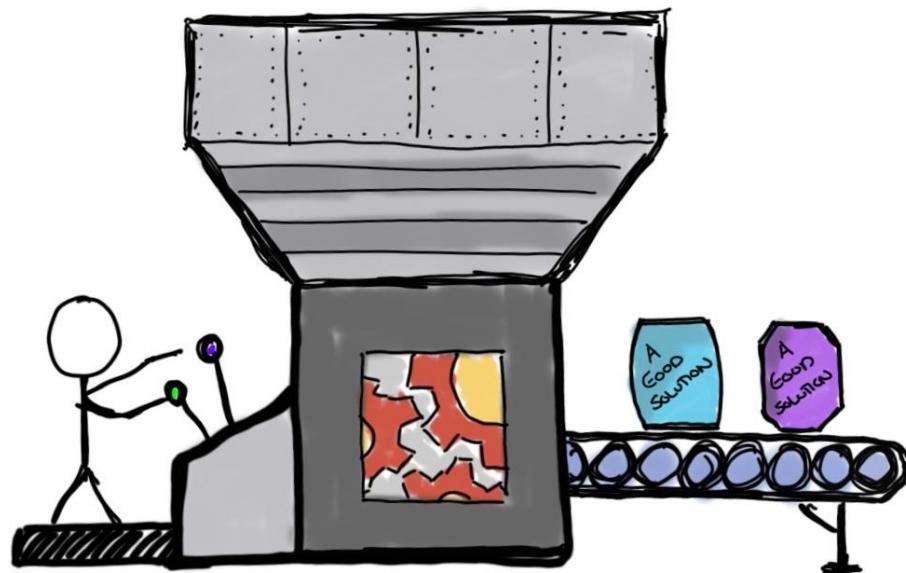
Geometry
Data
Constraints
Targets

Input



Output

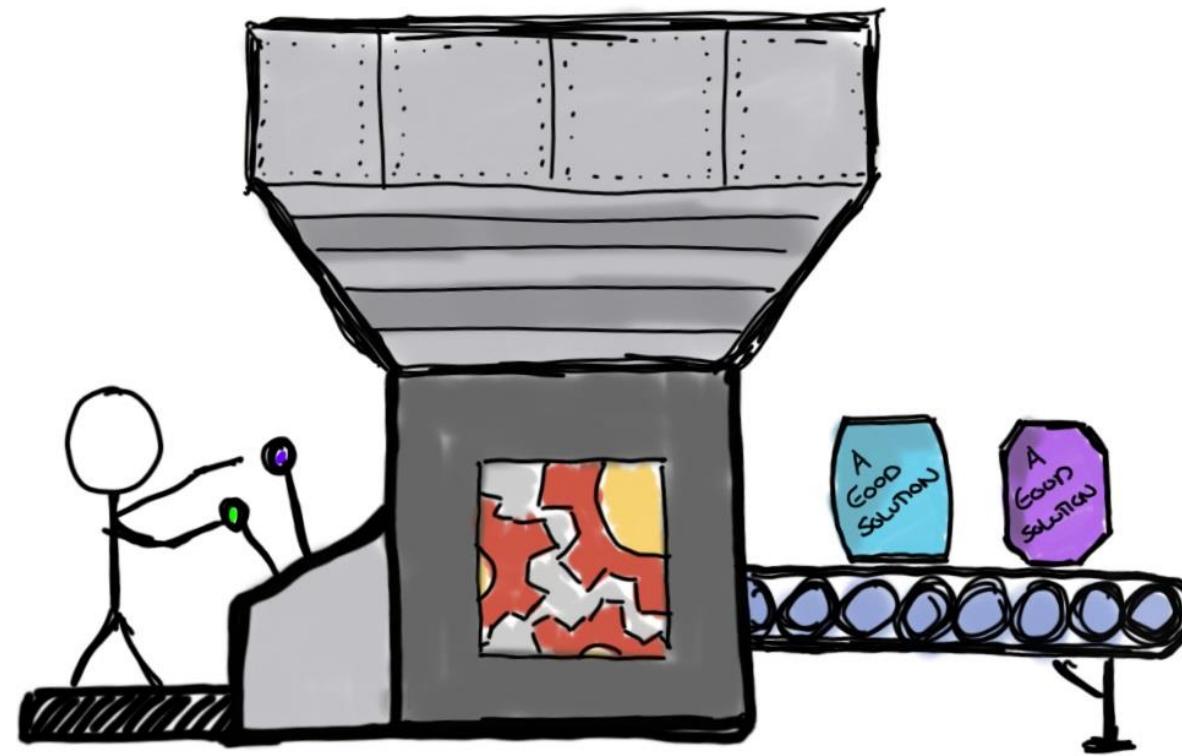
Insights
Metrics
Information
Confirmation



We planned for a successful outcome

| | |
|--|---|
| <p>MUST</p> <ul style="list-style-type: none">• Implement SGN 09 checks• Shear only• “Infinite” concrete• Two columns on studs / bars• Top x rows are bars• Revit object for plate• Fin plate centred on plate• Extensible for other codes■ Alert the user when design assumptions aren’t valid | <p>SHOULD</p> <ul style="list-style-type: none">• Alert the user if requirements for “infinite” concrete are not met (plus any other assumptions)• Resolve Axial load, moment, minor axis forces• X by Y arrangement of bars / studs• Variable number of top / bottom rows are bars• Revit object for plate + sub entities for bars, studs• Include basic clash checks for “thin” concrete (i.e. can the bent bars fit in)• Usage tracking |
| <p>COULD</p> <ul style="list-style-type: none">• Arbitrary substitution of studs for bars• Moment• Capacity reduction adjacent to edges / openings• Work for “thin” concrete i.e. plate through to other side of section• Other bar arrangements (hook-up, hook-side, studs only)• Use green book method for plate sizing• Template Revit checking views (governed by studs, governed by punching etc.) | <p>(First Version) WON’T</p> <ul style="list-style-type: none">• Design for hanger bars if there is an opening below• Web interface• Auto size• Non-fin plate connections |

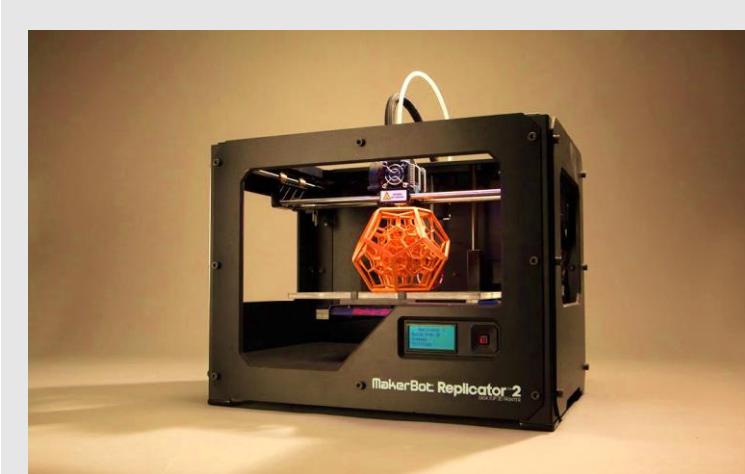
We developed our processing machine



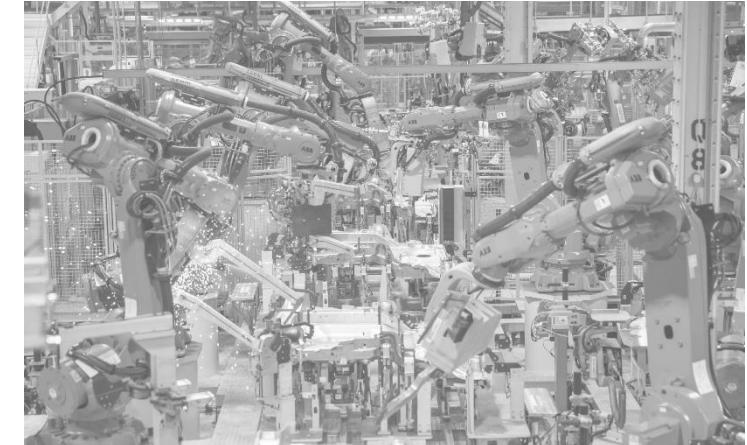
Left our comfort zone...



Hand crafted
*Engineering the products one
at a time*

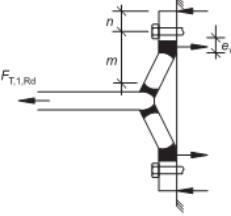
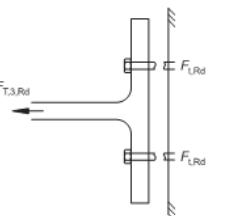
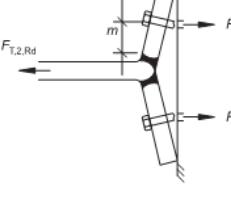


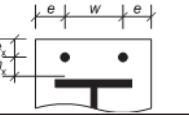
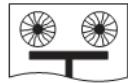
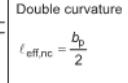
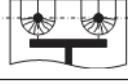
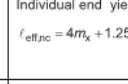
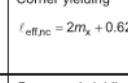
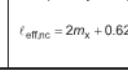
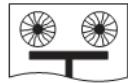
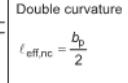
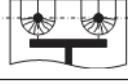
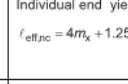
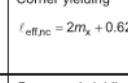
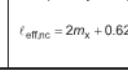
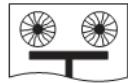
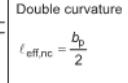
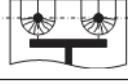
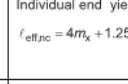
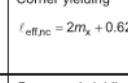
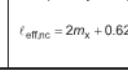
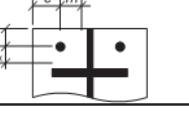
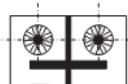
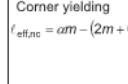
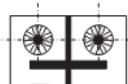
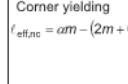
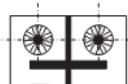
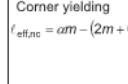
Mass bespoke
The model for how we need
to be consistently working



Mass production
*Engineering the process that
creates the products*

Reviewed guidance

| STEP 1 RESISTANCES OF BOLT ROWS IN THE TENSION ZONE | |
|--|---|
| RESISTANCES OF T-STUBS <p>The resistances of the equivalent T-stubs are evaluated separately for the end plate and the column flange. The resistances are calculated for three possible modes of failure. The resistance is taken as the minimum of the values for the three modes.</p> <p>The design resistance of the T-stub flange, for each of the modes, is given below.</p> <p>Mode 1 Complete Flange Yielding</p> <p>Using 'Method 2' in Table 6.2 of BS EN 1993-1-8:</p> $F_{T,1,Rd} = \frac{(8n - 2e_x) M_{pl,1,Rd}}{2mn - e_w(m+n)}$  <p>where: $M_{pl,1,Rd}$ and $M_{pl,2,Rd}$ are the plastic resistance moments of the equivalent T-stubs for Modes 1 and 2, given by: $M_{pl,1,Rd} = 0.25 \sum \ell_{eff,1} t_i^2 f_y / \gamma_M$ $M_{pl,2,Rd} = 0.25 \sum \ell_{eff,2} t_i^2 f_y / \gamma_M$</p> <p>$\ell_{eff,1}$ is the effective length of the equivalent T-stub for Mode 1, taken as the lesser of $\ell_{eff,cp}$ and $\ell_{eff,nc}$ (see Table 2.2 for effective lengths for individual rows and Table 2.3 for groups of rows)</p> <p>$\ell_{eff,2}$ is the effective length of the equivalent T-stub for Mode 2, taken as $\ell_{eff,nc}$ (see Table 2.2 for effective lengths for individual rows and Table 2.3 for groups of rows)</p> <p>t_i is the thickness of the T-stub flange ($= t_b$ or t_c)</p> <p>f_y is the yield strength of the T-stub flange (i.e. of the column or end plate)</p> <p>$\Sigma F_{l,Rd}$ is the total tension resistance for the bolts in the T-stub ($= 2F_{l,Rd}$ for a single row)</p> <p>$e_w = d_w / 4$</p> <p>d_w is the diameter of the washer or the width across the points of the bolt head, as relevant</p> <p>m is as defined in Figure 2.5</p> <p>n is the minimum of: e_c (edge distance of the column flange) e_e (edge distance of the end plate) $1.25m$ (for end plate or column flange, as appropriate)</p> | Mode 3 Bolt Failure $F_{T,3,Rd} = \sum F_{l,Rd}$  |
| <p>Mode 2 Bolt Failure with Flange Yielding</p> $F_{T,2,Rd} = \frac{2M_{pl,2,Rd} + n(\sum F_{l,Rd})}{m+n}$  | |

| STEP 1A T-STUB FLANGE IN BENDING | | | | | | | | | | | | | | | | | | | | | | |
|---|---|--|------------------|--|--|--|--|--|--|--|---|--|--|--|---------------------------------------|--|--|---|--|--|---|--|
| <p>Table 2.2 Effective lengths ℓ_{eff} for equivalent T-stubs for bolt row acting alone</p> <p>(a) Pair of bolts in an unstiffened end plate extension</p> <p>Note: Use m_x in place of m and e_x in place of n in the expressions for $F_{T,1,Rd}$ and $F_{T,2,Rd}$.</p> |  | | | | | | | | | | | | | | | | | | | | | |
| <table border="1"> <thead> <tr> <th>Circular patterns</th> <th>Non-circular patterns</th> <th>Double curvature</th> </tr> </thead> <tbody> <tr> <td></td> <td>Circular yielding $\ell_{eff,cp} = 2\pi m_x$</td> <td> yield lines</td> </tr> <tr> <td></td> <td>Individual end yielding $\ell_{eff,cp} = \pi m_x + 2e_x$</td> <td> Individual end yielding</td> </tr> <tr> <td></td> <td>Circular group yielding $\ell_{eff,cp} = \pi m_x + w$</td> <td> Corner yielding</td> </tr> <tr> <td></td> <td></td> <td>$\ell_{eff,nc} = 2m_x + 0.625e_x + e$</td> </tr> <tr> <td></td> <td></td> <td> Group end yielding</td> </tr> <tr> <td></td> <td></td> <td>$\ell_{eff,nc} = 2m_x + 0.625e_x + \frac{w}{2}$</td> </tr> </tbody> </table> | Circular patterns | Non-circular patterns | Double curvature |  | Circular yielding $\ell_{eff,cp} = 2\pi m_x$ |  yield lines |  | Individual end yielding $\ell_{eff,cp} = \pi m_x + 2e_x$ |  Individual end yielding |  | Circular group yielding $\ell_{eff,cp} = \pi m_x + w$ |  Corner yielding | | | $\ell_{eff,nc} = 2m_x + 0.625e_x + e$ | | |  Group end yielding | | | $\ell_{eff,nc} = 2m_x + 0.625e_x + \frac{w}{2}$ | |
| Circular patterns | Non-circular patterns | Double curvature | | | | | | | | | | | | | | | | | | | | |
|  | Circular yielding $\ell_{eff,cp} = 2\pi m_x$ |  yield lines | | | | | | | | | | | | | | | | | | | | |
|  | Individual end yielding $\ell_{eff,cp} = \pi m_x + 2e_x$ |  Individual end yielding | | | | | | | | | | | | | | | | | | | | |
|  | Circular group yielding $\ell_{eff,cp} = \pi m_x + w$ |  Corner yielding | | | | | | | | | | | | | | | | | | | | |
| | | $\ell_{eff,nc} = 2m_x + 0.625e_x + e$ | | | | | | | | | | | | | | | | | | | | |
| | |  Group end yielding | | | | | | | | | | | | | | | | | | | | |
| | | $\ell_{eff,nc} = 2m_x + 0.625e_x + \frac{w}{2}$ | | | | | | | | | | | | | | | | | | | | |
| <p>(b) Pair of bolts at end of column or on a stiffened end plate extension</p> <p>Note: The expressions below may also be used for a column without a stiffener except that the corner yielding pattern is not applicable.</p> |  | | | | | | | | | | | | | | | | | | | | | |
| <table border="1"> <thead> <tr> <th>Circular patterns</th> <th>Non-circular patterns</th> <th>Corner yielding</th> </tr> </thead> <tbody> <tr> <td></td> <td>Circular yielding $\ell_{eff,cp} = 2\pi m$</td> <td> Corner yielding</td> </tr> <tr> <td></td> <td>Individual end yielding, $\ell_{eff,cp} = \pi m + 2e_x$</td> <td>$\ell_{eff,nc} = \alpha m - (2m + 0.625e) + e_x$</td> </tr> <tr> <td></td> <td></td> <td> Corner yielding away from the stiffener/flange (m_x large)</td> </tr> <tr> <td></td> <td></td> <td>$\ell_{eff,nc} = 2m + 0.625e + e_x$</td> </tr> </tbody> </table> | Circular patterns | Non-circular patterns | Corner yielding |  | Circular yielding $\ell_{eff,cp} = 2\pi m$ |  Corner yielding |  | Individual end yielding, $\ell_{eff,cp} = \pi m + 2e_x$ | $\ell_{eff,nc} = \alpha m - (2m + 0.625e) + e_x$ | | |  Corner yielding away from the stiffener/flange (m_x large) | | | $\ell_{eff,nc} = 2m + 0.625e + e_x$ | | | | | | | |
| Circular patterns | Non-circular patterns | Corner yielding | | | | | | | | | | | | | | | | | | | | |
|  | Circular yielding $\ell_{eff,cp} = 2\pi m$ |  Corner yielding | | | | | | | | | | | | | | | | | | | | |
|  | Individual end yielding, $\ell_{eff,cp} = \pi m + 2e_x$ | $\ell_{eff,nc} = \alpha m - (2m + 0.625e) + e_x$ | | | | | | | | | | | | | | | | | | | | |
| | |  Corner yielding away from the stiffener/flange (m_x large) | | | | | | | | | | | | | | | | | | | | |
| | | $\ell_{eff,nc} = 2m + 0.625e + e_x$ | | | | | | | | | | | | | | | | | | | | |
| <p>See Notes on Page 14</p> | | | | | | | | | | | | | | | | | | | | | | |

The screenshot shows a Microsoft Visual Studio interface. On the left, the Test Explorer window displays a tree view of test cases under the category 'DesignCheck' (75 tests). The top item, 'DesignCheckUnitTests' (75), is selected. Below it, several sub-categories are listed, each with a count of failing tests (indicated by a red exclamation mark icon):

- DesignCheckUnitTests.NHBC.Part4.Section2 (11 failing)
- FoundationsInShrinkableSoils (11 failing)
- DesignCheckUnitTests.Structural.Composite(EC4).Base (1 failing)
- ShearStudDesignResistance (1 failing)
- DesignCheckUnitTests.Structural.Concrete(EC2).Base (19 failing)
- BarDesignTensileStrength (1 failing)
- BasicAnchorageLength (1 failing)
- Bent_or_HookedDesignAnchorageLength (1 failing)
- ConcreteStrengthReductionFactorCrackedInShear (1 failing)
- CrossBarWidth (1 failing)
- DesignCompressiveStrength (1 failing)
- LoopedDesignAnchorageLength (1 failing)
- MandrelBentDiameter (1 failing)
- NodeStrengthCCC (1 failing)
- NodeStrengthCTT (1 failing)
- PushingResistanceWithoutDeckReinforcement (1 failing)

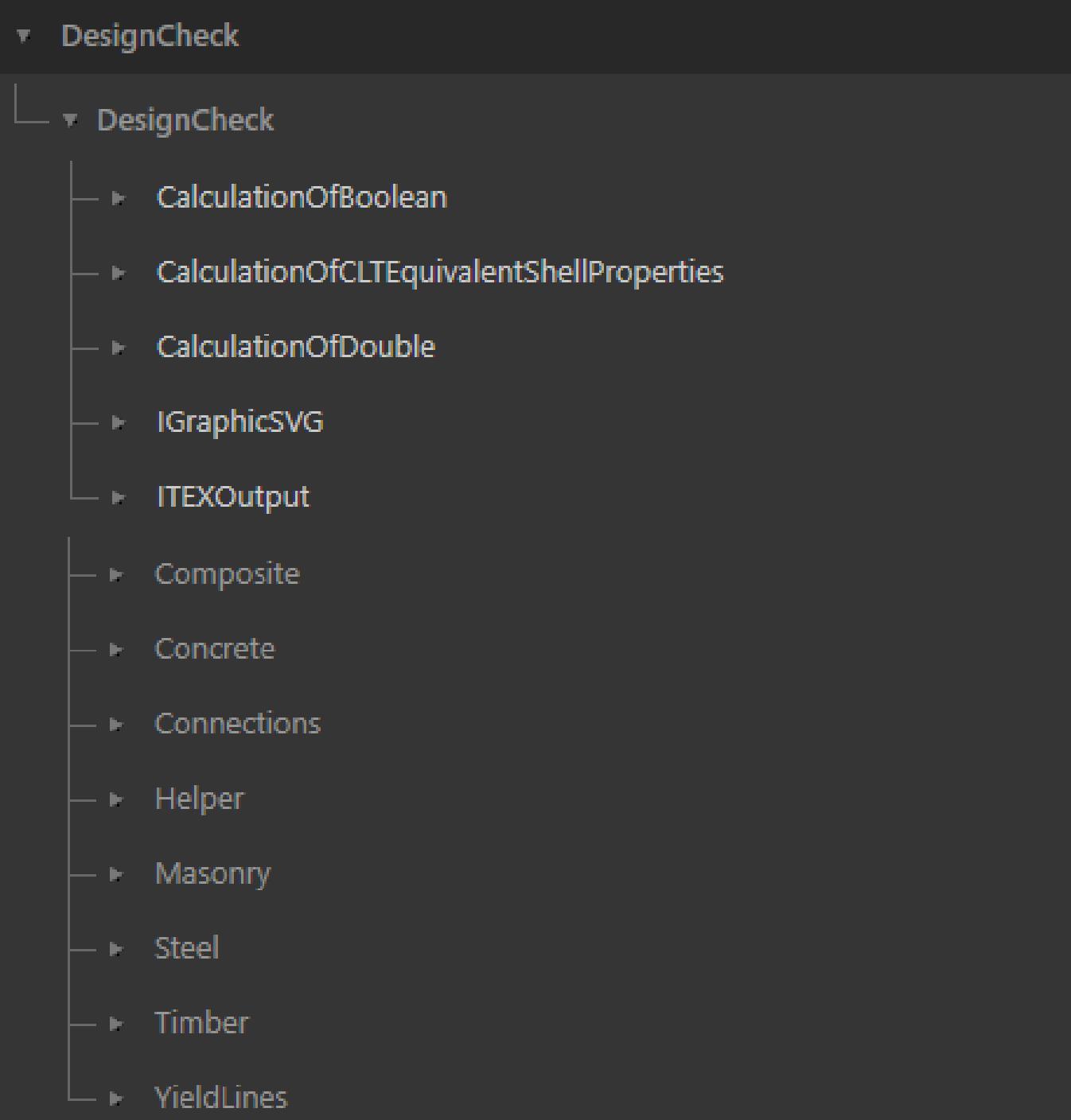
The main code editor window shows the 'OneWaySlab.cs' file. The code defines a class 'OneWaySlab' that implements the 'calculation_single<double>' interface. The class includes XML documentation for the 'YieldLines. OneWay' interface, specifying parameters L, n, and i_1, i_2.

```
1  using System;
2  using System.Collections.Generic;
3  using System.Linq;
4  using System.Text;
5  using System.Threading.Tasks;
6
7  namespace DesignCheck.Structural.YieldLines
8  {
9      /// <summary>
10     /// ***YieldLines.OneWay***
11     /// </summary>
12     public class OneWaySlab : calculation_single<double>
13     {
14         //Interface properties
15         public override int Revision { get; } = 0;
16         public override string RevisionHistory { get; } = "";
17         public override string Author { get; } = "Hugh Groves";
18         public override string Assumptions { get; }
19         public override string Description { get; } = "***YieldLines.OneWay***";
20         public override LevelOfReview LevelOfReview { get; } = LevelOfReview.WorkInProgress;
21
22         /// <summary>
23         /// ***YieldLines.OneWay***
24         /// </summary>
25         /// <param name="L">***YieldLines.OneWay.L***</param>
26         /// <param name="n">***YieldLines.n***</param>
27         /// <param name="i_1">***YieldLines.OneWay.i_1***</param>
28         /// <param name="i_2">***YieldLines.OneWay.i_2***</param>
29         public OneWaySlab(double L, double n, double i_1, double i_2)
30         {
31             AddTitle();
32
33             AddSubHeader("Inputs", false);
34             DeclareInput("L", L, "m", "***YieldLines.OneWay.L***");
35             DeclareInput("n", n, "kPa", "***YieldLines.n***");
36             DeclareInput("i_1", i_1, "", "***YieldLines.OneWay.i_1***");
37
38         }
39
40     }
41
42 }
```

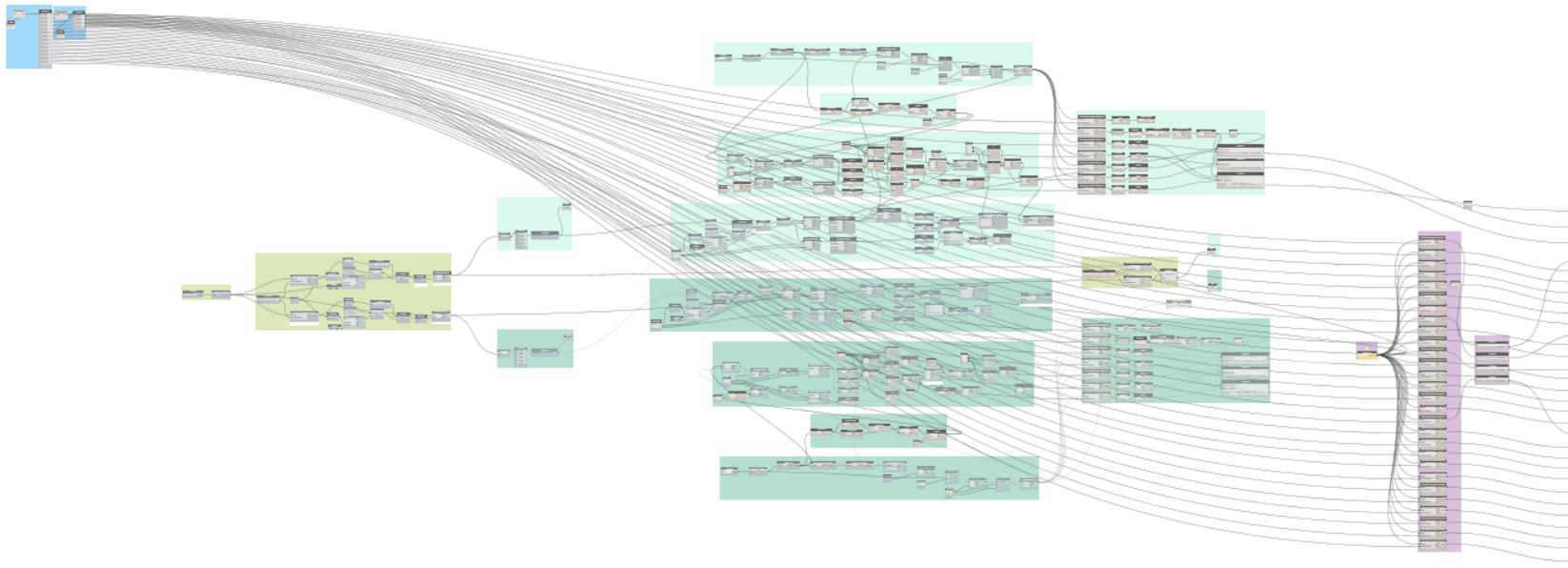
The output window at the bottom shows the build log:

```
2>C:\Users\ian.wise\Documents\DesignCheck\DesignCheck\BaseClass\Calculation.cs(1:
2>C:\Users\ian.wise\Documents\DesignCheck\DesignCheck\BaseClass\Calculation.cs(2:
2>C:\Users\ian.wise\Documents\DesignCheck\DesignCheck\BaseClass\Calculation.cs(2:
2>C:\Users\ian.wise\Documents\DesignCheck\DesignCheck\BaseClass\Calculation.cs(4:
2>C:\Users\ian.wise\Documents\DesignCheck\DesignCheck\BaseClass\Calculation.cs(5:
2>C:\Users\ian.wise\Documents\DesignCheck\DesignCheck\BaseClass\Calculation.cs(6:
2>  DesignCheck -> C:\Users\ian.wise\Documents\DesignCheck\DesignCheck\bin\Debug\DesignCheck.dll
3>----- Rebuild All started: Project: DesignCheckUnitTests, Configuration: Debug
3>C:\Users\ian.wise\Documents\DesignCheck\DesignCheckUnitTests\Structural\Concrete\Concrete.cs(1:
3>C:\Users\ian.wise\Documents\DesignCheck\DesignCheckUnitTests\Structural\Concrete\Concrete.cs(2:
3>C:\Users\ian.wise\Documents\DesignCheck\DesignCheckUnitTests\Structural\Concrete\Concrete.cs(3:
3>C:\Users\ian.wise\Documents\DesignCheck\DesignCheckUnitTests\Structural\Concrete\Concrete.cs(4:
3>  DesignCheckUnitTests -> C:\Users\ian.wise\Documents\DesignCheck\DesignCheck\bin\Debug\DesignCheckUnitTests.dll
===== Rebuild All: 3 succeeded, 0 failed, 0 skipped ======
```

Coded the
calculations



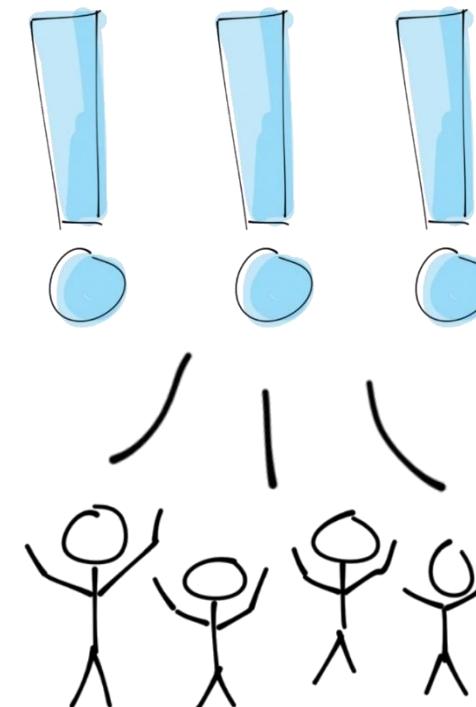
Compiled the
calculation
library



Parametric
spaghetti

```
3 class Element:  
4  
5     def __init__(self, id):  
6         self.id = id  
7  
8     def get_id(self):  
9         return self.id  
10  
11    class Beam(Element):  
12  
13        def __init__(self, id):  
14            super().__init__()  
15  
16  
17    class Shell(Element):  
18  
19        def __init__(self, id):  
20            super().__init__()  
21  
22    beam = Beam(1)  
23  
24    beam.get_id()  
25  
26    shell.get_id()
```

Removed external
libraries...





For simple
mass roll-out

Revit parameters used to drive the process:

| Name | Type | Usage | Notes |
|---------------------------------------|--------|-----------------------------------|------------------------------------|
| Connection_End_1_Type | String | Tag with "CiP" to activate Dynamo | <i>Standard template parameter</i> |
| Connection_End_2_Type | String | Tag with "CiP" to activate Dynamo | <i>Standard template parameter</i> |
| Connection_End_1_Shear_Vz_Major | Number | Vertical shear input to Dynamo | <i>Standard template parameter</i> |
| Connection_End_2_Shear_Vz_Major | Number | Vertical shear input to Dynamo | <i>Standard template parameter</i> |
| Reinforcement_Bars_Diameter_FF1 | Number | Wall rebar input to Dynamo | <i>Standard template parameter</i> |
| Reinforcement_Bars_Diameter_FF2 | Number | Wall rebar input to Dynamo | <i>Standard template parameter</i> |
| Reinforcement_Bars_Spacing_FF1 | Number | Wall rebar input to Dynamo | <i>Standard template parameter</i> |
| Reinforcement_Bars_Spacing_FF2 | Number | Wall rebar input to Dynamo | <i>Standard template parameter</i> |
| Reinforcement_Cover_Far_Face | Number | Wall rebar input to Dynamo | <i>Standard template parameter</i> |
| Specification_Material_Strength_Grade | Number | Wall concrete strength to Dynamo | <i>Standard template parameter</i> |



Create the
digital object

Document everything (especially the bits that don't work)

Bearing of plate on concrete

The general model has been to consider a cast in plate similarly to a base plate. With a typical column base plate a moment is resolved into a concentrated push-pull in the flanges of the column section. However, a fin plate is likely to induce a more even distribution of stress. This aspect has been reviewed by Ian Feltham from AT&R, who has suggested treating the concrete behind the cast in plate much like a typical reinforced concrete section. This has not been implemented due to time & budget constraints.

"I would follow your alternative treatment, considering the concrete behind the cast-in plate similarly to how you would consider a normal reinforced concrete section. Providing the limiting strain (usually 0.0035) can be met, the rectangular stress block, starting at a distance from the neutral axis, results in a force and moment very similar to those from a realistic stress-strain relation; only if there were brittle components in the system (for example, some form of brittle shear stud) would this be inappropriate." - Ian Feltham 07/2017

Bottom node of strut and tie diagram

There is limited guidance on how to dimension the CCC node at the base of the cast in plate. Under guidance from Ian Feltham, for this project the height of the node has been taken equal to the depth of the compression block and the width has been taken as the minimum of the compression block width, or the stud spacing + z (i.e. assuming a 30-degree spread of stress).

Following these recommendations has led to the CCC node failing at very low loads. The design of this CCC node will require additional review.

Shear stud interaction

SGN 09 does not provide guidance on how shear studs interact, and so there are no limitations on closely spaced shear studs beyond trying to adhere to good practice. More guidance would be welcomed to allow a tool to inform a user of poor spacings.

Stress block relative to fin plate

Current guidance does not relate compression block position to the fin plate, but to the bottom edge of the plate. In extreme cases where the plate extends a long way from the bottom of the fin plate the plate would clearly be too flexible for this diagram to form. Additional guidance is required.

Strut and tie diagram / additional links

Current practice is generally to avoid introducing additional links behind the cast in plate, however the guidance suggests that for $\cot(\theta) > 1.5$ links are required to form a modified strut and tie diagram. Current guidance suggests that the bearing stress block grows from the base of the cast in plate, leading to high strut angles and thus requiring links behind the plate.

As deep plates are often required to allow more shear studs to be introduced an alternative could be that the strut initially forms at a desirable / low energy angle, with the plate forming a compression block that grows from nearer the middle of the plate. Only for higher loads on a given plate arrangement would the modified strut and tie diagram be required. Clearly this approach would result in higher forces in the anchorage bars than for a deeper 'z', and so the engineer would need to choose between having larger anchorage bars or introducing additional links.

Further research would be required to guide a logical approach to the strut & tie diagram and stress block locations.

Axial force in wall

Impact of axial force in the wall is not addressed in guidance.

DesignCheck - Cast In Plates

Background

The DesignCheck framework is a vision of the digital future of structural engineering. The aim is to create an ecosystem of design tools for 'stand-alone' structural components that combines detailed and verified engineering calculations with simple user interaction through a variety of software platforms.

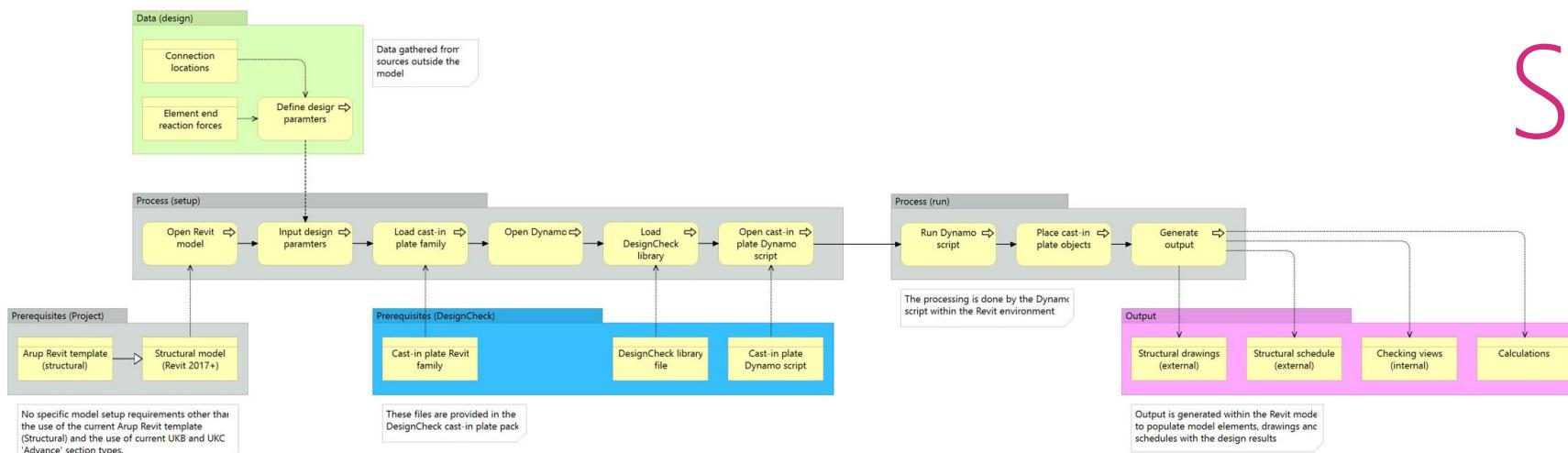
The fundamental basis is the creation of a centralised repository of code based design checking calculations that plug into a local users automated workflow. This could be Grasshopper and Dynamo based, or through more direct pure coding approaches. The work documented here represents the first step in building this framework using the practical example of cast-in plate connections through Dynamo in a structural Revit model.

iiA 17006: http://invest.intranet.arup.com/?lay_out=projsheet&pro_jid=17006&tab=projsheetdetailstabpage0

DesignCheck community site: <https://arup.sharepoint.com/sites/community-digital-design/designcheck>

Process

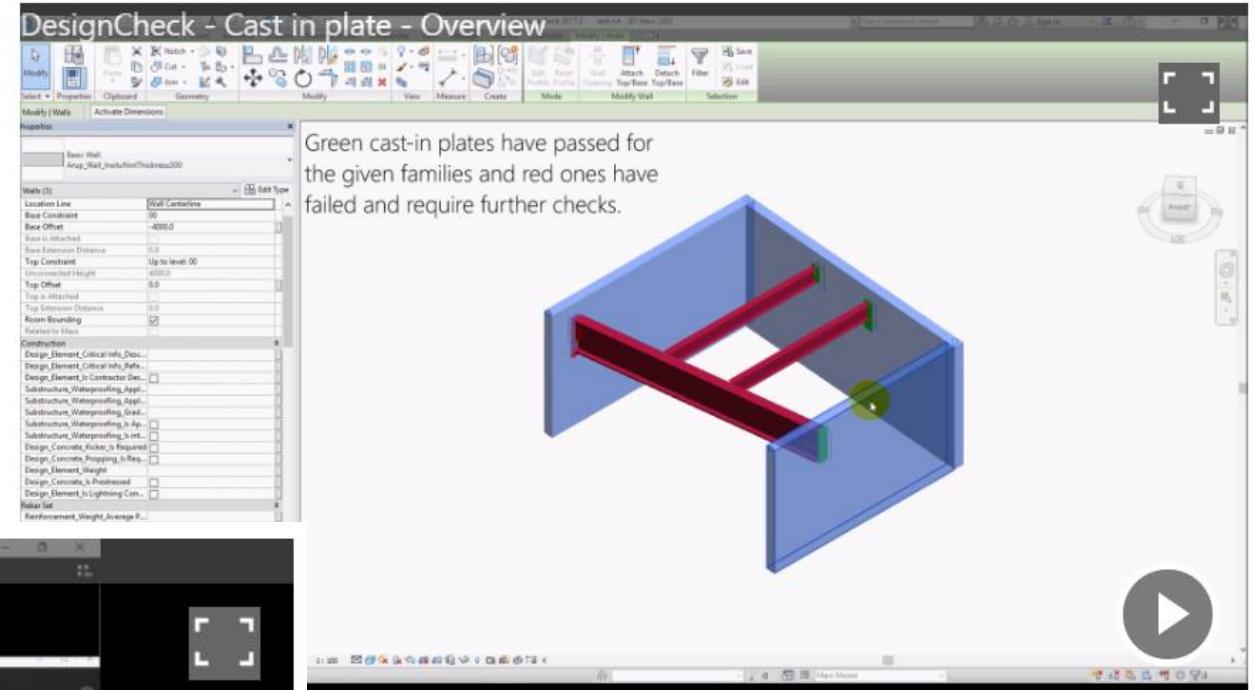
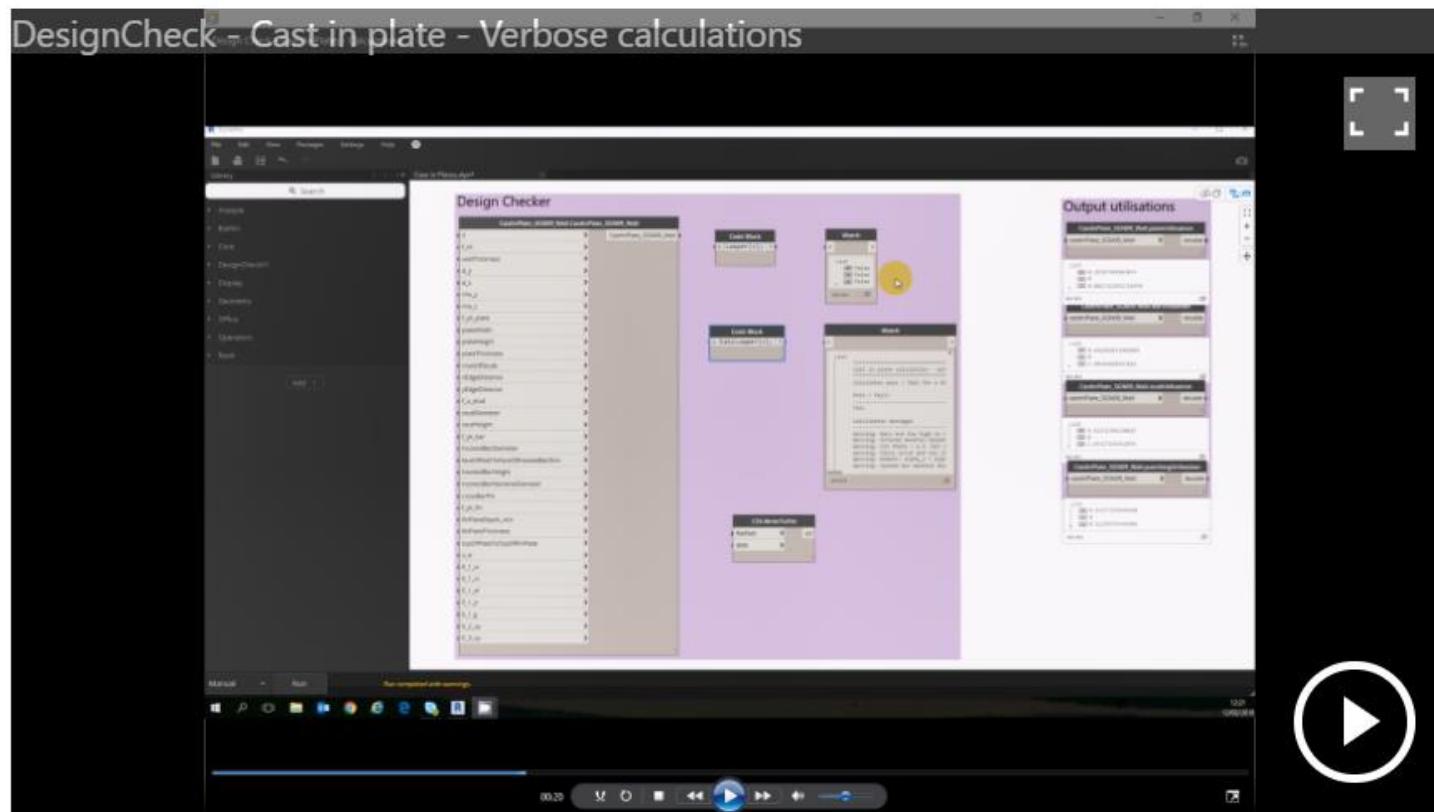
This tool has been developed around a defined workflow that is inherent in the operation of the tool. The archimate process map below (and downloadable further down this page) documents the steps required for successful execution of the tool and indicates the required inputs and outputs.



Share it

Showcase it

DesignCheck - Cast in plate - Verbose calculations



ARUP



Properties

3D View

3D View: (3D)

View Scale: 1:100 **Edit Type**

Scale Value: 1:100

Detail Level: Medium

Parts Visibility: Show Original

Visibility/Graphics Overrides: Edit...

Graphic Display Options: Edit...

Discipline: Coordination

Show Hidden Lines: By Discipline

Default Analysis Display Style: None

Sun Path:

Extents

Crop View:

Crop Region Visible:

Annotation Crop:

Far Clip Active:

Far Clip Offset: 304800.0

Section Box:

Camera

Rendering Settings: Edit...

Locked Orientation: Perspective

Eye Elevation: 7146.4

Target Elevation: -2000.0

Camera Position: Adjusting

Identity Data

View Template: <None>

View Name: (3D)

Dependency: Independent

Title on Sheet:

View Purpose:

Package:

Sub-Package:

Identity_LOI:

Phasing

Phase Filter: Show All

Phase: New Construction

Properties help

Apply

1:100

Main Model



0:00:10

*“What can we do with
the model output?”*

- Everyone we work with.

Type Properties

X

Family: Cast in Plate - Type Family

Load...

Type: 2x5 Studs, 2xB16 Anchors

Duplicate...

Rename...

Type Parameters

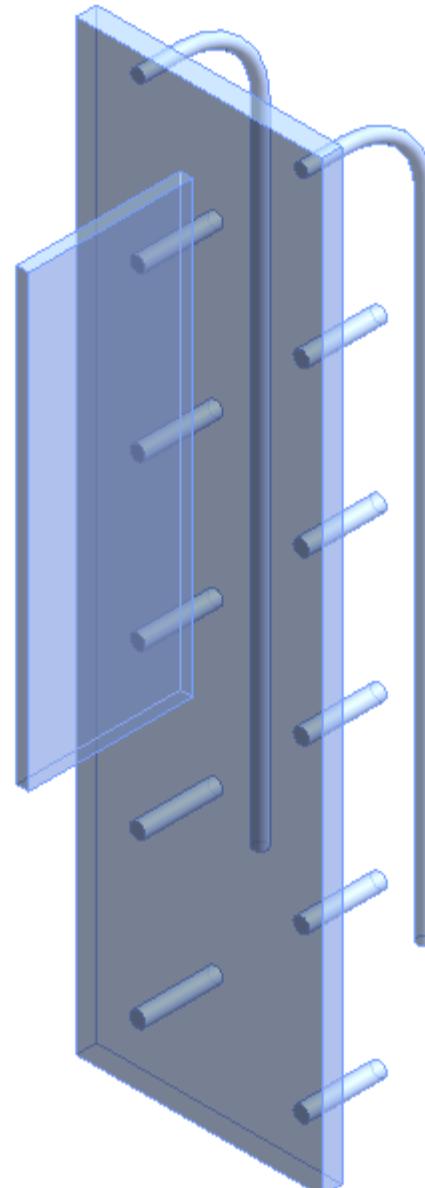
| Parameter | Value |
|---------------------------------|--------|
| Dimensions | |
| Cast In Plate Height | 1100.0 |
| Cast-In Plate Thickness | 25.0 |
| Cast-In Plate Width | 300.0 |
| Fin Plate Length | 200.0 |
| Fin Plate Offset From Centre of | 23.0 |
| Fin Plate Thickness | 15.0 |
| Minimum Fin Plate Depth | 550.0 |
| Rebar Anchor Diameter | 16.0 |
| Rebar Anchor Horizontal Length | 150.0 |
| Rebar Anchor Mandrel Diameter | 96.0 |
| Rebar Anchor Radius | 8.0 |
| Rebar Anchor Vertical Length | 900.0 |
| Shear Stud Diameter | 19.0 |
| Shear Stud Height | 95.0 |
| Shear Stud Y Spacing | 200.0 |
| Stud & Anchor X Edge Distance | 50.0 |

<< Preview

OK

Cancel

Apply



Review the
model

Review the
output text

```
*****
Cast in plate calculation - with reference to SGN09 & Eurocodes 2, 3, 4
*****
Calculates pass / fail for a SGN_09 style cast in plate i.e. two 90 degree hook bars, two columns of studs, set in an 'infinite & uninterrupted' wall

Pass / fail?
FAIL

Calculation messages
-----
Warning: Bars are too high in the plate. Taking account of vertical and horizontal tolerances of the fin plate the centroid of the bars must be at 45 degrees from the top of the fin plate. Plate strength calculations may not be valid. (CastInPlate_SGN09_Wall)
Warning: Entered mandrel diameter is less than minimum required by EC2 8.3(2) (CastInPlate_SGN09_Wall)
Warning: Cot theta > 1.5. Out of scope of calculation. Shear reinforcement likely required (CastInPlate_SGN09_Wall)
Warning: Falls strut and tie checks. Utilisation = 2.18998397536985 (CastInPlate_SGN09_Wall)
Warning: Remark: alpha_2 * alpha_3 + alpha_5 < 0.7, thus their product is now taken to be 0.7, clause (8.5) (StraightDesignAnchorageLength)
Warning: Hooked bar mandrel diameter is too small resulting in concrete crushing on the inside of the bend. Utilisation = 1.17 (CastInPlate_SGN09_Wall)
Warning: Plate thickness exceeds recommended geometry requirements to avoid secondary moments. (CastInPlate_SGN09_Wall)
Warning: Plate thickness exceeds recommended geometry requirements to avoid secondary moments. (CastInPlate_SGN09_Wall)
Remark: Plate weighs more than 20kg, consider manual handling (CastInPlate_SGN09_Wall)
Remark: Remark: alpha_2 * alpha_3 + alpha_5 < 0.7, thus their product is now taken to be 0.7, clause (8.5) (StraightDesignAnchorageLength)

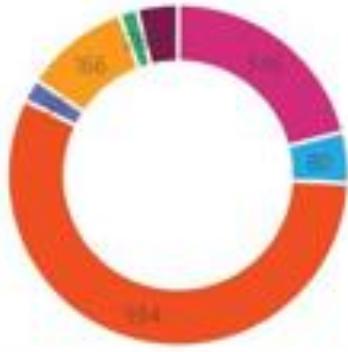
Utilisations
-----
Plate utilisation: 0.294
Bar utilisation (yield): 0.461
Stud utilisation: 0.413
Strut utilisation: 0.331
CTT node utilisation: 0.265
CCC node utilisation: 2.19
Concrete crushing (inside of mandrel) utilisation: 1.17
Anchorage utilisation: 0.244
Punching shear utilisation: 0.113

Assumptions
-----
Assumes not influenced by adjacent cast in plates. Assumes not above an opening. Assumes not adjacent to an edge. Assumes design to UK National Annex. Assumes persistent & transient load factors.

Verbose calculation
-----
Identification of tolerances (SGN09 Step 1)
Calculation of eccentricity of applied shear to face of the plate
E_1 = E_1_ct + E_1_cc + E_1_p + E_1_sf + E_1_g
E_1 = 35mm + 45mm + 10mm + 0mm + 10mm
E_1 = 100mm
```

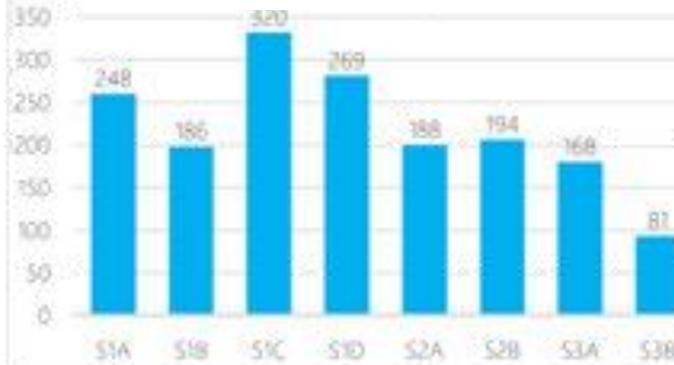
Review the metrics

Cast-in plates by type



- PP2
- CHP
- SPS
- CHS
- INS
- TIS
- TRS

Cast-in plates by zone

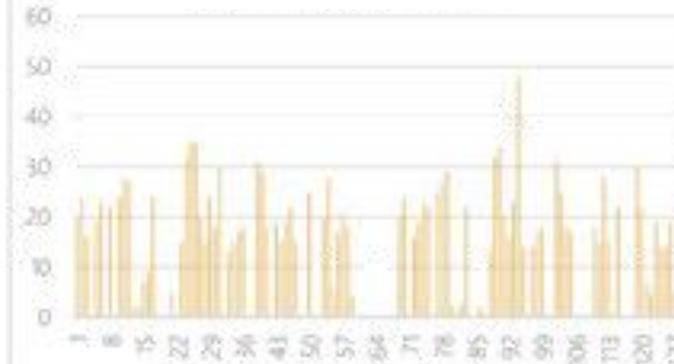


Cast-in plates by status



- Not modeled
- Test model
- Placeholder
- Geometry
- Design
- Review
- Construction

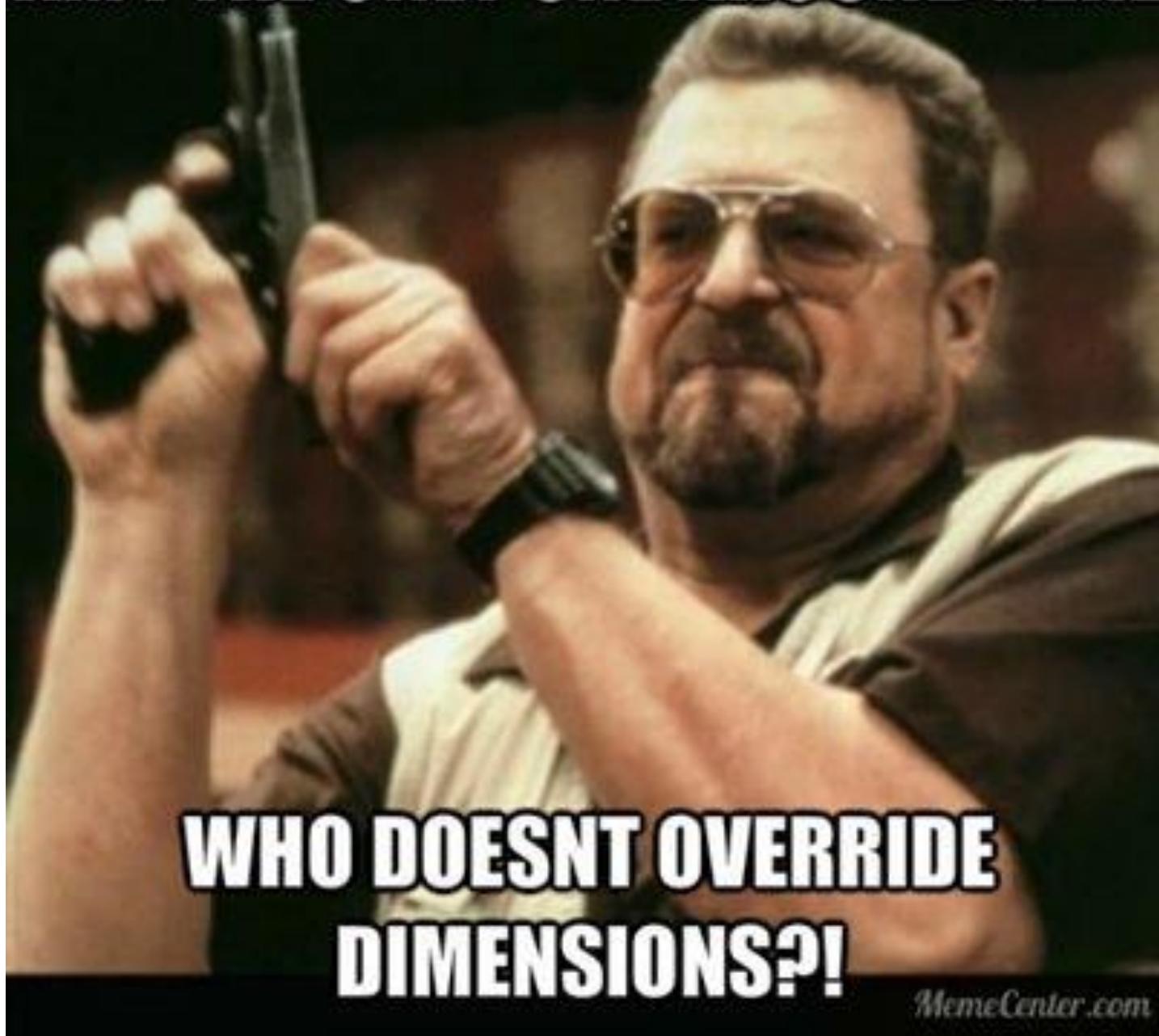
Cast-in plates by weight



Digital production

Modelling for success

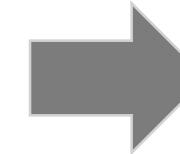
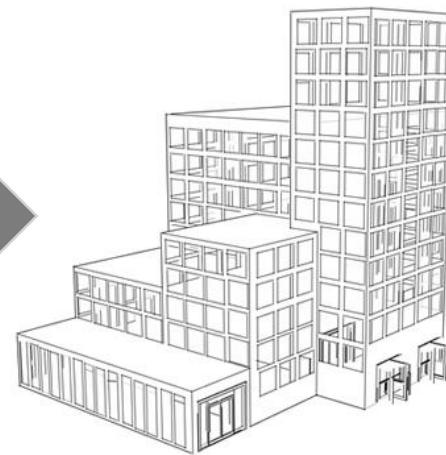
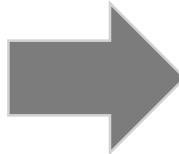
AM I THE ONLY ONE AROUND HERE





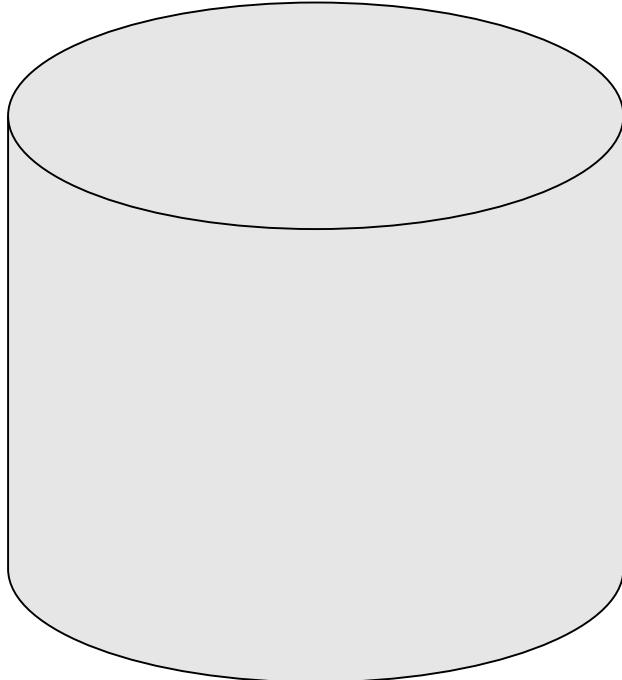
A model
first
approach

IN
Everything



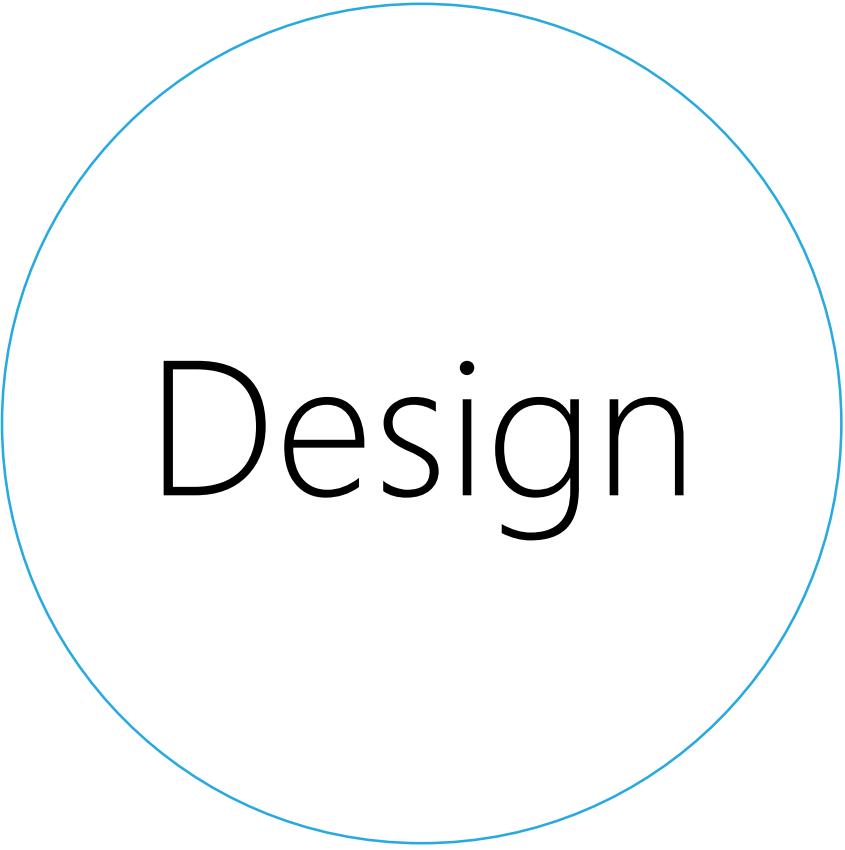
OUT
Everything

ARUP

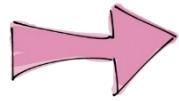
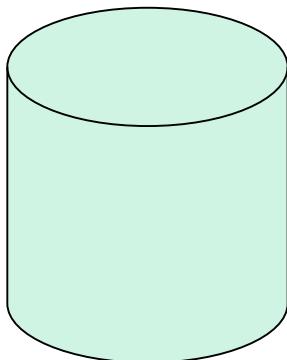


A single source
of the truth

No more 'double handing' of data

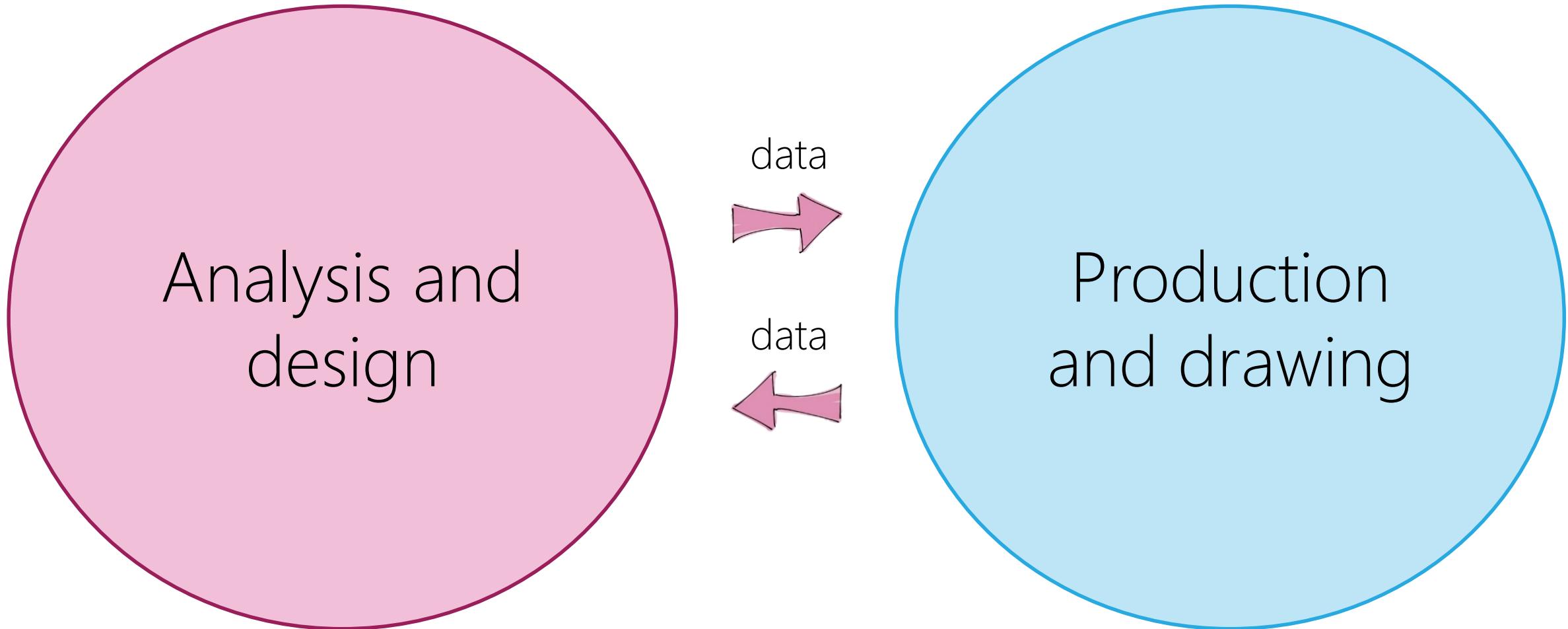


Design

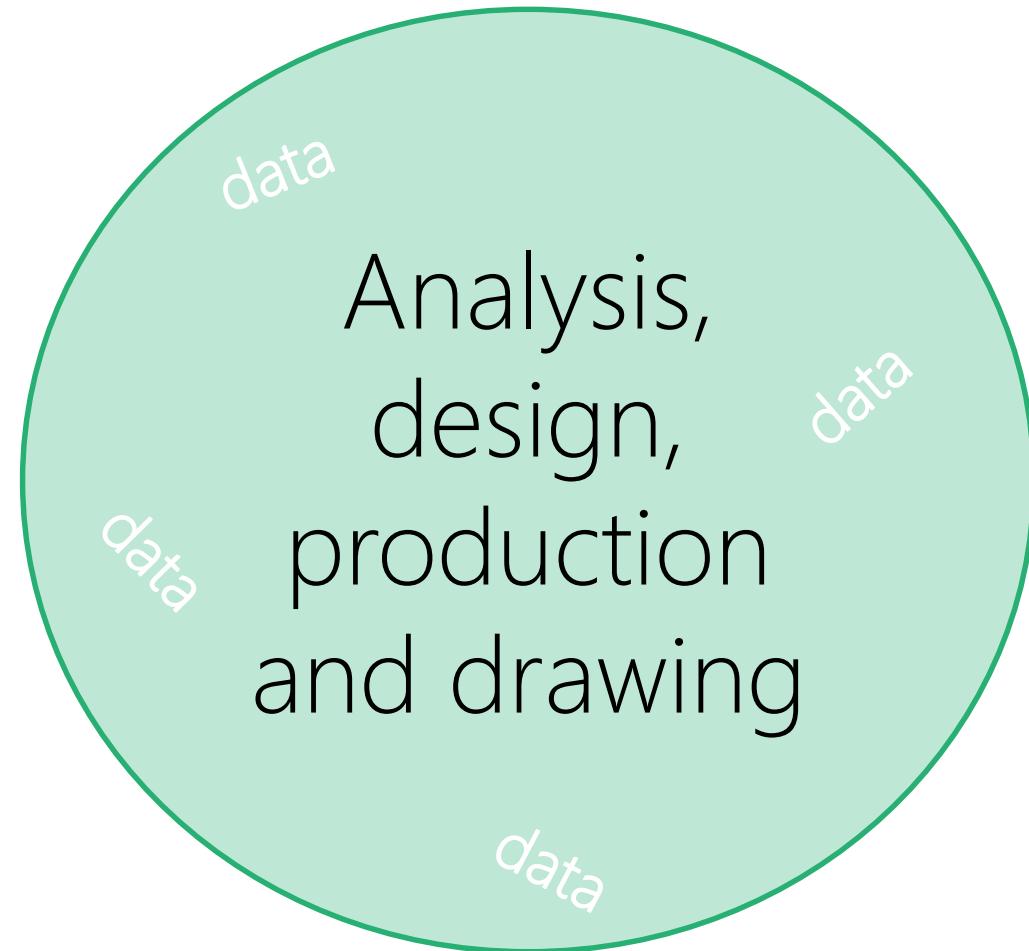


Everything the
client needs

Manual data transfer between two process streams 😞

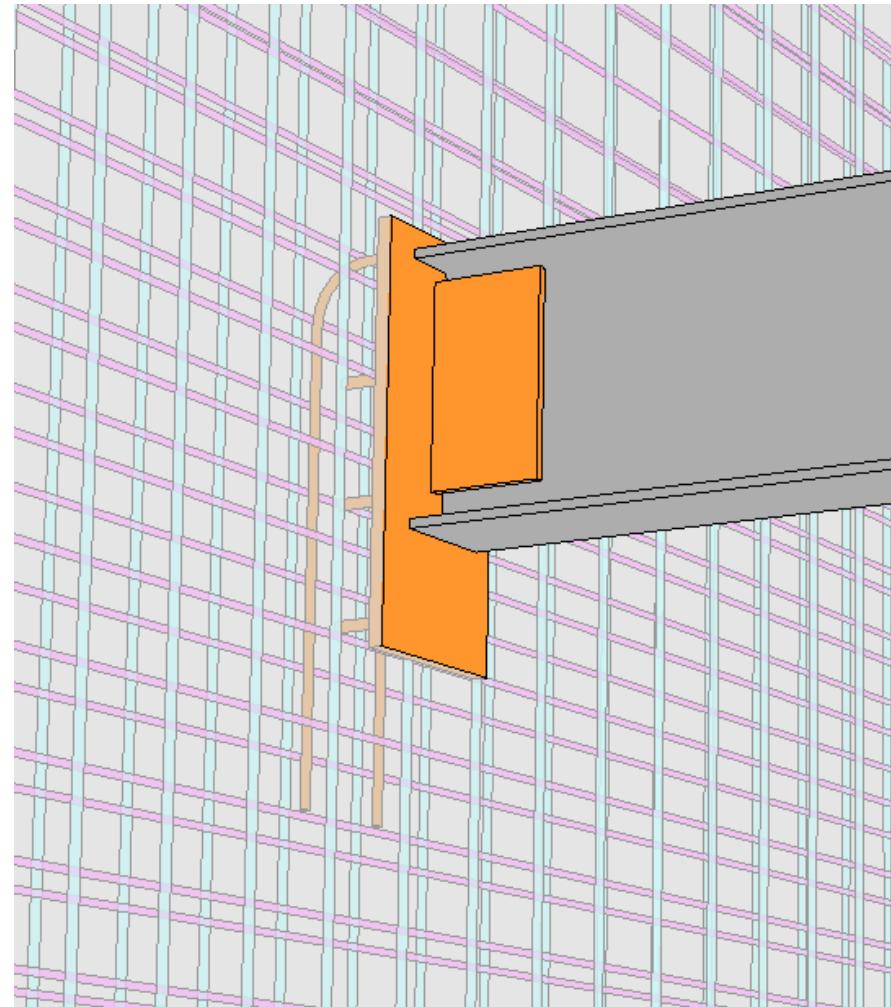


No data transfer in one flowing process stream





Direct experience
1-Click virtual reality
Spot what 'looks wrong'



Making time for thinking things through



Improving
Adding to our engine



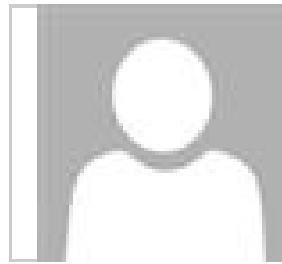
“This couldn’t
get any better”

-Us

ARUP



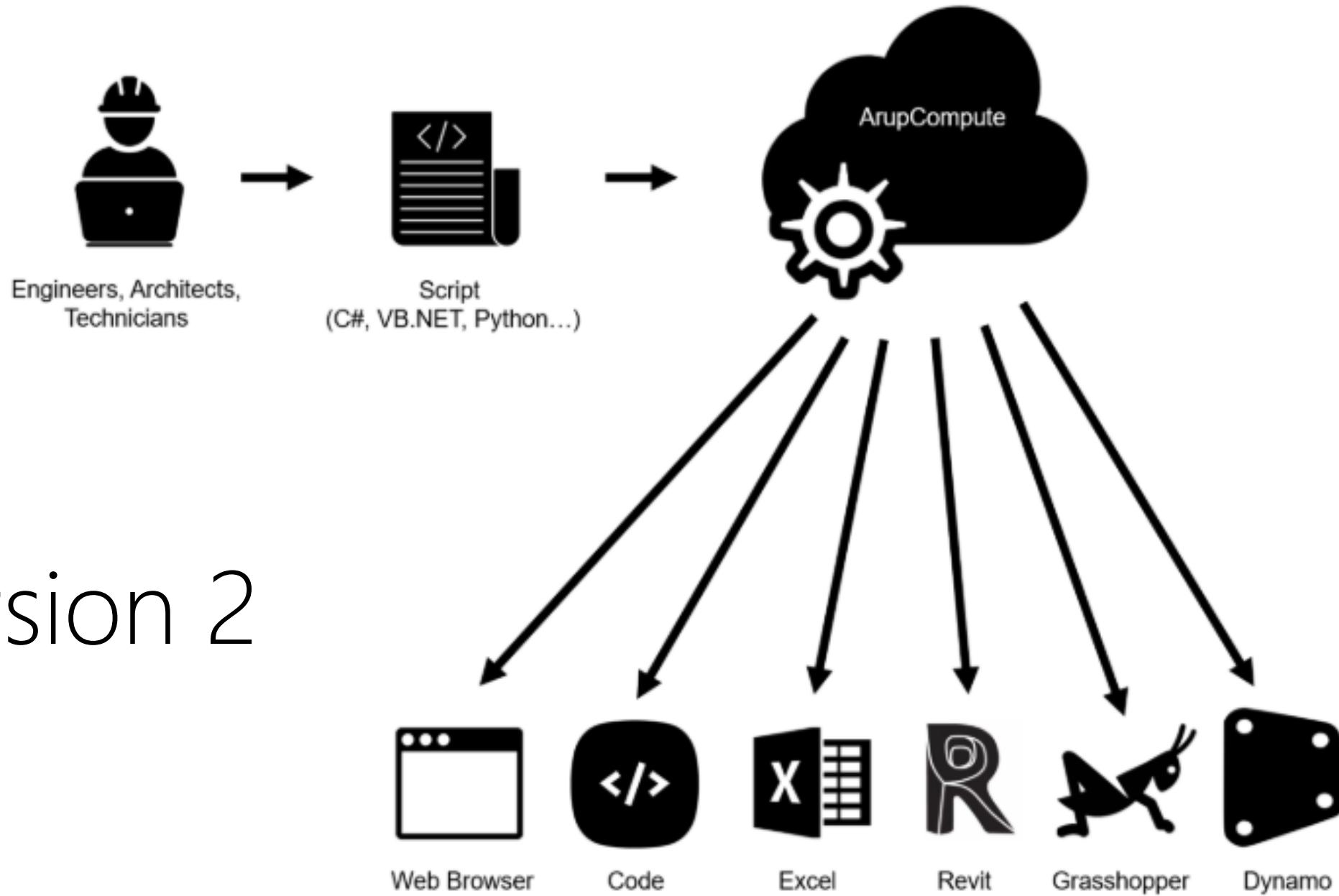
“Yeah it could”



Matteo Cominetti - People
Senior Programmer
Digital Technology, London Office

...get an Arup photo Matteo!

Version 2 API



Arup Compute https://arupcompute.azurewebsites.net

Automation Dynamo survey Analytics Tools Register - Tool All about the desk Analysis Results Disp Autodesk University Qasys GSA AWS Management C

ARUP COMPUTE

Filter calculations by name

DesignCheck 74

select a calculation to start

Matteo Cominetti

- All calculations
- Create new
- Recently run
- Favourites
- Integrations
- API
- About
- Report bugs
- Log out

This screenshot shows the Arup Compute web application interface. At the top, there's a navigation bar with links to various Autodesk tools like Automation, Dynamo, survey, Analytics, and Autodesk University. Below the header is a dark navigation bar with the 'ARUP COMPUTE' logo and a three-line menu icon. The main content area has a search bar for filtering calculations by name. A 'DesignCheck' section is visible with a count of 74. In the center, a large message says 'select a calculation to start'. To the right is a sidebar with a user profile for 'Matteo Cominetti' and a list of links: 'All calculations', 'Create new', 'Recently run', 'Favourites', 'Integrations', 'API', 'About', 'Report bugs', and 'Log out'. The bottom of the screen shows the Windows taskbar with icons for File Explorer, Google Chrome, and other system tools, along with the date and time (16:44, 03/10/2018).

The future

What's next?



Speckle integration?

Architect



Batch of changes

Engineer



Check and comment

Architect



Batch of changes

Engineer

Changes to geometry pushed to stream



Stream updated with results



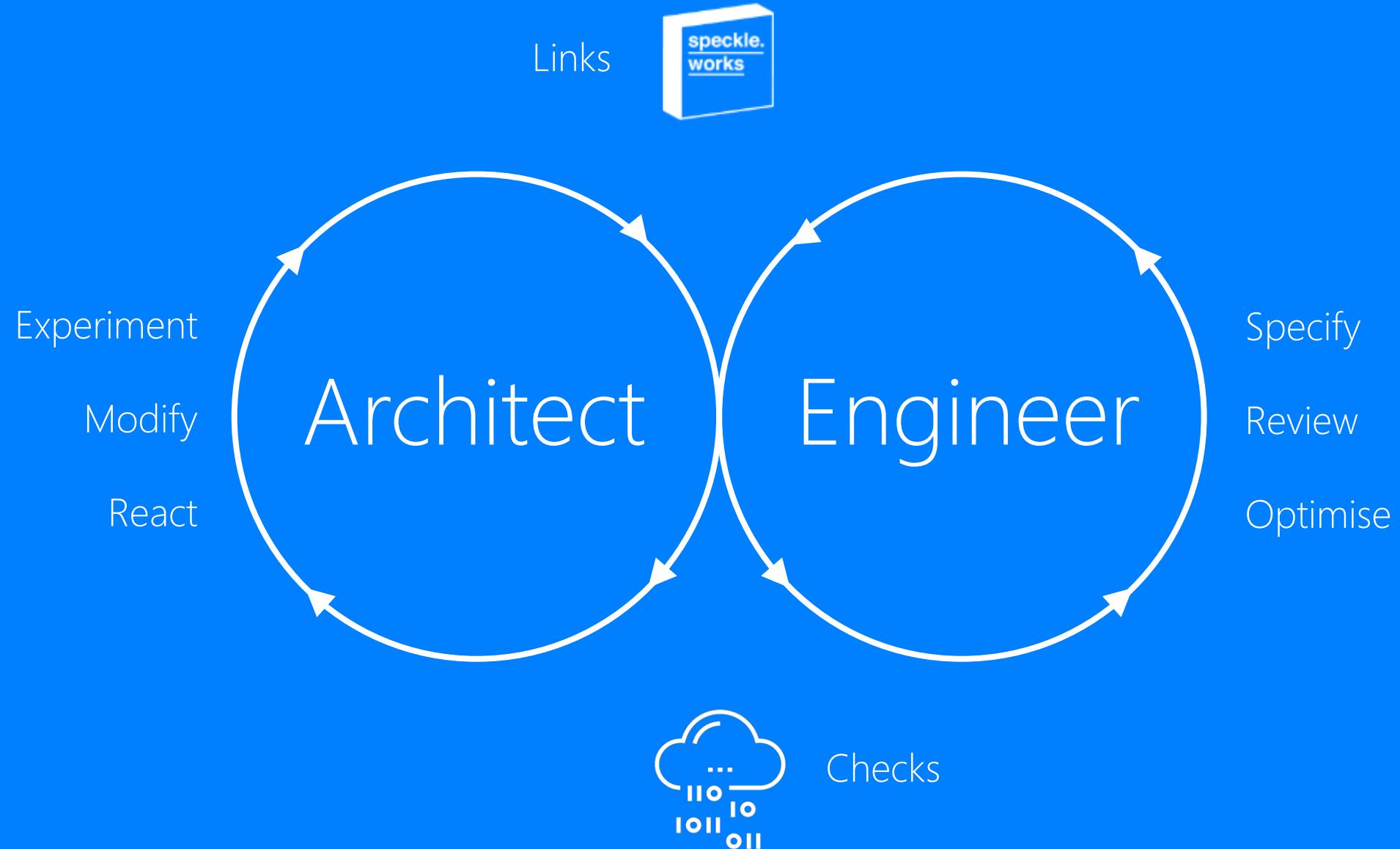
Speckle calls ArupCompute
API with updated inputs



Results sent back to Speckle



ArupCompute uses DesignCheck to
perform engineering calculations



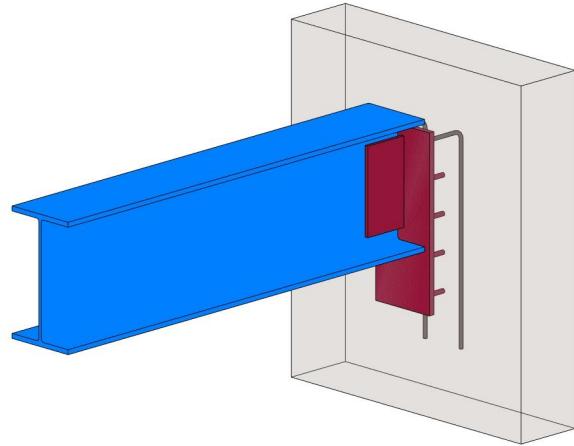
Data harvesting



Wolfram alpha

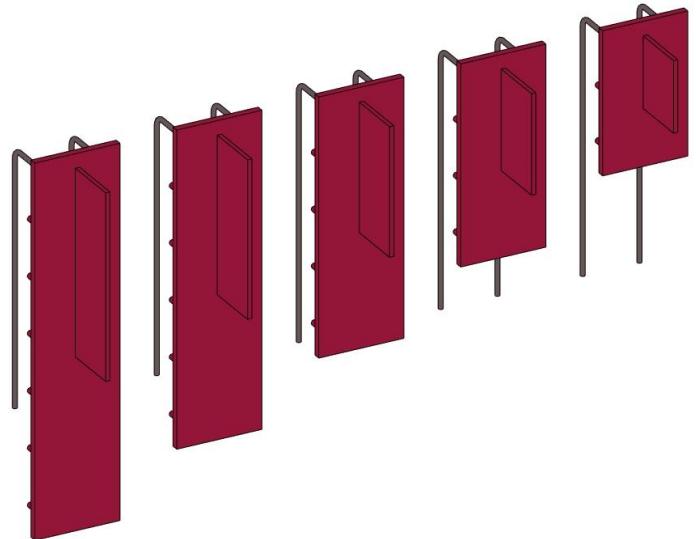
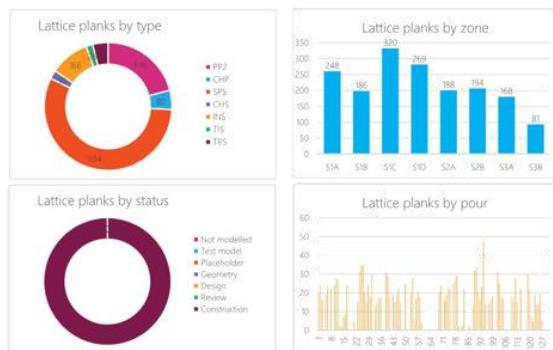
To conclude

If you only remember one bit

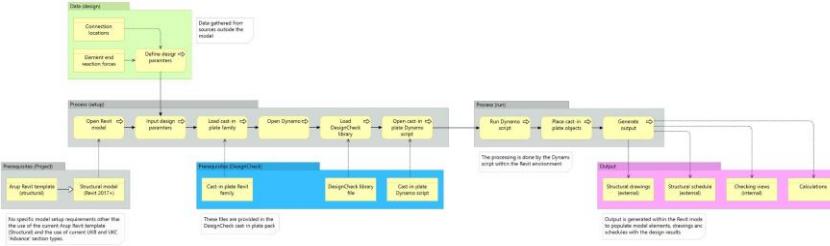


Comprehensive and
complete 3D modelling

Complete accuracy and
embedded information

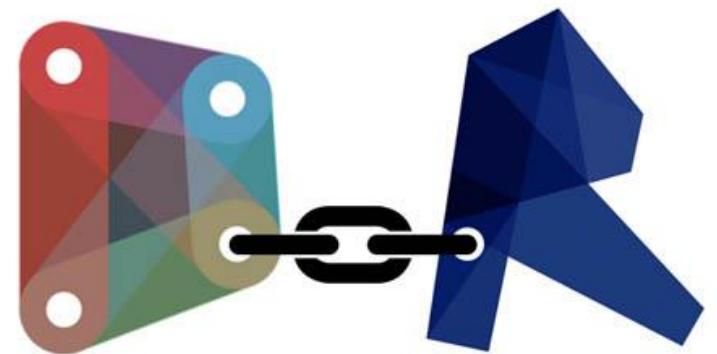
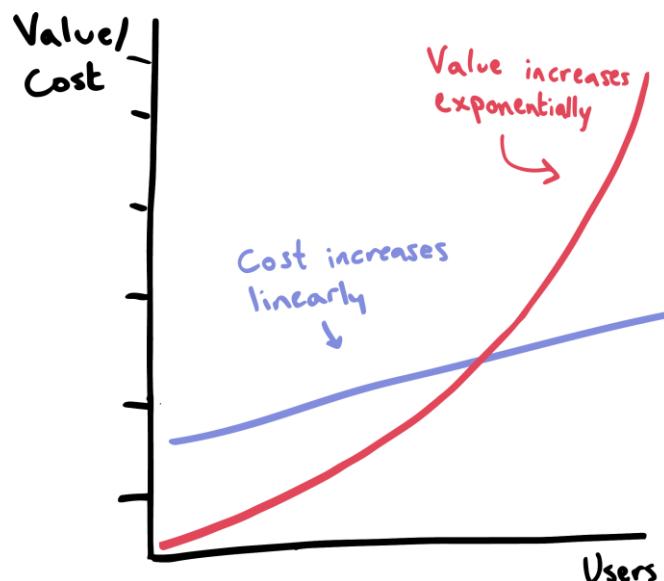


Better insight to improve
our design and delivery



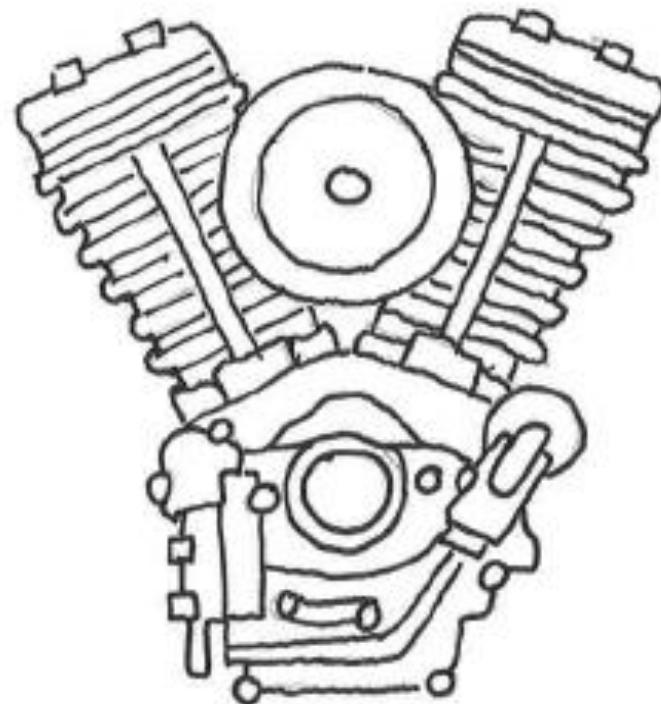
Improve our processes and embrace automation

Use a single data environment to exercise control

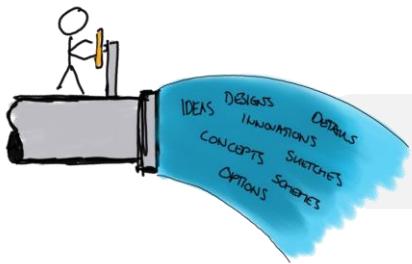


Build and enhance digital toolsets that work for us

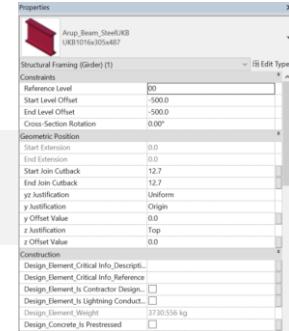
We took our engine...



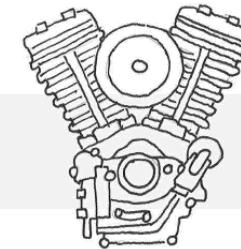
...and built our process around it



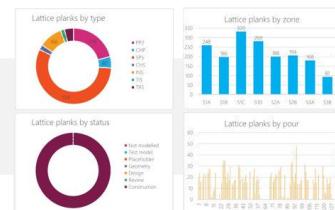
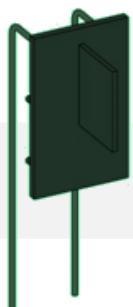
Geometry



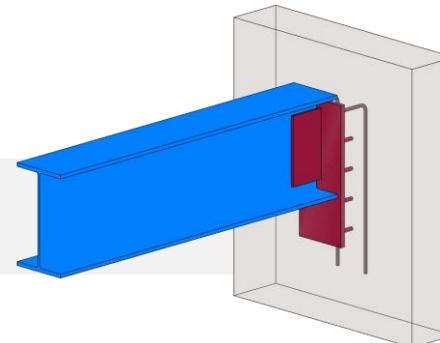
Parameters



Calculation



Results



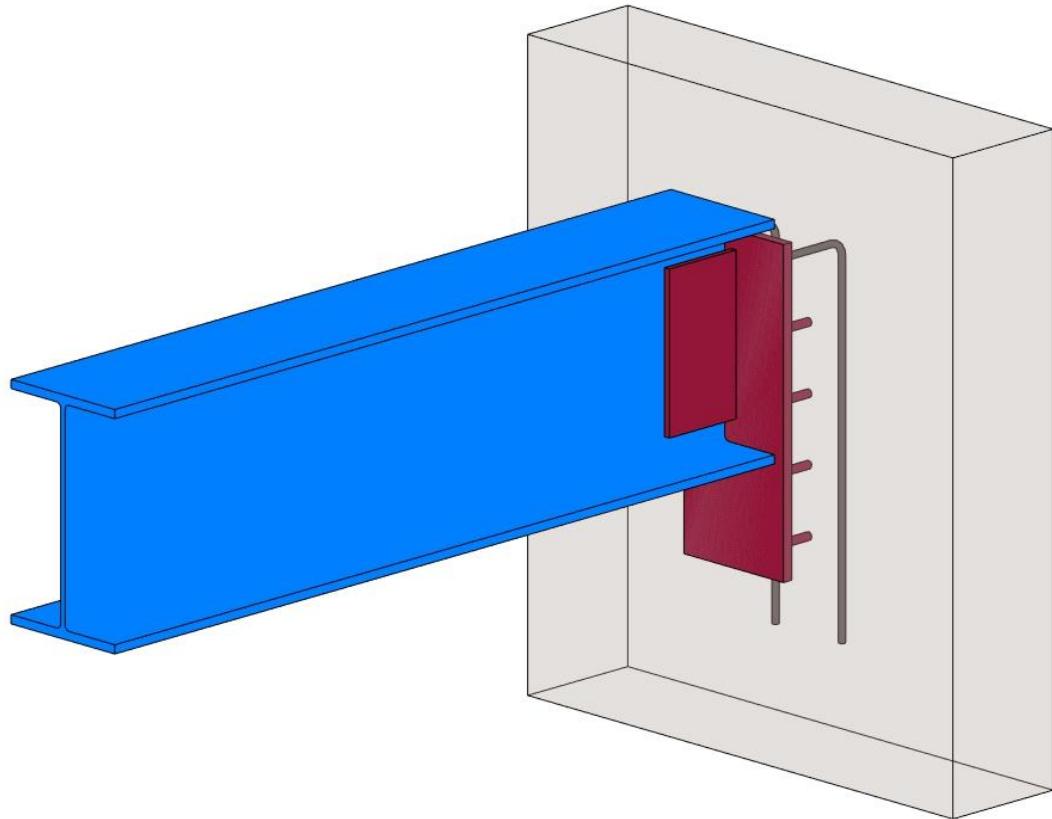
Insight

Communication

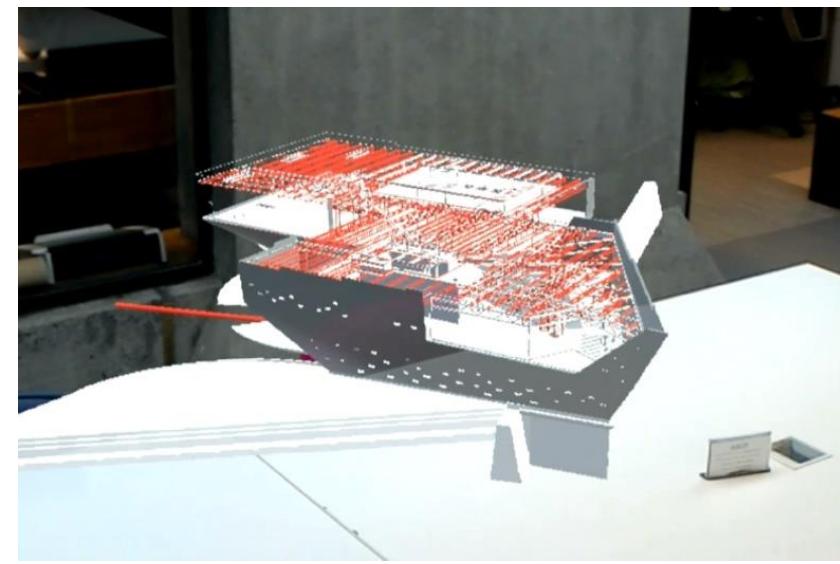
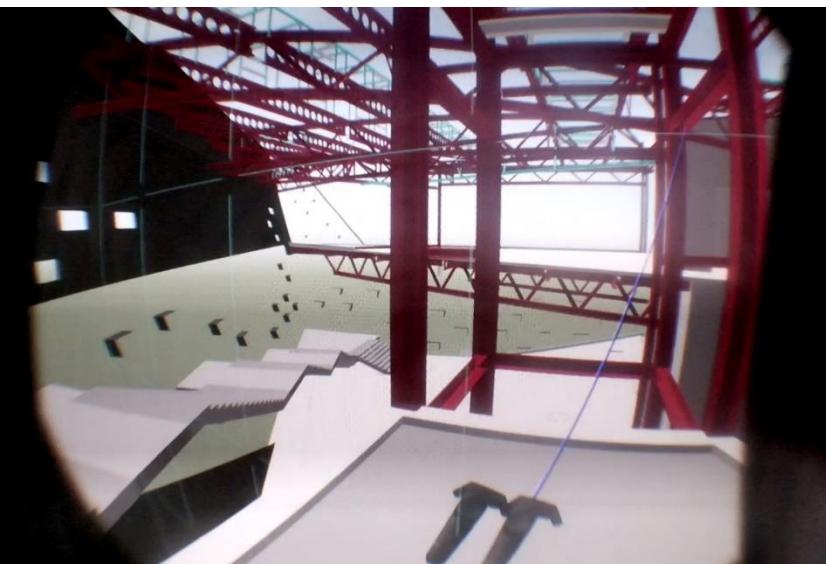
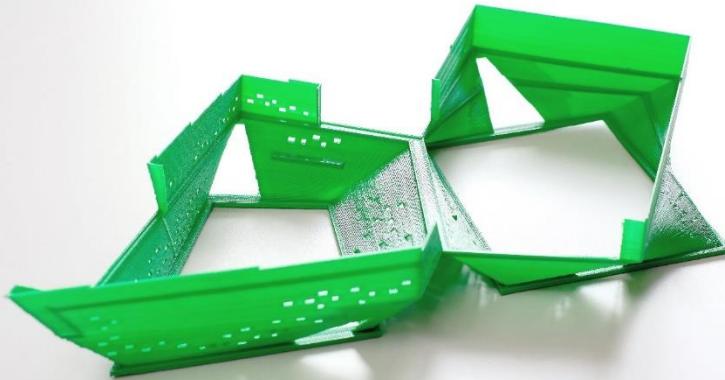


*"A late change in
requirements is a
competitive advantage."*

- Mary Poppendieck.



Design
automation



ARUP

Dynamo for Engineers, Design for All

Processes that work for us



Ian Wise | Senior Engineer | Arup

Steven Brown | Engineer | Arup

Hugh Groves | Engineer | Arup