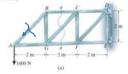
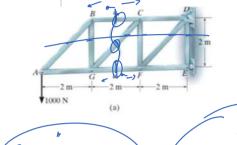
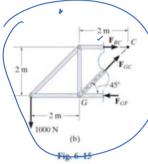
280 CHAPTER 6 STRUCTURAL ANALYSIS

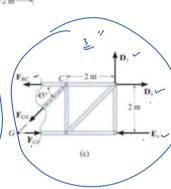
## 6.4 The Method of Sections

When we need to find the force in only a few members of a truss, we can analyze the truss using the method of sections [It is based on the principle that if the truss is in equilibrium then any segment of the truss is also in equilibrium. For example, consider the two truss members shown on the left in Fig. 6-14. If the forces within the members are to be determined, then an imaginary section, indicated by the blue line, can be used to cut each member into two parts and thereby "expose" each internal force as "external" to the free-body diagrams shown on the right. Clearly, it can be seen that equilibrium requires that the member in tension (T) be subjected to a "pull," whereas the member in compression (C) is subjected to a "push." [The method of sections can also be used to "cut" or section the members of an entire truss. If the section passes through the truss and the free-body diagram of internal force as "experiment of the section passes through the truss and the free-body diagram of internal force only three independent equilibrium gequations (\$F\_L = 0.\$ \( \text{LP}\_Q = 0.\) \$\( \text{S}\_Q = 0.\) \$







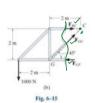


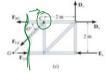
When applying the equilibrium equations, we should carefully consider ways of writing the equations so as to yield a direct solution for each of the unknowns, rather than having to solve simultaneous equations. For example, using the truss segment in Fig. 6-15b and summing moments about C would yield a direct solution for  $F_{GF}$  since  $F_{BC}$  and  $F_{GC}$  create zero moment about C. Likewise,  $F_{BC}$  can be directly obtained by summing moments about C. Finally,  $F_{GC}$  can be found directly from a force summation in the vertical direction since  $F_{GF}$  and  $F_{GC}$  reaction overtical components. This ability to determine directly the force in a particular truss member is one of the main advantages of using the method of sections."

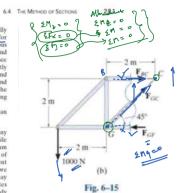
As in the method of joints, there are two ways in which we can determine the correct sense of an unknown member force:

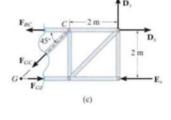
- The correct sense of an unknown member force:
   The correct sense of an unknown member force can in many cases be determined "by inspection." For example, F<sub>BC</sub> is a tensile force as represented in Fig. 6-15b since moment equilibrium about G requires that F<sub>BC</sub> create a moment opposite to that of the 1000-N force. Also, F<sub>BC</sub> is tensile since its vertical component must balance the 1000-N force which acts downward. In more complicated cases, the sense of an unknown member force may be assumed. If the solution yields a negative scalar, it indicates that the force's sense is opposite to that shown on the free-body diagram.
- Always assume that the unknown member forces at the cut section
  are tensile forces, i.e., "pulling" on the member. By doing this, the
  numerical solution of the equilibrium equations will yield positive
  scalars for members in tension and negative scalars for members in
  compression.

\*Notice that if the method of joints were used to determine, say, the force in member GC, it would be necessary to analyze joints A,B, and G in sequence.











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# Procedure for Analysis

The forces in the members of a truss may be determined by the method of sections using the following procedure.

Tree-Body Diagram.

Make a decision on how to "cut" or section the truss through the members where forces are to be determined.

Before isolating the appropriate section, it may first be necessary to determine the truss's support reactions. If this is done then the three equilibrium equations will be available to solve for member forces at the section.

Draw the free-body diagram of that segment of the sectioned truss which has the least number of forces acting on it.

Use one of the two methods described above for establishing the sense of the unknown member forces.

Equations of Equilibrium equations will be a supported to the two methods described above for establishing the sense of the unknown member forces.

sense of the unknown member forces.

Equations of Equilibrium.

• Moments should be summed about a point that lies at the intersection of the lines of action of two unknown forces, so that the third unknown force can be determined directly from the moment equation.

If I two of the unknown forces are parallel, forces may be summed perpendicular to the direction of these unknowns to determine directly the third unknown force.

## EXAMPLE 6.5

Determine the force in members GE, GC, and BC of the truss shown in Fig. 6–16 $\alpha$ . Indicate whether the members are in tension or

SOLUTION
Section as in Fig. 6-16a has been chosen since it cuts through the three members whose forces are to be determined. In order to use the method of sections, however, it is first necessary to determine the external reactions at A or D. Why? A free-body diagram of the entire truss is shown in Fig. 6-16b. Applying the equations of equilibrium, we have

we have  

$$\Rightarrow \Sigma F_x = 0;$$
  $400 \text{ N} - A_x = 0$   $A_y = 400 \text{ N}$   
 $\zeta + \Sigma M_A = 0;$   $-1200 \text{ N}(8 \text{ m}) - 400 \text{ N}(3 \text{ m}) + D_y(12 \text{ m}) = 0$   
 $D_z = 900 \text{ N}$ 

$$D_y = 900 \text{ N}$$
+ †  $\Sigma F_y = 0$ ;  $A_y - 1200 \text{ N} + 900 \text{ N} = 0$   $A_y = 300 \text{ N}$ 

Free-Body Diagram. For the analysis the free-body diagram of the left portion of the sectioned truss will be used, since it involves the least number of forces, Fig. 6-16c.

**Equations of Equilibrium.** Summing moments about point G eliminates  $\mathbf{F}_{GE}$  and  $\mathbf{F}_{GC}$  and yields a direct solution for  $F_{BC}$ .

$$\zeta + \Sigma M_G = 0;$$
  $-300 \text{ N}(4 \text{ m}) - 400 \text{ N}(3 \text{ m}) + F_{BC}(3 \text{ m}) = 0$   
 $F_{BC} = 800 \text{ N} \quad \text{(T)}$  Ans.

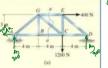
In the same manner, by summing moments about point C we obtain a direct solution for  $F_{GE}$ 

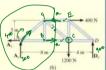
$$\zeta + \Sigma M_C = 0;$$
  $-300 \text{ N}(8 \text{ m}) + F_{GE}(3 \text{ m}) = 0$   
 $F_{GE} = 800 \text{ N} \quad \text{(C)}$  Am

Since  $\mathbf{F}_{BC}$  and  $\mathbf{F}_{GE}$  have no vertical components, summing forces in the y direction directly yields  $F_{GC}$ , i.e.,

$$+\uparrow \Sigma F_y = 0;$$
  $300 \text{ N} - \frac{3}{3}F_{GC} = 0$   $F_{GC} = 500 \text{ N}$  (T)

NOTE: Here it is possible to tell, by inspection, the proper direction for each unknown member force. For example,  $\Sigma M_C=0$  requires  $F_{GE}$  to be compressive because it must balance the moment of the 300-N force about C.







Expect health, as they high body and gap apply equilibrium equality  $\frac{1}{2}$  HA  $\circ$  ) 1 (po  $\times$  3)  $\frac{1}{2}$  1200  $\times$  8 - Dy  $\times$  12 = 0 Dy  $\times$  200 N  $\times$  1400 A  $\times$  3 A  $\times$  2 400 N  $\times$  1400 A  $\times$  3 A  $\times$  2 400 N  $\times$  2 Fy = 0  $\times$  4 A  $\times$  2 Du = 0  $\times$  2 A  $\times$  3 Se N

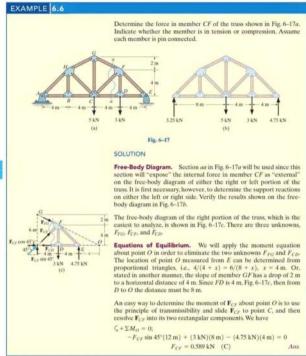
219=0

£Mc=0 2+ (300 x 8) - FGE x3 = 0 => FGE 800N(1) (300 x x / 1

2 f(4 = 0 2 + 1) - fsc x 3 = 0

1 f8 c = 8 00 N(T) 2/13 = 0 (++) 300 - 141 (3) = 0 3 fq = 500N(T)

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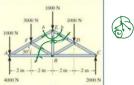


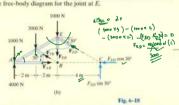
## EXAMPLE 6.7

Determine the force in member EB of the roof truss shown in Fig. 6–18 $\alpha$ . Indicate whether the member is in tension or compression.

### SOLUTION

Free-Body Diagrams. By the method of sections, any imaginary section that cuts through EB, Fig, 6–18a, will also have to cut through three other members for which the forces are unknown. For example, section are cuts through ED, EB, FB, and AB. If a free-body diagram of the left side of this section is considered. Fig, 6–18b, it is possible to obtain  $F_{EB}$  by summing moments about B to eliminate the other three unknowns; however,  $F_{EB}$  cannot be determined from the remaining two equilibrium cquations. One possible way of obtaining  $F_{EB}$  is first to determine  $F_{ED}$  from section ab, fig–6-18c, which is shown in Fig–6-18c, which is shown in Fig–6-18c. Where the force system is concurrent and our sectioned free-body diagram is the same as the free-body diagram for the joint at E.





2Fx=0

7=2000 N(4) 25/3=0 | 500 | 500 | -1000 | + 500 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 3000 N(X)

**Equations of Equilibrium.** In order to determine the moment of  $F_{ED}$  about point B, Fig. 6-18b, we will use the principle of transmissibility and slide the force to point C and then resolve it into its rectangular components as shown. Therefore,

$$\zeta + \Sigma M_B = 0; \qquad 1000 \ {\rm N}(4\ {\rm m}) + 3000 \ {\rm N}(2\ {\rm m}) - 4000 \ {\rm N}(4\ {\rm m}) \\ + F_{ED} \sin 30^\circ (4\ {\rm m}) = 0 \\ F_{ED} = 3000 \ {\rm N} \quad ({\rm C})$$

Considering now the free-body diagram of section bb, Fig. 6-18c, we have

$$\begin{array}{lll} \Rightarrow \Sigma F_x = 0; & F_{EF} \cos 30^{\circ} - 3000 \cos 30^{\circ} \text{N} = 0 \\ F_{EF} = 3000 \text{ N} & (\text{C}) \\ + \uparrow \Sigma F_y = 0; & 2(3000 \sin 30^{\circ} \text{N}) - 1000 \text{ N} - F_{EB} = 0 \\ F_{EB} = 2000 \text{ N} & (\text{T}) \end{array}$$