4

Analysis of Example Truss by a CAS

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§4.1. Computer Algebra Systems

Computer algebra systems, known by the acronym CAS, are programs designed to perform symbolic and numeric manipulations following the rules of mathematics. The development of such programs began in the mid 1960s. The first comprehensive system — the "granddaddy" of them all, called *Macsyma* (an acronym for Project **Mac Symbolic Ma**nipulator) — was developed using the programming language Lisp at MIT's famous Artificial Intelligence Laboratory over the period 1967 to 1980.

The number and quality of symbolic-manipulation programs has expanded dramatically since the availability of graphical workstations and personal computers has encouraged interactive and experimental programming. As of this writing the leading general-purpose contenders are *Maple* and *Mathematica*.² In addition there are a dozen or so more specialized programs, some of which are available free or at very reasonable cost. See **Notes and Bibliography** at the end of the Chapter.

§4.1.1. Why Mathematica?

In the present book *Mathematica* will be used for Chapters and Exercises that develop symbolic and numerical computation for matrix structural analysis and FEM implementations. *Mathematica* is a commercial product developed by Wolfram Research, web site: http://www.wolfram.com. The version used to construct the code fragments presented in this Chapter is 4.1, which was commercially released in 2001. (Update: The latest version is 8, released in 2011.) The main advantages of *Mathematica* for technical computing are:

- 1. Availability on a wide range of platforms that range from PCs and Macs through Unix workstations. At CU Boulder, it is available through a free campus license (see §4.1).
- 2. Up-to-date user interface. On all machines *Mathematica* offers a graphics user interface called the Notebook front-end. This is mandatory for serious work. It provides professional typesetting for results.
- 3. A powerful programming language.
- 4. Good documentation and abundance of application books at all levels.

One common disadvantage of CAS, and *Mathematica* is no exception, is computational inefficiency in numerical calculations compared with a low-level implementation in, for instance, C or Fortran. The relative penalty can reach several orders of magnitude. For instructional use, however, the penalty is acceptable when compared to *human efficiency*. This means the ability to get FEM programs up and running in very short time, with capabilities for symbolic manipulation and graphics as a bonus.

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¹ Some CAS vendors call that kind of activity "doing mathematics by computer." It is more appropriate to regard such programs as enabling tools that help humans with complicated and error-prone manipulations. Mettle *and* metal. As of now, only humans can do mathematics.

² Another commonly used program for engineering computations: *Matlab*, does only numerical computations although a [poorly done] interface to *Maple* can be purchased as a toolbox. Historical notes: *Macsyma* died as a commercial product in 1999, although the Lisp source code of some versions is freely available. The *Maple* developer company *Waterloo Maple Inc.*, also known as *Maplesoft*, was purchased in 2009 by the Japanese software retailer Cybernet Systems. As a result, the *Matlab* symbolic toolbox is likely to be replaced.

Eligibility: students, faculty, staff and departments of all CU campuses **Platforms**: Mac OSX, Windows, Linux, Solaris, AIX, HP-UX

How to Get It

- 1) Download and install software as per instructions at http://oit.colorado.edu/software-hardware/site-licenses/mathematica or request an installation CD from the Site Licensing office: sitelic@colorado.edu, 303-492-8995
- 2) Register your copy online at http://register.wolfram.com/ using campus license number L2437-5121 plus your computer MathID number. Be sure to use your CU mail address.
- 3) A password (unique to your computer) will be forwarded to you via email from Site Licensing

ITS Support

Question & tech support problems: send email to sitelic@colorado.edu Periodic hands-on workshops are available for those new to the software, click on Workshops on the Web page given above.

Details (for example: what is a MathID?) about licensing & support are posted at the above web site.

FIGURE 4.1. Instructions to get *Mathematica* for free from CU-OIT (Office of Information Technology). One correction: the CD version is no longer available.

§4.1.2. How to Get It

Starting 1 August 2007, a free one-year license from CU's Information Technology Services (ITS) is available. Students may renew this license as long as they are registered. See Figure 4.1 for the "How to Get It" instructions.

If you plan to keep *Mathematica* for a longer time, an academic version is available.³

One word of caution about the campus-licensed free copy: once installed on your laptop or desktop, it cannot be transferred to another computer since the license is forever keyed to the disk identification. (For example, it won't launch if the disk is erased or replaced.)

§4.1.3. Programming Style and Prerequisites

The following material assumes that you are a moderately experienced user of *Mathematica*, or are willing to learn to be one. See **Notes and Bibliography** for a brief discussion of tutorial and reference materials in case you are interested.

The best way to learn it from scratch is trying it as a calculator and using online help as needed. Practice with the program until you reach the level of writing functions, modules and scripts with relative ease. With the Notebook interface and a good primer it takes only a few hours. When approaching that level you may notice that functions in *Mathematica* display many aspects similar

³ This was written in 2001; this option does not seem to exist anymore.

to C.⁴ You can exploit this similarity if you are proficient in that language. But *Mathematica* functions do have some unique aspects, such as matching arguments by pattern, and the fact that internal variables are global unless otherwise made local.⁵

Modification of function arguments should be avoided because it may be difficult to trace side effects. The programming style enforced here outlaws output arguments and a function can only return its name. But since the name can be a list of arbitrary objects the restriction is not serious.⁶

Our objective is to develop a symbolic program written in *Mathematica* that solves the example plane truss as well as some symbolic versions thereof. The program will rely heavily on the development and use of *functions* implemented using the Module construct of *Mathematica*. Thus the style will be one of procedural programming.⁷ The program will not be particularly modular (in the computer science sense) because *Mathematica* is not suitable for that programming style.⁸

The code presented in §4.2 through §4.8 uses a few language constructs that may be deemed as advanced, and these are briefly noted in the text so that appropriate reference to the *Mathematica* reference manual can be made.

§4.1.4. Class Demo Scripts

The cell scripts shown in Figures 4.2 and 4.3 will be used to illustrate the organization of a Notebook file and the "look and feel" of some basic *Mathematica* commands. These scripts will be demonstrated in class from a laptop.

⁴ Simple functions can be implemented in Mathematica directly, for instance DotProduct[x_,y_]:=x.y; more complicated functions are handled by the Module construct. These constructs are called *rules* by computer scientists.

⁵ In *Mathematica* everything is a function, including programming constructs. Example: in C for is a loop-opening keyword, whereas in *Mathematica* For is a function that runs a loop according to its arguments.

⁶ Such restrictions on arguments and function returns are closer in spirit to C than Fortran although you can of course modify C-function arguments using pointers — exceedingly dangerous but often unavoidable.

The name Module should not be taken too seriously: it is far away from the concept of modules in Ada, Modula, Oberon or Fortran 90. But such precise levels of interface control are rarely needed in symbolic languages.

⁸ Indeed none of the CAS packages in popular use is designed for strong modularity because of historical and interactivity constraints.

Integration example

```
f[x_,α_,β_]:=(1+β*x^2)/(1+α*x+x^2);
F=Integrate[f[x,-1,2],{x,0,5}];
F=Simplify[F];
Print[F]; Print[N[F]];
F=NIntegrate[f[x,-1,2],{x,0,5}];
Print["F=",F//InputForm];
```

```
10 + Log[21]
13.0445
F=13.044522437723455
```

FIGURE 4.2. Example cell for class demo.

```
Fa=Integrate[f[z,a,b],{z,0,5}]; Fa=Simplify[Fa]; Print["Fa=",Fa];
Plot3D[Fa,{a,-1.5,1.5},{b,-10,10},ViewPoint->{-1,-1,1}];
Fa=FullSimplify[Fa]; (* very slow but you get *) Print["Fa=",Fa];
```

$$\begin{split} \operatorname{Fa} &= \frac{1}{2\sqrt{4 - a^2}} \left(i \, b \, \log \! \left(\frac{i \, a}{\sqrt{4 - a^2}} + 1 \right) a^2 + i \, b \, \log \! \left(\frac{-i \, a + \sqrt{4 - a^2} - 10 \, i}{\sqrt{4 - a^2}} \right) a^2 - i \, b \, \log \! \left(\frac{i \, a + \sqrt{4 - a^2} + 10 \, i}{\sqrt{4 - a^2}} \right) a^2 - \frac{i \, a}{\sqrt{4 - a^2}} \right) a^2 - \frac{i \, a}{\sqrt{4 - a^2}} b \, \log \! \left(5 \, a + 26 \right) a + 10 \, \sqrt{4 - a^2} \, b - i \, \left((a^2 - 2) \, b + 2 \right) \log \! \left(1 - \frac{i \, a}{\sqrt{4 - a^2}} \right) - 2 \, i \, b \, \log \! \left(\frac{i \, a}{\sqrt{4 - a^2}} + 1 \right) + 2 \, i \, \log \! \left(\frac{i \, a}{\sqrt{4 - a^2}} + 1 \right) - 2 \, i \, \log \! \left(\frac{i \, a + \sqrt{4 - a^2} + 10 \, i}{\sqrt{4 - a^2}} \right) + 2 \, i \, \log \! \left(\frac{i \, a + \sqrt{4 - a^2} + 10 \, i}{\sqrt{4 - a^2}} \right) - 2 \, i \, \log \! \left(\frac{i \, a + \sqrt{4 - a^2} + 10 \, i}{\sqrt{4 - a^2}} \right) \right) \end{split}$$

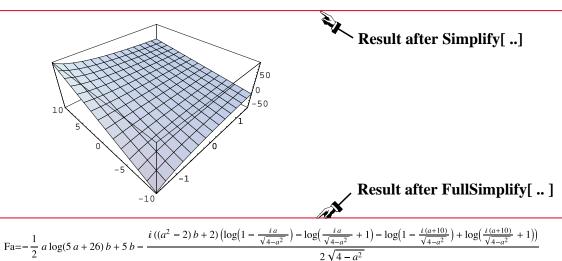


FIGURE 4.3. Another example cell for class demo. (Note: displayed results were obtained with *Mathematica* version 4.2. Integration answers from versions 5 and up are quite different.)

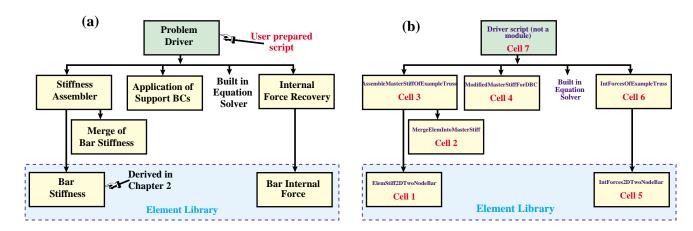


FIGURE 4.4. Example truss program: (a) organization by function; (b) organization by cell and module names.

§4.2. Program Organization

The overall organization of the example truss program is flowcharted in Figure 4.4. Figure 4.4(a) illustrates the program as divided into functional tasks. For example the driver program calls the stiffness assembler, which in turn calls two functions: form bar stiffness and merge into master stiffness. This kind of functional division would be provided by any programming language.

On the other hand, Figure 4.4(b) is *Mathematica* specific. It displays the names of the module that implement the functional tasks and the cells where they reside. (Those program objects: modules and cells, are described in the sections below.)

The following sections describe the code segments of Figure 4.4(b), one by one. For tutorial purposes this is done in a "bottom up" fashion, that is, going cell by cell, from left to right and bottom to top.

§4.3. The Element Stiffness Module

As our first FEM code segment, the top box of Figure 4.5 shows a module that evaluates and returns the 4 × 4 stiffness matrix of a plane truss member (two-node bar) in global coordinates. The text in that box of that figure is supposed to be placed on a Notebook cell. Executing the cell, by clicking on it and hitting an appropriate key (<Enter> on a Mac), gives the output shown in the bottom box. The contents of the figure is described in further detail below.

§4.3.1. Module Description

The stiffness module is called ElemStiff2DTwoNodeBar. Such descriptive names are permitted by the language. This reduces the need for detailed comments.

The module takes two arguments:

 $\{\{x1,y1\},\{x2,y2\}\}\$ A two-level list⁹ containing the $\{x,y\}$ coordinates of the bar end

A level-one list is a sequence of items enclosed in curly braces. For example: {x1,y1} is a list of two items. A level-two list is a list of level-one lists. An important example of a level-two list is a matrix.

```
ElemStiff2DTwoNodeBar[{x1_,y1_},{x2_,y2_}},{Em_,A_}] :=
  Module[\{c,s,dx=x2-x1,dy=y2-y1,L,Ke\},
    L=Sqrt[dx^2+dy^2]; c=dx/L; s=dy/L;
    Ke=(Em*A/L)* \{\{ c^2, c*s,-c^2,-c*s\},
                     c*s, s^2,-s*c,-s^2},
                     -c^2,-s*c, c^2, s*c},
                    {-s*c,-s^2, s*c, s^2}};
    Return[Ke]
 ];
Ke= ElemStiff2DTwoNodeBar[{{0,0},{10,10}},{100,2*Sqrt[2]}];
Print["Numerical elem stiff matrix:"]; Print[Ke//MatrixForm];
Ke= ElemStiff2DTwoNodeBar[{{0,0},{L,L}},{Em,A}];
Ke=Simplify[Ke,L>0];
Print["Symbolic elem stiff matrix:"]; Print[Ke//MatrixForm];
Numerical elem stiff matrix:
    10 10 -10 -10
    10 10 -10 -10
   -10 -10 10 10
   -10 -10 10 10
Symbolic elem stiff matrix:
    2\sqrt{2} L
           2\sqrt{2} L
                   -\frac{A\,Em}{2\sqrt{2}\,\,L}
            A Em
                    A Em
                            A Em
```

FIGURE 4.5. Module ElemStiff2DTwoNodeBar to form the element stiffness of a 2D 2-node truss element in global coordinates. Test statements (in blue) and test output.

nodes labelled as 1 and 2.10

 $\{Em,A\}$ A one-level list containing the bar elastic modulus, E and the member cross section area, A. See $\S4.3.3$ as to why name E cannot be used.

The use of the underscore after argument item names in the declaration of the Module is a requirement for pattern-matching in *Mathematica*. If, as recommended, you have learned functions and modules this language-specific feature should not come as a surprise.

The module name returns the 4×4 member stiffness matrix internally called Ke. The logic that leads to the formation of that matrix is straightforward and need not be explained in detail. Note, however, the elegant direct declaration of the matrix Ke as a level-two list, which eliminates the fiddling around with array indices typical of low-level programming languages. The specification format in fact closely matches the mathematical expression given as (2.18) in Chapter 2.

§4.3.2. Programming Remarks

The function in Figure 4.5 uses several intermediate variables with short names: dx, dy, s, c and L. It is strongly advisable to make these symbols *local* to avoid potential names clashes somewhere else. In the Module [. . .] construct this is done by listing those names in a list immediately

These are called the *local node numbers*, and replace the i, j of previous Chapters. This is a common FEM programming practice.

¹¹ The "global by default" choice is the worst one, but we must live with the rules of the language.

after the opening bracket. Local variables may be *initialized* when they are constants or simple functions of the argument items; for example on entry to the module dx=x2-x1 initializes variable dx to be the difference of x node coordinates, namely $\Delta x = x_2 - x_1$.

The Return statement fulfills the same purpose as in C or Fortran 90. *Mathematica* guides and textbooks advise against the use of that and other C-like constructs. The writer strongly disagrees: the Return statement makes clear what the Module gives back to its invoker and is self-documenting.

§4.3.3. Case Sensitivity

Mathematica, like most recent computer languages, is case sensitive so that for instance E is not the same as e. This is fine. But the language designer decided that names of system-defined objects such as built-in functions and constants must begin with a capital letter. Consequently the liberal use of names beginning with a capital letter may run into clashes. For example you cannot use E because of its built-in meaning as the base of natural logarithms.¹²

In the code fragments presented throughout this book, identifiers beginning with upper case are used for objects such as stiffness matrices, modulus of elasticity, and cross section area. This follows established usage in Mechanics. When there is danger of clashing with a protected system symbol, additional lower case letters are used. For example, Em is used for the elastic modulus instead of E because (as noted above) the latter is a reserved symbol.

§4.3.4. Testing the Member Stiffness Module

Following the definition of ElemStiff2DTwoNodeBar in Figure 4.5 there are several statements that constitute the *module test script*. This script calls the module and prints returned results. Two cases are tested. First, the stiffness of member (3) of the example truss, using all-numerical values. Next, some of the input arguments for the same member are given symbolic names so they stand for variables. For example, the elastic module is given as Em instead of 100 as in the foregoing test. The print output of the test is shown in the lower portion of Figure 4.5.

The first test returns the member stiffness matrix (?), as may be expected. The second test returns a symbolic form in which three symbols appear: the coordinates of end node 2, which is taken to be located at {L,L} instead of {10, 10}, A, which is the cross-section area and Em, which is the elastic modulus. Note that the returning matrix Ke is subject to a Simplify step before printing it, the reason for which is the subject of an Exercise. The ability to carry along variables is of course a fundamental capability of any CAS, and the main reason for which such programs are used.

§4.4. Merging a Member into the Master Stiffness

The next code fragment, listed in Figure 4.6, is used in the assembly step of the DSM. Module MergeElemIntoMasterStiff receives the 4×4 element stiffness matrix formed by FormElemStiff2DNodeBar and "merges" it into the master stiffness. It takes three arguments:

Ke The 4×4 member stiffness matrix to be merged. This is a level-two list.

eftab The column of the Element Freedom Table or EFT, defined in §3.5.1, appropriate to the member being merged. Recall that the EFT lists the global equation numbers

In retrospect this appears to have been a highly questionable decision. System defined names should have been identified by a reserved prefix or postfix to avoid surprises, as done in *Macsyma* or *Maple*. *Mathematica* issues a warning message, however, if an attempt to redefine a "protected symbol" is made.

```
MergeElemIntoMasterStiff[Ke_,eftab_,Kin_]:=Module[
{i,j,ii,jj,K=Kin},
   For [i=1, i<=4, i++, ii=eftab[[i]];
      For [j=i, j<=4, j++, jj=eftab[[j]];
    K[[jj,ii]]=K[[ii,jj]]+=Ke[[i,j]]</pre>
   ]; Return[K]
1:
K=Table[0,{6},{6}];
Print["Initialized master stiffness matrix:"];
Print[K//MatrixForm]
Ke=ElemStiff2DTwoNodeBar[{{0,0},{10,10}},{100,2*Sqrt[2]}];
Print["Member stiffness matrix:"]; Print[Ke//MatrixForm];
K=MergeElemIntoMasterStiff[Ke,{1,2,5,6},K];
Print["Master stiffness after member merge:"];
Print[K//MatrixForm];
Initialized master stiffness matrix:
  0 0 0 0 0 0
  0 0 0 0 0 0
 0 \quad 0 \quad 0 \quad 0 \quad 0
 0 0 0 0 0 0
 0 0 0 0 0 0
 0 0 0 0 0 0
 Member stiffness matrix:
   10 10 -10 -10
  10 10 -10 -10
  -10 -10 10 10
  -10 -10 10 10
Master stiffness after member merge:
   10 \quad 10 \quad 0 \quad 0 \quad -10 \quad -10
  10 10 0 0 -10 -10
   0 0 0 0 0 0
   0 0 0 0
               0 0
  -10 -10 0 0
                10 10
  -10 -10 0 0
                10
```

FIGURE 4.6. Module MergeElemIntoMasterStiff to merge a 4×4 bar element stiffness into the master stiffness matrix. Test statements (in blue) and test output.

for the four member degrees of freedom, cf. (3.26) in Chapter 3. This is a level-one list consisting of 4 integers.

Kinp The incoming 6×6 master stiffness matrix. This is a level-two list.

MergeElemIntoMasterStiff returns, as module name, the updated master stiffness matrix internally called K with the member stiffness merged in. We encounter here a novelty: an input-output argument. Because a formal argument cannot be modified, the situation is handled by copying the incoming Kinp into K (a local variable declared in the list that follows the Module name) on entry. That is the copy updated and returned via the Return statement. The implementation has a strong C flavor with two nested For loops. Because the iterators are very simple, nested Do loops could have been used as well.

The statements after the module provide a simple test. Before the first call to this function, the master stiffness matrix must be initialized to a zero 6×6 array. This is done in the first test statement using the Table function. The test member stiffness matrix is that of member (3) of the example truss, and is obtained by calling ElemStiff2DTwoNodeBar. The EFT is {1,2,5,6} since element freedoms 1,2,3,4 map into global freedoms 1,2,5,6. Running the test statements yields the listing

```
AssembleMasterStiffOfExampleTruss[]:=
Module[\{Ke,K=Table[0,\{6\},\{6\}]\}
    Ke=ElemStiff2DTwoNodeBar[\{\{0,0\},\{10,0\}\},\{100,1\}];
    K= MergeElemIntoMasterStiff[Ke, {1,2,3,4},K];
Ke=ElemStiff2DTwoNodeBar[{{10,0},{10,10}},{100,1/2}];
    K= MergeElemIntoMasterStiff[Ke,{3,4,5,6},K];
    Ke=ElemStiff2DTwoNodeBar[{{0,0},{10,10}},{100,2*Sqrt[2]}];
    K= MergeElemIntoMasterStiff[Ke, {1,2,5,6},K];
    Return[K]
  1:
K=AssembleMasterStiffOfExampleTruss[];
Print["Master stiffness of example truss:"]; Print[K//MatrixForm];
Master stiffness of example truss:
  20 \quad 10 \quad -10 \quad 0 \quad -10 \quad -10
  10 10 0 0 -10 -10
  -10 \quad 0 \quad 10 \quad 0 \quad 0
  0 0
           0 5
                   0
                       -5
  -10 -10 0 0
                   10
                       10
  -10 -10 0 -5
                   10
                        15
```

FIGURE 4.7. Module AssembleMasterStiffOfExampleTruss that forms the 6×6 master stiffness matrix of the example truss. Test statements (in blue) and test output.

given in Figure 4.6. The output is as expected.

§4.5. Assembling the Master Stiffness

The module AssembleMasterStiffOfExampleTruss, listed in the top box of Figure 4.7, makes use of the foregoing two modules: ElemStiff2DTwoNodeBar and MergeElemIntoMasterStiff, to form the master stiffness matrix of the example truss. The initialization of the stiffness matrix array in K to zero is done by the Table function of Mathematica, which is handy for initializing lists. The remaining statements are self explanatory. The module is similar in style to C functions with no arguments. All the example truss data is "wired in."

The output from the test script is shown in the lower box of Figure 4.7. The output stiffness matches that in Equation (3.12), as can be expected if all code fragments used so far work correctly.

§4.6. Modifying the Master System

Following the assembly process the master stiffness equations $\mathbf{Ku} = \mathbf{f}$ must be modified to account for single-freedom displacement boundary conditions. This is done through the computer-oriented equation modification process outlined in §3.5.2.

Module ModifiedMasterStiffForDBC and ModifiedMasterForcesForDBC carry out this modification for **K** and **f**, respectively. These two modules are listed in the top box of Figure 4.8, along with test statements. The logic of both functions is considerably simplified by assuming that *all prescribed displacements are zero*. That is, the BCs are homogeneous. (The more general case of nonzero prescribed values is treated in Part III of the book.) The module has two arguments:

A list of the prescribed degrees of freedom identified by their global number. For the example truss this list contains three entries: {1, 2, 4}.

K The master stiffness matrix \mathbf{K} produced by the assembly process.

```
ModifiedMasterStiffForDBC[pdof ,K ] := Module[
  {i,j,k,nk=Length[K],np=Length[pdof],Kmod=K},
      For [k=1,k<=np,k++, i=pdof[[k]];
         For [j=1,j<=nk,j++, Kmod[[i,j]]=Kmod[[j,i]]=0];
           Kmod[[i,i]]=1];
   Return[Kmod]
ModifiedMasterForcesForDBC[pdof_,f_] := Module[
  {i,k,np=Length[pdof],fmod=f},
      For [k=1,k<=np,k++, i=pdof[[k]]; fmod[[i]]=0];</pre>
   Return[fmod]
K=Array[Kij,{6,6}]; Print["Assembled master stiffness:"];
Print[K//MatrixForm];
K=ModifiedMasterStiffForDBC[{1,2,4},K];
Print["Master stiffness modified for displacement B.C.:"];
Print[K//MatrixForm];
f=Array[fi,{6}]; Print["Force vector:"]; Print[f];
f=ModifiedMasterForcesForDBC[{1,2,4},f];
Print["Force vector modified for displacement B.C.:"]; Print[f];
Assembled master stiffness:
Kij[1,1] Kij[1,2] Kij[1,3] Kij[1,4] Kij[1,5] Kij[1,6]
 \text{Kij}[2,1] \text{Kij}[2,2] \text{Kij}[2,3] \text{Kij}[2,4] \text{Kij}[2,5] \text{Kij}[2,6]
 Kij[3,1] Kij[3,2] Kij[3,3] Kij[3,4] Kij[3,5] Kij[3,6]
 Kij[4,1] Kij[4,2] Kij[4,3] Kij[4,4] Kij[4,5] Kij[4,6]
 Kij[5,1] Kij[5,2] Kij[5,3] Kij[5,4] Kij[5,5] Kij[5,6]
| Kij[6, 1] Kij[6, 2] Kij[6, 3] Kij[6, 4] Kij[6, 5] Kij[6, 6] |
Master stiffness modified for displacement B.C.:
\begin{pmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 \end{pmatrix}
 0 0 Kij[3, 3] 0 Kij[3, 5] Kij[3, 6]
 0 0 0 1 0
 0 0 Kij[5, 3] 0 Kij[5, 5] Kij[5, 6]
0 0 Kij[6,3] 0 Kij[6,5] Kij[6,6]
Force vector:
{fi[1], fi[2], fi[3], fi[4], fi[5], fi[6]}
Force vector modified for displacement B.C.:
{0,0,fi[3],0,fi[5],fi[6]}
```

FIGURE 4.8. Modules ModifiedMasterStiffForDBC and ModifiedMasterForceForDBC that modify the master stiffness matrix and force vector of a truss to impose displacement BCs.

Test statements (in blue) and test output.

The function clears appropriate rows and columns of **K**, places ones on the diagonal, and returns the modified **K** as function value. The only slightly fancy thing in this module is the use of the *Mathematica* function Length to extract the number of prescribed displacement components: Length [pdof] here will return the value 3, which is the length of the list pdof. Similarly nk=Length [K] assigns 6 to nk, which is the order of matrix **K**. Although for the example truss these values are known *a priori*, the use of Length serves to illustrate a technique that is heavily used in more general code.

Module ModifiedMasterForcesForDBC has similar structure and logic and need not be described in detail. It is important to note that for homogeneous BCs the modules are independent of each other and may be called in any order. On the other hand, if there were nonzero prescribed displacements present the force modification must be done *before* the stiffness modification. This is because stiffness coefficients that are cleared in the latter are needed for modifying the force vector.

```
IntForce2DTwoNodeBar[\,\{\{x1\_,y1\_\}\,,\{x2\_,y2\_\}\}\,,\{Em\_,A\_\}\,,eftab\_,u\_]\,:=\,
  Module[ \{c,s,dx=x2-x1,dy=y2-y1,L,ix,iy,jx,jy,ubar,e\},
    L=Sqrt[dx^2+dy^2]; c=dx/L; s=dy/L; {ix,iy,jx,jy}=eftab;
    ubar={c*u[[ix]]+s*u[[iy]],-s*u[[ix]]+c*u[[iy]],
          c*u[[jx]]+s*u[[jy]],-s*u[[jx]]+c*u[[jy]]};
    e=(ubar[[3]]-ubar[[1]])/L; Return[Em*A*e]
p =IntForce2DTwoNodeBar[{{0,0},{10,10}},{100,2*Sqrt[2]},
         {1,2,5,6},{0,0,0,0,0.4,-0.2}];
Print["Member int force (numerical):"]; Print[N[p]];
p =IntForce2DTwoNodeBar[{{0,0},{L,L}},{Em,A},
         {1,2,5,6},{0,0,0,0,ux3,uy3}];
Print["Member int force (symbolic):"]; Print[Simplify[p]];
Member int force (numerical):
2.82843
Member int force (symbolic):
A Em (ux3 + uy3)
      2 L
```

FIGURE 4.9. Module IntForce2DTwoNodeBar for computing the internal force in a bar element. Test statements (in blue) and test output.

The test statements are purposedly chosen to illustrate another feature of *Mathematica*: the use of the Array function to generate subscripted symbolic arrays of one and two dimensions. The test output is shown in the bottom box of Figure 4.8, which should be self explanatory. The force vector and its modified form are printed as row vectors to save space.

§4.7. Recovering Internal Forces

Since *Mathematica* provides built-in matrix operations for solving a linear system of equations and multiplying matrices by vectors, we do not need to write application functions for the solution of the modified stiffness equations, and for the recovery of nodal forces as $\mathbf{f} = \mathbf{K}\mathbf{u}$. Consequently, the last application functions we need are those for internal force recovery.

Function IntForce2DTwoNodeBar listed in the top box of Figure 4.9 computes the internal force in an individual bar element. It is somewhat similar in argument sequence and logic to ElemStiff2DTwoNodeBar of Figure 4.5. The first two arguments are identical. Argument eftab provides the Element Freedom Table array for the element. The last argument, u, is the vector of computed node displacements.

The logic of IntForce2DTwoNodeBar is straightforward and follows the method outlined in §3.2.1. Member joint displacements $\bar{\mathbf{u}}^{(e)}$ in local coordinates $\{\bar{x}, \bar{y}\}$ are recovered in array ubar, then the longitudinal strain $e = (\bar{u}_{xj} - \bar{u}_{xi})/L$ and the internal (axial) force p = EAe is returned as function value. As coded the function contains redundant operations because entries 2 and 4 of ubar (that is, components \bar{u}_{yi} and \bar{u}_{yj}) are not actually needed to get p, but were kept to illustrate the general backtransformation of global to local displacements.

Running this function with the test statements shown after the module produces the output shown in the bottom box of Figure 4.9. The first test is for member (3) of the example truss using the actual nodal displacements (3.17). It also illustrates the use of the *Mathematica* built in function N to produce output in floating-point form. The second test does a symbolic calculation in which several argument values are fed in variable form.

FIGURE 4.10. Module IntForceOfExampleTruss that computes internal forces in the 3 members of the example truss. Test statements (in blue) and test output.

The top box of Figure 4.10 lists a higher-level function, IntForceOfExampleTruss, which has a single argument: u. This is the complete 6-vector of joint displacements u. This function calls IntForce2DTwoNodeBar three times, once for each member of the example truss, and returns the three member internal forces thus computed as a 3-component list.

The test statements listed after IntForcesOfExampleTruss feed the displacement solution (3.24) to the module. Running the test produces the output shown in the lower box of Figure 4.10. The internal forces are $F^{(1)} = 0$, $F^{(2)} = -1$ and $F^{(3)} = 2\sqrt{2} = 2.82843$, in agreement with the values found in §3.4.2.

§4.8. Putting the Pieces Together

After all this development and testing effort, documented in Figures 4.5 through 4.10, we are ready to make use of all these bits and pieces of code to analyze the example plane truss. This is actually done with the logic shown in Figure 4.11. This particular piece of code is called the *driver script*. Note that it is *not* a Module. It uses the seven previously described modules

```
ElemStiff2DTwoNodeBar
MergeElemIntoMasterStiff
AssembleMasterStiffOfExampleTruss
ModifiedMasterStiffForDBC
ModifiedMasterForcesForDBC
IntForce2DTwoNodeTruss
IntForcesOfExampleTruss
```

(4.1)

These functions must have been defined ("compiled") at the time the driver scripts described below are run. A simple way to making sure that all of them are defined is to put all these functions in the same Notebook file and to mark them as *initialization cells*. These cells may be executed by picking up Kernel \rightarrow Initialize \rightarrow Execute Initialization.¹³ (An even simpler scheme would to group them all in one cell, but that would make placing separate test statements messy.)

For a hierarchical version of (4.1), see the last CAETE slide.

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¹³ In *Mathematica* versions 6 and higher, there is an initialization shortcut.

```
f={0,0,0,0,2,1};
K=AssembleMasterStiffOfExampleTruss[];
Kmod=ModifiedMasterStiffForDBC[{1,2,4},K];
fmod=ModifiedMasterForcesForDBC[{1,2,4},f];
u=Simplify[Inverse[Kmod].fmod];
Print["Computed nodal displacements:"]; Print[u];
f=Simplify[K.u];
Print["External node forces including reactions:"]; Print[f];
p=Simplify[IntForcesOfExampleTruss[u]];
Print["Internal member forces:"]; Print[p];
Computed nodal displacements:
\left\{0, 0, 0, 0, \frac{2}{5}, -\frac{1}{5}\right\}
External node forces including reactions:
\{-2, -2, 0, 1, 2, 1\}
Internal member forces:
\{0, -1, 2\sqrt{2}\}
```

FIGURE 4.11. Driver script for numerical analysis of example truss and its output.

§4.8.1. The Driver Script

The code listed in the top box of Figure 4.11 first assembles the master stiffness matrix through AssembleMasterStiffOfExampleTruss. Next, it applies the displacement boundary conditions through ModifiedMasterStiffForDBC and ModifiedMasterForcesForDBC. Note that the modified stiffness matrix is placed into Kmod rather than K to save the original form of the master stiffness for the reaction force recovery later. The complete displacement vector is obtained by the matrix calculation

This statement takes advantage of two built-in *Mathematica* functions. Inverse returns the inverse of its matrix argument.¹⁴ The dot operator denotes matrix multiplication (here, matrix-vector multiply.) The enclosing Simplify function in Figure 4.11 simplifies the expression of vector u in case of symbolic calculations; it is actually redundant if all computations are numerical.

The remaining statements recover the node vector including reactions via the matrix-vector multiply f = K.u (recall that K contains the unmodified master stiffness matrix) and the member internal forces p through IntForcesOfExampleTruss. The code prints u, f and p as row vectors to save space.

Running the script of the top box of Figure 4.11 produces the output shown in the bottom box of that figure. The results confirm the hand calculations of Chapter 3.

§4.8.2. Is All of This Worthwhile?

At this point you may wonder whether all of this work is worth the trouble. After all, a hand calculation (typically helped by a programable calculator) would be quicker in terms of flow time. Typing and debugging the *Mathematica* fragments displayed here took the writer about six hours (although about two thirds of this was spent in editing and getting the fragment listings into the Chapter.) For larger problems, however, *Mathematica* would certainly beat hand-plus-calculator

This is a highly inefficient way to solve $\mathbf{K}\mathbf{u} = \mathbf{f}$ if the linear system becomes large. It is done here for simplicity.

```
f = \{0,0,0,0,fx3,fy3\}; \\ K = Assemble Master Stiff for Example Truss[]; \\ Kmod = Modified Master Stiff For DBC[\{1,2,4\},K]; \\ fmod = Modified Master Forces For DBC[\{1,2,4\},f]; \\ u = Simplify [Inverse [Kmod].fmod]; \\ Print["Computed nodal displacements:"]; Print[u]; \\ f = Simplify [K.u]; \\ Print["External node forces including reactions:"]; Print[f]; \\ p = Simplify [Int Forces Of Example Truss[u]]; \\ Print["Internal member forces:"]; Print[p]; \\ \\ Computed nodal displacements: \\ \left\{0,0,0,0,\frac{1}{10}(3 fx3-2 fy3),\frac{1}{5}(-fx3+fy3)\right\} \\ External node forces including reactions: \\ \left\{-fx3,-fx3,0,fx3-fy3,fx3,fy3\right\} \\ Internal member forces: \\ \left\{0,-fx3+fy3,\sqrt{2} fx3\right\}
```

FIGURE 4.12. Driver script for symbolic analysis of example truss and its output.

computations, the cross-over typically appearing for 10 to 20 equations. For up to about 500 equations and using floating-point arithmetic, *Mathematica* gives answers within minutes on a fast PC or Mac with sufficient memory but eventually runs out of steam at about 1000 equations. For a range of 1000 to about 50000 equations, *Matlab*, using built-in sparse solvers, would be the best compromise between human and computer flow time. Beyond 50000 equations a program in a low-level language, such as C or Fortran, would be most efficient in terms of computer time. ¹⁵

One distinct advantage of computer algebra systems emerges when you need to *parametrize* a small problem by leaving one or more problem quantities as variables. For example suppose that the applied forces on node 3 are to be left as f_{x3} and f_{y3} . You replace the last two components of array p as shown in the top box of Figure 4.12, execute the cell and shortly get the symbolic answer shown in the bottom box of that figure. This is the answer to an infinite number of numerical problems. Although one may try to undertake such studies by hand, the likelyhood of errors grows rapidly with the complexity of the system. Symbolic manipulation systems can amplify human abilities in this regard, as long as the algebra "does not explode" because of combinatorial complexity. Examples of such nontrivial calculations will appear throughout the following Chapters.

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¹⁵ The current record for FEM structural applications is about 100 million equations, done on a massively parallel super-computer (ASCI Red at SNL). Fluid mechanics problems with over 500 million equations have been solved.

Notes and Bibliography

The hefty *Mathematica Book* [814] is a reference manual. Since the contents are available online (click on **Help** in topbar), buying the printed book makes little sense. ¹⁶

The best available tutorial to learn the basic (procedural) level of *Mathematica* is *Schaum's Outlines: Mathematica*, McGraw-Hill, New York, second edition 2009. Covers up to version 7.0. Contains 750 exercises with answers. Can be inexpensively purchased from Amazon at www.amazon.com. Learning that level, which is sufficient for this course, should take only a few hours.

There is a another nice tutorial by Glynn and Gray [310], list: \$35, dated 1999. (Theodore Gray invented the Notebook front-end that appeared in version 2.2.) It has the disadvantage of covering only up to fersion 3.0 and may be difficult to find as used book. More up to date and comprehensive is the recent appeared *Mathematica Navigator* in two volumes [657,658]; list: \$69.95 but used copies are discounted, down to about \$20. The recently appeared *Mathematica Cookbook* by S. Mangano, published by O'Reilly, is being reviewed as of this writing; if useful, it will be placed in the reference list. This one covers more advanced programmin techniques that are not needed for this course.

Beyond these, there are many books at all levels that expound on the use of *Mathematica* for various applications ranging from pure mathematics to physics and engineering. A web search on September 2003 done on www3.addall.com hit 150 book titles containing *Mathematica*, compared to 111 for *Maple* and 148 for *Matlab*. A google search (August 2005) hits 3,820,000 pages containing *Mathematica*, but here *Matlab* wins with 4,130,000.

Wolfram Research hosts the MathWorld web site at http://mathworld.wolfram.com, maintained by Eric W. Weisstein. It is essentially a hyperlinked, on-line version of his *Encyclopædia of Mathematics* [796].

To close the topic of symbolic versus numerical computation, here is a nice summary by A. Grozin, posted on the Usenet:

"Computer Algebra Systems (CASs) are programs [that] operate with formulas. Mathematica is a powerful CAS (though quite expensive). Other CASs are, e.g., Maple, REDUCE, MuPAD <...>. There are also quite powerful free CASs: Maxima and Axiom. In all of these systems, it is possible to do some numerical calculations (e.g., to evaluate the formula you have derived at some numerical values of all parameters). But it is a very bad idea to do large-scale numerical work in such systems: performance will suffer. In some special cases (e.g., numerical calculations with very high precision, impossible at the double-precision level), you can use Mathematica to do what you need, but there are other, faster ways to do such things.

There are a number of programs to do numerical calculations with usual double-precision numbers. One example is Matlab; there are similar free programs, e.g., Octave, Scilab, R, ... Matlab is very good and fast in doing numerical linear algebra: if you want to solve a system of 100 linear equations whose coefficients are all numbers, use Matlab; if coefficients contain letters (symbolic quantities) and you want the solution as formulas, use Mathematica or some other CAS. Matlab can do a limited amount of formula manipulations using its "symbolic toolbox," which is an interface to a cut-down Maple. It's a pain to use this interface: if you want Maple, just use Maple."

References

Referenced items have been moved to Appendix R.

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The fifth edition, covering version 5, lists for \$49.95 but older editions are heavily discounted on the web, some under\$2. There are no printed manual versions for versions 6 and higher.

Homework Exercises for Chapter 4 Analysis of Example Truss by a CAS

Before doing any of these Exercises, download the *Mathematica* Notebook file ExampleTruss.nb from the course web site. (Go to Chapter 4 Index and click on the link). Open this Notebook file using version 4.1 or later one. The first six cells contain the modules and test statements listed in the top boxes of Figures 4.5 through 4.10. These six are marked as *initialization cells*. Before running driver scripts, they should be executed by picking up Kernel \rightarrow Evaluation \rightarrow Execute Initialization (or equivalent in newer versions).

Verify that the output of those six cells agrees with that shown in the bottom boxes of those figures. Then execute the driver script in Cell 7 by clicking on the cell and pressing the appropriate key: <Enter> on a Mac, <Shift-Enter> on a Windows PC. Compare the output with that shown in 4.11. If the output checks out, you may proceed to the Exercises.

EXERCISE 4.1 [C:10] Explain why the Simplify command in the test statements of Figure 4.5 says L>0. (One way to figure this out is to just say Ke=Simplify[Ke] and look at the output. Related question: why does Mathematica refuse to simplify Sqrt[Lexp2] to L unless one specifies the sign of L in the Simplify command?

EXERCISE 4.2 [C:10] Explain the logic of the For loops in the merge function MergeElemIntoMasterStiff of Figure 4.6. What does the operator += do? (If you are a C programmer, all of this should be easy.)

EXERCISE 4.3 [C:10] Explain the reason behind the use of Length in the modules of Figure 4.6. Why not simply set nk and np to 6 and 3, respectively?

EXERCISE 4.4 [C:15] Of the seven modules listed in Figures 4.5 through 4.10, with names collected in (4.1), two can be used only for the example truss, three can be used for any plane truss, and two can be used for other structures analyzed by the DSM. Identify which ones, and briefly state the reasons for your classification.

EXERCISE 4.5 [C:20] Modify the modules AssembleMasterStiffOfExampleTruss, IntForcesOfExampleTruss, and the driver script of Figure 4.11, to solve numerically the three-node, two-member truss of Exercise 3.6. Verify that the output reproduces the solution given for that problem. Procedural

recommendation: modify cells but keep a copy of the original Notebook handy in case things go wrong.

EXERCISE 4.6 [C:25] Expand the logic of ModifiedMasterForcesForDBC to permit specified nonzero displacements. Specify these in a second argument called pval, which contains a list of prescribed values paired with pdof.

```
xynode={{0,0},{10,0},{10,10}}; elenod={{1,2},{2,3},{3,1}};
unode={{0,0},{0,0},{2/5,-1/5}}; amp=5; p={};
For [t=0,t<=1,t=t+1/5,
    For [e=1,e<=Length[elenod],e++, {i,j}=elenod[[e]];
    xyi=xynode[[i]];ui=unode[[i]];xyj=xynode[[j]];uj=unode[[j]];
    p=AppendTo[p,Graphics[Line[{xyi+amp*t*ui,xyj+amp*t*uj}]]];
    ];
];
Show[p,Axes->False,AspectRatio->Automatic];
```

FIGURE E4.1. Mystery scriptfor Exercise 4.7.

EXERCISE 4.7 [C:20] Explain what the program of Figure E4.1 does, and the logic behind what it does. (You may want to put it in a cell and execute it.) What modifications would be needed so it can be used for any plane struss?