

# Geomatics Guidance Note 7, part 2 Coordinate Conversions & Transformations including Formulas



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**Geomatics  
Guidance Note Number 7, part 2**

**Coordinate Conversions and Transformations including Formulas**

**Revised - February 2019**

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## **Preface**

The EPSG Geodetic Parameter Dataset, abbreviated to the **EPSG Dataset**, is a repository of parameters required to:

- define a *coordinate reference system* (CRS) which ensures that coordinates describe position unambiguously.
- define transformations and conversions that allow coordinates to be changed from one CRS to another CRS. Transformations and conversions are collectively called *coordinate operations*.

The EPSG Dataset is maintained by the IOGP Geomatics Committee's Geodesy Subcommittee. It conforms to ISO 19111:2007 – *Spatial referencing by coordinates*<sup>1</sup>. It is distributed in three ways:

- the **EPSG Registry**, in full the *EPSG Geodetic Parameter Registry*, a web-based delivery platform in which the data is held in GML using the CRS entities described in ISO 19136.
- the **EPSG Database**, in full the *EPSG Geodetic Parameter Database*, a relational database structure where the entities which form the components of CRSs and coordinate operations are in separate tables, distributed as an MS Access database;
- in a relational data model as **SQL scripts** which enable a user to create an Oracle, MySQL, PostgreSQL or other relational database and populate that database with the EPSG Dataset;

IOGP Geomatics Guidance Note 7 is a multi-part document for users of the EPSG Dataset.

- *Part 1, Using the Dataset*, sets out detailed information about the Dataset and its content, maintenance and terms of use.
- *Part 2, Formulas*, (this document), provides a detailed explanation of formulas necessary for executing coordinate conversions and transformations using the coordinate operation methods supported in the EPSG Dataset. Geodetic parameters in the Dataset are consistent with these formulas.
- *Part 3, Registry Developer Guide*, is primarily intended to assist computer application developers who wish to use the API of the Registry to query and retrieve entities and attributes from the dataset.
- *Part 4, Database Developer Guide*, is primarily intended to assist computer application developers who wish to use the Database or its relational data model to query and retrieve entities and attributes from the dataset.

The complete texts may be found at <http://www.epsg.org/GuidanceNotes>. The terms of use of the dataset are also available at <http://www.epsg.org/TermsOfUse>.

In addition to these documents, the Registry's user interface contains online help and the Database user interface includes context-sensitive help accessed by left-clicking on any label.

This Part 2 of the multipart Guidance Note is primarily intended to assist computer application developers in using the coordinate operation methods supported by the EPSG Dataset. It may also be useful to other users of the data. Map projection methods are described in section 3 below. Other coordinate conversions, transformations and point motion operations are described in section 4.

Further test data to supplement the examples in this document may be found in IOGP Publication 430, Guidelines for Geospatial Integrity of Geoscience Software (GIGS), part 3, Test Dataset.

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<sup>1</sup> Conformance will be updated to ISO 19111:2019 during 2019.

## 1 **Implementation notes**

### 1.1 **Ellipsoid parameters**

In the formulas in this Guidance Note, the basic ellipsoidal parameters are represented by symbols and derived as follows:

#### **Primary ellipsoid parameters**

<u>Parameter Name</u>	<u>Symbol</u>	<u>Description</u>
semi-major axis	a	Length of the semi-major axis of the reference ellipsoid, the radius of the equator.
semi-minor axis	b	Length of the semi-minor axis of the reference ellipsoid, the distance along the ellipsoid axis between equator and pole.
inverse flattening	1/f	$= a/(a - b)$

#### **Derived ellipsoid parameters**

<u>Parameter Name</u>	<u>Symbol</u>	<u>Description</u>
flattening	f	$= 1 / (1/f)$
eccentricity	e	$= \sqrt{(2f - f^2)}$
second eccentricity	e'	$= \sqrt{[e^2 / (1 - e^2)]}$
radius of curvature in the meridian	$\rho$	radius of curvature of the reference ellipsoid in the plane of the meridian at latitude $\phi$ , where $\rho = a(1 - e^2)/(1 - e^2 \sin^2 \phi)^{3/2}$
radius of curvature in the prime vertical	$\nu$	radius of curvature of the reference ellipsoid perpendicular to the meridian at latitude $\phi$ , where $\nu = a / (1 - e^2 \sin^2 \phi)^{1/2}$
radius of authalic sphere	$R_A$	radius of sphere having same surface area as the ellipsoid. $R_A = a * [(1 - \{(1 - e^2) / (2e)\}) * \{LN[(1 - e) / (1 + e)]\}) * 0.5]^{1/2}$
radius of conformal sphere	$R_C$	$= \sqrt{(\rho \nu)} = [a \sqrt{(1 - e^2) / (1 - e^2 \sin^2 \phi)}]$ This is a function of latitude and therefore not constant. When used for spherical map projections the use of $\phi_0$ (or $\phi_1$ as relevant to method) for $\phi$ is suggested, except if the map projection is equal area when $R_A$ (see above) should be used.

### 1.2 **Arctangent function**

Unless there is a note to the contrary, and if the expression for which the arctangent is sought has a numerator over a denominator, the formulas are arranged so that the atan2 function should be used. For atan(y/x), the arctangent is normally returned as an angle between  $-\pi/2$  and  $+\pi/2$  (between  $-90^\circ$  and  $+90^\circ$ ), whereas the atan2(y,x) output accounts for the quadrant resulting in output between  $-\pi$  and  $+\pi$  ( $-180^\circ$  and  $+180^\circ$ ).

Readers should note that using the atan2 function the order of the arguments for atan(y/x) varies across different programming languages and tools. This Guidance Note uses the convention atan2(y,x) for atan(y/x).

Conditions not resolved by the atan2 function, but requiring adjustment for almost any program, are as follows:

- 1) If A and B are both zero, the arctangent is indeterminate, but may normally be given an arbitrary value of 0, and
- 2) If A or B is infinite, the arctangent is  $\pm\pi/2$  ( $\pm 90^\circ$ ), the sign depending on other conditions. In any case, the final longitude should be adjusted, if necessary, so that it is an angle between  $-\pi$  (or  $-180^\circ$ ) and  $+\pi$  (or  $+180^\circ$ ). This is done by adding or subtracting multiples of  $360^\circ$  (or  $2\pi$ ) as required (also see above longitude 'wrap-around').



### **1.3 Angular units**

All angles are assumed to be in radians unless otherwise stated.

### **1.4 Longitude 'wrap-around'**

The formulas in this Guidance Note assume longitudes are described using the range  $-\pi$  to  $+\pi$  radians ( $-180 \leq \lambda \leq +180$  degrees). If the area of interest crosses the  $180^\circ$  meridian and an alternative longitude range convention is being used, longitudes need to be converted to fall into this  $-180 \leq \lambda \leq +180$  degrees range. This may be achieved by applying the following:

If  $(\lambda - \lambda_0) \leq -180^\circ$  then  $\lambda = \lambda + 360^\circ$ . This may be required when  $\lambda_0 > 0^\circ$ .

If  $(\lambda - \lambda_0) \geq 180^\circ$  then  $\lambda = \lambda - 360^\circ$ . This may be required when  $\lambda_0 < 0^\circ$ .

In the formulas in this Guidance Note the symbol  $\lambda_C$  or  $\lambda_F$  may be used rather than  $\lambda_0$ , but the same principle applies.

### **1.5 Offsets**

Several transformation methods which utilise offsets in coordinate values are recognised. The offset methods may be in n-dimensions. These include longitude rotations, geographic CRS ellipsoidal coordinate offsets, Cartesian grid offsets and vertical offsets.

Mathematically, if the origin of a one-dimensional coordinate system is shifted along the positive axis and placed at a point with ordinate A, then the transformation formula is:

$$X_{\text{new}} = X_{\text{old}} - A$$

However it is common practice in coordinate reference system transformations to apply the shift as an addition, with the sign of the shift parameter value having been suitably reversed to compensate for the practice. This practice has been adopted for the EPSG Dataset. Hence transformations allow calculation of coordinates in the target system by adding a correction parameter to the coordinate values of the point in the source system:

$$X_t = X_s + A$$

where  $X_s$  and  $X_t$  are the values of the coordinates in the source and target coordinate systems and A is the value of the transformation parameter to transform source coordinate reference system coordinate to target coordinate reference system coordinate.

Offset methods are reversible. For the reverse transformation, the offset parameter value is applied with its sign reversed.

### **Example**

Examples of input and output for the Longitude Rotation method (EPSG method code 9601) may be found in test procedure 5208 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### **1.6 Coordinate transformation direction**

Note that it is very important to ensure that the signs of the parameter values used in the transformations are correct in respect of the transformation being executed. Preferably one should always express transformations in terms of "From"....."To"..... thus avoiding the confusion which may result from interpreting a dash as a minus sign or vice versa.

## **2 Coordinate Conversions and Transformations - Introduction**

### **2.1 Basic concepts**

Coordinate conversions and coordinate transformations change coordinate values in one coordinate reference system to coordinate values in another coordinate reference system. A **coordinate system** is a set of mathematical rules for specifying how coordinates are to be assigned to points. It includes the definition of the coordinate axes, the units to be used and the geometry of the axes. A coordinate system is an abstract concept, unrelated to the Earth. A coordinate system is related to the Earth through a **datum**. The combination of coordinate system and datum is a **coordinate reference system (CRS)**. If a different datum is used, the coordinates of a point to change. Colloquially the term coordinate system has historically been used to mean coordinate reference system.

Coordinates may be changed from one coordinate reference system to another through the application of a **coordinate operation**. Two types of coordinate operation may be distinguished:

- **coordinate conversion**, where no change of datum is involved and the parameters are chosen and thus error free.
- **coordinate transformation**, where the target CRS is based on a different datum to the source CRS. Transformation parameters are empirically determined and thus subject to measurement errors.

Certain coordinate operation methods do not readily fit the ISO 19111 classification of being either a coordinate conversion (no change of datum involved) or a coordinate transformation. These methods (for example polynomials and affine transformations) change coordinates directly from one coordinate reference system to another and may be applied with or without change of datum, depending upon whether the source and target coordinate reference systems are based on the same or different datums. EPSG follows the general mathematical usage of these methods and classifies them as transformations. In practice, most usage of these methods does in fact include a change of datum.

Coordinates may also change within a coordinate reference system due to plate motion or other tectonic activity. **Plate motion operations** describe this change of coordinate value with time. It is emphasised that this change is within a coordinate reference system, in contrast with coordinate conversions and coordinate transformations, which are between coordinate reference systems. However, for convenience of using the same data model, point motion operations are modelled as if they were a type of coordinate operation.

Geographic coordinates (latitude and longitude) are calculated on a model of the Earth. The definition of the model is part of a geodetic datum definition.

A projected coordinate reference system is the result of the application of a **map projection** to a geographic coordinate reference system. A map projection is a type of coordinate conversion. It uses an identified method with specific formulas and a set of parameters specific to that coordinate conversion method. Map projection formulas are given in the following part of this document.

Some transformation methods operate directly between geographic coordinates. Others are between geocentric coordinates (3-dimensional Cartesian coordinates where the coordinate system origin is at the centre of the earth). The last part of this Guidance Note covers conversions and transformations between geographic coordinate reference systems, both directly and indirectly through geocentric systems. This last part also describes transformations of vertical coordinates.

Coordinate handling software may execute more complicated operations, concatenating a number of steps linking together geographic, projected and/or engineering coordinates referenced to different datums. Other than as mentioned above, these concatenated operations are beyond the scope of this document.

## 2.2 Map Projection methods

Setting aside the large number of map projection methods which may be employed for atlas maps, equally small scale illustrative exploration maps, and wall maps of the world or continental areas, the EPSG Dataset provides reference parameter values for orthomorphic or conformal map projections which are used for medium or large scale topographic or exploration mapping. Here accurate positions are important and sometimes users may wish to scale accurate positions, distances or areas from the maps.

Small scale maps normally assume a spherical earth and the inaccuracies inherent in this assumption are of no consequence at the usual scale of these maps. For medium and large scale sheet map projections, or maps and coordinates held digitally to a high accuracy, it is essential that due regard is paid to the actual shape of the Earth. Such coordinate reference systems are therefore invariably based on an ellipsoid and its derived map projections. The EPSG Dataset and this supporting conversion documentation usually considers only map projection methods for the ellipsoid. Projection formulas for the spherical earth are simpler but the spherical figure is inadequate to represent positional data with great accuracy at large map scales for the real earth. Projections of the sphere are only suitable for illustrative maps at scale of 1:1 million or less where precise positional definition is not critical.

Though not exhaustive the following list of named map projection methods are those which are most frequently encountered for medium and large scale mapping, some of them much less frequently than others since they are designed to serve only one particular country. They are grouped according to their possession of similar construction properties. Except where indicated all are conformal.

Lambert Conical Conformal	Conical
with one standard parallel	
with two standard parallels	
Mercator	Cylindrical
with one standard parallel	
with two standard parallels	
Cassini-Soldner (note: not conformal)	Transverse Cylindrical
Transverse Mercator Group	Transverse Cylindrical
Transverse Mercator (including south oriented version)	
Universal Transverse Mercator	
Gauss-Kruger	
Gauss-Boaga	
Oblique Mercator Group	Oblique Cylindrical
Hotine Oblique Mercator	
Laborde Oblique Mercator	
Stereographic	Azimuthal
Polar	
Oblique and equatorial	

The map projection formulas which follow are largely adapted from "Map Projections - A Working Manual" by J.P.Snyder, published by the U.S. Geological Survey as Professional Paper No.1395<sup>2</sup>. As well as providing an extensive overview of most map projections in current general use, and the formulas for their construction for both the spherical and ellipsoidal earth, this excellent publication provides computational hints and details of the accuracies attainable by the formulas. It is strongly recommended that all those who have to deal with map projections for medium and large scale mapping should follow its guidance.

There are a number of different formulas available in the literature for map projections other than those quoted by Snyder. Some are closed formulas; others, for ease of calculation, may depend on series expansions and their precision will generally depend on the number of terms used for computation. Generally those formulas which follow in this chapter will provide results which are accurate to within a few centimetres, which is normally adequate for exploration mapping purposes. Coordinate expression and computations for engineering operations are usually consistently performed in grid terms.

The importance of one further variable should be noted. This is the unit of linear measurement used in the definition of projected coordinate reference systems. For metric map projections the unit of measurement is restricted to this unit. For non-metric map projections the metric ellipsoid semi-major axis needs to be converted to the projected coordinate reference system linear unit before use in the formulas below. The relevant ellipsoid is obtained through the datum part of the projected coordinate reference system.

#### Reversibility

Different formulas are required for forward and reverse map projection conversions: the forward formula cannot be used for the reverse conversion. However both forward and reverse formulas are explicitly given in the sections below as parts of a single conversion method. As such, map projection methods are described in the EPSG Dataset as being reversible. Forward and reverse formulas for each conversion method use the projection parameters appropriate to that method with parameter values unchanged.

### **2.3 Map Projection parameters**

A map projection grid is related to the geographical graticule of an ellipsoid through the definition of a coordinate conversion method and a set of parameters appropriate to that method. Different conversion methods may require different parameters. Any one coordinate conversion method may take several different sets of associated parameter values, each set related to a particular map projection zone applying to a particular country or area of the world. Before setting out the formulas involving these parameters, which enable the coordinate conversions for the projection methods listed above, it is as well to understand the nature of the parameters.

The plane of the map and the ellipsoid surface may be assumed to have one particular point in common. This point is referred to as the **natural origin**. It is the point from which the values of both the geographic coordinates on the ellipsoid and the grid coordinates on the projection are deemed to increment or decrement for computational purposes. Alternatively it may be considered as the point which in the absence of application of false coordinates has grid coordinates of (0,0). For example, for projected coordinate reference systems using the Cassini-Soldner or Transverse Mercator methods, the natural origin is at the intersection of a chosen parallel and a chosen meridian. The chosen parallel will frequently but not necessarily be the equator. The chosen meridian will usually be central to the mapped area. For the stereographic projection the origin is at the centre of the projection where the plane of the map is imagined to be tangential to the ellipsoid.

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<sup>2</sup> This was originally published with the title "Map Projections Used by the US Geological Survey". In some cases the formulas given are insufficient for global use. In these cases EPSG has modified the formulas. Note that the origin of most map projections is given false coordinates (FE and FN or E<sub>F</sub> and N<sub>F</sub> or Ec and Nc) to avoid negative coordinates. In the EPSG formulas these values are included where appropriate so that the projected coordinates of points result directly from the quoted formulas.

Since the natural origin may be at or near the centre of the projection and under normal coordinate circumstances would thus give rise to negative coordinates over parts of the map, this origin is usually given false coordinates which are large enough to avoid this inconvenience. Hence each natural origin will normally have **False Easting, FE** and **False Northing, FN** values. For example, the false easting for the origins of all Universal Transverse Mercator zones is 500000m. As the UTM origin lies on the equator, areas north of the equator do not need and are not given a false northing but for mapping southern hemisphere areas the equator origin is given a false northing of 10,000,000m, thus ensuring that no point in the southern hemisphere will take a negative northing coordinate. Figure 7 illustrates the UTM arrangements.

These arrangements suggest that if there are false easting and false northing for the real or natural origin, there is also a **Grid Origin** which has coordinates (0,0). In general this point is of no consequence though its geographic position may be computed if needed. For example, for the WGS 84 / UTM zone 31N coordinate reference system which has a natural origin at 0°N, 3°E where false easting is 500000m E (and false northing is 0m N), the grid origin is at 0°N, 1°29'19.478"W. Sometimes however, rather than base the easting and northing coordinate reference system on the natural origin by giving it **FE** and **FN** values, it may be convenient to select a **False Origin** at a specific meridian/parallel intersection and attribute the false coordinates **Easting at False Origin, E<sub>F</sub>** and **Northing at False Origin, N<sub>F</sub>** to this. The related easting and northing of the natural origin may then be computed if required.

Longitudes are most commonly expressed relative to the **Prime Meridian** of Greenwich but some countries, particularly in former times, have preferred to relate their longitudes to a prime meridian through their national astronomic observatory, usually sited in or near their capital city, for example Paris for France, Bogota for Colombia. The meridian of the projection zone origin is known as the **Longitude of Origin**. For certain projection types it is often termed the **Central Meridian** or abbreviated as **CM** and provides the direction of the northing axis of the projected coordinate reference system.

Because of the steadily increasing distortion in the scale of the map with increasing distance from the origin, central meridian or other line on which the scale is the nominal scale of the projection, it is usual to limit the extent of a projection to within a few degrees of latitude or longitude of this point or line. Thus, for example, a UTM or other Transverse Mercator projection zone will normally extend only 2 or 3 degrees from the central meridian. Beyond this area another **zone** of the projection, with a new origin and central meridian, needs to be used or created. The UTM system has a specified 60 numbered zones, each 6 degrees wide, covering the ellipsoid between the 84 degree North and 80 degree South latitude parallels. Other Transverse Mercator projection zones may be constructed with different central meridians, and different origins chosen to suit the countries or states for which they are used. A number of these are included in the EPSG Dataset. Similarly a Lambert Conic Conformal zone distorts most rapidly in the north-south direction and may, as in Texas, be divided into latitudinal bands.

In order to further limit the scale distortion within the coverage of the zone or projection area, some projections introduce a **scale factor** at the origin (on the central meridian for Transverse Mercator projections), which has the effect of reducing the nominal scale of the map here and making it have the nominal scale some distance away. For example in the case of the UTM and some other Transverse Mercator projections a scale factor of slightly less than unity is introduced on the central meridian thus making it unity on two roughly north-south lines either side of the central one, and reducing its departure from unity beyond these. The scale factor is a required parameter whether or not it is unity and is usually symbolised as **k<sub>0</sub>**.

Thus for projections in the Transverse Mercator group, the parameters which are required to completely and unambiguously define the projection method are:

- Latitude of natural origin ( $\phi_0$ )
- Longitude of natural origin (the central meridian) ( $\lambda_0$ )
- Scale factor at natural origin (on the central meridian) ( $k_0$ )
- False easting (FE)
- False northing (FN)

This set of parameters may also be used by other projection methods. However for some of these other methods, including the Lambert Conic Conformal, the projection may be defined using alternative parameters. It is possible to replace the latitude of natural origin and scale factor by instead giving the latitude of the two parallels along which scale is true, the ‘standard parallels’. This results in the following set of parameters, which we call variant B:

- Latitude of first standard parallel ( $\phi_1$ )
- Latitude of second standard parallel ( $\phi_2$ )
- Longitude of natural origin (the central meridian) ( $\lambda_o$ )
- False easting (FE)
- False northing (FN)

For the Lambert Conic Conformal projection on the ellipsoid the single parallel at which scale is a minimum – the latitude of natural origin – is slightly poleward of midway between the two standard parallels. It is usual to choose values which are round numbers for the two standard parallels, which results in the latitude of natural origin having an irregular value. It is then more convenient to define the false grid coordinates at some other point itself having round number values. This results in the following set of parameters (variant C):

- Latitude of first standard parallel ( $\phi_1$ )
- Latitude of second standard parallel ( $\phi_2$ )
- Latitude of false origin ( $\phi_F$ )
- Longitude of false origin ( $\lambda_F$ )
- False easting ( $E_F$ )
- False northing ( $N_F$ )

The limiting cases of the conic aspect are a cylinder tangential at the equator and a plane tangential at a pole, but the conic formulas should not be used for these cases as some trigonometrical functions break down. Instead different formulas or methods exist. For the Lambert Conic Conformal these are the Mercator and Polar Stereographic methods respectively. For the Mercator projection the two standard parallels have the same latitude in opposite hemispheres. It is therefore only necessary to have one of them as a defining parameter. Similarly in the other limiting case, the Polar Stereographic projection, there is only one standard parallel. In these projection methods, for variant B the set of parameters reduces to:

- Latitude of standard parallel ( $\phi_1$ )
- Longitude of natural origin (the central meridian) ( $\lambda_o$ )
- False easting (FE)
- False northing (FN)

with a similar reduction for variant C.

Variants A and B are alternative ways of defining one projection. Regardless of whether variant A or variant B is used for the projection definition the conversion of geographic coordinates will produce the same grid coordinates. However in the case of variant C the conversion of the same geographic coordinates will produce different grid coordinates to those of variants A and B, except in the special and unusual case when the latitude of the false origin is chosen to exactly coincide with the natural origin.

It is EPSG policy to document the parameters and their values for these variants as they are defined by the information source. To accomplish this EPSG recognises the variants as different methods, each with their own name and formulas:



<u>General Method</u>	<u>Variant A</u>	<u>Variant B</u>	<u>Variant C</u>
Lambert Conic Conformal	Lambert Conic Conformal (1SP)	(not used)	Lambert Conic Conformal (2SP)
Mercator	Mercator (variant A) alias Mercator (1SP)	Mercator (variant B)	Mercator (variant C)
Polar Stereographic	Polar Stereographic (variant A)	Polar Stereographic (variant B)	Polar Stereographic (variant C)

This concept is continued with some other projection methods in which variants are found, but in these the relationship between the alternative sets of parameters may be less straightforward:

<u>General Method</u>	<u>Variant A</u>	<u>Variant B</u>	<u>Variant C</u>
Oblique Mercator	Hotine Oblique Mercator (variant A) aliases Hotine Oblique Mercator, Rectified Skew Orthomorphic	Hotine Oblique Mercator (variant B) aliases Oblique Mercator, Rectified Skew Orthomorphic	Laborde Oblique Mercator alias Laborde Madagascar

For the Oblique Mercator it is also possible to define the azimuth of the initial line through the latitude and longitude of two widely spaced points along that line. This approach is not included in the EPSG Dataset.

The following tables give the definitions of projection parameters used in the projection formulas which follow. For Ellipsoid parameters see the general Implementation Notes in the previous section.

**TABLE 1.**  
**Parameters used in map projection conversions**

Parameter Name	Symbol	Description
Angle from Rectified to Skew Grid	$\gamma_C$	The angle at the natural origin of an oblique projection through which the natural coordinate reference system is rotated to make the projection north axis parallel with true north.
Azimuth of initial line	$\alpha_C$	The azimuthal direction (north zero, east of north being positive) of the great circle which is the centre line of an oblique projection. The azimuth is given at the projection centre.
Central meridian		See Longitude of natural origin
Easting at false origin	$E_F$	The easting value assigned to the false origin.
Easting at projection centre	$E_C$	The easting value assigned to the projection centre.
False easting	$FE$	The value assigned to the abscissa (east or west) axis of the projection grid at the natural origin.
False northing	$FN$	The value assigned to the ordinate (north or south) axis of the projection grid at the natural origin.
False origin		A specific parallel/meridian intersection other than the natural origin to which the grid coordinates $E_F$ and $N_F$ , are assigned.
Grid origin		The point which has coordinates (0,0). It is offset from the natural origin by the false easting and false northing. In some projection methods it may alternatively be offset from the false origin by Easting at false origin and Northing at false origin. In general this point is of no consequence.
Initial line		The line on the surface of the earth model which forms the axis for the grid of an oblique projection.
Initial longitude	$\lambda_I$	The longitude of the western limit of the first zone of a Transverse Mercator zoned grid system.
Latitude of 1st standard parallel	$\phi_1$	For a conic projection with two standard parallels, this is the latitude of one of the parallels at which the cone intersects with the ellipsoid. It is normally but not necessarily that nearest to the pole. Scale is true along this parallel.
Latitude of 2nd standard parallel	$\phi_2$	For a conic projection with two standard parallels, this is the latitude of one of the parallels at which the cone intersects with the ellipsoid. It is normally but not necessarily that nearest to the equator. Scale is true along this parallel.
Latitude of false origin	$\phi_F$	The latitude of the point which is not the natural origin and at which grid coordinate values easting at false origin and northing at false origin are defined.
Latitude of natural origin	$\phi_O$	The latitude of the point from which the values of both the geographic coordinates on the ellipsoid and the grid coordinates on the projection are deemed to increment or decrement for computational purposes. Alternatively it may be considered as the latitude of the point which in the absence of application of false coordinates has grid coordinates of (0,0).
Latitude of projection centre	$\phi_C$	For an oblique projection, this is the latitude of the point at which the azimuth of the initial line is defined.
Latitude of pseudo standard parallel	$\phi_P$	Latitude of the parallel on which the conic or cylindrical projection is based. This latitude is not geographic, but is defined on the conformal sphere AFTER its rotation to obtain the oblique aspect of the projection.



Parameter Name	Symbol	Description
Latitude of standard parallel	$\phi_F$	For polar aspect azimuthal projections, the parallel on which the scale factor is defined to be unity.
Longitude of false origin	$\lambda_F$	The longitude of the point which is not the natural origin and at which grid coordinate values easting at false origin and northing at false origin are defined.
Longitude of natural origin	$\lambda_O$	The longitude of the point from which the values of both the geographic coordinates on the ellipsoid and the grid coordinates on the projection are deemed to increment or decrement for computational purposes. Alternatively it may be considered as the longitude of the point which in the absence of application of false coordinates has grid coordinates of (0,0). Sometimes known as "central meridian (CM)".
Longitude of origin	$\lambda_O$	For polar aspect azimuthal projections, the meridian along which the northing axis increments and also across which parallels of latitude increment towards the north pole.
Longitude of projection centre	$\lambda_C$	For an oblique projection, this is the longitude of the point at which the azimuth of the initial line is defined.
Natural origin		The point from which the values of both the geographic coordinates on the ellipsoid and the grid coordinates on the projection are deemed to increment or decrement for computational purposes. Alternatively it may be considered as the point which in the absence of application of false coordinates has grid coordinates of (0,0). For example, for projected coordinate reference systems using the Transverse Mercator method, the natural origin is at the intersection of a chosen parallel and a chosen central meridian.
Northing at false origin	$N_F$	The northing value assigned to the false origin.
Northing at projection centre	$N_C$	The northing value assigned to the projection centre.
Origin		See natural origin, false origin and grid origin.
Projection centre		On an oblique cylindrical or conical projection, the point at which the direction of the cylinder or cone and false coordinates are defined.
Scale factor at natural origin	$k_O$	The factor by which the map grid is reduced or enlarged during the projection process, defined by its value at the natural origin.
Scale factor on initial line	$k_C$	The factor by which an oblique projection's map grid is reduced or enlarged during the projection process, defined by its value along the centre line of the cylinder or cone.
Scale factor on pseudo standard parallel	$k_P$	The factor by which the map grid is reduced or enlarged during the projection process, defined by its value at the pseudo-standard parallel.
Zone width	$W$	The longitude width of a zone of a Transverse Mercator zoned grid system.

**TABLE 2**  
**Summary of Coordinate Operation Parameters required for selected Map Projection methods**

<b><u>Coordinate Operation Parameter Name</u></b>	<b><u>Coordinate Operation Method</u></b>								
	Mercator (variant A)	Mercator (variant B)	Cassini-Soldner	Transverse Mercator	Hotine Oblique Mercator (variant A)	Hotine Oblique Mercator (variant B)	Lambert Conical (1 SP)	Lambert Conical (2 SP)	Oblique Stereographic
Latitude of false origin								1	
Longitude of false origin								2	
Latitude of 1st standard parallel		1						3	
Latitude of 2nd standard parallel								4	
Easting at false origin								5	
Northing at false origin								6	
Latitude of projection centre					1	1			
Longitude of projection centre					2	2			
Scale factor on initial line					3	3			
Azimuth of initial line					4	4			
Angle from Rectified to Skewed grid					5	5			
Easting at projection centre						6			
Northing at projection centre						7			
Latitude of natural origin	1 =equator		1	1			1		1
Longitude of natural origin	2	2	2	2			2		2
Scale factor at natural origin	3			3			3		3
False easting	4	3	3	4	6		4		4
False northing	5	4	4	5	7		5		5

### 3 **Map projection formulas**

#### 3.1 **Conic Map Projections**

##### 3.1.1 **Lambert Conic Conformal**

The Lambert Conic Conformal may often be adopted for territories with limited latitudinal extent but wide longitudinal width. But if the latitudinal extent is also large there may still be a need to use two or more zones if the scale distortion at the extremities of the one zone becomes too large to be tolerable.

Conical projections with one standard parallel are normally considered to maintain the nominal map scale along the parallel of latitude which is the line of contact between the imagined cone and the ellipsoid. For a one standard parallel Lambert the natural origin of the projected coordinate reference system is the intersection of the standard parallel with the longitude of origin (central meridian). See Figure 1 below. To maintain the conformal property the spacing of the parallels is variable and increases with increasing distance from the standard parallel, while the meridians are all straight lines radiating from a point on the prolongation of the ellipsoid's minor axis.

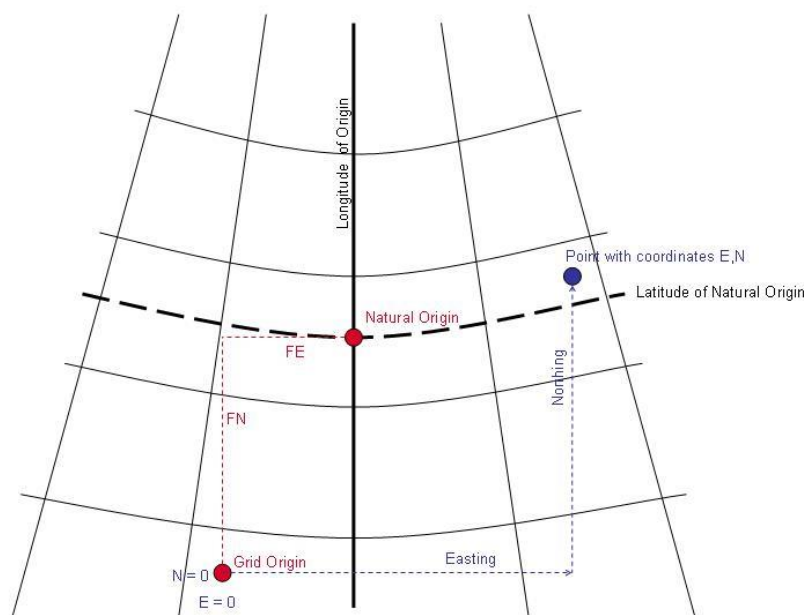


Figure 1 – Lambert Conic Conformal projection (1SP)

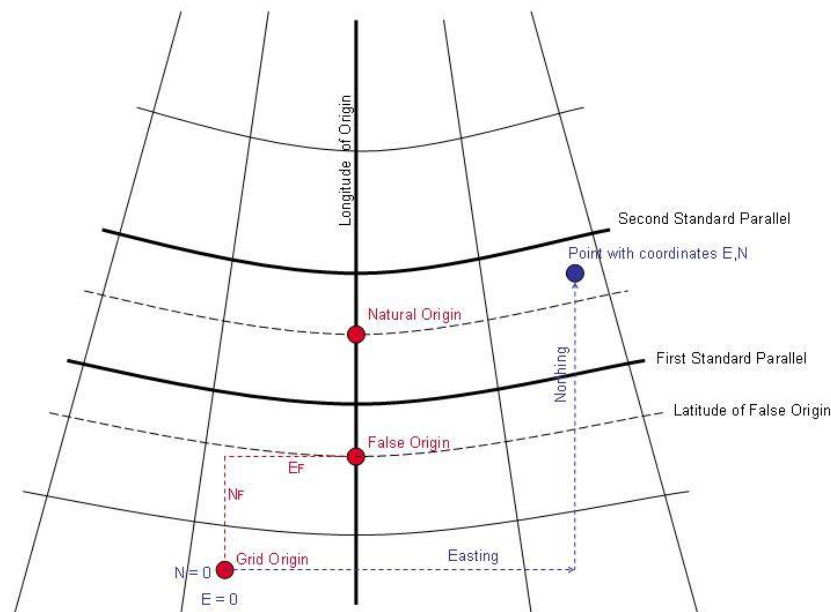


Figure 2 – Lambert Conic Conformal projection (2SP)

Sometimes however, although a one standard parallel Lambert is normally considered to have unity scale factor on the standard parallel, a scale factor of slightly less than unity is introduced on this parallel. This is a regular feature of the mapping of some former French territories and has the effect of making the scale factor unity on two other parallels either side of the standard parallel. The projection thus, strictly speaking, becomes a Lambert Conic Conformal projection with **two** standard parallels. From the one standard parallel and its scale factor it is possible to derive the equivalent two standard parallels and then treat the projection as a two standard parallel Lambert conical conformal, but this procedure is seldom adopted. Since the two parallels obtained in this way will generally not have integer values of degrees or degrees minutes and seconds it is instead usually preferred to select two specific parallels on which the scale factor is to be unity, as for several State Plane Coordinate systems in the United States (see figure 2).

The choice of the two standard parallels will usually be made according to the latitudinal extent of the area which it is wished to map, the parallels usually being chosen so that they each lie a proportion inboard of the north and south margins of the mapped area. Various schemes and formulas have been developed to make this selection such that the maximum scale distortion within the mapped area is minimised, e.g. Kavraisky in 1934, but whatever two standard parallels are adopted the formulas are the same.

EPSG recognises two variants of the Lambert Conic Conformal, the methods called Lambert Conic Conformal (1SP) and Lambert Conic Conformal (2SP). These variants are similar to the Mercator and Polar Stereographic variants A and C respectively. (The Mercator and Polar Stereographic methods may be logically considered as end-member cases of the Lambert Conic Conformal where the apex of the cone is at infinity or a pole respectively, but the Lambert formulas should not be used for these cases as some trigonometrical functions break down).

### 3.1.1.1 Lambert Conic Conformal (2SP)

(EPSG Dataset coordinate operation method code 9802)

To derive the projected Easting and Northing coordinates of a point with geographic coordinates ( $\phi, \lambda$ ) the formulas for the Lambert Conic Conformal **two standard parallel case** are:

$$\begin{aligned}\text{Easting, } E &= E_F + r \sin \theta \\ \text{Northing, } N &= N_F + r_F - r \cos \theta\end{aligned}$$

where  $m = \cos\phi / (1 - e^2 \sin^2\phi)^{0.5}$  for  $m_1, \phi_1$ , and  $m_2, \phi_2$  where  $\phi_1$  and  $\phi_2$  are the latitudes of the two standard parallels  
 $t = \tan(\pi/4 - \phi/2) / [(1 - e \sin\phi)/(1 + e \sin\phi)]^{e/2}$  for  $t_1, t_2, t_F$  and  $t$  using  $\phi_1, \phi_2, \phi_F$  and  $\phi$  respectively  
 $n = (\ln m_1 - \ln m_2) / (\ln t_1 - \ln t_2)$   
 $F = m_1 / (n t_1^n)$   
 $r = a F t^n$  for  $r_F$  and  $r$ , where  $r_F$  is the radius of the parallel of latitude of the false origin  
 $\theta = n(\lambda - \lambda_F)$

The reverse formulas to derive the latitude and longitude of a point from its Easting and Northing values are:

$$\begin{aligned}\phi &= \pi/2 - 2 \operatorname{atan}\{t'[(1 - e \sin\phi)/(1 + e \sin\phi)]^{e/2}\} \\ \lambda &= \theta'/n + \lambda_F\end{aligned}$$

where

$$\begin{aligned}r' &= \pm\{(E - E_F)^2 + [r_F - (N - N_F)]^2\}^{0.5}, \text{ taking the sign of } n \\ t' &= (r'/(aF))^{1/n} \\ \theta' &= \operatorname{atan2}[(E - E_F), (r_F - (N - N_F))]\end{aligned}$$

and  $n, F$ , and  $r_F$  are derived as for the forward calculation.

Note that the formula for  $\phi$  requires iteration. First calculate  $t'$  and then a trial value for  $\phi$  using  $\phi = \pi/2 - 2 \operatorname{atan}(t')$ . Then use the full equation for  $\phi$  substituting the trial value into the right hand side of the equation. Thus derive a new value for  $\phi$ . Iterate the process until  $\phi$  does not change significantly. The solution should quickly converge, in 3 or 4 iterations.

#### Example:

For Projected Coordinate Reference System: NAD27 / Texas South Central

Parameters:

Ellipsoid:	Clarke 1866	$a = 6378206.400$ metres	=	20925832.16 US survey feet
		$1/f = 294.97870$		
	then	$e = 0.08227185$		$e^2 = 0.00676866$
Latitude of false origin	$\phi_F$	27°50'00"N	=	0.48578331 rad
Longitude of false origin	$\lambda_F$	99°00'00"W	=	-1.72787596 rad
Latitude of 1st standard parallel	$\phi_1$	28°23'00"N	=	0.49538262 rad
Latitude of 2nd standard parallel	$\phi_2$	30°17'00"N	=	0.52854388 rad
Easting at false origin	$E_F$	2000000.00		US survey feet
Northing at false origin	$N_F$	0.00		US survey feet

Forward calculation for:

Latitude	$\phi$	=	28°30'00.00"N	=	0.49741884 rad
Longitude	$\lambda$	=	96°00'00.00"W	=	-1.67551608 rad

first gives :

$$\begin{array}{llll} m_1 & = & 0.88046050 & m_2 & = & 0.86428642 \\ t & = & 0.59686306 & t_F & = & 0.60475101 \\ t_1 & = & 0.59823957 & t_2 & = & 0.57602212 \\ n & = & 0.48991263 & F & = & 2.31154807 \\ r & = & 37565039.86 & r_F & = & 37807441.20 \\ \theta & = & 0.02565177 & & & \end{array}$$

$$\begin{array}{llll} \text{Then Easting} & E & = & 2963503.91 \text{ US survey feet} \\ \text{Northing} & N & = & 254759.80 \text{ US survey feet} \end{array}$$

Reverse calculation for same easting and northing first gives:

$$\begin{array}{ll} \theta' & = & 0.025651765 \\ t' & = & 0.59686306 \\ r' & = & 37565039.86 \end{array}$$

$$\begin{array}{llll} \text{Then Latitude} & \varphi & = & 28^\circ 30' 00.000'' \text{N} \\ \text{Longitude} & \lambda & = & 96^\circ 00' 00.000'' \text{W} \end{array}$$

Further examples of input and output may be found in test procedure 5103 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 3.1.1.2 Lambert Conic Conformal (1SP)

(EPSG Dataset coordinate operation method code 9801)

For the Lambert Conic Conformal single standard parallel variant the formulas for the Lambert Conic Conformal (2SP) variant can be used with minor modifications:

$$E = FE + r \sin \theta$$

$$N = FN + r_0 - r \cos \theta, \text{ using the natural origin rather than the false origin.}$$

where

$$n = \sin \varphi_0$$

$$r = a F t^n k_0 \quad \text{for } r_0, \text{ and } r$$

$t$  is found for  $t_0$ ,  $\varphi_0$  and  $t$ ,  $\varphi$  and  $m$ ,  $F$ , and  $\theta$  are found as for the two standard parallel variant.

The reverse formulas for  $\varphi$  and  $\lambda$  are as for the two standard parallel variant above, with  $n$ ,  $F$  and  $r_0$  as before and

$$\theta' = \text{atan2}\{(E - FE), [r_0 - (N - FN)]\}$$

$$r' = \pm \{(E - FE)^2 + [r_0 - (N - FN)]^2\}^{0.5}, \text{ taking the sign of } n$$

$$t' = (r' / (a k_0 F))^{1/n}$$

**Example:**

For Projected Coordinate Reference System: JAD69 / Jamaica National Grid

Parameters:

Ellipsoid:	Clarke 1866	a = 6378206.400 metres	1/f = 294.97870
	then	e = 0.08227185	e <sup>2</sup> = 0.00676866
Latitude of natural origin	$\phi_0$	18°00'00"N	= 0.31415927 rad
Longitude of natural origin	$\lambda_0$	77°00'00"W	= -1.34390352 rad
Scale factor at natural origin	$k_0$	1.000000	
False easting	FE	250000.00	metres
False northing	FN	150000.00	metres

Forward calculation for:

Latitude	$\phi$	=	17°55'55.80"N	=	0.31297535 rad
Longitude	$\lambda$	=	76°56'37.26"W	=	-1.34292061 rad

first gives

$m_0$	=	0.95136402	$t_0$	=	0.72806411
F	=	3.39591092	n	=	0.30901699
R	=	19643955.26	$r_0$	=	19636447.86
$\theta$	=	0.00030374	t	=	0.728965259

Then	Easting	E	=	255966.58 metres
	Northing	N	=	142493.51 metres

Reverse calculation for the same easting and northing first gives

$\theta'$	=	0.000303736
$t'$	=	0.728965259
$m_0$	=	0.95136402
$r'$	=	19643955.26

Then	Latitude	$\phi$	=	17°55'55.80"N
	Longitude	$\lambda$	=	76°56'37.26"W

Further examples of input and output may be found in test procedure 5102 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 3.1.1.3 Lambert Conic Conformal (1SP West Orientated)

(EPSG Dataset coordinate operation method code 9826)

In older mapping of Denmark and Greenland the Lambert Conic Conformal (1SP) is used with axes positive north and **west**. To derive the projected Westing and Northing coordinates of a point with geographic coordinates ( $\phi, \lambda$ ) the formulas are as for the standard Lambert Conic Conformal (1SP) variant above (EPSG Dataset coordinate operation method code 9801) except for:

$$W = FE - r * \sin \theta$$

In this formula the term FE retains its definition, i.e. in the Lambert Conic Conformal (West Orientated) method it increases the Westing value at the natural origin. In this method it is effectively false westing (FW).

The reverse formulas to derive the latitude and longitude of a point from its Westing and Northing values are as for the standard Lambert Conic Conformal (1SP) variant except for:

$$\theta' = \text{atan2}[(FE - W), \{r_0 - (N - FN)\}]$$

$$r' = \pm[(FE - W)^2 + \{r_0 - (N - FN)\}^2]^{0.5}, \text{ taking the sign of } n$$

### 3.1.1.4 Lambert Conic Conformal (2SP Belgium) (EPSG Dataset coordinate operation method code 9803)

In 1972, in order to retain approximately the same grid coordinates after a change of geodetic datum, a modified form of the two standard parallel case was introduced in Belgium. In 2000 this modification was replaced through use of the regular Lambert Conic Conformal (2SP) method with appropriately modified parameter values.

In the 1972 modification the formulas for the regular Lambert Conic Conformal (2SP) variant given above are used except for:

$$\text{Easting, } E = E_F + r \sin(\theta - a)$$

$$\text{Northing, } N = N_F + r_F - r \cos(\theta - a)$$

and for the reverse formulas

$$\lambda = [(\theta' + a)/n] + \lambda_F$$

where  $a = 29.2985$  arc-seconds.

#### **Example:**

For Projected Coordinate Reference System: Belge 1972 / Belge Lambert 72

Parameters:

Ellipsoid:	International 1924	$a = 6378388$ metres	$1/f = 297.0$
	then	$e = 0.08199189$	$e^2 = 0.006722670$
Latitude of false origin	$\phi_F$	90°00'00"N	= 1.57079633 rad
Longitude of false origin	$\lambda_F$	4°21'24.983"E	= 0.07604294 rad
Latitude of 1 <sup>st</sup> standard parallel	$\phi_1$	49°50'00"N	= 0.86975574 rad
Latitude of 2 <sup>nd</sup> standard parallel	$\phi_2$	51°10'00"N	= 0.89302680 rad
Easting at false origin	$E_F$	150000.01	metres
Northing at false origin	$N_F$	5400088.44	metres

Forward calculation for:

Latitude	$\phi$	=	50°40'46.461"N	=	0.88452540 rad
Longitude	$\lambda$	=	5°48'26.533"E	=	0.10135773 rad

first gives :

$m_1$	=	0.64628304	$m_2$	=	0.62834001
$t$	=	0.35913403	$t_F$	=	0.00
$t_1$	=	0.36750382	$t_2$	=	0.35433583
$n$	=	0.77164219	$F$	=	1.81329763
$r$	=	5248041.03	$r_F$	=	0.00
$\theta$	=	0.01953396	$a$	=	0.00014204

Then	Easting	$E$	=	251763.20 metres
	Northing	$N$	=	153034.13 metres

Reverse calculation for same easting and northing first gives:

$$\theta' = 0.01939192$$



$$\begin{aligned} t' &= 0.35913403 \\ r' &= 5248041.03 \end{aligned}$$

$$\begin{aligned} \text{Then Latitude } \varphi &= 50^{\circ}40'46.461''\text{N} \\ \text{Longitude } \lambda &= 5^{\circ}48'26.533''\text{E} \end{aligned}$$

### 3.1.1.5 Lambert Conic Conformal (2SP Michigan) (EPSG Dataset coordinate operation method code 1051)

In 1964, the US state of Michigan redefined its State Plane CS27 zones, changing them from being Transverse Mercator zones orientated north-south to being Lambert Conic Conformal zones orientated east-west to better reflect the geography of the state. In making this change it was decided to scale the ellipsoid to the average height of the state, 800 feet above sea level, in order to eliminate the need for height reduction in engineering surveys. (In the later CS83 definitions this modification was eliminated). In the 1964 modification the formulas for the regular Lambert Conic Conformal (2SP) variant are used except for:

in the forward formulas

$$r = aK F t^n \quad \text{for } r_F \text{ and } r$$

and in the reverse formulas

$$t' = (r'/(aKF))^{1/n}$$

where K = is the ellipsoid scaling factor.

#### **Example:**

For Projected Coordinate Reference System: NAD27 / Michigan Central

Parameters:

Ellipsoid:	Clarke 1866	a = 6378206.400 metres	= 20925832.164 US Survey feet
		1/f = 294.978698	
	then	e = 0.08227185	e <sup>2</sup> = 0.00676866
Latitude of false origin	$\varphi_F$	43°19'00"N	= 0.756018454 rad
Longitude of false origin	$\lambda_F$	84°20'00"W	= -1.471894336 rad
Latitude of 1 <sup>st</sup> standard parallel	$\varphi_1$	44°11'00"N	= 0.771144641 rad
Latitude of 2 <sup>nd</sup> standard parallel	$\varphi_2$	45°42'00"N	= 0.797615468 rad
Easting at false origin	$E_F$	2000000.0	ftUS
Northing at false origin	$N_F$	0	ftUS
Ellipsoid scaling factor	K	1.0000382	

Forward calculation for:

$$\begin{aligned} \text{Latitude } \varphi &= 43^{\circ}45'00.000''\text{N} &= 0.763581548 \text{ rad} \\ \text{Longitude } \lambda &= 83^{\circ}10'00.000''\text{W} &= -1.451532161 \text{ rad} \end{aligned}$$

first gives :

$$\begin{aligned} m_1 &= 0.718295175 & m_2 &= 0.699629151 \\ t &= 0.429057680 & t_F &= 0.433541026 \\ t_1 &= 0.424588396 & t_2 &= 0.409053868 \\ n &= 0.706407410 & F &= 1.862317735 \\ r &= 21436775.51 & r_F &= 21594768.40 \\ \theta &= 0.014383991 & & \end{aligned}$$

$$\begin{array}{llll} \text{Then} & \text{Easting} & X & = & 2308335.75 \text{ ftUS} \\ & \text{Northing} & Y & = & 160210.48 \text{ ftUS} \end{array}$$

Reverse calculation for same easting and northing first gives:

$$\begin{array}{ll} \theta' & = & 0.014383991 \\ t' & = & 0.429057680 \\ r' & = & 21436775.51 \end{array}$$

$$\begin{array}{llll} \text{Then} & \text{Latitude} & \varphi & = & 43^{\circ}45'00.000''\text{N} \\ & \text{Longitude} & \lambda & = & 83^{\circ}10'00.000''\text{E} \end{array}$$

### 3.1.1.6 Lambert Conic Near-Conformal

(EPSG Dataset coordinate operation method code 9817)

The Lambert Conformal Conic with one standard parallel formulas, as published by the Army Map Service, are still in use in several countries. The AMS uses series expansion formulas for ease of computation, as was normal before the electronic computer made such approximate methods unnecessary. Where the expansion series have been carried to enough terms the results are the same to the centimetre level as through the Lambert Conic Conformal (1SP) formulas above. However in some countries the expansion formulas were truncated to the third order and the map projection is not fully conformal. The full formulas are used in Libya but from 1915 for France, Morocco, Algeria, Tunisia and Syria the truncated formulas were used. In 1943 in Algeria and Tunisia, from 1948 in France, from 1953 in Morocco and from 1973 in Syria the truncated formulas were replaced with the full formulas.

To compute the Lambert Conic Near-Conformal the following formulas are used. First compute constants for the projection:

$$\begin{array}{ll} n & = & f / (2 - f) \\ A & = & 1 / (6 \rho_0 v_0) \text{ where } \rho_0 \text{ and } v_0 \text{ are computed as in the Implementation Notes in this document.} \\ A' & = & a [ 1 - n + 5 (n^2 - n^3) / 4 + 81 (n^4 - n^5) / 64 ] * \pi / 180 \\ B' & = & 3 a [ n - n^2 + 7 (n^3 - n^4) / 8 + 55 n^5 / 64 ] / 2 \\ C' & = & 15 a [ n^2 - n^3 + 3 (n^4 - n^5) / 4 ] / 16 \\ D' & = & 35 a [ n^3 - n^4 + 11 n^5 / 16 ] / 48 \\ E' & = & 315 a [ n^4 - n^5 ] / 512 \\ r_0 & = & k_0 v_0 / \tan \varphi_0 \\ s_0 & = & A' \varphi_0 - B' \sin 2\varphi_0 + C' \sin 4\varphi_0 - D' \sin 6\varphi_0 + E' \sin 8\varphi_0 \\ & & \text{where in the first term } \varphi_0 \text{ is in degrees, in the other terms } \varphi_0 \text{ is in radians.} \end{array}$$

Then for the computation of easting and northing from latitude and longitude:

$$\begin{array}{ll} s & = & A' \varphi - B' \sin 2\varphi + C' \sin 4\varphi - D' \sin 6\varphi + E' \sin 8\varphi \\ & & \text{where in the first term } \varphi \text{ is in degrees, in the other terms } \varphi \text{ is in radians.} \\ m & = & [s - s_0] \\ M & = & k_0 (m + Am^3) \quad (\text{see footnote}^3) \\ r & = & r_0 - M \\ \theta & = & (\lambda - \lambda_0) \sin \varphi_0 \\ \text{and} & & \\ E & = & FE + r \sin \theta \\ N & = & FN + M + r \sin \theta \tan (\theta / 2) \end{array}$$

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<sup>3</sup> This is the term that is truncated to the third order. To be equivalent to the Lambert Conic Conformal (1SP) (variant A) it would be  $M = k_0 (m + Am^3 + Bm^4 + Cm^5 + Dm^6)$ . B, C and D are not detailed here.

The reverse formulas for  $\varphi$  and  $\lambda$  from E and N are:

$$\begin{aligned}\theta' &= \text{atan2} \{ (E - FE), [r_0 - (N - FN)] \} \\ r' &= \pm \{ (E - FE)^2 + [r_0 - (N - FN)]^2 \}^{0.5}, \text{ taking the sign of } \varphi_0 \\ M' &= r_0 - r'\end{aligned}$$

If an exact solution is required, it is necessary to solve for m and  $\varphi$  using iteration of the two equations  
 Firstly:

$$m' = m' - [M' - k_0 m' - k_0 A (m')^3] / [-k_0 - 3 k_0 A (m')^2]$$

using M' for m' in the first iteration. This will usually converge (to within 1mm) in a single iteration.

Then

$$\varphi' = \varphi' + \{ m' + s_0 - [A' \varphi' (180/\pi) - B' \sin 2\varphi' + C' \sin 4\varphi' - D' \sin 6\varphi' + E' \sin 8\varphi'] \} / A' (\pi/180)$$

first using  $\varphi' = \varphi_0 + m'/A' (\pi/180)$ .

However the following non-iterative solution is accurate to better than 0.001" (3mm) within 5 degrees latitude of the projection origin and should suffice for most purposes:

$$\begin{aligned}m' &= M' - [M' - k_0 M' - k_0 A (M')^3] / [-k_0 - 3 k_0 A (M')^2] \\ \varphi' &= \varphi_0 + m'/A' (\pi/180) \\ s' &= A' \varphi' - B' \sin 2\varphi' + C' \sin 4\varphi' - D' \sin 6\varphi' + E' \sin 8\varphi' \\ &\quad \text{where in the first term } \varphi' \text{ is in degrees, in the other terms } \varphi' \text{ is in radians.} \\ ds' &= A'(180/\pi) - 2B' \cos 2\varphi' + 4C' \cos 4\varphi' - 6D' \cos 6\varphi' + 8E' \cos 8\varphi' \\ \varphi &= \varphi' - [(m' + s_0 - s') / (-ds')] \text{ radians}\end{aligned}$$

Then after solution of  $\varphi$  using either method above

$$\lambda = \lambda_0 + \theta' / \sin \varphi_0 \text{ where } \lambda_0 \text{ and } \lambda \text{ are in radians}$$

### Example:

For Projected Coordinate Reference System: Deir ez Zor / Levant Zone

Parameters:

Ellipsoid:	Clarke 1880 (IGN)	a = 6378249.2 metres	1/f = 293.4660213
	then		n = 0.001706682563
Latitude of natural origin	$\varphi_0$	34°39'00"N	= 0.604756586 rad
Longitude of natural origin	$\lambda_0$	37°21'00"E	= 0.651880476 rad
Scale factor at natural origin	$k_0$	0.99962560	
False easting	FE	300000.00	metres
False northing	FN	300000.00	metres

Forward calculation for:

Latitude	$\varphi$	= 37°31'17.625"N	= 0.654874806 rad
Longitude	$\lambda$	= 34°08'11.291"E	= 0.595793792 rad

first gives :

A	=	4.1067494 * 10 <sup>-15</sup>	A'	=	111131.8633
B'	=	16300.64407	C'	=	17.38751
D'	=	0.02308	E'	=	0.000033
s <sub>0</sub>	=	3835482.233	r <sub>0</sub>	=	9235264.405
s	=	4154101.458	m	=	318619.225
M	=	318632.72	r	=	8916631.685
$\theta$	=	-0.03188875			

Then    Easting        E    =    15707.96 metres (c.f. E = 15708.00 using full formulas)  
          Northing      N    =    623165.96 metres (c.f. N = 623167.20 using full formulas)

Reverse calculation for same easting and northing first gives:

$\theta'$     =   -0.031888749  
 $r'$     =   8916631.685  
 $M'$     =   318632.717

Using the non-iterative solution:

$m'$     =   318619.222  
 $\phi'$     =   0.654795830  
 $s'$     =   4153599.259  
 $ds'$    =   6358907.456

Then    Latitude         $\phi$     =   0.654874806 rad    =   37°31'17.625"N  
          Longitude       $\lambda$     =   0.595793792 rad    =   34°08'11.291"E

### 3.1.2 **Krovak**

The normal case of the Lambert Conic Conformal is for the axis of the cone to be coincident with the minor axis of the ellipsoid, that is the axis of the cone is normal to the ellipsoid at a geographic pole. For the Oblique Conic Conformal the axis of the cone is normal to the ellipsoid at a defined location and its extension cuts the ellipsoid minor axis at a defined angle. The map projection method is similar in principle to the Oblique Mercator. It is used in the Czech Republic and Slovakia under the name ‘Krovak’ projection, where like the Laborde Oblique Mercator cylindrical projection in Madagascar the rotation to north is made in spherical rather than plane coordinates. The geographic coordinates on the ellipsoid are first reduced to conformal coordinates on the conformal (Gaussian) sphere. These spherical coordinates are then rotated to north and the rotated spherical coordinates then projected onto the oblique cone and converted to grid coordinates. The pseudo standard parallel is defined on the conformal sphere after its rotation. It is then the parallel on this sphere at which the map projection is true to scale; on the ellipsoid it maps as a complex curve. A scale factor may be applied to the map projection to increase the useful area of coverage.

Four variants of the classical Krovak method developed in 1922 are recognised:

- i) For the Czech Republic and Slovakia the original 1922 method by **Krovak** results in coordinates which increment to the south and west.
- ii) A 21<sup>st</sup> century modification to this,
- iii) **Krovak (North Orientated)**, causes the coordinate system axes to increment to the east and north, making the system more readily incorporated in GIS systems.
- iv) A second modification to the original method for distortions in the surveying network, **Krovak Modified**.
- v) A third modification, **Krovak Modified (North Orientated)**, applies both of the first and second modifications, that is it compensates for surveying network distortions and also results in coordinates for the Czech Republic and Slovakia incrementing to the east and north.

### 3.1.2.1 **Krovak**

(EPSG Dataset coordinate operation method code 9819)

The defining parameters for the Krovak method are:

$\varphi_C$	= latitude of projection centre, the latitude of the point used as reference for the calculation of the Gaussian conformal sphere
$\lambda_O$	= longitude of origin
$\alpha_C$	= rotation in plane of meridian of origin of the conformal coordinates = co-latitude of the cone axis at its point of intersection with the conformal sphere
$\varphi_P$	= latitude of pseudo standard parallel
$k_P$	= scale factor on pseudo standard parallel
FE	= False Easting
FN	= False Northing

For the projection devised by Krovak in 1922 for Czechoslovakia the axes are positive south (X) and west (Y). The symbols FE and FN represent the westing and southing coordinates at the grid natural origin. This is at the intersection of the cone axis with the surface of the conformal sphere.

From these the following constants for the projection may be calculated:

A	=	$a (1 - e^2)^{0.5} / [1 - e^2 \sin^2(\varphi_C)]$
B	=	$\{1 + [e^2 \cos^4 \varphi_C / (1 - e^2)]\}^{0.5}$
$\gamma_O$	=	$\text{asin} [\sin(\varphi_C) / B]$
$t_O$	=	$\tan(\pi / 4 + \gamma_O / 2) \cdot [(1 + e \sin(\varphi_C)) / (1 - e \sin(\varphi_C))]^{e.B/2} / [\tan(\pi / 4 + \varphi_C / 2)]^B$
n	=	$\sin(\varphi_P)$
$r_O$	=	$k_P A / \tan(\varphi_P)$

To derive the projected Southing and Westing coordinates of a point with geographic coordinates ( $\varphi, \lambda$ ) the formulas for the Krovak are:

U	=	$2 (\text{atan} \{ t_O \tan^B(\varphi / 2 + \pi / 4) / [(1 + e \sin(\varphi)) / (1 - e \sin(\varphi))]^{e.B/2} \} - \pi / 4)$
V	=	$B (\lambda_O - \lambda)$ where $\lambda_O$ and $\lambda$ must both be referenced to the same prime meridian,
T	=	$\text{asin} [\cos(\alpha_C) \sin(U) + \sin(\alpha_C) \cos(U) \cos(V)]$
D	=	$\text{asin} [\cos(U) \sin(V) / \cos(T)]$
$\theta$	=	$n D$
r	=	$r_O \tan^n(\pi / 4 + \varphi_P / 2) / \tan^n(T / 2 + \pi / 4)$
Xp	=	$r \cos \theta$
Yp	=	$r \sin \theta$

Then

Southing	X	=	Xp + FN
Westing	Y	=	Yp + FE

Note also that the formula for D is satisfactory for the normal use of the projection within the pseudo-longitude range on the conformal sphere of  $\pm 90$  degrees from the central line of the projection. Should there be a need to exceed this range (which is not necessary for application in the Czech and Slovak Republics) then for the calculation of D use:

$$\begin{aligned} \sin(D1) &= \cos(U) * \sin(V) / \cos(T) \\ \cos(D1) &= \{[\cos(\alpha_C) * \sin(T) - \sin(U)] / [\sin(\alpha_C) * \cos(T)]\} \\ D &= \text{atan2}(\sin(D1), \cos(D1)) \end{aligned}$$

The reverse formulas to derive the latitude and longitude of a point from its Southing (X) and Westing (Y) values are:

$$\begin{aligned}
 X_{p'} &= \text{Southing} - \text{FN} \\
 Y_{p'} &= \text{Westing} - \text{FE} \\
 r' &= [(X_{p'})^2 + (Y_{p'})^2]^{0.5} \\
 \theta' &= \text{atan2}(Y_{p'}, X_{p'}) \\
 D' &= \theta' / \sin(\phi_P) \\
 T' &= 2\{\text{atan}[(r_O / r')^{1/n} \tan(\pi / 4 + \phi_P / 2)] - \pi / 4\} \\
 U' &= \text{asin}[\cos(\alpha_C) \sin(T') - \sin(\alpha_C) \cos(T') \cos(D')] \\
 V' &= \text{asin}[\cos(T') \sin(D') / \cos(U')]
 \end{aligned}$$

Latitude  $\phi$  is found by iteration using  $U'$  as the value for  $\phi_{j-1}$  in the first iteration

$$\phi_j = 2 \left( \text{atan} \left\{ \tan^{-1/B} \tan^{1/B} \left( U'/2 + \pi / 4 \right) \left[ (1 + e \sin(\phi_{j-1})) / (1 - e \sin(\phi_{j-1})) \right]^{e/2} \right\} - \pi / 4 \right)$$

3 iterations will usually suffice. Then:

$$\lambda = \lambda_O - V' / B \quad \text{where } \lambda \text{ is referenced to the same prime meridian as } \lambda_O.$$

### Example

For Geographic CRS *S-JTSK* and Projected CRS *S-JTSK (Ferro) / Krovak*

Parameters:

Ellipsoid:	Bessel 1841	$a = 6377397.155$ metres	$1/f = 299.15281$
	then	$e = 0.081696831$	$e^2 = 0.006674372$

Latitude of projection centre	$\phi_C$	49°30'00"N	= 0.863937979 rad
Longitude of origin	$\lambda_O$	42°30'00"E of Ferro	= 0.741764932 rad
Co-latitude of cone axis	$\alpha_C$	30°17'17.30311"	= 0.528627763 rad
Latitude of pseudo standard parallel	$\phi_P$	78°30'00"N	= 1.370083463 rad
Scale factor on pseudo standard parallel	$k_P$	0.9999	
False Easting	FE	0.00	metres
False Northing	FN	0.00	metres

Projection constants:

$A$	=	6380703.611	$B$	=	1.000597498
$\gamma_O$	=	0.863239103	$t_O$	=	1.003419164
$n$	=	0.979924705	$r_O$	=	1298039.005

Forward calculation for:

Latitude	$\phi$	=	50°12'32.442"N	=	0.876312568 rad
Longitude	$\lambda_G$	=	16°50'59.179"E of Greenwich		

Firstly, because the projection definition includes longitudes referenced to the Ferro meridian, the longitude of the point needs to be transformed to be referenced to the Ferro meridian<sup>4</sup> using the Longitude Rotation method (EPSG method code 9601, see Implementation Notes on Offsets).

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<sup>4</sup> An alternative approach would be to define the projection with longitude of origin value of 24°50'E referenced to the Greenwich meridian. This approach would eliminate the need to transform the longitude of every point from Greenwich to Ferro before conversion, which arguably is more practical. It is not formerly documented here because the published projection definition is referenced to Ferro.

Longitude of point  $\lambda_G$  = 16°50'59.179"E of Greenwich  
when referenced to  
Greenwich meridian

Longitude of Ferro = 17°40'00" west of Greenwich  
and then

Longitude of point  $\lambda$  = 34°30'59.179"E of Ferro = 0.602425500 rad  
when referenced to  
Ferro meridian

Then the forward calculation first gives:

U = 0.875596949  
V = 0.139422687  
T = 1.386275049  
D = 0.506554623  
 $\theta$  = 0.496385389  
r = 1194731.014  
Xp = 1050538.643  
Yp = 568990.997

and Southing X = 1050538.64 metres  
Westing Y = 568991.00 metres

Note that in the Czech Republic and Slovakia southing (X) values for the Krovak grid are greater than 900000m and westing (Y) values are less than 900000m.

Reverse calculation for the same Southing and Westing gives

Xp' = 1050538.643  
Yp' = 568990.997  
r' = 1194731.014  
 $\theta'$  = 0.496385389  
D' = 0.506554623  
T' = 1.386275049  
U' = 0.875596949  
V' = 0.139422687

Then by iteration

$\phi_1$  = 0.876310601 rad  $\phi_2$  = 0.876312560 rad  $\phi_3$  = 0.876312566 rad  
and

Latitude  $\phi$  = 0.876312566 rad = 50°12'32.442"N

Longitude of point  $\lambda$  = 0.602425500 rad = 34°30'59.179"E of Ferro  
when referenced to  
Ferro meridian

Then using the Longitude Rotation method (EPSG method code 9601)

Longitude of Ferro = 17°40'00" west of Greenwich  
Then

Longitude of point  $\lambda_G$  = 16°50'59.179"E of Greenwich  
when referenced to  
Greenwich meridian

### 3.1.2.2 **Krovak (North Orientated)**

(EPSG Dataset coordinate operation method code 1041)

Having axes positive south and west, as is the case with the projection defined by Krovak in 1922, causes problems in some GIS systems. To resolve this problem the Czech and Slovak authorities modified the Krovak projection method to have a coordinate system with axes incrementing to the east and north.

To derive the projected Easting and Northing coordinates of a point with geographic coordinates ( $\phi, \lambda$ ) the formulas are the same as those for the Krovak (south-orientated) method (EPSG method code 9819) described in the previous section as far as the calculation of Southing and Westing. Then

$$\begin{array}{ll} \text{Easting}_{\text{Krovak (North Orientated)}} & X = -\text{Westing}_{\text{Krovak}} \\ \text{Northing}_{\text{Krovak (North Orientated)}} & Y = -\text{Southing}_{\text{Krovak}} \end{array}$$

Note that in addition to changing the sign of the coordinate values, the axes abbreviations X and Y are transposed from south to east and from west to north respectively. Note also that for this north-orientated variant, although the grid coordinates increment to the east and north their numerical values are negative in the Czech Republic and Slovakia area. For the north-orientated grid easting (X) values are greater than  $-900000\text{m}$  i.e. between  $-900000\text{m}$  and  $-400000\text{m}$  and northing (Y) values are less than  $-900000\text{m}$  (between  $-900000\text{m}$  and  $-1400000\text{m}$ ).

For the reverse conversion to derive the latitude and longitude of a point from its Easting (X) and Northing (Y) values, first:

$$\begin{array}{ll} \text{Southing}_{\text{Krovak}} & = -\text{Northing}_{\text{Krovak (North Orientated)}} \\ \text{Westing}_{\text{Krovak}} & = -\text{Easting}_{\text{Krovak (North Orientated)}} \end{array}$$

after which latitude and longitude are derived as described for the south-orientated Krovak method in the previous section.

### **Example**

For Projected Coordinate Reference System: S-JTSK (Ferro) / Krovak East North

Forward calculation for the same point as in the south-orientated case in section 3.1.2.1 above:

$$\begin{array}{lll} \text{Latitude } \phi & = & 50^{\circ}12'32.442''\text{N} \\ \text{Longitude } \lambda_G & = & 34^{\circ}30'59.179''\text{E of Ferro} \end{array}$$

proceeds as in the south-orientated case as far as the calculation of Southing and Westing:

$$\begin{array}{ll} U & = 0.875596949 \\ V & = 0.139422687 \\ T & = 1.386275049 \\ D & = 0.506554623 \\ \theta & = 0.496385389 \\ r & = 1194731.014 \\ X_p & = 1050538.643 \\ Y_p & = 568990.997 \\ \text{Southing} & = 1050538.643 \\ \text{Westing} & = 568990.997 \end{array}$$

Then

$$\begin{array}{lll} \text{Easting } X & = & -568991.00 \text{ metres} \\ \text{Northing } Y & = & -1050538.64 \text{ metres} \end{array}$$



Reverse calculation for the same Easting and Northing (–568990.995, –1050538.634) first gives

$$\begin{aligned}\text{Southing} &= 1050538.643 \\ \text{Westing} &= 568990.997\end{aligned}$$

then proceeds as in the calculation for the (south-orientated) Krovak method to give

$$\begin{aligned}\text{Latitude } \varphi &= 50^{\circ}12'32.442''\text{N} \\ \text{Longitude } \lambda &= 34^{\circ}30'59.179''\text{E of Ferro}\end{aligned}$$

### 3.1.2.3 **Krovak Modified**

(EPSG Dataset coordinate operation method code 1042)

After the initial introduction of the S-JTSK (Ferro) / Krovak projected CRS, further geodetic measurements were observed and calculations made in the projected CRS resulted in changed grid coordinate values. The relationship through the Krovak projection between the original S-JTSK geographic CRS coordinates and the new projected CRS coordinates was no longer exact. In 2009 the Czech authorities introduced a modification which modelled the survey network distortions.

To derive the projected Easting and Northing coordinates of a point with geographic coordinates ( $\varphi, \lambda$ ) the formulas for the Krovak Modified method are the same as those for the original (unmodified, south-orientated) Krovak method as far as the calculation of  $X_p$  and  $Y_p$ .

Then:

$$\begin{aligned}X_r &= X_p - X_o \\ Y_r &= Y_p - Y_o\end{aligned}$$

and

$$\begin{aligned}dX &= C_1 + C_3.X_r - C_4.Y_r - 2.C_6.X_r.Y_r + C_5.(X_r^2 - Y_r^2) + C_7.X_r.(X_r^2 - 3.Y_r^2) \\ &\quad - C_8.Y_r.(3.X_r^2 - Y_r^2) + 4.C_9.X_r.Y_r.(X_r^2 - Y_r^2) + C_{10}.(X_r^4 + Y_r^4 - 6.X_r^2.Y_r^2) \\ dY &= C_2 + C_3.Y_r + C_4.X_r + 2.C_5.X_r.Y_r + C_6.(X_r^2 - Y_r^2) + C_8.X_r.(X_r^2 - 3.Y_r^2) \\ &\quad + C_7.Y_r.(3.X_r^2 - Y_r^2) - 4.C_{10}.X_r.Y_r.(X_r^2 - Y_r^2) + C_9.(X_r^4 + Y_r^4 - 6.X_r^2.Y_r^2)\end{aligned}$$

where  $X_o$ ,  $Y_o$  and  $C_1$  through  $C_{10}$  are additional defining parameters for the modified method.

Finally:

$$\begin{aligned}\text{Southing } X &= X_p - dX + FN \\ \text{Westing } Y &= Y_p - dY + FE\end{aligned}$$

For the reverse conversion to derive the latitude and longitude of a point from its Southing (X) and Westing (Y) values:

$$\begin{aligned}X_r' &= (\text{Southing} - FN) - X_o \\ Y_r' &= (\text{Westing} - FE) - Y_o \\ dX' &= C_1 + C_3.X_r' - C_4.Y_r' - 2.C_6.X_r'.Y_r' + C_5.(X_r'^2 - Y_r'^2) + C_7.X_r'.(X_r'^2 - 3.Y_r'^2) \\ &\quad - C_8.Y_r'.(3.X_r'^2 - Y_r'^2) + 4.C_9.X_r'.Y_r'.(X_r'^2 - Y_r'^2) + C_{10}.(X_r'^4 + Y_r'^4 - 6.X_r'^2.Y_r'^2) \\ dY' &= C_2 + C_3.Y_r' + C_4.X_r' + 2.C_5.X_r'.Y_r' + C_6.(X_r'^2 - Y_r'^2) + C_8.X_r'.(X_r'^2 - 3.Y_r'^2) \\ &\quad + C_7.Y_r'.(3.X_r'^2 - Y_r'^2) - 4.C_{10}.X_r'.Y_r'.(X_r'^2 - Y_r'^2) + C_9.(X_r'^4 + Y_r'^4 - 6.X_r'^2.Y_r'^2) \\ X_p' &= (\text{Southing} - FN) + dX' \\ Y_p' &= (\text{Westing} - FE) + dY'\end{aligned}$$

After which latitude and longitude are derived from  $X_p'$  and  $Y_p'$  as described for the classic Krovak method (EPSG method code 9819) described above.

It may be noted that the calculation of  $dX$  and  $dY$  in the forward formulas (and  $dX'$  and  $dY'$  in the reverse formulas) is a reversible polynomial of degree 4 – see section 4.6. The coefficients  $C_1$  through  $C_{10}$  can be mapped to the coefficients  $A_0$  through  $A_{14}$  and  $B_0$  through  $B_{14}$  given in section describing polynomials as:

$A_0 = C_1$	$A_7 = -3.C_8$	$B_0 = C_2$	$B_7 = 3.C_7$
$A_1 = C_3$	$A_8 = -3.C_7$	$B_1 = C_4$	$B_8 = -3.C_8$
$A_2 = -C_4$	$A_9 = C_8$	$B_2 = C_3$	$B_9 = -C_7$
$A_3 = C_5$	$A_{10} = C_{10}$	$B_3 = C_6$	$B_{10} = C_9$
$A_4 = -2.C_6$	$A_{11} = 4.C_9$	$B_4 = 2.C_5$	$B_{11} = -4.C_{10}$
$A_5 = -C_5$	$A_{12} = -6.C_{10}$	$B_5 = -C_6$	$B_{12} = -6.C_9$
$A_6 = C_7$	$A_{13} = -4.C_9$	$B_6 = C_8$	$B_{13} = 4.C_{10}$
	$A_{14} = C_{10}$		$B_{14} = C_9$

and the scaling parameter  $m$  is unity. The polynomial coefficient values have been empirically derived so in principle this is a transformation which induces inaccuracy into the output coordinates. However the polynomial is decreed to be exact and for practical purposes the Krovak Modified method is treated as a map projection. The embedding of this polynomial means that the Krovak Modified method is not truly conformal.

### Example

For Projected Coordinate Reference System: S-JTSK/05 (Ferro) / Modified Krovak

Parameters:

Ellipsoid:	Bessel 1841	$a = 6377397.155$ metres	$1/f = 299.15281$
	then	$e = 0.081696831$	$e^2 = 0.006674372$

Latitude of projection centre	$\varphi_C$	49°30'00"N	= 0.863937979 rad
Longitude of origin	$\lambda_O$	42°30'00"E of Ferro	
Co-latitude of cone axis	$\alpha_C$	30°17'17.30311"	= 0.528627763 rad.
Latitude of pseudo standard parallel	$\varphi_P$	78°30'00"N	= 1.370083463 rad
Scale factor on pseudo standard parallel	$k_P$	0.9999	
False Easting	FE	5000000.00	metres
False Northing	FN	5000000.00	metres
Ordinate 1 of evaluation point	$X_O$	1089000.00	metres
Ordinate 2 of evaluation point	$Y_O$	654000.00	metres
$C_1$		2.946529277E-02	
$C_2$		2.515965696E-02	
$C_3$		1.193845912E-07	
$C_4$		-4.668270147E-07	
$C_5$		9.233980362E-12	
$C_6$		1.523735715E-12	
$C_7$		1.696780024E-18	
$C_8$		4.408314235E-18	
$C_9$		-8.331083518E-24	
$C_{10}$		-3.689471323E-24	

Forward calculation for the same point as in the Krovak (south-orientated) method example:

Latitude	$\varphi$	= 50°12'32.442"N
Longitude	$\lambda_G$	= 34°30'59.179"E of Ferro

proceeds as in the original Krovak method as far as the calculation of  $X_p$  and  $Y_p$ .

Then:

$$\begin{aligned} X_r &= -38461.369 \\ Y_r &= -85009.005 \\ dX &= -0.077 \\ dY &= 0.088 \end{aligned}$$

$$\begin{aligned} \text{and Southing } X &= 6050538.71 \text{ metres} \\ \text{Westing } Y &= 5568990.91 \text{ metres} \end{aligned}$$

For the reverse calculation of these coordinates:

$$\begin{aligned} X_{r'} &= -38461.292 & Y_{r'} &= -85009.093 \\ dX' &= -0.077 & dY' &= 0.088 \\ X_{p'} &= 1050538.631 & Y_{p'} &= 568990.995 \\ r' &= 1194731.002 & \theta' &= 0.496385393 \\ D' &= 0.506554627 & T' &= 1.386275051 \\ U' &= 0.875596951 & V' &= 0.139422687 \end{aligned}$$

Then by iteration

$$\begin{aligned} \phi_1 &= 0.876310603 \text{ rad} & \phi_2 &= 0.876312562 \text{ rad} & \phi_3 &= 0.876312568 \text{ rad} \\ \text{Latitude } \phi &= 0.876312568 \text{ rad} & &= 50^\circ 12' 32.442'' \text{N} \\ \text{Longitude } \lambda &= 0.602425500 \text{ rad} & &= 34^\circ 30' 59.179'' \text{E of Ferro} \end{aligned}$$

### 3.1.2.4 Krovak Modified (North Orientated)

(EPSG Dataset coordinate operation method code 1043)

A further modification of the Krovak method is to apply the axis orientation change as described to the Krovak Modified method described in the previous section. The calculations for Southing and Westing is as for the Krovak Modified method (EPSG method code 1042), but then:

$$\begin{aligned} \text{Easting}_{\text{Krovak Modified (North Orientated)}} &X = - \text{Westing}_{\text{Krovak Modified}} \\ \text{Northing}_{\text{Krovak Modified (North Orientated)}} &Y = - \text{Southing}_{\text{Krovak Modified}} \end{aligned}$$

As with the Krovak (North Orientated) projection, the axis abbreviations X and Y are transposed from south to east and from west to north respectively.

For the reverse calculation of easting and northing to latitude and longitude,

$$\begin{aligned} \text{Southing}_{\text{Krovak Modified}} &= - \text{Northing}_{\text{Krovak Modified (North Orientated)}} \\ \text{Westing}_{\text{Krovak Modified}} &= - \text{Easting}_{\text{Krovak Modified (North Orientated)}} \end{aligned}$$

and the calculation then follows that of the Krovak Modified method.

### Example

For Projected Coordinate Reference System: S-JTSK/05 (Ferro) / Modified Krovak East North

Forward calculation for the same point as in the Krovak method in section 3.1.2.1 above:

$$\begin{aligned} \text{Latitude } \phi &= 50^\circ 12' 32.442'' \text{N} \\ \text{Longitude } \lambda_G &= 34^\circ 30' 59.179'' \text{E of Ferro} \end{aligned}$$

proceeds as in the example for the Krovak Modified method described in the previous section as far as the calculation of Southing and Westing.

$$\begin{array}{llll} \text{Then} & \text{Easting} & X & = -5568990.91 \text{ metres} \\ & \text{Northing} & Y & = -6050538.71 \text{ metres} \end{array}$$

Reverse calculation for the same Easting and Northing first gives

$$\begin{array}{ll} \text{Southing} & = 6050538.71 \\ \text{Westing} & = 5568990.91 \end{array}$$

and then continues as in the calculation for the Krovak Modified method to give

$$\begin{array}{llll} \text{Latitude} & \varphi & = & 50^{\circ}12'32.442''\text{N} \\ \text{Longitude} & \lambda_G & = & 34^{\circ}30'59.179''\text{E of Ferro} \end{array}$$

### 3.1.3 **Albers Equal Area**

(EPSG Dataset coordinate operation method code 9822)

To derive the projected coordinates of a point, geodetic latitude ( $\varphi$ ) is converted to authalic latitude ( $\beta$ ). The formulas to convert geodetic latitude and longitude ( $\varphi, \lambda$ ) to Easting (E) and Northing (N) are:

$$\begin{array}{ll} \text{Easting (E)} & = E_F + (\rho \sin \theta) \\ \text{Northing (N)} & = N_F + \rho_0 - (\rho \cos \theta) \end{array}$$

where

$$\begin{array}{l} \theta = n (\lambda - \lambda_0) \\ \rho = [a (C - n \alpha)^{0.5}] / n \\ \rho_0 = [a (C - n \alpha_0)^{0.5}] / n \end{array}$$

and

$$\begin{array}{l} C = m_1^2 + (n \alpha_1) \\ n = (m_1^2 - m_2^2) / (\alpha_2 - \alpha_1) \\ m_1 = \cos \varphi_1 / (1 - e^2 \sin^2 \varphi_1)^{0.5} \\ m_2 = \cos \varphi_2 / (1 - e^2 \sin^2 \varphi_2)^{0.5} \\ \alpha = (1 - e^2) \{ [\sin \varphi / (1 - e^2 \sin^2 \varphi)] - [1/(2e)] \ln [(1 - e \sin \varphi) / (1 + e \sin \varphi)] \} \\ \alpha_0 = (1 - e^2) \{ [\sin \varphi_0 / (1 - e^2 \sin^2 \varphi_0)] - [1/(2e)] \ln [(1 - e \sin \varphi_0) / (1 + e \sin \varphi_0)] \} \\ \alpha_1 = (1 - e^2) \{ [\sin \varphi_1 / (1 - e^2 \sin^2 \varphi_1)] - [1/(2e)] \ln [(1 - e \sin \varphi_1) / (1 + e \sin \varphi_1)] \} \\ \alpha_2 = (1 - e^2) \{ [\sin \varphi_2 / (1 - e^2 \sin^2 \varphi_2)] - [1/(2e)] \ln [(1 - e \sin \varphi_2) / (1 + e \sin \varphi_2)] \} \end{array}$$

The reverse formulas to derive the geodetic latitude and longitude of a point from its Easting and Northing values are:

$$\begin{array}{l} \varphi = \beta' + [(e^2/3 + 31e^4/180 + 517e^6/5040) \sin 2\beta'] + [(23e^4/360 + 251e^6/3780) \sin 4\beta'] \\ \quad + [(761e^6/45360) \sin 6\beta'] \end{array}$$

$$\lambda = \lambda_0 + (\theta / n)$$

where

$$\begin{array}{l} \beta' = \text{asin}(\alpha' / \{1 - [(1 - e^2) / 2e] \ln [(1 - e) / (1 + e)]\}) \\ \alpha' = [C - (\rho^2 n^2 / a^2)] / n \\ \rho = \{(E - E_F)^2 + [\rho_0 - (N - N_F)]^2\}^{0.5} \\ \theta = \text{atan2} \{(E - E_F), [\rho_0 - (N - N_F)]\} \end{array}$$

and C, n and  $\rho_0$  are as in the forward equations.

### Example

See USGS Professional Paper 1395, "Map Projections - A Working Manual" by John P. Snyder for an example including values of intermediate variables. Further examples of input and output may be found in test procedure 5109 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

#### 3.1.4 **Bonne**

(EPSG Dataset coordinate operation method code 9827)

The Bonne projection was frequently adopted for 18<sup>th</sup> and 19<sup>th</sup> century mapping, but being equal area rather than conformal its use for topographic mapping was replaced during the 20<sup>th</sup> century by conformal map projection methods.

The formulas to convert geodetic latitude and longitude ( $\phi$ ,  $\lambda$ ) to Easting and Northing are:

$$E = (\rho \sin T) + FE$$

$$N = (a m_0 / \sin \phi_0 - \rho \cos T) + FN$$

where

$$m = \cos \phi / (1 - e^2 \sin^2 \phi)^{0.5}$$

with  $\phi$  in radians and  $m_0$  for  $\phi_0$ , the latitude of the origin, derived in the same way.

$$M = a[(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)\phi - (3e^2/8 + 3e^4/32 + 45e^6/1024 + \dots)\sin 2\phi + (15e^4/256 + 45e^6/1024 + \dots)\sin 4\phi - (35e^6/3072 + \dots)\sin 6\phi + \dots]$$

with  $\phi$  in radians and  $M_0$  for  $\phi_0$ , the latitude of the origin, derived in the same way.

$$\rho = a m_0 / \sin \phi_0 + M_0 - M$$

$$T = a m (\lambda - \lambda_0) / \rho \quad \text{with } \lambda \text{ and } \lambda_0 \text{ in radians}$$

For the reverse calculation:

$$X = E - FE$$

$$Y = N - FN$$

$$\rho = \pm [X^2 + (a m_0 / \sin \phi_0 - Y)^2]^{0.5} \quad \text{taking the sign of } \phi_0$$

$$M = a m_0 / \sin \phi_0 + M_0 - \rho$$

$$\mu = M / [a (1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)]$$

$$e_1 = [1 - (1 - e^2)^{0.5}] / [1 + (1 - e^2)^{0.5}]$$

$$\phi = \mu + (3e_1/2 - 27e_1^3/32 + \dots)\sin 2\mu + (21e_1^2/16 - 55e_1^4/32 + \dots)\sin 4\mu + (151e_1^3/96 + \dots)\sin 6\mu + (1097e_1^4/512 - \dots)\sin 8\mu + \dots$$

$$m = \cos \phi / (1 - e^2 \sin^2 \phi)^{0.5}$$

If  $\phi_0$  is not negative

$$\lambda = \lambda_0 + \rho \{ \text{atan2}[X, (a m_0 / \sin \phi_0 - Y)] \} / a m$$

but if  $\phi_0$  is negative

$$\lambda = \lambda_0 + \rho \{ \text{atan2}[-X, -(a m_0 / \sin \phi_0 - Y)] \} / a m$$

In either case, if  $\phi = \pm 90^\circ$ ,  $m = 0$  and the equation for  $\lambda$  is indeterminate, so use  $\lambda = \lambda_0$ .

### 3.1.4.1 **Bonne (South Orientated)**

(EPSG Dataset coordinate operation method code 9828)

In Portugal a special case of the method with coordinate system axes positive south and west has been used for older mapping. The formulas are as for the general case above except:

$$W = FE - (\rho \sin T)$$

$$S = FN - (a \sin \varphi_0 / \sin \varphi - \rho \cos T)$$

In these formulas the terms FE and FN retain their definition, i.e. in the Bonne (South Orientated) method they increase the Westing and Southing value at the natural origin. In this method they are effectively false westing (FW) and false southing (FS) respectively.

For the reverse formulas, those for the standard Bonne method above apply, with the exception that:

$$X = FE - W$$

$$Y = FN - S$$

### 3.1.5 **American Polyconic**

(EPSG Dataset coordinate operation method code 9818)

This projection was popular before the advent of modern computers due to its ease of mechanical construction, particularly in the United States. It is neither conformal nor equal area, and is distortion-free only along the longitude of origin. A modified form of the polyconic projection was adopted in 1909 for the International Map of the World series of 1/1,000,000 scale topographic maps. A general study of the polyconic family of projections by Oscar Adams of the US Geological Survey was published in 1919 (and reprinted in 1934).

The formulas to derive the projected Easting and Northing coordinates are:

If  $\varphi = 0$ :

$$\text{Easting, } E = FE + a(\lambda - \lambda_0)$$

$$\text{Northing, } N = FN - M_0$$

If  $\varphi$  is not zero:

$$\text{Easting, } E = FE + v \cot \varphi \sin L$$

$$\text{Northing, } N = FN + M - M_0 + v \cot \varphi (1 - \cos L)$$

where  $L = (\lambda - \lambda_0) \sin \varphi$

$$v = a / (1 - e^2 \sin^2 \varphi)^{0.5}$$

$$M = a[(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)\varphi - (3e^2/8 + 3e^4/32 + 45e^6/1024 + \dots)\sin 2\varphi + (15e^4/256 + 45e^6/1024 + \dots)\sin 4\varphi - (35e^6/3072 + \dots)\sin 6\varphi + \dots]$$

with  $\varphi$  in radians and  $M_0$  for  $\varphi_0$ , the latitude of the origin, derived in the same way.

The reverse formulas to convert Easting and Northing projected coordinates to latitude and longitude require iteration. This iteration will not converge if  $(\lambda - \lambda_0) > 90^\circ$  but the projection should not be used in that range.

First  $M_0$  is calculated using the formula for  $M$  given in the forward case. Then:

If  $M_O = (N-FN)$  then:

$$\phi = 0$$

$$\lambda = \lambda_O + (E-FE)/a$$

If  $M_O$  does not equal  $(N-FN)$  then:

$$A = [M_O + (N-FN)] / a$$

$$B = A^2 + \{[(E-FE)^2] / a^2\}$$

$$C = (1 - e^2 \sin^2 \phi_n)^{0.5} \tan \phi_n \text{ initially using } \phi_n = A \text{ for the first iteration}$$

$M$  is found

$$H = Mn$$

$$J = H / a$$

$$\phi'' = \phi' - [A(C J + 1) - J - 0.5C(J^2 + B)] / \{e^2 \sin 2\phi' (J^2 + B - 2A J) / 4C + (A - J) (C H - [2 / \sin 2\phi']) - H\}$$

Then after solution of  $\phi$

$$\lambda = \lambda_O + \{a \sin[(E-FE) C / a]\} / \sin \phi$$

### Example

Examples of input and output may be found in test procedure 5107 of the Test Dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

## 3.2 Cylindrical Map Projections

### 3.2.1 Mercator

(EPSG Dataset coordinate operation method codes 9804 and 9805)

The Mercator map projection is a special limiting case of the Lambert Conic Conformal map projection with the equator as the single standard parallel. All other parallels of latitude are straight lines and the meridians are also straight lines at right angles to the equator, equally spaced. It is the basis for the transverse and oblique forms of the projection. It is little used for land mapping purposes but is in almost universal use for navigation charts. As well as being conformal, it has the particular property that straight lines drawn on it are lines of constant bearing. Thus navigators may derive their course from the angle the straight course line makes with the meridians.

Three variants of the Mercator projection are recognised, differentiated by the parameters used in the projection definition:

- Variant A, also known as Mercator (1SP). The projection is defined with the equator as the single standard parallel, with scale factor on the equator also defined. False grid coordinates are applied at the natural origin of the projection, the intersection of the equator and the longitude of origin. See figure 3 below. In the few cases in which the Mercator projection is used for terrestrial applications or land mapping, such as in Indonesia prior to the introduction of the Universal Transverse Mercator, this is the normal method of definition.
- Variant B, defined through the latitude of two parallels equidistant either side of the equator upon which the grid scale is true. False grid coordinates are applied at the natural origin of the projection, the intersection of the equator and the longitude of origin. See figure 3 below. Variants A and B are alternative ways of defining the same projection – coordinates of a point will have the same values regardless of whether the definition is variant A or variant B.
- Variant C, defined through the latitude of two parallels equidistant either side of the equator upon which the grid scale is true, as in variant (B). However in variant C false grid coordinates are applied

at a point other than the natural origin of the projection, called the false origin. See figure 4 below. If the values of the standard parallels and false grid coordinates are the same, coordinates for variant C will differ from those for variant B.

Variants A and B are special cases of the more general variant C. The defining parameters for the three Mercator variants are summarised in the following table:

EPSG Dataset coordinate operation method code: <u>Parameter</u>	Method		
	<u>1 SP</u>	<u>2 SPs</u>	
	<u>Variant A</u>	<u>Variant B</u>	<u>Variant C</u>
	<u>9804</u>	<u>9805</u>	<u>xxxx</u>
Latitude of natural origin ( $\phi_0$ )	(x) (note 1)		
Scale at natural origin ( $k_0$ )	x		
Latitude of standard parallel ( $\phi_1$ )		x (note 2)	x (note 2)
Latitude of false origin ( $\phi_F$ )			x
Longitude of natural origin ( $\lambda_0$ )	x	x	x
False easting (easting at natural origin) (FE)	x	x	
False northing (northing at natural origin) (FN)	x	x	
Easting at false origin ( $E_F$ )			x
Northing at false origin ( $N_F$ )			x

Notes:

1. In the formulas that follow the parameter *latitude of natural origin* ( $\phi_0$ ) is not used. However for the Mercator variant A (1SP) method, for completeness in CRS labelling the EPSG Dataset includes this parameter, which must have a value of zero.
2. For each of variants B and C (the 2SP cases), there is a pair of standard parallels with equal latitude value but different signs. Only one needs to be defined.

Several equations are common between the variants.



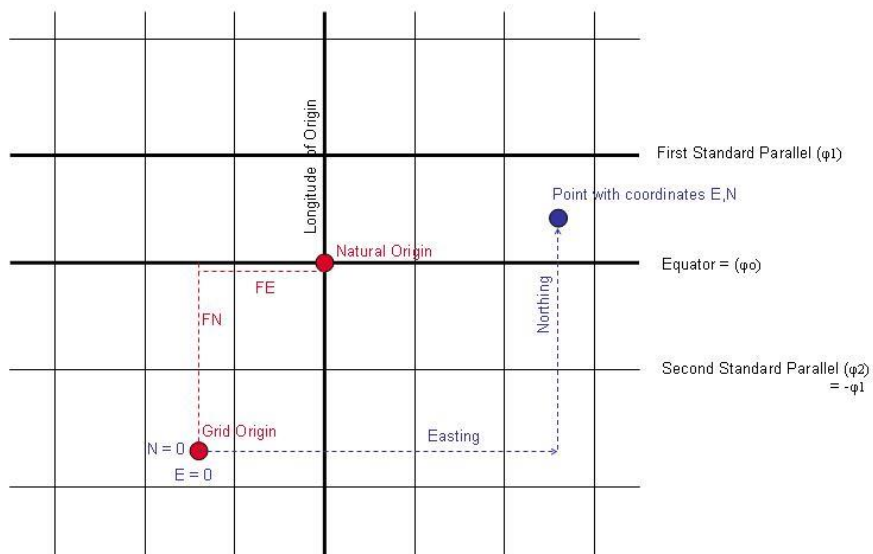


Figure 3 – Mercator variants A and B

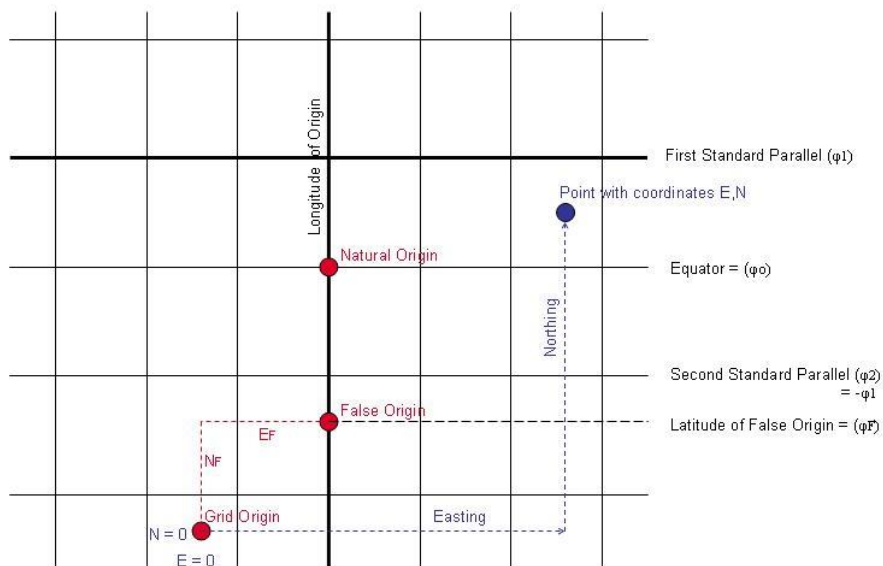


Figure 4 – Mercator variant C

The formulas to derive projected Easting and Northing coordinates are:

For variants B and C, the two standard parallel cases with false coordinates defined at the natural origin of the projection (variant B) or the false origin (variant C), first calculate  $k_O$ , the scale factor at the equator or natural origin, from

$$k_O = \cos \varphi_1 / (1 - e^2 \sin^2 \varphi_1)^{0.5}$$

where  $\varphi_1$  is the absolute value of the first standard parallel (i.e. positive).

Then, for both of variants A and B,

$$E = FE + a k_O (\lambda - \lambda_O)$$

$$N = FN + a k_O \ln \{ \tan(\pi/4 + \varphi/2) [(1 - \operatorname{esin} \varphi)/(1 + \operatorname{esin} \varphi)]^{(e/2)} \}$$

where symbols are as listed above and logarithms are natural.

but for variant C only:

$$M = a k_O \ln \{ \tan(\pi/4 + \varphi_F/2) [(1 - \operatorname{esin} \varphi_F)/(1 + \operatorname{esin} \varphi_F)]^{(e/2)} \}$$

$$E = E_F + a k_O (\lambda - \lambda_F)$$

$$N = (N_F - M) + a k_O \ln \{ \tan(\pi/4 + \varphi/2) [(1 - \operatorname{esin} \varphi)/(1 + \operatorname{esin} \varphi)]^{(e/2)} \}$$

The reverse formulas to derive latitude and longitude from E and N values are:

$$\begin{aligned} \varphi = \chi + & (e^2/2 + 5e^4/24 + e^6/12 + 13e^8/360) \sin(2\chi) \\ & + (7e^4/48 + 29e^6/240 + 811e^8/11520) \sin(4\chi) \\ & + (7e^6/120 + 81e^8/1120) \sin(6\chi) + (4279e^8/161280) \sin(8\chi) \end{aligned}$$

where for both variants A and B:

$$\chi = \pi/2 - 2 \operatorname{atan}(t)$$

$$t = B^{(FN-N)/(a \cdot k_O)}$$

B = base of the natural logarithm, 2.7182818...

and for the 2 SP variant B,  $k_O$  is calculated as for the forward transformation above.

Then:

$$\lambda = [(E - FE)/ak_O] + \lambda_O$$

For variant C:

$$t = B^{(NF - M - N)/(a \cdot k_O)}$$

where, B is base of the natural logarithm and M and  $k_O$  are calculated as for the forward transformation.

Then

$$\lambda = [(E - E_F)/ak_O] + \lambda_F$$

## Examples:

### 1. Mercator (variant A)

Also known as Mercator (1SP)

(EPSG Dataset coordinate operation method code 9804)

For Projected Coordinate Reference System: Makassar / NEIEZ

Parameters:

Ellipsoid:	Bessel 1841	a = 6377397.155metres	1/f = 299.15281
	then	e = 0.081696831	
Latitude of natural origin	$\phi_0$	0°00'00"N	= 0.0 rad
Longitude of natural origin	$\lambda_0$	110°00'00"E	= 1.91986218 rad
Scale factor at natural origin	$k_0$	0.997	
False easting	FE	3900000.00	metres
False northing	FN	900000.00	metres

Forward calculation for:

Latitude	$\phi$	=	3°00'00.00"S	=	-0.05235988 rad
Longitude	$\lambda$	=	120°00'00.00"E	=	2.09439510 rad

gives	Easting	E	=	5009726.58 metres
	Northing	N	=	569150.82 metres

Reverse calculation for same easting and northing first gives:

t	=	1.0534121
$\chi$	=	-0.0520110

Then	Latitude	$\phi$	=	3°00'00.000"S
	Longitude	$\lambda$	=	120°00'00.000"E

Further examples of input and output may be found in test procedure 5111 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 2. Mercator variant B

(EPSG Dataset coordinate operation method code 9805)

For Projected Coordinate Reference System: Pulkovo 1942 / Caspian Sea Mercator

Parameters:

Ellipsoid:	Krassowsky 1940	a = 6378245.0 metres	1/f = 298.3
	then	e = 0.08181333	e <sup>2</sup> = 0.00669342
Latitude of 1 <sup>st</sup> standard parallel	$\phi_1$	42°00'00"N	= 0.73303829 rad
Longitude of natural origin	$\lambda_0$	51°00'00"E	= 0.89011792 rad
False easting	FE	0.00	metres
False northing	FN	0.00	metres

then scale factor at natural origin  $k_0$  (at latitude of natural origin at 0°N) = 0.744260894.

Forward calculation for:

Latitude	$\phi$	=	53°00'00.00"N	=	0.9250245 rad
Longitude	$\lambda$	=	53°00'00.00"E	=	0.9250245 rad

gives Northing N = 5171848.07 metres  
 Easting E = 165704.29 metres

Reverse calculation for same northing and easting first gives:

t = 0.336391288  
 $\chi$  = 0.921795958

Then Latitude  $\varphi$  = 53°00'00.000"N  
 Longitude  $\lambda$  = 53°00'00.000"E

Further examples of input and output may be found in test procedure 5112 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 3. Mercator variant C

(EPSG Dataset coordinate operation method code 1044)

Parameters:

Ellipsoid: Krassowsky 1940 a = 6378245.0 metres 1/f = 298.3  
 then e = 0.08181333 e<sup>2</sup> = 0.00669342

Latitude of 1 <sup>st</sup> standard parallel	$\varphi_1$	42°00'00"N	=	0.73303829 rad
Longitude of natural origin	$\lambda_F$	51°00'00"E	=	0.89011792 rad
Latitude of false origin	$\varphi_F$	42°00'00"N	=	0.73303829 rad
Easting at false origin	$E_F$	0.00	metres	
Northing at false origin	$N_F$	0.00	metres	

First calculate projection constants:

$k_0$  = 0.744260894  
 M = 3819897.85

Forward calculation for:

Latitude  $\varphi$  = 53°00'00.00"N = 0.9250245 rad  
 Longitude  $\lambda$  = 53°00'00.00"E = 0.9250245 rad

gives Easting E = 165704.29 metres  
 Northing N = 1351950.22 metres

(Note the significant difference in northings from the variant B example).

Reverse calculation for same easting and northing first gives:

t = 0.336391288  
 $\chi$  = 0.921795958

Then Latitude  $\varphi$  = 53°00'00.000"N  
 Longitude  $\lambda$  = 53°00'00.000"E

### 3.2.1.1 Mercator (Spherical)

(EPSG Dataset coordinate operation method code 1026)

The formulas to derive projected Easting and Northing coordinates from latitude  $\phi$  and longitude  $\lambda$  using a sphere rather than ellipsoid as model of the earth are:

$$E = FE + R (\lambda - \lambda_0)$$

$$N = FN + R \ln[\tan(\pi/4 + \phi/2)]$$

where  $\lambda_0$  is the longitude of natural origin and FE and FN are false easting and false nothing.

R is the radius of the sphere and will normally be one of the CRS parameters. If the figure of the earth used is an ellipsoid rather than a sphere then R should be calculated as the radius of the conformal sphere at the projection origin at latitude  $\phi_0$  using the formula for  $R_c$  given in the Implementation Notes in this document.

If latitude  $\phi = 90^\circ$ , N is infinite. The above formula for N will fail near to the pole, and should not be used poleward of  $88^\circ$ .

The reverse formulas to derive latitude and longitude on the sphere from E and N values are:

$$D = -(N - FN) / R = (FN - N) / R$$

$$\phi = \pi/2 - 2 \operatorname{atan}(e^D) \text{ where } e = \text{base of natural logarithms, } 2.7182818\dots$$

$$\lambda = [(E - FE)/R] + \lambda_0$$

Note that in these formulas, the parameter *latitude of natural origin* ( $\phi_0$ ) is not used. However for the Mercator (Spherical) method, for completeness in CRS labelling the EPSG Dataset includes this parameter, which must have a value of zero.

#### Example

Parameters:

Sphere:	R = 6371007.0 metres			
Latitude of natural origin	$\phi_0$	0°00'00.000"N	=	0.0 rad
Longitude of natural origin	$\lambda_0$	0°00'00.000"E	=	0.0 rad
False easting	FE	0.00		metres
False northing	FN	0.00		metres

Forward calculation for:

Latitude	$\phi$	=	24°22'54.433"N	=	0.425542460 rad
Longitude	$\lambda$	=	100°20'00.000"W	=	-1.751147016 rad

whence

$$E = -11\,156\,569.90 \text{ m}$$

$$N = 2\,796\,869.94 \text{ m}$$

Reverse calculation for the same point (-11 156 569.90 m E, 2 796 869.94m N) first gives:

$$D = -0.438999665$$

Then	Latitude	$\phi$	=	0.425542460 rad	=	24°22'54.433"N
	Longitude	$\lambda$	=	-1.751147016 rad	=	100°20'00.000"W

### 3.2.1.2 Popular Visualisation Pseudo-Mercator ("Web Mercator")

(EPSG Dataset coordinate operation method code 1024)

This method is utilised by some popular web mapping and visualisation applications. Strictly speaking the inclusion of 'Mercator' in the method name is misleading: it is *not* a Mercator projection, it is a different map projection and uses its own distinct formula: it is a separate method. Unlike either the spherical or ellipsoidal Mercator projection methods, this method is not conformal: scale factor varies as a function of azimuth, which creates angular distortion. Despite angular distortion there is no convergence in the meridian, so the graticule has a similar appearance to the graticule of a Mercator projection, but the graticules of the two projections do not overlay. See IOGP publication 373-23 (Web Mercator) for further information.

The formulas to derive projected easting and northing coordinates from ellipsoidal latitude  $\phi$  and longitude  $\lambda$  are:

$$E = FE + a (\lambda - \lambda_0)$$

$$N = FN + a \ln[\tan(\pi/4 + \phi/2)]$$

where  $a$  is the ellipsoid semi-major axis,  $\lambda_0$  is the longitude of natural origin,  $FE$  and  $FN$  are false easting and false nothing and logarithms are natural.

If latitude  $\phi = 90^\circ$ ,  $N$  is infinite. The above formula for  $N$  will fail near to the pole, and should not be used poleward of  $88^\circ$ . Web applications often truncate the projection at approximately  $\pm 85.06^\circ$ , a value which coincides with the boundary of tiles constructed on the map grid.

The reverse formulas to derive latitude and longitude on the ellipsoid from  $E$  and  $N$  values are:

$$D = -(N - FN) / a = (FN - N) / a$$

$$\phi = \pi/2 - 2 \operatorname{atan}(e^D) \text{ where } e = \text{base of natural logarithms, } 2.7182818\dots$$

$$\lambda = [(E - FE)/a] + \lambda_0$$

Note that in these formulas the parameter *latitude of natural origin* ( $\phi_0$ ) is not used. However for completeness in CRS labelling the EPSG Dataset includes this parameter, which must have a value of zero.

#### Distortion

$q_\alpha$  is the scale factor at a given azimuth  $\alpha$ . It is a function of the radius of curvature of the ellipsoid at that azimuth.

$$q_\alpha = a / (R' \cos \phi)$$

where

$$R' = \rho v / (v \cos^2 \alpha + \rho \sin^2 \alpha)$$

$$\rho = a (1 - e^2) / (1 - e^2 \sin^2 \phi)^{3/2}$$

$$v = a / (1 - e^2 \sin^2 \phi)^{1/2}$$

$\rho$  and  $v$  are the radii of curvature of the ellipsoid at latitude  $\phi$  in the plane of the meridian and perpendicular to the meridian respectively;

Then when the azimuth is  $0^\circ$ ,  $180^\circ$ ,  $90^\circ$  or  $270^\circ$  the scale factors in the meridian ( $h$ ) and on the parallel ( $k$ ) are:

$$h = q_0 = q_{180} = a / (\rho \cos \phi)$$

$$k = q_{90} = q_{270} = a / (v \cos \phi)$$

$h$  and  $k$  are not equal, which demonstrates the non-conformality of the Pseudo-Mercator method.

Maximum angular distortion  $\omega$  is a function of latitude and is found from:

$$\omega = 2 \operatorname{asin}\{[\operatorname{ABS}(h - k)] / (h + k)\}$$

## Example

For Projected Coordinate Reference System: WGS 84 / Pseudo-Mercator (EPSG CRS code 3857)

Parameters:

Ellipsoid:	WGS 84	a = 6378137.0 metres	1/f = 298.2572236
Latitude of natural origin	$\phi_0$	0°00'00.000"N	= 0.0 rad
Longitude of natural origin	$\lambda_0$	0°00'00.000"E	= 0.0 rad
False easting	FE	0.00	metres
False northing	FN	0.00	metres

Forward calculation for the same coordinate values as used for the Mercator (Spherical) example:

Latitude	$\phi$	=	24°22'54.433"N	=	0.425542460 rad
Longitude	$\lambda$	=	100°20'00.000"W	=	-1.751147016 rad

whence

$$E = -11\,169\,055.58 \text{ m} \quad N = 2\,800\,000.00 \text{ m}$$

(in comparison, the spherical Mercator coordinates in 1.3.3.1 are E = -11 156 569.90m, N = 2 796 869.9m)

and

$$h = 1.1034264$$

$$k = 1.0972914$$

$$\omega = 0^\circ 19' 10.01''$$

Reverse calculation for a point 10km north on the grid (-11 169 055.58 m E, 2 810 000.00m N) first gives:

$$D = -0.44056752$$

Then	Latitude	$\phi$	=	0.426970023 rad	=	24°27'48.889"N
	Longitude	$\lambda$	=	-1.751147016 rad	=	100°20'00.000"W

In comparison, the same WGS 84 ellipsoidal coordinates when converted to the WGS 84 / World Mercator projected coordinate reference system (EPSG CRS code 3395) using the ellipsoidal Mercator (1SP) variant A method described above results in a grid distance between the two points of 9944.4m, a scale difference of ~0.5%.

<u>WGS 84</u>		<u>WGS 84 / Pseudo-Mercator</u> (EPSG CRS code 3857)		<u>WGS 84 / World Mercator</u> (EPSG CRS code 3395)	
Latitude	Longitude	Easting	Northing	Easting	Northing
24°27'48.889"N	100°20'00.000"W	-11169055.58m	2810000.00m	-11169055.58m	2792311.49m
24°22'54.433"N	100°20'00.000"W	-11169055.58m	2800000.00m	-11169055.58m	2782367.06m
		0.00m	10000.00m	0.00m	9944.43m

### 3.2.2 Cassini-Soldner

(EPSG Dataset coordinate operation method code 9806)

The Cassini-Soldner projection is the ellipsoidal version of the Cassini projection for the sphere. It is not conformal but as it is relatively simple to construct it was extensively used in the last century and is still useful for mapping areas with limited longitudinal extent. It has now largely been replaced by the conformal Transverse Mercator which it resembles. Like this, it has a straight central meridian along which the scale is true, all other meridians and parallels are curved, and the scale distortion increases rapidly with increasing distance from the central meridian.

The formulas to derive projected Easting and Northing coordinates are:

$$\text{Easting, } E = FE + v[A - TA^3/6 - (8 - T + 8C)TA^5/120]$$

$$\text{Northing, } N = FN + X$$

$$\text{where } X = M - M_0 + v \tan \varphi [A^2/2 + (5 - T + 6C)A^4/24]$$

$$A = (\lambda - \lambda_0) \cos \varphi$$

$$T = \tan^2 \varphi$$

$$C = e^2 \cos^2 \varphi / (1 - e^2)$$

$$v = a / (1 - e^2 \sin^2 \varphi)^{0.5}$$

and  $M$ , the distance along the meridian from equator to latitude  $\varphi$ , is given by

$$M = a[(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)\varphi - (3e^2/8 + 3e^4/32 + 45e^6/1024 + \dots)\sin 2\varphi + (15e^4/256 + 45e^6/1024 + \dots)\sin 4\varphi - (35e^6/3072 + \dots)\sin 6\varphi + \dots]$$

with  $\varphi$  in radians.

$M_0$  is the value of  $M$  calculated for the latitude of the natural origin  $\varphi_0$ . This may not necessarily be chosen as the equator.

To compute latitude and longitude from Easting and Northing the reverse formulas are:

$$\varphi = \varphi_1 - (v_1 \tan \varphi_1 / \rho_1)[D^2/2 - (1 + 3T_1)D^4/24]$$

$$\lambda = \lambda_0 + [D - T_1 D^3/3 + (1 + 3T_1)T_1 D^5/15]/\cos \varphi_1$$

where

$$v_1 = a / (1 - e^2 \sin^2 \varphi_1)^{0.5}$$

$$\rho_1 = a(1 - e^2)/(1 - e^2 \sin^2 \varphi_1)^{1.5}$$

$\varphi_1$  is the latitude of the point on the central meridian which has the same Northing as the point whose coordinates are sought, and is found from:

$$\varphi_1 = \mu_1 + (3e_1/2 - 27e_1^3/32 + \dots)\sin 2\mu_1 + (21e_1^2/16 - 55e_1^4/32 + \dots)\sin 4\mu_1 + (151e_1^3/96 + \dots)\sin 6\mu_1 + (1097e_1^4/512 - \dots)\sin 8\mu_1 + \dots$$

where

$$e_1 = [1 - (1 - e^2)^{0.5}]/[1 + (1 - e^2)^{0.5}]$$

$$\mu_1 = M_1/[a(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)]$$

$$M_1 = M_0 + (N - FN)$$

$$T_1 = \tan^2 \varphi_1$$

$$D = (E - FE)/v_1$$

### Example

For Projected Coordinate Reference System: Trinidad 1903 / Trinidad Grid

Parameters:

Ellipsoid:	Clarke 1858	$a = 20926348 \text{ ft}$	$=$	$31706587.88 \text{ Clarke's links}$
		$b = 20855233 \text{ ft}$		
	then	$1/f = 294.2606764$		$e^2 = 0.006785146$

Latitude of natural origin	$\varphi_0$	$10^\circ 26' 30'' \text{N}$	$=$	$0.182241463 \text{ rad}$
Longitude of natural origin	$\lambda_0$	$61^\circ 20' 00'' \text{W}$	$=$	$-1.070468608 \text{ rad}$
False easting	FE	$430000.00$		Clarke's links
False northing	FN	$325000.00$		Clarke's links

Forward calculation for:

Latitude	$\varphi$	$=$	$10^\circ 00' 00.00'' \text{N}$	$=$	$0.17453293 \text{ rad}$
Longitude	$\lambda$	$=$	$62^\circ 00' 00.00'' \text{W}$	$=$	$-1.08210414 \text{ rad}$



first gives :

$$\begin{array}{llll} A & = & -0.01145876 & C & = & 0.00662550 \\ T & = & 0.03109120 & M & = & 5496860.24 \\ v & = & 31709831.92 & M_0 & = & 5739691.12 \end{array}$$

$$\begin{array}{llll} \text{Then Easting} & E & = & 66644.94 \text{ Clarke's links} \\ \text{Northing} & N & = & 82536.22 \text{ Clarke's links} \end{array}$$

Reverse calculation for same easting and northing first gives:

$$\begin{array}{llll} e_1 & = & 0.00170207 & D & = & -0.01145875 \\ T_1 & = & 0.03109544 & M_1 & = & 5497227.34 \\ v_1 & = & 31709832.34 & \mu_1 & = & 0.17367306 \\ \phi_1 & = & 0.17454458 & \rho_1 & = & 31501122.40 \end{array}$$

$$\begin{array}{llll} \text{Then Latitude} & \phi & = & 10^{\circ}00'00.000''\text{N} \\ \text{Longitude} & \lambda & = & 62^{\circ}00'00.000''\text{W} \end{array}$$

Further examples of input and output may be found in test procedure 5108 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 3.2.2.1 Hyperbolic Cassini-Soldner

(EPSG Dataset coordinate operation method code 9833)

The grid for the island of Vanua Levu, Fiji, uses a modified form of the standard Cassini-Soldner projection known as the Hyperbolic Cassini-Soldner.

Easting is calculated as for the standard Cassini-Soldner above. The standard Cassini-Soldner formula to derive projected Northing is modified to:

$$\text{Northing, } N = FN + X - (X^3/6\rho v)$$

where  $\rho = a(1 - e^2)/(1 - e^2\sin^2\phi)^{1.5}$  and  $X$  and  $v$  are as in the standard Cassini-Soldner above.

For the reverse calculation of latitude and longitude from easting and northing the standard Cassini-Soldner formula given in the previous section need to be modified to account for the hyperbolic correction factor  $(X^3/6\rho v)$ . *Specifically for the Fiji Vanua Levu grid* the following may be used. The standard Cassini-Soldner formula given in the previous section are used except that the equation for  $M_1$  is modified to

$$M_1 = M_0 + (N - FN) + q$$

where

$$\begin{array}{ll} \phi_1' & = \phi_0 + (N - FN)/315320 \\ \rho_1' & = a(1 - e^2)/(1 - e^2\sin^2\phi_1')^{1.5} \\ v_1' & = a/(1 - e^2\sin^2\phi_1')^{0.5} \\ q' & = (N - FN)^3 / 6 \rho_1' v_1' \\ q & = (N - FN + q')^3 / 6 \rho_1' v_1' \end{array}$$

### Example

For Projected Coordinate Reference System: Vanua Levu 1915 / Vanua Levu Grid

Parameters:

Ellipsoid:	Clarke 1880	a = 20926202 ft	=	317063.667 chains
		b = 20854895 ft		
	then	1/f = 293.4663077		e <sup>2</sup> = 0.006803481
Latitude of natural origin	$\phi_0$	16°15'00"S	=	-0.283616003 rad
Longitude of natural origin	$\lambda_0$	179°20'00"E	=	3.129957125 rad
False easting	FE	12513.318		chains
False northing	FN	16628.885		chains

Forward calculation for:

Latitude	$\phi$	=	16°50'29.2435"S	=	-0.293938867 rad
Longitude	$\lambda$	=	179°59'39.6115"E	=	3.141493807 rad

first gives :

A	=	0.011041875	C	=	0.006275088
T	=	0.091631819	M	=	-92590.02
v	=	317154.24	M <sub>0</sub>	=	-89336.59
$\rho$	=	315176.48	X	=	-3259.28

Then	Easting	E	=	16015.2890 chains
	Northing	N	=	13369.6601 chains

Reverse calculation for same easting and northing first gives: `

$\phi_1'$	=	0.293952249	$q'$	=	-0.058
$v_1'$	=	317154.25	q	=	-0.058
$\rho_1'$	=	315176.50			
$e_1$	=	0.001706681	D	=	0.011041854
$T_1$	=	0.091644092	M <sub>1</sub>	=	-92595.87
$v_1$	=	317154.25	$\mu_1$	=	-0.292540098
$\rho_1$	=	315176.51	$\phi_1$	=	-0.293957437

Then	Latitude	$\phi$	=	16°50'29.2435"S
	Longitude	$\lambda$	=	179°59'39.6115"E

### 3.2.3 **Transverse Mercator**

#### 3.2.3.1 **General Case**

(EPSG Dataset coordinate operation method code 9807)

The Transverse Mercator projection in its various forms is the basis for the most widely used projected coordinate reference system for world topographical and offshore mapping. All versions have the same basic characteristics and formulas. The differences which distinguish the different forms of the projection which are applied in different countries arise from variations in the choice of values for the coordinate conversion parameters, namely the latitude of the natural origin, the longitude of the natural origin (central meridian), the scale factor at the natural origin (on the central meridian), and the values of False Easting and False Northing, which embody the units of measurement, given to the origin. Additionally there are variations in the width of the longitudinal zones for the projections used in different territories.

The following table indicates the variations in the coordinate conversion parameters which distinguish the different forms of the Transverse Mercator projection and are used in the EPSG Dataset Transverse Mercator map projection operations:

TABLE 4  
**Transverse Mercator**

<b>Coordinate Operation Method Name</b>	<b>Areas used</b>	<b>Latitude of natural origin</b>	<b>Longitude of natural origin</b>	<b>CM Scale Factor</b>	<b>Zone width</b>	<b>False Easting</b>	<b>False Northing</b>
Transverse Mercator	Various, world wide	Various	Various	Various	Usually less than 6°	Various	Various
Gauss-Kruger	Former USSR, Yugoslavia, Germany, S. America, China	Usually 0°	Various, according to area of cover	Usually 1.000000	Often 6° or 3°, sometimes less than 3°	Various but often 500000 prefixed by zone number	Various
Gauss Boaga	Italy	Various	Various	0.9996	6°	Various	0m
Transverse Mercator (south oriented)	Southern Africa	0°	2° intervals E of 11°E	1.000000	2°	0m	0m
UTM North hemisphere	World wide equator to 84°N	Always 0°	6° intervals E of 177°W	Always 0.9996	Always 6°	500000 m	0m
UTM South hemisphere	World wide north of 80°S to equator	Always 0°	6° intervals E of 177°W	Always 0.9996	Always 6°	500000 m	10000000 m

In the EPSG Dataset the Transverse Mercator, Gauss-Kruger and Gauss-Boaga coordinate conversion methods are considered to be the same under the name “Transverse Mercator”. The most familiar and commonly used Transverse Mercator in the oil industry is the Universal Transverse Mercator (UTM), a global system of 120 TM zones where many of the defining parameters have the same value in each zone.

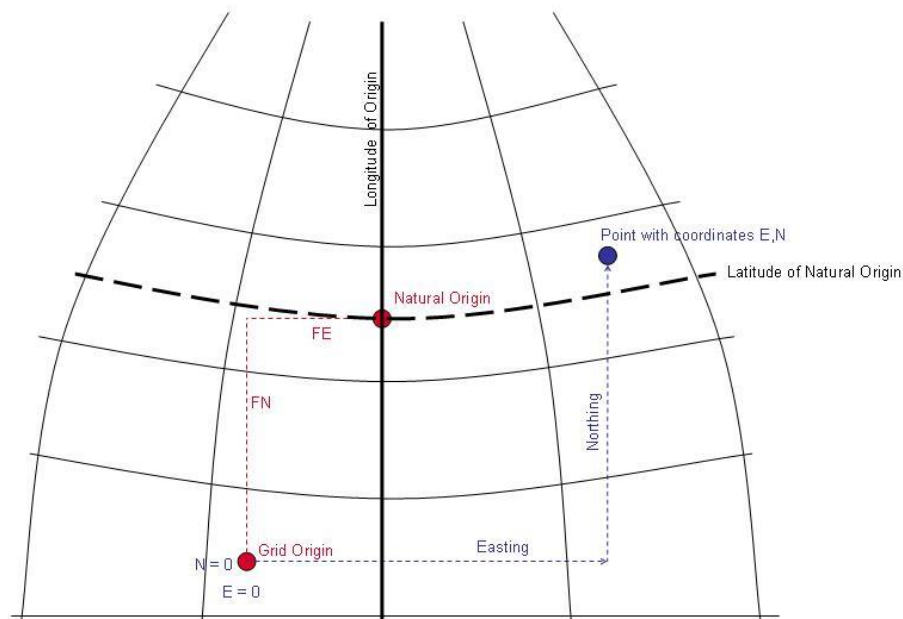


Figure 5 – Transverse Mercator in the northern hemisphere

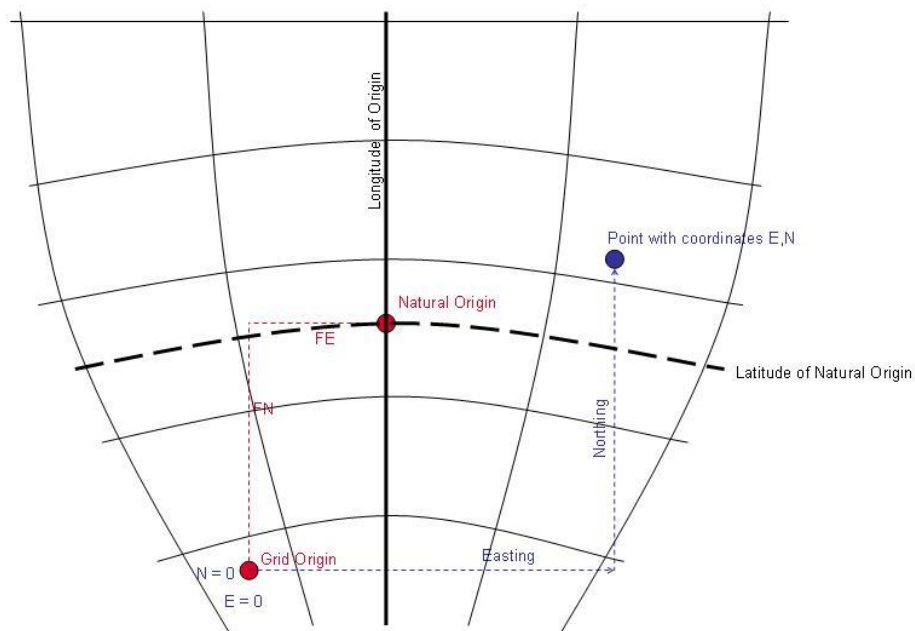


Figure 6 – Transverse Mercator in the southern hemisphere

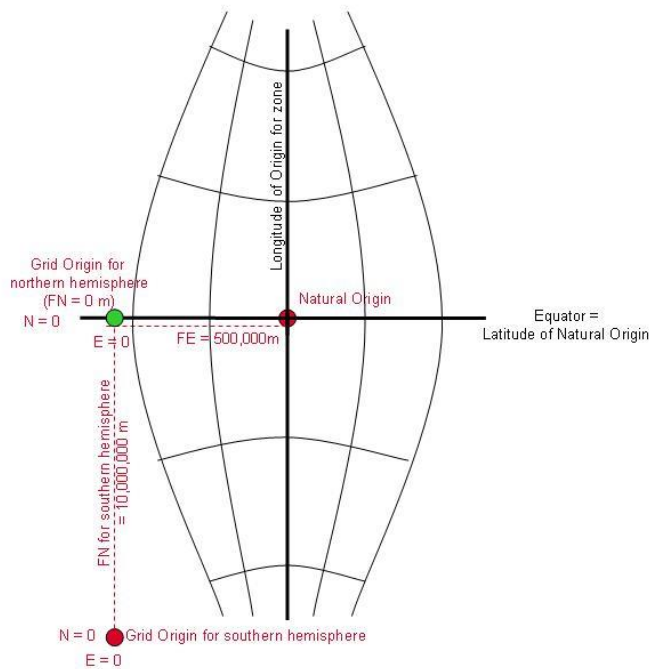


Figure 7 – One zone of the Universal Transverse Mercator system

From April 2010 the EPSG Dataset supports two distinct formulas for the Transverse Mercator projection (EPSG Dataset coordinate operation method code 9807):

- Our original formulas which are based on those published by Snyder in US Geological Survey Professional Paper #1395 (referred to below as ‘USGS formulas’)
- Formulas which are based on those of Krüger and published in Finland as Recommendations for Public Administration (JHS) 154 (referred to below as ‘JHS formulas’).

Within  $\pm 4^\circ$  of longitude east or west of the projection longitude of origin, the results from these two formulas can be considered the same. Within this longitude range differences in results are never more than 3mm (forward conversion) and 0.0005" (reverse conversion), with these differences greatest at the outer limits of this  $\pm 4^\circ$  longitude band and also at high latitudes. Both USGS and JHS formulas are satisfactory within  $\pm 4^\circ$  east or west of the longitude of origin, the area for which the characteristics of the projection method are best suited. However the JHS formulas are more robust over a wider area – to  $\pm 40^\circ$  of the longitude of origin – and have better forward and reverse round trip closure. For these reasons the JHS formulas are now recommended.

It must be recognised that although the JHS formulae allow more accurate coordinate conversion over this wide band, the increasing distortions in distance, area and angle which are inherent in the Transverse Mercator projection method well away from the longitude of origin cannot be avoided. These distortions make the method quite unsuitable in these outer areas for many purposes, for example engineering and large scale mapping. *Warning: more than  $\pm 4^\circ$  of longitude east or west of the projection longitude of origin, the USGS and JHS formulas produce different results. Their use should not be mixed, else location integrity will be compromised.*

JHS formulas:

For the calculation of easting and northing from latitude and longitude, first calculate\_constants for the projection:

n	=	f / (2-f)
B	=	[a/(1+n)] (1 + n <sup>2</sup> /4 + n <sup>4</sup> /64)
h <sub>1</sub>	=	n/2 – (2/3)n <sup>2</sup> + (5/16)n <sup>3</sup> + (41/180)n <sup>4</sup>
h <sub>2</sub>	=	(13/48)n <sup>2</sup> – (3/5)n <sup>3</sup> + (557/1440)n <sup>4</sup>
h <sub>3</sub>	=	(61/240)n <sup>3</sup> – (103/140)n <sup>4</sup>
h <sub>4</sub>	=	(49561/161280)n <sup>4</sup>

Then the meridional arc distance from equator to the projection origin (M<sub>O</sub>) is computed from:

If  $\varphi_0 = 0$  then  $M_0 = 0$

else if  $\varphi_0 \equiv 90^\circ\text{N} \equiv \pi/2$  radians then  $M_0 = B (\pi/2)$

else if  $\varphi_0 \equiv 90^\circ\text{S} \equiv -\pi/2$  radians then  $M_0 = B (-\pi/2)$

else

Q <sub>0</sub>	=	asinh(tan $\varphi_0$ ) – [e atanh(e sin $\varphi_0$ )]
$\beta_0$	=	atan(sinh Q <sub>0</sub> )
$\xi_{00}$	=	asin (sin $\beta_0$ )

Note: The previous two steps are taken from the generic calculation flow given below for latitude  $\varphi$ , but here for  $\varphi_0$  may be simplified to  $\xi_{00} = \beta_0 = \text{atan}(\sinh Q_0)$ .

$\xi_{01}$	=	h <sub>1</sub> sin(2 $\xi_{00}$ )
$\xi_{02}$	=	h <sub>2</sub> sin(4 $\xi_{00}$ )
$\xi_{03}$	=	h <sub>3</sub> sin(6 $\xi_{00}$ )
$\xi_{04}$	=	h <sub>4</sub> sin(8 $\xi_{00}$ )
$\xi_0$	=	$\xi_{00} + \xi_{01} + \xi_{02} + \xi_{03} + \xi_{04}$
M <sub>O</sub>	=	B $\xi_0$

end

Note: if the projection grid origin is very close to but not exactly at the pole (within 2" or 50m), the tangent function in the equation above for Q<sub>0</sub> is unstable and may fail. M<sub>O</sub> may instead be calculated (as in the USGS formula below) from:

$$M_0 = a[(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)\varphi_0 - (3e^2/8 + 3e^4/32 + 45e^6/1024 + \dots)\sin 2\varphi_0 + (15e^4/256 + 45e^6/1024 + \dots)\sin 4\varphi_0 - (35e^6/3072 + \dots)\sin 6\varphi_0 + \dots]$$

with  $\varphi_0$  in radians

Then

Q	=	asinh(tan $\varphi$ ) – [e atanh(e sin $\varphi$ )]			
$\beta$	=	atan(sinh Q)			
$\eta_0$	=	atanh [cos $\beta$ sin( $\lambda - \lambda_0$ )]			
$\xi_0$	=	asin (sin $\beta$ cosh $\eta_0$ )			
$\xi_1$	=	h <sub>1</sub> sin(2 $\xi_0$ ) cosh(2 $\eta_0$ )	$\eta_1$	=	h <sub>1</sub> cos(2 $\xi_0$ ) sinh(2 $\eta_0$ )
$\xi_2$	=	h <sub>2</sub> sin(4 $\xi_0$ ) cosh(4 $\eta_0$ )	$\eta_2$	=	h <sub>2</sub> cos(4 $\xi_0$ ) sinh(4 $\eta_0$ )
$\xi_3$	=	h <sub>3</sub> sin(6 $\xi_0$ ) cosh(6 $\eta_0$ )	$\eta_3$	=	h <sub>3</sub> cos(6 $\xi_0$ ) sinh(6 $\eta_0$ )
$\xi_4$	=	h <sub>4</sub> sin(8 $\xi_0$ ) cosh(8 $\eta_0$ )	$\eta_4$	=	h <sub>4</sub> cos(8 $\xi_0$ ) sinh(8 $\eta_0$ )
$\xi$	=	$\xi_0 + \xi_1 + \xi_2 + \xi_3 + \xi_4$	$\eta$	=	$\eta_0 + \eta_1 + \eta_2 + \eta_3 + \eta_4$

and

$$\text{Easting, } E = FE + k_0 B \eta$$

$$\text{Northing, } N = FN + k_0 (B \xi - M_0)$$

For the reverse formulas to convert Easting and Northing projected coordinates to latitude and longitude first calculate constants of the projection where  $n$  is as for the forward conversion, as are  $B$  and  $M_0$ :

$h_1'$	=	$n/2 - (2/3)n^2 + (37/96)n^3 - (1/360)n^4$
$h_2'$	=	$(1/48)n^2 + (1/15)n^3 - (437/1440)n^4$
$h_3'$	=	$(17/480)n^3 - (37/840)n^4$
$h_4'$	=	$(4397/161280)n^4$

Then

$\eta'$	=	$(E - FE) / (B k_0)$			
$\xi'$	=	$[(N - FN) + k_0 M_0] / (B k_0)$			
$\xi_1'$	=	$h_1' \sin(2\xi') \cosh(2\eta')$	$\eta_1'$	=	$h_1' \cos(2\xi') \sinh(2\eta')$
$\xi_2'$	=	$h_2' \sin(4\xi') \cosh(4\eta')$	$\eta_2'$	=	$h_2' \cos(4\xi') \sinh(4\eta')$
$\xi_3'$	=	$h_3' \sin(6\xi') \cosh(6\eta')$	$\eta_3'$	=	$h_3' \cos(6\xi') \sinh(6\eta')$
$\xi_4'$	=	$h_4' \sin(8\xi') \cosh(8\eta')$	$\eta_4'$	=	$h_4' \cos(8\xi') \sinh(8\eta')$
$\xi_0'$	=	$\xi' - (\xi_1' + \xi_2' + \xi_3' + \xi_4')$	$\eta_0'$	=	$\eta' - (\eta_1' + \eta_2' + \eta_3' + \eta_4')$
$\beta'$	=	$\text{asin}(\sin \xi_0' / \cosh \eta_0')$			
$Q'$	=	$\text{asinh}(\tan \beta')$			
$Q''$	=	$Q' + [e \text{ atanh}(e \tanh Q')]$			
	=	$Q' + [e \text{ atanh}(e \tanh Q'')] \text{ which should be iterated until the change in } Q'' \text{ is insignificant}$			

Then

$$\phi = \text{atan}(\sinh Q'')$$

$$\lambda = \lambda_0 + \text{asin}(\tanh(\eta_0') / \cos \beta')$$

USGS formulas (presented here only for backward compatibility within 4° of longitude of origin):

$$\text{Easting, } E = FE + k_0 v [A + (1 - T + C)A^3/6 + (5 - 18T + T^2 + 72C - 58e'^2)A^5/120]$$

$$\text{Northing, } N = FN + k_0 \{M - M_0 + v \tan \phi [A^2/2 + (5 - T + 9C + 4C^2)A^4/24 + (61 - 58T + T^2 + 600C - 330e'^2)A^6/720]\}$$

where  $T = \tan^2 \phi$

$$C = e^2 \cos^2 \phi / (1 - e^2)$$

$$A = (\lambda - \lambda_0) \cos \phi, \text{ with } \lambda \text{ and } \lambda_0 \text{ in radians}$$

$$v = a / (1 - e^2 \sin^2 \phi)^{0.5}$$

$$M = a[(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)\phi - (3e^2/8 + 3e^4/32 + 45e^6/1024 + \dots)\sin 2\phi + (15e^4/256 + 45e^6/1024 + \dots)\sin 4\phi - (35e^6/3072 + \dots)\sin 6\phi + \dots]$$

with  $\phi$  in radians and  $M_0$  for  $\phi_0$ , the latitude of the origin, derived in the same way.

The reverse formulas to convert Easting and Northing projected coordinates to latitude and longitude are:

$$\phi = \phi_1 - (v_1 \tan \phi_1 / \rho_1) [D^2/2 - (5 + 3T_1 + 10C_1 - 4C_1^2 - 9e'^2)D^4/24 + (61 + 90T_1 + 298C_1 + 45T_1^2 - 252e'^2 - 3C_1^2)D^6/720]$$

$$\lambda = \lambda_0 + [D - (1 + 2T_1 + C_1)D^3/6 + (5 - 2C_1 + 28T_1 - 3C_1^2 + 8e'^2 + 24T_1^2)D^5/120] / \cos \phi_1$$

where

$$v_1 = a / (1 - e^2 \sin^2 \varphi_1)^{0.5}$$

$$\rho_1 = a(1 - e^2) / (1 - e^2 \sin^2 \varphi_1)^{1.5}$$

$\varphi_1$  may be found as for the Cassini projection from:

$$\varphi_1 = \mu_1 + (3e_1/2 - 27e_1^3/32 + \dots)\sin 2\mu_1 + (21e_1^2/16 - 55e_1^4/32 + \dots)\sin 4\mu_1 \\ + (151e_1^3/96 + \dots)\sin 6\mu_1 + (1097e_1^4/512 - \dots)\sin 8\mu_1 + \dots$$

and where

$$e_1 = [1 - (1 - e^2)^{0.5}] / [1 + (1 - e^2)^{0.5}]$$

$$\mu_1 = M_1 / [a(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)]$$

$$M_1 = M_0 + (N - FN)/k_0$$

$$T_1 = \tan^2 \varphi_1$$

$$C_1 = e'^2 \cos^2 \varphi_1$$

$$e'^2 = e^2 / (1 - e^2)$$

$$D = (E - FE) / (v_1 k_0)$$

For areas south of the equator the value of latitude  $\varphi$  will be negative and the formulas above, to compute the E and N, will automatically result in the correct values. Note that the false northings of the origin, if the equator, will need to be large to avoid negative northings and for the UTM projection is in fact 10,000,000m. Alternatively, as in the case of Argentina's Transverse Mercator (Gauss-Kruger) zones, the origin is at the south pole with a northings of zero. However each zone central meridian takes a false easting of 500000m prefixed by an identifying zone number. This ensures that instead of points in different zones having the same eastings, every point in the country, irrespective of its projection zone, will have a unique set of projected system coordinates. Strict application of the above formulas, with south latitudes negative, will result in the derivation of the correct Eastings and Northings.

Similarly, in applying the reverse formulas to determine a latitude south of the equator, a negative sign for  $\varphi$  results from a negative  $\varphi_1$  which in turn results from a negative  $M_1$ .

## Examples

For Projected Coordinate Reference System OSGB 1936 / British National Grid

Parameters:

Ellipsoid:	Airy 1830	$a = 6377563.396$ metres	$1/f = 299.32496$
	then	$e^2 = 0.00667054$	$e'^2 = 0.00671534$
Latitude of natural origin	$\varphi_0$	49°00'00"N	= 0.85521133 rad
Longitude of natural origin	$\lambda_0$	2°00'00"W	= -0.03490659 rad
Scale factor at natural origin	$k_0$	0.9996012717	
False easting	FE	400000.00	metres
False northing	FN	-100000.00	metres

## JHS formulas:

Forward calculation for:

Latitude	$\varphi$	=	50°30'00.00"N	=	0.88139127 rad
Longitude	$\lambda$	=	00°30'00.00"E	=	0.00872665 rad

Constants of the projection:

$n$	=	0.00167322	$B$	=	6366914.609
$h_1$	=	0.0008347452	$h_2$	=	0.0000007554



$$\begin{aligned}
 h_3 &= 1.18487\text{E-}09 & h_4 &= 2.40864\text{E-}12 \\
 Q_0 &= 0.9787671618 \\
 \xi_{00} &= 0.8518980373 & \xi_{01} &= 0.0008273732 \\
 \xi_{02} &= -0.0000001986 & \xi_{03} &= -1.0918\text{E-}09 \\
 \xi_{04} &= 1.2218\text{E-}12 & M_0 &= 5429228.602 \\
 \\ 
 Q &= 1.0191767215 & \beta &= 0.8781064142 \\
 \eta_0 &= 0.0278629616 & \xi_0 &= 0.8785743280 \\
 \eta_1 &= -0.0000086229 & \xi_1 &= 0.0008215669 \\
 \eta_2 &= -0.0000000786 & \xi_2 &= -0.0000002768 \\
 \eta_3 &= 1.05551\text{E-}10 & \xi_3 &= -1.01855\text{E-}09 \\
 \eta_4 &= 3.97791\text{E-}13 & \xi_4 &= 1.67447\text{E-}12 \\
 \eta &= 0.0278542603 & \xi &= 0.8793956171
 \end{aligned}$$

Then Easting  $E = 577274.99$  metres  
 Northing  $N = 69740.50$  metres

Reverse calculation for same easting and northing first gives:

Then	$\underline{h}_1'$	=	0.0008347455		$\underline{h}_2'$	=	0.0000000586
	$\underline{h}_3'$	=	1.65563\text{E-}10		$\underline{h}_4'$	=	2.13692\text{E-}13
Then	$\xi'$	=	0.87939562		$\eta'$	=	0.0278542603
	$\xi_1'$	=	0.0008213109		$\eta_1'$	=	-0.0000086953
	$\xi_2'$	=	-0.0000000217		$\eta_2'$	=	-0.0000000061
	$\xi_3'$	=	-1.41881\text{E-}10		$\eta_3'$	=	1.486\text{E-}11
	$\xi_4'$	=	1.49609\text{E-}13		$\eta_4'$	=	3.50657\text{E-}14
	$\xi_0'$	=	0.8785743280		$\eta_0'$	=	0.0278629616
	$\beta'$	=	0.8781064142		$Q'$	=	1.0191767215
	$Q''$ 1 <sup>st</sup> iteration	=	1.0243166838		$Q''$ 2 <sup>nd</sup> iteration	=	1.0243306667
	$Q''$ 3 <sup>rd</sup> iteration	=	1.0243307046		$Q''$ 4 <sup>th</sup> iteration	=	1.0243307047

Then Latitude  $\phi = 50^\circ 30' 00.000''\text{N}$   
 Longitude  $\lambda = 00^\circ 30' 00.000''\text{E}$

#### USGS formulas:

Forward calculation for:

Latitude  $\phi = 50^\circ 30' 00.00''\text{N} = 0.88139127$  rad  
 Longitude  $\lambda = 00^\circ 30' 00.00''\text{E} = 0.00872665$  rad

first gives :

$$\begin{aligned}
 A &= 0.02775415 & C &= 0.00271699 \\
 T &= 1.47160434 & M &= 5596050.46 \\
 v &= 6390266.03 & M_0 &= 5429228.60
 \end{aligned}$$

Then Easting  $E = 577274.99$  metres  
 Northing  $N = 69740.50$  metres

Reverse calculation for same easting and northing first gives:

$$\begin{aligned}
 e_1 &= 0.00167322 & \mu_1 &= 0.87939562 \\
 M_1 &= 5599036.80 & v_1 &= 6390275.88
 \end{aligned}$$

$$\begin{array}{llll} \varphi_1 & = & 0.88185987 & D & = & 0.02775243 \\ \rho_1 & = & 6372980.21 & C_1 & = & 0.00271391 \\ T_1 & = & 1.47441726 & & & \end{array}$$

$$\begin{array}{llll} \text{Then Latitude } \varphi & = & 50^{\circ}30'00.000''\text{N} \\ \text{Longitude } \lambda & = & 00^{\circ}30'00.000''\text{E} \end{array}$$

Comparison of results from JHS and USGS formulas:

<u>Forward conversion, JHS minus USGS</u>								
	Latitude = 0°		Latitude = 25°		Latitude = 50°		Latitude = 75°	
	dE	dN	dE	dN	dE	dN	dE	dN
	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)
$\lambda = \lambda_0 + 1^{\circ}$	0	0	0	0	0	0	0	-1
$\lambda = \lambda_0 + 2^{\circ}$	0	0	0	0	0	0	0	-1
$\lambda = \lambda_0 + 3^{\circ}$	0	0	0	0	0	0	0	-1
$\lambda = \lambda_0 + 4^{\circ}$	1	0	0	0	0	0	0	-1
$\lambda = \lambda_0 + 5^{\circ}$	3	0	-1	0	-1	0	0	-1
$\lambda = \lambda_0 + 10^{\circ}$	404	0	-117	23	-75	-10	12	0
$\lambda = \lambda_0 + 20^{\circ}$	52772	0	-15803	5730	-9383	-2453	1584	161

<u>Reverse conversion, JHS minus USGS</u>								
	Latitude = 0°		Latitude = 25°		Latitude = 50°		Latitude = 75°	
	dN (")	dE (")	dN (")	dE (")	dN (")	dE (")	dN (")	dE (")
$\lambda = \lambda_0 + 1^{\circ}$	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
$\lambda = \lambda_0 + 2^{\circ}$	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
$\lambda = \lambda_0 + 3^{\circ}$	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
$\lambda = \lambda_0 + 4^{\circ}$	0.0000	0.0000	0.0000	-0.0001	0.0000	-0.0001	0.0000	-0.0002
$\lambda = \lambda_0 + 5^{\circ}$	0.0000	-0.0001	0.0000	-0.0002	0.0000	-0.0006	0.0000	-0.0011
$\lambda = \lambda_0 + 10^{\circ}$	0.0000	-0.0013	0.0024	-0.0322	0.0035	-0.0857	0.0018	-0.1408
$\lambda = \lambda_0 + 20^{\circ}$	0.0000	-1.7750	0.6895	-4.7213	0.0600	-12.7781	0.5258	-20.9142

Further examples of input and output may be found in test procedure 5101 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 3.2.3.2 Transverse Mercator Zoned Grid System (EPSG Dataset coordinate operation method code 9824)

When the growth in distortion away from the projection origin is of concern, a projected coordinate reference system cannot be used far from its origin. A means of creating a grid system over a large area but also limiting distortion is to have several grid zones with most defining parameters being made common. Coordinates throughout the system are repeated in each zone. To make coordinates unambiguous the easting is prefixed by the relevant zone number. This procedure was adopted by German mapping in the 1930's through the Gauss-Kruger systems and later by American military mapping through the Universal Transverse Mercator (or UTM) grid system. (Note: subsequent civilian adoption of the systems usually ignores the zone prefix to easting. Where this is the case the formulas below do not apply: use the standard TM formula separately for each zone).

The parameter Longitude of natural origin ( $\lambda_0$ ) is changed from being a defining parameter to a derived parameter, replaced by two other defining parameters, the Initial Longitude ( $\lambda_i$ ) (the western limit of zone 1)

and the Zone Width (W). Each of the remaining four Transverse Mercator defining parameters – Latitude of natural origin, Scale factor at natural origin, False easting and False northing – have the same parameter values in every zone.

The standard Transverse Mercator formulas above are modified as follows:

Zone number,  $Z$ , =  $[\text{INT}((\lambda - \lambda_i) / W) \bmod (360 / W)] + 1$  with  $\lambda$ ,  $\lambda_i$  and  $W$  in degrees.

In the formula above,  $\bmod = (a - \text{floor}(a / b) * b)$  where 'floor' is defined as rounding toward negative infinity.  $Z$  is in the range  $1 \leq Z \leq (360/W)$  and for UTM  $1 \leq Z \leq 60$ .

Then,

$$\lambda_o = \lambda_i + (Z W) - (W/2)$$

For the forward calculation,

$$\text{Easting, } E = Z * 10^6 + FE + k_o.v [A + (1 - T + C)A^3/6 + (5 - 18T + T^2 + 72C - 58e'^2)A^5/120]$$

and in the reverse calculation for longitude,

$$D = (E - [FE + Z * 10^6]) / (v_1 k_o)$$

### 3.2.3.3 **Transverse Mercator (South Orientated)** (EPSG Dataset coordinate operation method code 9808)

For the mapping of parts of southern Africa a south oriented Transverse Mercator map projection method is used. Here the coordinate axes are called Westings and Southings and increment to the West and South from the origin respectively. See figure 8 for a diagrammatic illustration (and compare this with figures 5 and 6).

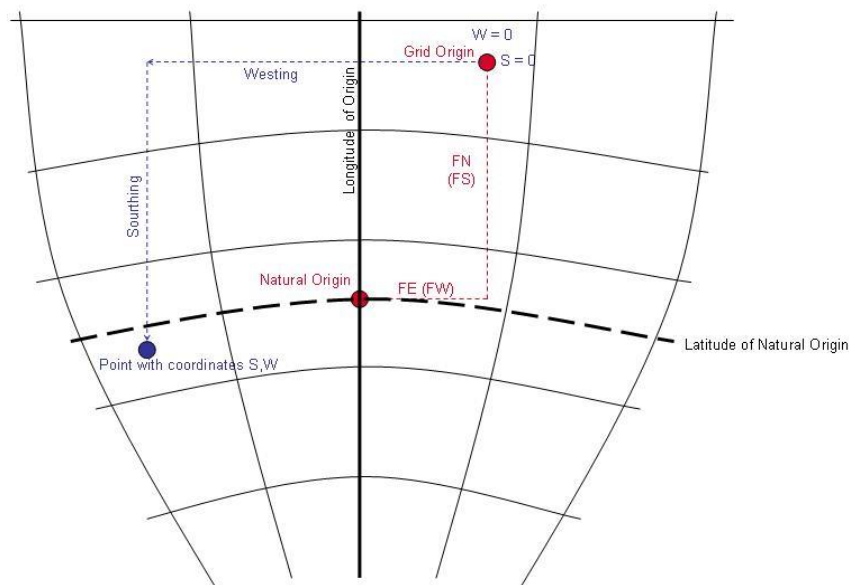


Figure 8 – Transverse Mercator (South Orientated) in the southern hemisphere

The general case of the Transverse Mercator formulas given above need to be modified to cope with this arrangement with

$$\text{Westing, } W = FE - k_0 v[A + (1 - T + C)A^3/6 + (5 - 18T + T^2 + 72C - 58e'^2)A^5/120]$$

$$\text{Southing, } S = FN - k_0\{M - M_0 + v \tan \phi[A^2/2 + (5 - T + 9C + 4C^2)A^4/24 + (61 - 58T + T^2 + 600C - 330e'^2)A^6/720]\}$$

In these formulas the terms FE and FN retain their definition, i.e. in the Transverse Mercator (South Orientated) method they increase the Westing and Southing value at the natural origin. In this method they are effectively false westing (FW) and false southing (FS) respectively.

For the reverse formulas, those for the general case of the Transverse Mercator given above apply, with the exception that:

$$M_1 = M_0 - (S - FN)/k_0$$

and  $D = -(W - FE)/(v_1 k_0)$ , with  $v_1 = v$  for  $\phi_1$

### Example

For Projected Coordinate Reference System Hartebeesthoek94 / Lo29

Parameters:

Ellipsoid:	WGS 84	a = 6378137 metres	1/f = 298.25722356
	then	e <sup>2</sup> = 0.006694380	e' <sup>2</sup> = 0.006739497
Latitude of natural origin	$\phi_0$	0°00'00"N	= 0.0 rad
Longitude of natural origin	$\lambda_0$	29°00'00"E	= 0.506145483 rad
Scale factor at natural origin	$k_0$	1.0	
False westing	FW	0.00	metres
False southing	FS	0.00	metres

Forward calculation for:

Latitude	$\phi$	= 25°43'55.302"S	= -0.449108618 rad
Longitude	$\lambda$	= 28°16'57.479"E	= 0.493625066 rad

first gives :

A	=	-0.11278823	C	=	0.005469120
T	=	0.232281740	M	=	-2847147.08
v	=	6382165.01	M <sub>0</sub>	=	0.00

Then	Westing	Y	=	71984.49 metres
	Southing	X	=	2847342.74 metres

Reverse calculation for same easting and northing first gives:

e <sub>1</sub>	=	0.001679220	$\mu_1$	=	-0.447171650
M <sub>1</sub>	=	-2847342.74	v <sub>1</sub>	=	6382165.53
$\phi_1$	=	-0.449139443	D	=	-0.011279007
$\rho_1$	=	6347451.58	C <sub>1</sub>	=	0.005468958
T <sub>1</sub>	=	0.232318356			

Then	Latitude	$\phi$	=	25°43'55.302"S
	Longitude	$\lambda$	=	28°16'57.479"E

Further examples of input and output may be found in test procedure 5113 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 3.2.4 **Oblique Mercator**

It has been noted that the Transverse Mercator map projection method is employed for the topographical mapping of longitudinal bands of territories, limiting the amount of scale distortion by limiting the extent of the projection either side of the central meridian. Sometimes the shape, general trend and extent of some countries makes it preferable to apply a single zone of the same kind of projection but with its central line aligned with the trend of the territory concerned rather than with a meridian. So, instead of a meridian forming this true scale central line for one of the various forms of Transverse Mercator, or the equator forming the line for the Mercator, a line with a particular azimuth traversing the territory is chosen and the same principles of construction are applied to derive what is now an Oblique Mercator. Such a single zone projection suits areas which have a large extent in one direction but limited extent in the perpendicular direction and whose trend is oblique to the bisecting meridian - such as East and West Malaysia and the Alaskan panhandle. The ellipsoidal development of the Oblique Mercator was originally applied at the beginning of the 20<sup>th</sup> century by Rosenmund to the mapping of Switzerland. Further developments were made in the 1920's by Laborde for the mapping of Madagascar and Reunion, and in the 1940's by Hotine for the mapping of Malaysia, for which he introduced the name Rectified Skew Orthomorphic.

These approaches differ slightly from each other in the use of intermediate surfaces used. Hotine projected the ellipsoid conformally onto a sphere of constant total curvature, called the 'aposphere', before projection onto the plane and then rotation of the grid to north. Like Hotine, Laborde used a triple projection technique to map the ellipsoid to the plane but in the Laborde method the rotation to north is made on the intermediate conformal sphere rather than in the projection plane. These different approaches give similar results, but unlike the Transverse Mercator method where different approaches are not significantly different within the normal area of use of the projection (sub-centimetre within 4° of the central meridian), with the Oblique Mercator formulas beyond a few hundred kilometres from the projection centre, particularly in the direction along the initial line, results from the Hotine development diverge rapidly from those given by Laborde's formulas.

EPSG therefore documents both the Hotine and Laborde Oblique Mercator methods. For the Hotine method we utilise Snyder's work which modified Hotine's hyperbolic functions to use exponential functions. And for the Hotine method we recognise two variants differentiated only by the point at which false grid coordinates are defined. Referring to figure 9 below, if the false grid coordinates are defined at the intersection of the initial line and the aposphere (the equator on one of the intermediate surfaces inherent in the method), that is at the natural origin of the coordinate system, the map projection method is known as variant A. If the false grid coordinates are defined at the projection centre the projection method is known as variant B. The defining parameters for these methods and variants are summarised in the following table:

	Method:	Hotine Variant A	Hotine Variant B	Laborde
EPSG Dataset coordinate operation method code:		9812	9815	9813
<b><u>Parameter</u></b>				
Latitude of the projection centre ( $\phi_C$ )		x	x	x
Longitude of the projection centre ( $\lambda_C$ )		x	x	x
Azimuth of the initial line ( $\alpha_C$ )		x	x	x
Angle from the rectified grid to the skew (oblique) grid ( $\gamma_C$ )		x	x	
Scale factor on the initial line ( $k_C$ )		x	x	x
False Easting (easting at the natural origin) (FE)		x		
False Northing (northing at the natural origin) (FN)		x		
Easting at the projection centre ( $E_C$ )			x	x
Northing at the projection centre ( $N_C$ )			x	x

(Note: if the Hotine (variant B) method is used as an approximation to the Laborde method, the additional parameter required by that method, the angle from the rectified grid to the skew (oblique) grid  $\gamma_C$ , takes the same value as the azimuth of the initial line passing through the projection centre, i.e.  $\gamma_C = \alpha_C$ )

Snyder describes a further variation of the Hotine method where the initial line is defined by two points through which it passes. This approach is not currently included in the EPSG Dataset.

Specific references for the formulas originally used in the individual cases of these projections are:

Switzerland: "Die Änderung des Projektionssystems der schweizerischen Landesvermessung." M. Rosenmund 1903. Also "Die projektionen der Schweizerischen Plan und Kartenwerke." J. Bollinger 1967 and "Formulas and constants for the calculation of the Swiss conformal cylindrical projection and for the transformation between coordinate systems." Federal Office of Topography (SwissTopo) 2008.  
 Madagascar: "La nouvelle projection du Service Geographique de Madagascar". J. Laborde 1928.  
 Malaysia: Series of Articles in numbers 62-66 of the Empire Survey Review of 1946 and 1947 by M. Hotine.

### 3.2.4.1 Hotine Oblique Mercator

(EPSG dataset coordinate operation method codes 9812 and 9815)

The Hotine Oblique Mercator coordinate system is defined by:

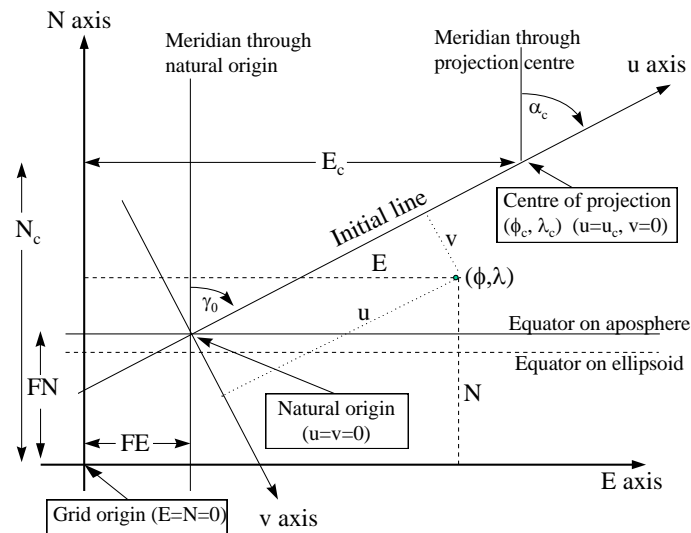


Figure 9. – Hotine Oblique Mercator Projection

The initial line central to the map area of given azimuth  $\alpha_c$  passes through a defined centre of the projection  $(\phi_c, \lambda_c)$ . The point where the projection of this line cuts the equator on the aposphere is the origin of the  $(u, v)$  coordinate system. The  $u$  axis is along the initial line and the  $v$  axis is perpendicular to ( $90^\circ$  clockwise from) this line.

In applying the formulas for both variants of the Hotine Oblique Mercator projection the first set of coordinates computed are referred to the  $(u, v)$  coordinate axes defined with respect to the initial line. These coordinates are then 'rectified' to the usual Easting and Northing by applying an orthogonal conversion. Hence the alternative name as the Rectified Skew Orthomorphic. The angle from rectified to skewed grid may be defined such that grid north coincides with true north at the natural origin of the projection, that is where the initial line of the projection intersects equator on the aposphere. In some circumstances, particularly where the projection is used in non-equatorial areas such as the Alaskan panhandle, the angle from rectified to skewed grid is defined to be identical to the azimuth of the initial line at the projection centre; this results in grid and true north coinciding at the projection centre rather than at the natural origin.

To ensure that all coordinates in the map area have positive grid values, false coordinates are applied. These may be given values ( $E_C$ ,  $N_C$ ) if applied at the projection centre [variant B] or be applied as false easting (FE) and false northing (FN) at the natural origin [variant A].

The formulas can be used for the following cases:

Alaska State Plane Zone 1  
 Hungary EOVS  
 East and West Malaysia Rectified Skew Orthomorphic grids  
 Swiss Cylindrical projection

The Swiss and Hungarian systems are a special case of variant B where the azimuth of the line through the projection centre is 90 degrees. The variant B formulas may also be used as an approximation to the Laborde Oblique Mercator method and has been used in this way for the Madagascar grid (see following section).

From the defining parameters the following constants for the map projection may be calculated for both variants of the Hotine Oblique Mercator:

$$\begin{aligned} B &= \{1 + [e^2 \cos^4 \varphi_C / (1 - e^2)]\}^{0.5} \\ A &= a B k_C (1 - e^2)^{0.5} / (1 - e^2 \sin^2 \varphi_C) \\ t_0 &= \tan(\pi/4 - \varphi_C / 2) / [(1 - e \sin \varphi_C) / (1 + e \sin \varphi_C)]^{e/2} \\ D &= B (1 - e^2)^{0.5} / [\cos \varphi_C (1 - e^2 \sin^2 \varphi_C)^{0.5}] \\ \text{To avoid problems with computation of F, if } D < 1 \text{ make } D^2 &= 1 \\ F &= D + (D^2 - 1)^{0.5} \cdot \text{SIGN}(\varphi_C) \\ H &= F t_0^B \\ G &= (F - 1/F) / 2 \\ \gamma_0 &= \text{asin}[\sin(\alpha_C) / D] \\ \lambda_0 &= \lambda_C - [\text{asin}(G \tan \gamma_0)] / B \end{aligned}$$

Then for variant B only (EPSG method code 9815), two further constants for the map projection, the ( $u_C$ ,  $v_C$ ) coordinates for the centre point ( $\varphi_C$ ,  $\lambda_C$ ), are calculated from:

$$v_C = 0$$

In general

$$u_C = (A / B) \text{atan2}[(D^2 - 1)^{0.5}, \cos(\alpha_C)] * \text{SIGN}(\varphi_C)$$

but for the special cases where  $\alpha_C = 90$  degrees (e.g. Hungary, Switzerland) then

$$u_C = A (\lambda_C - \lambda_0)$$

**Forward case:** To compute (E,N) from a given ( $\varphi, \lambda$ ), for both variants of the Hotine Oblique Mercator method:

$$\begin{aligned} t &= \tan(\pi / 4 - \varphi / 2) / [(1 - e \sin \varphi) / (1 + e \sin \varphi)]^{e/2} \\ Q &= H / t^B \\ S &= (Q - 1/Q) / 2 \\ T &= (Q + 1/Q) / 2 \\ V &= \sin(B (\lambda - \lambda_0)) \\ U &= (-V \cos(\gamma_0) + S \sin(\gamma_0)) / T \\ v &= A \ln[(1 - U) / (1 + U)] / (2 B) \end{aligned}$$

Then either (a) for variant A, (where the FE and FN values have been specified with respect to the natural origin of the ( $u$ ,  $v$ ) axes, EPSG method code 9812):

$$u = A \text{atan2}\{(S \cos \gamma_0 + V \sin \gamma_0), \cos[B (\lambda - \lambda_0)]\} / B$$

and the rectified skew coordinates are then derived from:

$$\begin{aligned} E &= v \cos(\gamma_C) + u \sin(\gamma_C) + FE \\ N &= u \cos(\gamma_C) - v \sin(\gamma_C) + FN \end{aligned}$$

or (b) for variant B (where the false easting and northing values ( $E_C$ ,  $N_C$ ) have been specified with respect to the centre of the projection ( $\varphi_C$ ,  $\lambda_C$ ) (EPSG method code 9815):

In general:

$$u = (A \operatorname{atan2}\{(S \cos \gamma_O + V \sin \gamma_O), \cos[B(\lambda - \lambda_O)]\} / B) - [ABS(u_C) * \operatorname{SIGN}(\varphi_C)]$$

but in the special case where  $\alpha_C = 90$  degrees:

if  $\lambda = \lambda_C$ ,  $u = 0$

$$\text{else } u = (A \operatorname{atan2}\{(S \cos \gamma_O + V \sin \gamma_O), \cos[B(\lambda - \lambda_O)]\} / B) - [ABS(u_C) * \operatorname{SIGN}(\varphi_C) * \operatorname{SIGN}(\lambda_C - \lambda)]$$

and in all cases of this variant B the rectified skew coordinates are then derived from:

$$\begin{aligned} E &= v \cos(\gamma_C) + u \sin(\gamma_C) + E_C \\ N &= u \cos(\gamma_C) - v \sin(\gamma_C) + N_C \end{aligned}$$

**Reverse case:** To compute ( $\varphi, \lambda$ ) from a given (E,N) :

For variant A:

$$\begin{aligned} v' &= (E - FE) \cos(\gamma_C) - (N - FN) \sin(\gamma_C) \\ u' &= (N - FN) \cos(\gamma_C) + (E - FE) \sin(\gamma_C) \end{aligned}$$

or for variant B:

$$\begin{aligned} v' &= (E - E_C) \cos(\gamma_C) - (N - N_C) \sin(\gamma_C) \\ u' &= (N - N_C) \cos(\gamma_C) + (E - E_C) \sin(\gamma_C) + [ABS(u_C) * \operatorname{SIGN}(\varphi_C)] \end{aligned}$$

then for both variants:

$$Q' = e^{-(B v' / A)} \text{ where } e \text{ is the base of natural logarithms.}$$

$$S' = (Q' - 1 / Q') / 2$$

$$T' = (Q' + 1 / Q') / 2$$

$$V' = \sin(B u' / A)$$

$$U' = (V' \cos(\gamma_O) + S' \sin(\gamma_O)) / T'$$

$$t' = \{H / [(1 + U') / (1 - U')]^{0.5}\}^{1/B}$$

$$\chi = \pi / 2 - 2 \operatorname{atan}(t')$$

$$\begin{aligned} \varphi &= \chi + \sin(2\chi) (e^2 / 2 + 5 e^4 / 24 + e^6 / 12 + 13 e^8 / 360) \\ &\quad + \sin(4\chi) (7 e^4 / 48 + 29 e^6 / 240 + 811 e^8 / 11520) \\ &\quad + \sin(6\chi) (7 e^6 / 120 + 81 e^8 / 1120) + \sin(8\chi) (4279 e^8 / 161280) \end{aligned}$$

$$\lambda = \lambda_O - \operatorname{atan2}[(S' \cos \gamma_O - V' \sin \gamma_O), \cos(B u' / A)] / B$$

### Examples:

For Projected Coordinate Reference System Timbalai 1948 / R.S.O. Borneo (m) using variant B: (EPSG Dataset coordinate operation method code 9815).



Parameters:

Ellipsoid:	Everest 1830 (1967 Definition)	a = 6377298.556 metres	1/f = 300.8017
	then	e = 0.081472981	e <sup>2</sup> = 0.006637847
Latitude of projection centre	$\phi_C$	4°00'00"N	= 0.069813170 rad
Longitude of projection centre	$\lambda_C$	115°00'00"E	= 2.007128640 rad
Azimuth of initial line	$\alpha_C$	53°18'56.9537"	= 0.930536611 rad
Angle from Rectified to Skew Grid	$\gamma_C$	53°07'48.3685"	= 0.927295218 rad
Scale factor on initial line	$k_C$	0.99984	
Easting at projection centre	$E_C$	590476.87	metres
Northings at projection centre	$N_C$	442857.65	metres

Constants for the map projection:

B	=	1.003303209	F	=	1.072121256
A	=	6376278.686	H	=	1.000002991
t <sub>0</sub>	=	0.932946976	$\gamma_0$	=	0.927295218
D	=	1.002425787	$\lambda_0$	=	1.914373469
D <sup>2</sup>	=	1.004857458			
u <sub>c</sub>	=	738096.09	v <sub>c</sub>	=	0.00

Forward calculation for:

Latitude	$\phi$	=	5°23'14.1129"N	=	0.094025313 rad
Longitude	$\lambda$	=	115°48'19.8196"E	=	2.021187362 rad

first gives :

t	=	0.910700729	Q	=	1.098398182
S	=	0.093990763	T	=	1.004407419
V	=	0.106961709	U	=	0.010967247
v	=	-69702.787	u	=	163238.163

Then	Easting	E	=	679245.73 metres
	Northing	N	=	596562.78 metres

Reverse calculation for same easting and northing first gives:

v'	=	-69702.787	u'	=	901334.257
Q'	=	1.011028053			
S'	=	0.010967907	T'	=	1.000060146
V'	=	0.141349378	U'	=	0.093578324
t'	=	0.910700729	$\chi$	=	0.093404829

Then	Latitude	$\phi$	=	5°23'14.113"N
	Longitude	$\lambda$	=	115°48'19.820"E

If the same projection is defined using variant A then:

False easting	FE	=	0.0 metres
False northing	FN	=	0.0 metres
Then	u	=	901334.257

and all other values are as for variant B.

Further examples of input and output may be found in test procedures 5105 and 5106 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 3.2.4.2 Laborde Oblique Mercator

(EPSG dataset coordinate operation method code 9813).

The defining parameters for the Laborde Oblique Mercator projection method are:

$\varphi_C$  = latitude of the projection centre  
 $\lambda_C$  = longitude of the projection centre  
 $\alpha_C$  = azimuth (true) of the initial line passing through the projection centre  
 $k_C$  = scale factor on the initial line of the projection  
 $E_C$  = Easting at the projection centre  
 $N_C$  = Northing at the projection centre

All angular units should be converted to radians prior to use and all longitudes reduced to the Paris Meridan using the Paris Longitude of 2.5969212963 grads (2°20'14.025"E) east of Greenwich.

From these defining parameters the following constants for the map projection may be calculated:

$$B = \{1 + [e^2 \cos^4 \varphi_C] / (1 - e^2)\}^{0.5}$$

$$\varphi_s = \arcsin[\sin \varphi_C / B]$$

$$R = a \cdot k_C \{ (1 - e^2)^{0.5} / [1 - e^2 \sin^2 \varphi_C] \}$$

$$C = \ln[\tan(\pi/4 + \varphi_s / 2)] - B \cdot \ln\{\tan(\pi/4 + \varphi_C / 2) ([1 - e \sin \varphi_C] / [1 + e \sin \varphi_C])^{(e/2)}\}$$

**Forward case:** To compute (E,N) from a given ( $\varphi, \lambda$ )

$$L = B \cdot (\lambda - \lambda_C)$$

$$q = C + B \cdot \ln\{\tan(\pi/4 + \varphi/2) ([1 - e \sin \varphi] / [1 + e \sin \varphi])^{(e/2)}\}$$

$$P = 2 \cdot \operatorname{atan}(\mathbf{e}^q) - \pi/2 \quad \text{where } \mathbf{e} \text{ is the base of natural logarithms.}$$

$$U = \cos P \cdot \cos L \cdot \cos \varphi_s + \sin P \cdot \sin \varphi_s$$

$$V = \cos P \cdot \cos L \cdot \sin \varphi_s - \sin P \cdot \cos \varphi_s$$

$$W = \cos P \cdot \sin L$$

$$d = (U^2 + V^2)^{0.5}$$

$$\text{if } d <> 0 \text{ then } L' = 2 \cdot \operatorname{atan}(V/(U+d)) \text{ and } P' = \operatorname{atan}(W/d)$$

$$\text{if } d = 0 \text{ then } L' = 0 \text{ and } P' = \operatorname{sign}(W) \cdot \pi/2$$

$$H = -L' + i \cdot \ln(\tan(\pi/4 + P'/2)) \quad \text{where } i^2 = -1$$

$$G = (1 - \cos(2 \cdot \alpha_C) + i \cdot \sin(2 \cdot \alpha_C)) / 12$$

$$E = E_C + R \cdot \operatorname{IMAGINARY}(H + G \cdot H^3)$$

$$N = N_C + R \cdot \operatorname{REAL}(H + G \cdot H^3)$$

**Reverse case:** To compute ( $\varphi, \lambda$ ) from a given (E,N):

$$G = (1 - \cos(2 \cdot \alpha_C) + i \cdot \sin(2 \cdot \alpha_C)) / 12 \quad \text{where } i^2 = -1$$

To solve for Latitude and Longitude, a re-iterative solution is required, where the first two elements are

$$H_0 = (N - FN) / R + i \cdot (E - FE) / R \quad \text{ie } k = 0$$

$$H_1 = H_0 / (H_0 + G \cdot H_0^3), \text{ i.e. } k = 1,$$

and in subsequent reiterations, k increments by 1

$$H_{k+1} = (H_0 + 2 \cdot G \cdot H_k^3) / (3 \cdot G \cdot H_k^2 + 1)$$

Re-iterate until  $\operatorname{ABSOLUTE}(\operatorname{REAL}([H_0 - H_k - G \cdot H_k^3])) < 1E-11$

$$L' = -1 \cdot \operatorname{REAL}(H_k)$$

$$P' = 2 \cdot \operatorname{atan}(\mathbf{e}^{\operatorname{IMAGINARY}(H_k)}) - \pi/2 \quad \text{where } \mathbf{e} \text{ is the base of natural logarithms.}$$

$$U' = \cos P' \cdot \cos L' \cdot \cos \varphi_s + \sin P' \cdot \sin L' \cdot \sin \varphi_s$$

$$V' = \sin P'$$

$$W' = \cos P' \cdot \cos L' \cdot \sin \varphi_s - \sin P' \cdot \sin L' \cdot \cos \varphi_s$$

$$d = (U'^2 + V'^2)^{0.5}$$

$$\text{if } d <> 0 \text{ then } L = 2 \operatorname{atan}[V'/(U'+d)] \text{ and } P = \operatorname{atan}(W'/d)$$

if  $d = 0$  then  $L = 0$  and  $P = \text{SIGN}(W') \cdot \pi/2$   
 $\lambda = \lambda_C + (L/B)$

$$q' = \{\ln[\tan(\pi/4 + P/2)] - C\}/B$$

The final solution for latitude requires a second re-iterative process, where the first element is

$\phi'_0 = 2 \cdot \text{atan}(\mathbf{e}^{q'}) - \pi/2$  where  $\mathbf{e}$  is the base of natural logarithms.

And the subsequent elements are

$$\phi'_k = 2 \cdot \text{atan}\left(\left(\frac{1 + \mathbf{e} \cdot \sin[\phi'_{k-1}]}{1 - \mathbf{e} \cdot \sin[\phi'_{k-1}]}\right)^{(e/2)} \cdot \mathbf{e}^{q'}\right) - \pi/2 \text{ for } k = 1 \rightarrow$$

Iterate until  $\text{ABSOLUTE}(\phi'_k - \phi'_{k-1}) < 1\text{E-}11$

$$\phi = \phi'_k$$

### Example:

For Projected Coordinate Reference System Tananarive (Paris) / Laborde Grid.

Parameters:

Ellipsoid:	International 1924	$a = 6378388$ metres	$1/f = 297$
		then $e = 0.081991890$	$e^2 = 0.006722670$

Latitude of projection centre	$\phi_C$	21 grads S	= -0.329867229 rad
Longitude of projection centre	$\lambda_C$	49 grads E of Paris	= 51.5969213 grads E of Greenwich
			= 0.810482544 rad
Azimuth of initial line	$\alpha_C$	21 grads	= 0.329867229 rad
Scale factor on initial line	$k_C$	0.9995	
Easting at projection centre	$E_C$	400000	metres
Northings at projection centre	$N_C$	800000	metres

Constants for the map projection:

$B$	=	1.002707541	$\phi_s$	=	-0.328942879
$R$	=	6358218.319	$C$	=	-0.0002973474

Forward calculation for:

Latitude	$\phi$	=	16°11'23.280"S	
			17.9886666667 grads S	= -0.282565315 rad
Longitude	$\lambda$	=	44°27'27.260"E of Greenwich	
			46.800381173 grads E of Paris	= 0.735138668 rad

first gives :

$L$	=	-0.034645081	$q$	=	-0.285595283	$P$	=	-0.281790207
$U$	=	0.998343010	$V$	=	-0.046948995	$W$	=	-0.033271994
$d$	=	0.999446334	$L'$	=	-0.046992297	$P'$	=	-0.033278135
$H$	=	0.046992297 -	$G$	=	0.017487082 +			
		0.033284279i			0.051075588i			

Then Easting	$E$	=	188333.848 metres
Northing	$N$	=	1098841.091 metres

Reverse calculation for same easting and northing first gives:

$G$	=	0.017487082 +	$H_0$	=	0.047000760 -	$H_1$	=	0.999820949 -
		0.051075588i			0.033290167i			0.000001503i
$H_k$	=	0.046992297 -	$L'$	=	-0.046992297	$P'$	=	-0.033278136

$$\begin{aligned}
 &0.033284279i \\
 U' &= 0.959982752 & V' &= -0.033271994 & W' &= -0.278075693 \\
 d &= 0.960559165 & L &= -0.3464508142 & P &= -0.281790207 \\
 q' &= -0.284527565 & \varphi'_0 &= -0.280764449
 \end{aligned}$$

$$\begin{aligned}
 \text{Then Latitude } \varphi &= -0.282565315 \text{ rad} = 17.9886666667 \text{ grads S} \\
 &= 16^\circ 11' 23.280'' \text{S} \\
 \text{Longitude } \lambda &= 0.735138668 \text{ rad} = 46.8003811733 \text{ grads East of Paris} \\
 &= 44^\circ 27' 27.260'' \text{E of Greenwich}
 \end{aligned}$$

Comparing the Hotine Oblique Mercator (variant B) method as an approximation of the Laborde formula:

Latitude	Greenwich Longitude	Using Laborde formula		Using Hotine variant B		dX (m)	dY (m)
		Northing X	Easting Y	Northing X	Easting Y		
18°54'S	47°30'E	799665.521	511921.054	799665.520	511921.054	0.00	0.00
16°12'S	44°24'E	1097651.447	182184.982	1097651.426	182184.985	0.02	0.00
25°40'S	45°18'E	50636.222	285294.334	50636.850	285294.788	0.63	0.45
12°00'S	49°12'E	1561109.146	701354.056	1561109.350	701352.935	0.20	1.12

Within 450 kilometres of the projection origin near Antananarivo, Laborde's formulas can be approximated to better than 2cm by the Hotine method, which is satisfactory for most purposes. But at 600 kilometres from the origin along the initial line the Hotine method approximates Laborde's formulas to no better than 1 metre.

### 3.2.5 Equidistant Cylindrical

(EPSG Dataset coordinate operation method code 1028)

The characteristics of the Equidistant Cylindrical projection are that the scale is true along two standard parallels equidistant from the equator (or at the equator if that is the standard parallel) and along the meridians. The formulas usually given for this method employ spherical equations with a mean radius of curvature sphere calculated from the latitude of standard parallel. This is a compromise, often satisfactory for the low resolution purposes to which it is put. However in the spherical implementation the distance is not true along the meridians nor along the standard parallel(s). Spherical formulas are given in the following section.

The ellipsoidal forward equations to convert latitude and longitude to easting and northing are

$$\begin{aligned}
 E &= FE + v_1 \cos \varphi_1 (\lambda - \lambda_0) \\
 N &= FN + M
 \end{aligned}$$

where

$$v_1 = a / (1 - e^2 \sin^2 \varphi_1)^{0.5} \text{ (see the Implementation Notes in this document)}$$

and

$$M = a(1 - e^2) \int_0^\varphi (1 - e^2 \sin^2 \varphi)^{-\frac{3}{2}} d\varphi \quad (1)$$

or

$$M = a \left[ \int_0^\varphi (1 - e^2 \sin^2 \varphi)^{\frac{1}{2}} d\varphi - e^2 \sin \varphi \cos \varphi / (1 - e^2 \sin^2 \varphi)^{\frac{1}{2}} \right] \quad (2)$$

The first calculation (1) of M above contains an elliptic integral of the third kind. The alternative calculation (2) of M contains an elliptic integral of the second kind. If software supports the functions for these integrals, then the functions can be used directly. Otherwise, the value of M can be computed through a

series equation. The following series equation is adequate for any ellipsoid with a flattening of 1/290 or less, which covers all earth-based ellipsoids of record.

$$\begin{aligned}
 M = & a[(1 - \frac{1}{4}e^2 - \frac{3}{64}e^4 - \frac{5}{256}e^6 - \frac{175}{16384}e^8 - \frac{441}{65536}e^{10} - \frac{4851}{1048576}e^{12} - \frac{14157}{4194304}e^{14})\varphi \\
 & + (-\frac{3}{8}e^2 - \frac{3}{32}e^4 - \frac{45}{1024}e^6 - \frac{105}{4096}e^8 - \frac{2205}{131072}e^{10} - \frac{6237}{524288}e^{12} - \frac{297297}{33554432}e^{14})\sin 2\varphi \\
 & + (\frac{15}{256}e^4 + \frac{45}{1024}e^6 + \frac{525}{16384}e^8 + \frac{1575}{65536}e^{10} + \frac{155925}{8388608}e^{12} + \frac{495495}{33554432}e^{14})\sin 4\varphi \\
 & + (-\frac{35}{3072}e^6 - \frac{175}{12288}e^8 - \frac{3675}{262144}e^{10} - \frac{13475}{1048576}e^{12} - \frac{385385}{33554432}e^{14})\sin 6\varphi \\
 & + (\frac{315}{131072}e^8 + \frac{2205}{524288}e^{10} + \frac{43659}{8388608}e^{12} + \frac{189189}{33554432}e^{14})\sin 8\varphi \\
 & + (-\frac{693}{1310720}e^{10} - \frac{6237}{5242880}e^{12} - \frac{297297}{167772160}e^{14})\sin 10\varphi \\
 & + (\frac{1001}{8388608}e^{12} + \frac{11011}{33554432}e^{14})\sin 12\varphi \\
 & + (-\frac{6435}{234881024}e^{14})\sin 14\varphi]
 \end{aligned}$$

The inverse equations are

$$\begin{aligned}
 \lambda &= \lambda_0 + X / (v_1 \cos \varphi_1) = \lambda_0 + X (1 - e^2 \sin^2 \varphi_1)^{1/2} / (a \cos \varphi_1) \\
 \varphi &= \mu + (\frac{3}{2}n - \frac{27}{32}n^3 + \frac{269}{512}n^5 - \frac{6607}{24576}n^7)\sin 2\mu \\
 &+ (\frac{21}{16}n^2 - \frac{55}{32}n^4 + \frac{6759}{4096}n^6)\sin 4\mu \\
 &+ (\frac{151}{96}n^3 - \frac{417}{128}n^5 + \frac{87963}{20480}n^7)\sin 6\mu \\
 &+ (\frac{1097}{512}n^4 - \frac{15543}{2560}n^6)\sin 8\mu \\
 &+ (\frac{8011}{2560}n^5 - \frac{69119}{6144}n^7)\sin 10\mu \\
 &+ (\frac{293393}{61440}n^6)\sin 12\mu \\
 &+ (\frac{6845701}{860160}n^7)\sin 14\mu
 \end{aligned}$$

where

X = E - FE

Y = N - FN

$$\mu = Y / [a(1 - \frac{1}{4}e^2 - \frac{3}{64}e^4 - \frac{5}{256}e^6 - \frac{175}{16384}e^8 - \frac{441}{65536}e^{10} - \frac{4851}{1048576}e^{12} - \frac{14157}{4194304}e^{14})]$$

$$n = \frac{1 - (1 - e^2)^{1/2}}{1 + (1 - e^2)^{1/2}}$$

### Example

For Projected Coordinate Reference System: WGS84 / World Equidistant Cylindrical  
Parameters:

Ellipsoid: WGS 1984    a = 6378137.0 metres    1/f = 298.257223563  
then    e = 0.08181919084262

Latitude of first standard parallel	$\varphi_1$	=	0°00'00.000"N	=	0.0 rad
Longitude of natural origin	$\lambda_0$	=	0°00'00.000"E	=	0.0 rad
False easting	FE	=	0.00	metres	
False northing	FN	=	0.00	metres	

Forward calculation for:

$$\begin{array}{llll} \text{Latitude} & \varphi & = & 55^{\circ}00'00.000''\text{N} & = & 0.959931086 \text{ rad} \\ \text{Longitude} & \lambda & = & 10^{\circ}00'00.000''\text{E} & = & 0.174532925 \text{ rad} \end{array}$$

First gives

$$\begin{array}{llll} \text{Radius of curvature in prime vertical at } \varphi_1 & v_1 & = & 6378137.0 \\ \text{Radius of curvature of parallel at } \varphi_1 & v_1 \cos \varphi_1 & = & 6378137.0 \\ \text{Meridional arc distance from equator to } \varphi & M & = & 6097230.3131 \end{array}$$

whence

$$\begin{array}{l} E = 1113194.91 \text{ m} \\ N = 6097230.31 \text{ m} \end{array}$$

Reverse calculation for the same Easting and Northing (1113194.91 E, 6097230.31 N) first gives:

$$\begin{array}{ll} \text{Rectifying latitude (radians)} & \mu = 0.9575624671 \\ \text{Second flattening} & n = 0.001679220386 \end{array}$$

$$\begin{array}{llll} \text{Then Latitude} & \varphi & = & 55^{\circ}00'00.000''\text{N} \\ \text{Longitude} & \lambda & = & 10^{\circ}00'00.000''\text{E} \end{array}$$

### 3.2.5.1 Equidistant Cylindrical (Spherical)

(EPSG Dataset coordinate operation method code 1029)

This method has one of the simplest formulas available. If the latitude of natural origin ( $\varphi_1$ ) is at the equator the method is also known as Plate Carrée. It is not used for rigorous topographic mapping because its distortion characteristics are unsuitable. Formulas are included to distinguish this map projection method from an approach sometimes mistakenly called by the same name and used for simple computer display of geographic coordinates – see Pseudo Plate Carrée below.

For the forward calculation of the Equidistant Cylindrical method:

$$\begin{array}{l} E = FE + R (\lambda - \lambda_0) \cos(\varphi_1) \\ N = FN + R \varphi \end{array}$$

where  $\varphi_1$ ,  $\lambda_0$ ,  $\varphi$  and  $\lambda$  are expressed in radians.

$R$  is the radius of the sphere and will normally be one of the CRS parameters. If the figure of the earth used is an ellipsoid rather than a sphere then  $R$  should be calculated as the radius of the conformal sphere at the projection origin at latitude  $\varphi_1$  using the formula for  $R_c$  given in the Implementation Notes in this document. Note however that if applying spherical formula to ellipsoidal coordinates, the equidistant projection properties are not preserved.

For the reverse calculation:

$$\begin{array}{l} \varphi = (N - FN) / R \\ \lambda = \lambda_0 + \{[E - FE] / [R \cos(\varphi_1)]\} \end{array}$$

where  $R$  is as for the forward method.

### **3.2.5.2 Pseudo Plate Carrée**

(EPSG Dataset coordinate operation method code 9825)

This is not a map projection in the true sense as the coordinate system units are angular (for example, decimal degrees) and therefore of variable linear scale. It is used only for depiction of graticule (latitude/longitude) coordinates on a computer display. The origin is at latitude ( $\varphi$ ) = 0, longitude ( $\lambda$ ) = 0. See above for the formulas for the proper Plate Carrée map projection method.

For the forward calculation:

$$X = \lambda$$

$$Y = \varphi$$

For the reverse calculation:

$$\varphi = Y$$

$$\lambda = X$$

### **3.2.6 Lambert Cylindrical Equal Area**

(EPSG Dataset coordinate operation method code 9835)

See USGS Professional Paper 1395, "Map Projections - A Working Manual" by John P. Snyder for formulas and example.

#### **3.2.6.1 Lambert Cylindrical Equal Area (Spherical)**

(EPSG Dataset coordinate operation method code 9834)

For the forward calculation for the normal aspect of the projection in which  $\varphi_1$  is the latitude of the standard parallel:

$$E = FE + R (\lambda - \lambda_0) \cos(\varphi_1)$$

$$N = FN + R \sin(\varphi) / \cos(\varphi_1)$$

where  $\varphi_1$ ,  $\varphi$  and  $\lambda$  are expressed in radians

R is the radius of the sphere and will normally be one of the CRS parameters. If the figure of the earth used is an ellipsoid rather than a sphere then R should be calculated as the radius of the authalic sphere using the formula for  $R_A$  given in the Implementation Notes in this document.

For the reverse calculation:

$$\varphi = \arcsin\{[(N - FN) / R] \cos(\varphi_1)\}$$

$$\lambda = \lambda_0 + \{[E - FE] / [R \cos(\varphi_1)]\}$$

where R is as for the forward method.

See USGS Professional Paper 1395, "Map Projections - A Working Manual" by John P. Snyder for formulas for oblique and polar aspects and examples.

### 3.3 Azimuthal Map Projections

#### 3.3.1 Stereographic

The Stereographic projection may be imagined to be a projection of the earth's surface onto a plane in contact with the earth at a single tangent point from a projection point at the opposite end of the diameter through that tangent point.

This projection is best known in its polar form and is frequently used for mapping polar areas where it complements the Universal Transverse Mercator used for lower latitudes. Its spherical form has also been widely used by the US Geological Survey for planetary mapping and the mapping at small scale of continental hydrocarbon provinces. In its transverse or oblique ellipsoidal forms it is useful for mapping limited areas centred on the point where the plane of the projection is regarded as tangential to the ellipsoid., e.g. the Netherlands. The tangent point is the origin of the projected coordinate system and the meridian through it is regarded as the central meridian. In order to reduce the scale error at the extremities of the projection area it is usual to introduce a scale factor of less than unity at the origin such that a unitary scale factor applies on a near circle centred at the origin and some distance from it.

The coordinate conversion from geographic to projected coordinates is executed via the distance and azimuth of the point from the centre point or origin. For a sphere the formulas are relatively simple. For the ellipsoid the parameters defining the conformal sphere at the tangent point as origin are first derived. The conformal latitudes and longitudes are substituted for the geodetic latitudes and longitudes of the spherical formulas for the origin and the point.

An alternative approach is given by Snyder, where, instead of defining a single conformal sphere at the origin point, the conformal latitude at each point on the ellipsoid is computed. The conformal longitude is then always equivalent to the geodetic longitude. This approach is a valid alternative to that given here, but gives slightly different results away from the origin point. The USGS formula is therefore considered by OGP to be a different coordinate operation method to that described here.

##### 3.3.1.1 Oblique and Equatorial Stereographic

(EPSG Dataset coordinate operation method code 9809)

Given the geodetic origin of the projection at the tangent point ( $\phi_0, \lambda_0$ ), the parameters defining the conformal sphere are:

$$\begin{aligned} R &= (\rho_0 v_0)^{0.5} \\ n &= \{1 + [(e^2 \cos^4 \phi_0) / (1 - e^2)]\}^{0.5} \\ c &= (n + \sin \phi_0) (1 - \sin \chi_0) / [(n - \sin \phi_0) (1 + \sin(\chi_0))] \end{aligned}$$

where:  $\sin \chi_0 = (w_1 - 1) / (w_1 + 1)$   
 $w_1 = [S_1 (S_2)^e]^n$   
 $S_1 = (1 + \sin \phi_0) / (1 - \sin \phi_0)$   
 $S_2 = (1 - e \sin \phi_0) / (1 + e \sin \phi_0)$

The conformal latitude and longitude of the origin ( $\chi_0, \Lambda_0$ ) are then computed from :

$$\begin{aligned} \chi_0 &= \sin^{-1} [(w_2 - 1) / (w_2 + 1)] \\ \text{where } S_1 \text{ and } S_2 &\text{ are as above and } w_2 = c [S_1 (S_2)^e]^n = c w_1 \\ \Lambda_0 &= \lambda_0 \end{aligned}$$



For any point with geodetic coordinates ( $\varphi, \lambda$ ) the equivalent conformal latitude and longitude ( $\chi, \Lambda$ ) are then computed from

$$\Lambda = n(\lambda - \Lambda_0) + \Lambda_0$$

and

$$\chi = \sin^{-1} [(w - 1)/(w + 1)]$$

where  $w = c [S_a (S_b)^e]^n$

$$S_a = (1 + \sin\varphi) / (1 - \sin\varphi)$$

$$S_b = (1 - e \cdot \sin\varphi) / (1 + e \cdot \sin\varphi)$$

Then

$$E = FE + 2 R k_0 \cos\chi \sin(\Lambda - \Lambda_0) / B$$

and

$$N = FN + 2 R k_0 [\sin\chi \cos\chi_0 - \cos\chi \sin\chi_0 \cos(\Lambda - \Lambda_0)] / B$$

where  $B = [1 + \sin\chi \sin\chi_0 + \cos\chi \cos\chi_0 \cos(\Lambda - \Lambda_0)]$

The reverse formulas to compute the geodetic coordinates from the grid coordinates involves computing the conformal values, then the isometric latitude and finally the geodetic values.

The parameters of the conformal sphere and conformal latitude and longitude at the origin are computed as above. Then for any point with Stereographic grid coordinates (E,N) :

$$\chi = \chi_0 + 2 \tan^{-1} \{ [(N - FN) - (E - FE) \tan(j/2)] / (2 R k_0) \}$$

$$\Lambda = j + 2 i + \Lambda_0$$

where  $g = 2 R k_0 \tan(\pi/4 - \chi_0/2)$

$$h = 4 R k_0 \tan \chi_0 + g$$

$$i = \text{atan2} \{ (E - FE), [h + (N - FN)] \}$$

$$j = \text{atan2} \{ (E - FE), [g - (N - FN)] \} - i$$

Geodetic longitude  $\lambda = (\Lambda - \Lambda_0) / n + \Lambda_0$

Isometric latitude  $\psi = 0.5 \ln \{ (1 + \sin\chi) / [c (1 - \sin\chi)] \} / n$

First approximation  $\varphi_1 = 2 \text{atan}(e^\psi) - \pi/2$  where  $e$ =base of natural logarithms.

$$\psi_i = \text{isometric latitude at } \varphi_i$$

where  $\psi_i = \ln \{ [\tan(\varphi_i/2 + \pi/4)] [(1 - e \sin\varphi_i)/(1 + e \sin\varphi_i)]^{(e/2)} \}$

Then iterate  $\varphi_{i+1} = \varphi_i - (\psi_i - \psi) \cos \varphi_i (1 - e^2 \sin^2 \varphi_i) / (1 - e^2)$

until the change in  $\varphi$  is sufficiently small.

If the projection is the equatorial case,  $\varphi_0$  and  $\chi_0$  will be zero degrees and the formulas are simplified as a result, but the above formulas remain valid.

For the polar version,  $\phi_0$  and  $\chi_0$  will be 90 degrees and the formulas become indeterminate. See below for formulas for the polar case.

For stereographic projections centred on points in the southern hemisphere, the signs of E, N,  $\lambda_0$  and  $\lambda$  must be reversed to be used in the equations and  $\phi$  will be negative anyway as a southerly latitude.

### Example:

For Projected Coordinate Reference System: Amersfoort / RD New

Parameters:

Ellipsoid:	Bessel 1841	a = 6377397.155 metres	1/f = 299.15281
	then	e = 0.08169683	
Latitude of natural origin	$\phi_0$	52°09'22.178"N	= 0.910296727 rad
Longitude of natural origin	$\lambda_0$	5°23'15.500"E	= 0.094032038 rad
Scale factor at natural origin	$k_0$	0.9999079	
False easting	FE	155000.00	metres
False northing	FN	463000.00	metres

Forward calculation for:

Latitude	$\phi$	=	53°N	=	0.925024504 rad
Longitude	$\lambda$	=	6°E	=	0.104719755 rad

first gives the conformal sphere constants:

$\rho_0 = 6374588.71$	$v_0 = 6390710.613$	
$R = 6382644.571$	$n = 1.000475857$	$c = 1.007576465$

where	$S_1 = 8.509582274$	$S_2 = 0.878790173$	$w_1 = 8.428769183$
	$\sin \chi_0 = 0.787883237$		

$w_2 = 8.492629457$	$\chi_0 = 0.909684757$	$\Lambda_0 = \lambda_0 = 0.094032038 \text{ rad}$
---------------------	------------------------	---

For the point ( $\phi, \lambda$ )	$\chi = 0.924394997$	$\Lambda = 0.104724841 \text{ rad}$
-----------------------------------	----------------------	-------------------------------------

hence  $B = 1.999870665$

and  $E = 196105.283 \text{ m}$   $N = 557057.739 \text{ m}$

Reverse calculation for the same Easting and Northing (196105.28E, 557057.74N) first gives:

$g = 4379954.188$   $h = 37197327.960$   $i = 0.001102255$   $j = 0.008488122$

then  $\Lambda = 0.10472467$  whence  $\lambda = 0.104719584 \text{ rad} = 6^\circ\text{E}$

Also	$\chi = 0.924394767$	and	$\psi = 1.089495123$
Then	$\phi_1 = 0.921804948$		$\psi_1 = 1.084170164$
	$\phi_2 = 0.925031162$		$\psi_2 = 1.089506925$
	$\phi_3 = 0.925024504$		$\psi_3 = 1.089495505$
	$\phi_4 = 0.925024504$		

Then	Latitude	$\phi$	=	53°00'00.000"N
	Longitude	$\lambda$	=	6°00'00.000"E

Further examples of input and output may be found in test procedure 5104 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### 3.3.1.2 Polar Stereographic

For the polar stereographic projection, three variants are recognised, differentiated by their defining parameters. In the basic variant (**variant A**) the latitude of origin is either the north or the south pole, at which is defined a scale factor at the natural origin, the meridian along which the northing axis increments and along which intersecting parallels increment *towards the north pole* (the longitude of origin), and false grid coordinates. In **variant B** instead of the scale factor at the pole being defined, the (non-polar) latitude at which the scale is unity – the standard parallel – is defined. In **variant C** the latitude of a standard parallel along which the scale is unity is defined; the intersection of this parallel with the longitude of origin is the false origin, at which grid coordinate values are defined.

<u>Parameter</u>	<u>Method</u>		
	<u>Variant A</u>	<u>Variant B</u>	<u>Variant C</u>
Latitude of natural origin ( $\phi_O$ )	(note 1)	(note 2)	(note 2)
Latitude of standard parallel ( $\phi_F$ )		x	x
Longitude of origin ( $\lambda_O$ )	x	x	x
Scale at natural origin ( $k_O$ )	x		
False easting (easting at natural origin = pole) (FE)	x	x	
False northing (northing at natural origin = pole) (FN)	x	x	
Easting at false origin ( $E_F$ )			x
Northing at false origin ( $N_F$ )			x

In all three variants the formulae for the **south pole case** are straightforward but some require modification for the **north pole case** to allow the longitude of origin going towards (as opposed to away from) the natural origin and for the anticlockwise increase in longitude value when viewed from the north pole (see figure 10). Several equations are common between the variants and cases.

Notes:

1. In variant A the parameter *Latitude of natural origin* is used only to identify which hemisphere case is required. The only valid entries are  $\pm 90^\circ$  or equivalent in alternative angle units.
2. For variants B and C, whilst it is mathematically possible for the standard parallel to be in the opposite hemisphere to the pole at which is the projection natural origin, such an arrangement would be unsatisfactory from a cartographic perspective as the rate of change of scale would be excessive in the area of interest. The EPSG Dataset therefore excludes the hemisphere of pole as a defining parameter for these variants. In the formulas that follow for these variants B and C, the hemisphere of pole is taken to be that of the hemisphere of the standard parallel.

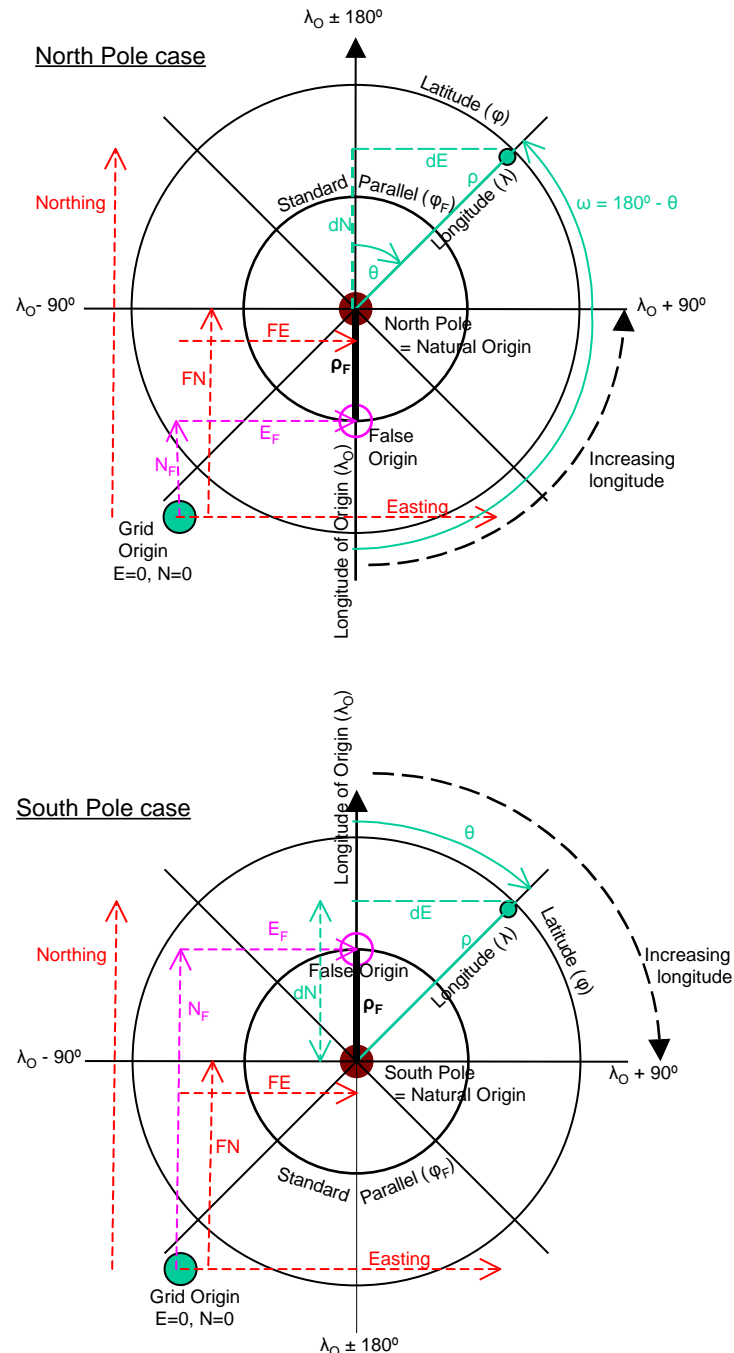


Figure 10 – Polar Stereographic Projection

Polar Stereographic (Variant A) (EPSG Dataset coordinate operation method code 9810).

For the forward conversion from latitude and longitude, for the south pole case

$$dE = \rho \sin(\theta)$$

and

$$dN = \rho \cos(\theta)$$

where  $\theta = (\lambda - \lambda_O)$

Then

$$E = dE + FE = FE + \rho \sin(\lambda - \lambda_O)$$

$$N = dN + FN = FN + \rho \cos(\lambda - \lambda_O)$$

where

$$t = \tan(\pi/4 + \varphi/2) / \{[(1 + e \sin \varphi) / (1 - e \sin \varphi)]^{(e/2)}\}$$

$$\rho = 2 a k_0 t / \{[(1+e)^{(1+e)}(1-e)^{(1-e)}]^{0.5}\}$$

For the north pole case,

$$dE = \rho \sin(\theta) = \rho \sin(\omega)$$

$$dN = \rho \cos(\theta) = -\rho \cos(\omega)$$

where, as shown in figure 10,  $\omega$  = longitude  $\lambda$  measured anticlockwise in the projection plane.

$\rho$  and  $E$  are found as for the south pole case but

$$t = \tan(\pi/4 - \varphi/2) \{[(1 + e \sin \varphi) / (1 - e \sin \varphi)]^{(e/2)}\}$$

$$N = FN - \rho \cos(\lambda - \lambda_0)$$

For the reverse conversion from easting and northing to latitude and longitude,

$$\begin{aligned} \varphi = \chi &+ (e^2/2 + 5e^4/24 + e^6/12 + 13e^8/360) \sin(2\chi) \\ &+ (7e^4/48 + 29e^6/240 + 811e^8/11520) \sin(4\chi) \\ &+ (7e^6/120 + 81e^8/1120) \sin(6\chi) + (4279e^8/161280) \sin(8\chi) \end{aligned}$$

where  $\rho' = [(E - FE)^2 + (N - FN)^2]^{0.5}$

$$t' = \rho' \{[(1+e)^{(1+e)}(1-e)^{(1-e)}]^{0.5}\} / 2 a k_0$$

and for the south pole case

$$\chi = 2 \operatorname{atan}(t') - \pi/2$$

but for the north pole case

$$\chi = \pi/2 - 2 \operatorname{atan}(t')$$

Then for both north and south cases if  $E = FE$ ,  $\lambda = \lambda_0$

else for the south pole case

$$\lambda = \lambda_0 + \operatorname{atan2}[(E - FE), (N - FN)]$$

and for the north pole case

$$\lambda = \lambda_0 + \operatorname{atan2}[(E - FE), (FN - N)]$$

### Example:

For Projected Coordinate Reference System: WGS 84 / UPS North

Parameters:

Ellipsoid: WGS 84     $a = 6378137.0$  metres     $1/f = 298.2572236$   
 then     $e = 0.081819191$

Latitude of natural origin	$\varphi_0$	90°00'00.000"N	=	1.570796327 rad
Longitude of natural origin	$\lambda_0$	0°00'00.000"E	=	0.0 rad
Scale factor at natural origin	$k_0$	0.994		
False easting	FE	2000000.00		metres
False northing	FN	2000000.00		metres

Forward calculation for:

Latitude	$\varphi$	=	73°N	=	1.274090354 rad
Longitude	$\lambda$	=	44°E	=	0.767944871 rad

$$t = 0.150412808$$

$$\rho = 1900814.564$$

whence

$$E = 3320416.75 \text{ m}$$

$$N = 632668.43 \text{ m}$$

Reverse calculation for the same Easting and Northing (3320416.75 E, 632668.43 N) first gives:

$$\begin{aligned}\rho' &= 1900814.566 \\ t' &= 0.150412808 \\ \chi &= 1.2722090\end{aligned}$$

$$\begin{array}{llll} \text{Then} & \text{Latitude} & \varphi & = 73^{\circ}00'00.000''\text{N} \\ & \text{Longitude} & \lambda & = 44^{\circ}00'00.000''\text{E} \end{array}$$

Polar Stereographic (Variant B) (EPSG Dataset coordinate operation method code 9829).

For the forward conversion from latitude and longitude:  
 for the south pole case

$$\begin{aligned}t_F &= \tan(\pi/4 + \varphi_F/2) / \{[(1 + e \sin \varphi_F) / (1 - e \sin \varphi_F)]^{(e/2)}\} \\ m_F &= \cos \varphi_F / (1 - e^2 \sin^2 \varphi_F)^{0.5} \\ k_O &= m_F \{[(1+e)^{(1+e)}(1-e)^{(1-e)}]^{0.5} / (2*t_F)\}\end{aligned}$$

then  $t$ ,  $\rho$ ,  $E$  and  $N$  are found as in the south pole case of variant A.

For the north pole case,  $m_F$  and  $k_O$  are found as for the south pole case above, but

$$t_F = \tan(\pi/4 - \varphi_F/2) \{[(1 + e \sin \varphi_F) / (1 - e \sin \varphi_F)]^{(e/2)}\}$$

Then  $t$ ,  $\rho$ ,  $E$  and  $N$  are found as in variant A.

For the reverse conversion from easting and northing to latitude and longitude, first  $k_O$  is found from  $m_F$  and  $t_F$  as in the forward conversion above, then  $\varphi$  and  $\lambda$  are found as for variant A.

### Example:

For Projected Coordinate Reference System: WGS 84 / Australian Antarctic Polar Stereographic

Parameters:

$$\begin{array}{llll} \text{Ellipsoid:} & \text{WGS 84} & a = 6378137.0 \text{ metres} & 1/f = 298.2572236 \\ & & \text{then } e = 0.081819191 & \end{array}$$

$$\begin{array}{llll} \text{Latitude of standard parallel} & \varphi_F & 71^{\circ}00'00.000''\text{S} & = -1.239183769 \text{ rad} \\ \text{Longitude of origin} & \lambda_O & 70^{\circ}00'00.000''\text{E} & = 1.221730476 \text{ rad} \\ \text{False easting} & FE & 6000000.00 & \text{metres} \\ \text{False northing} & FN & 6000000.00 & \text{metres} \end{array}$$

Forward calculation for:

$$\begin{array}{llll} \text{Latitude} & \varphi & = 75^{\circ}00'00.000''\text{S} & = -1.308996939 \text{ rad} \\ \text{Longitude} & \lambda & = 120^{\circ}00'00.000''\text{E} & = 2.094395102 \text{ rad} \end{array}$$

$$\begin{aligned}t_F &= 0.168407325 \\ m_F &= 0.326546781 \\ k_O &= 0.97276901 \\ t &= 0.132508348 \\ \rho &= 1638783.238\end{aligned}$$

whence

$$\begin{aligned}E &= 7255380.79 \text{ m} \\ N &= 7053389.56 \text{ m}\end{aligned}$$

Reverse calculation for the same Easting and Northing (7255380.79 E, 7053389.56 N) first gives:

$$\begin{array}{llll} t_F = 0.168407325 & m_F = 0.326546781 & \text{and } k_O = 0.97276901 \\ \text{then } \rho' = 1638783.236 & t' = 0.132508347 & \chi = -1.3073146 \end{array}$$

Then    Latitude       $\varphi$     =    75°00'00.000"S  
           Longitude     $\lambda$     =    120°00'00.000"E

Polar Stereographic (Variant C) (EPSG Dataset coordinate operation method code 9830).

For the forward conversion from latitude and longitude, for the south pole case

$$E = E_F + \rho \sin(\lambda - \lambda_O)$$

$$N = N_F - \rho_F + \rho \cos(\lambda - \lambda_O)$$

where

$$m_F \text{ is found as in variant B} = \cos \varphi_F / (1 - e^2 \sin^2 \varphi_F)^{0.5}$$

$$t_F \text{ is found as in variant B} = \tan(\pi/4 + \varphi_F/2) / \{[(1 + e \sin \varphi_F) / (1 - e \sin \varphi_F)]^{(e/2)}\}$$

$$t \text{ is found as in variants A and B} = \tan(\pi/4 + \varphi/2) / \{[(1 + e \sin \varphi) / (1 - e \sin \varphi)]^{(e/2)}\}$$

$$\rho_F = a m_F$$

$$\rho = \rho_F t / t_F$$

For the north pole case,  $m_F$ ,  $\rho_F$ ,  $\rho$  and  $E$  are found as for the south pole case but

$$t_F \text{ is found as in variant B} = \tan(\pi/4 - \varphi_F/2) / \{[(1 + e \sin \varphi_F) / (1 - e \sin \varphi_F)]^{(e/2)}\}$$

$$t \text{ is found as in variants A and B} = \tan(\pi/4 - \varphi/2) / \{[(1 + e \sin \varphi) / (1 - e \sin \varphi)]^{(e/2)}\}$$

$$N = N_F + \rho_F - \rho \cos(\lambda - \lambda_O)$$

For the reverse conversion from easting and northing to latitude and longitude,

$$\varphi = \chi + (e^2/2 + 5e^4/24 + e^6/12 + 13e^8/360) \sin(2\chi)$$

$$+ (7e^4/48 + 29e^6/240 + 811e^8/11520) \sin(4\chi)$$

$$+ (7e^6/120 + 81e^8/1120) \sin(6\chi) + (4279e^8/161280) \sin(8\chi)$$

(as for variants A and B)

where for the south pole case

$$\rho' = [(E - E_F)^2 + (N - N_F + \rho_F)^2]^{0.5}$$

$$t' = \rho' t_F / \rho_F$$

$$\chi = 2 \operatorname{atan}(t') - \pi/2$$

and where  $m_F$  and  $t_F$  are as for the forward conversion

For the reverse conversion north pole case,  $m_F$ ,  $t_F$  and  $\rho_F$  are found as for the north pole case of the forward conversion, and

$$\rho' = [(E - E_F)^2 + (N - N_F - \rho_F)^2]^{0.5}$$

$$t' \text{ is found as for the south pole case of the reverse conversion} = \rho' t_F / \rho_F$$

$$\chi = \pi/2 - 2 \operatorname{atan}(t')$$

Then for for both north and south pole cases

if  $E = E_F$ ,  $\lambda = \lambda_O$

else for the south pole case

$$\lambda = \lambda_O + \operatorname{atan2}[(E - E_F), (N - N_F + \rho_F)]$$

and for the north pole case

$$\lambda = \lambda_O + \operatorname{atan2}[(E - E_F), (N_F + \rho_F - N)]$$

### Example:

For Projected Coordinate Reference System: Petrels 1972 / Terre Adelie Polar Stereographic

Parameters:

Ellipsoid:	International 1924	a = 6378388.0 metres	1/f = 297.0
	then	e = 0.081991890	
Latitude of false origin	$\phi_F$	67°00'00.000"S	= -1.169370599 rad
Longitude of origin	$\lambda_O$	140°00'00.000"E	= 2.443460953 rad
Easting at false origin	$E_F$	300000.00	metres
Northing at false origin	$N_F$	200000.00	metres

Forward calculation for:

Latitude	$\phi$	=	66°36'18.820"S	=	-1.162480524 rad
Longitude	$\lambda$	=	140°04'17.040"E	=	2.444707118 rad

$$\begin{aligned} m_F &= 0.391848769 \\ \rho_F &= 2499363.488 \\ t_F &= 0.204717630 \\ t &= 0.208326304 \\ \rho &= 2543421.183 \end{aligned}$$

whence

$$\begin{aligned} E &= 303169.52 \text{ m} \\ N &= 244055.72 \text{ m} \end{aligned}$$

Reverse calculation for the same Easting and Northing (303169.522 E, 244055.721 N) first gives:

$$\begin{aligned} \rho' &= 2543421.183 \\ t' &= 0.208326304 \\ \chi &= -1.1600190 \\ \text{Then Latitude } \phi &= 66°36'18.820"S \\ \text{Longitude } \lambda &= 140°04'17.040"E \end{aligned}$$

### 3.3.2 Lambert Azimuthal Equal Area

(EPSG Dataset coordinate operation method code 9820)

#### Oblique aspect

To derive the projected coordinates of a point, geodetic latitude ( $\phi$ ) is converted to authalic latitude ( $\beta$ ). The formulae to convert geodetic latitude and longitude ( $\phi, \lambda$ ) to Easting and Northing are:

$$\begin{aligned} \text{Easting, } E &= FE + \{(B * D) [\cos \beta \sin(\lambda - \lambda_O)]\} \\ \text{Northing, } N &= FN + (B / D) \{(\cos \beta_O \sin \beta) - [\sin \beta_O \cos \beta \cos(\lambda - \lambda_O)]\} \end{aligned}$$

where

$$\begin{aligned} B &= R_q (2 / \{1 + \sin \beta_O \sin \beta + [\cos \beta_O \cos \beta \cos(\lambda - \lambda_O)]\})^{0.5} \\ D &= a [\cos \phi_O / (1 - e^2 \sin^2 \phi_O)^{0.5}] / (R_q \cos \beta_O) \\ R_q &= a (q_P / 2)^{0.5} \\ \beta &= \text{asin}(q / q_P) \\ \beta_O &= \text{asin}(q_O / q_P) \\ q &= (1 - e^2) ([\sin \phi / (1 - e^2 \sin^2 \phi)] - \{[1/(2e)] \ln [(1 - e \sin \phi) / (1 + e \sin \phi)]\}) \\ q_O &= (1 - e^2) ([\sin \phi_O / (1 - e^2 \sin^2 \phi_O)] - \{[1/(2e)] \ln [(1 - e \sin \phi_O) / (1 + e \sin \phi_O)]\}) \\ q_P &= (1 - e^2) ([\sin \phi_P / (1 - e^2 \sin^2 \phi_P)] - \{[1/(2e)] \ln [(1 - e \sin \phi_P) / (1 + e \sin \phi_P)]\}) \\ &\text{where } \phi_P = \pi/2 \text{ radians, thus} \\ q_P &= (1 - e^2) ([1 / (1 - e^2)] - \{[1/(2e)] \ln [(1 - e) / (1 + e)]\}) \end{aligned}$$



The reverse formulas to derive the geodetic latitude and longitude of a point from its Easting and Northing values are:

$$\varphi = \beta' + [(e^2/3 + 31e^4/180 + 517e^6/5040) \sin 2\beta'] + [(23e^4/360 + 251e^6/3780) \sin 4\beta'] + [(761e^6/45360) \sin 6\beta']$$

$$\lambda = \lambda_0 + \text{atan2} \{ (E-FE) \sin C, [D \rho \cos \beta_0 \cos C - D^2 (N-FN) \sin \beta_0 \sin C] \}$$

where

$$\beta' = \text{asin} \{ (\cos C \sin \beta_0) + [(D (N-FN) \sin C \cos \beta_0) / \rho] \}$$

$$C = 2 \text{ asin}(\rho / 2 R_q)$$

$$\rho = \{ [(E-FE)/D]^2 + [D (N-FN)]^2 \}^{0.5}$$

and D, R<sub>q</sub>, and β<sub>0</sub> are as in the forward equations.

### Example

For Projected Coordinate Reference System: ETRS89 / ETRS-LAEA

Parameters:

Ellipsoid: GRS 1980    a = 6378137.0 metres    1/f = 298.2572221  
    then    e = 0.081819191

Latitude of natural origin	φ <sub>0</sub>	52°00'00.000"N	=	0.907571211 rad
Longitude of natural origin	λ <sub>0</sub>	10°00'00.000"E	=	0.174532925 rad
False easting	FE	4321000.00	metres	
False northing	FN	3210000.00	metres	

Forward calculation for:

Latitude	φ	=	50°00'00.000"N	=	0.872664626 rad
Longitude	λ	=	5°00'00.000"E	=	0.087266463 rad

First gives

q <sub>p</sub> =	1.995531087	q <sub>0</sub> =	1.569825704
q =	1.525832247	R <sub>q</sub> =	6371007.181
β <sub>0</sub> =	0.905397517	β =	0.870458708
D =	1.000425395	B =	6374393.455

whence

E = 3962799.45 m  
 N = 2999718.85 m

Reverse calculation for the same Easting and Northing (3962799.45 E, 2999718.85 N) first gives:

ρ = 415276.208  
 C = 0.065193736  
 β' = 0.870458708

Then	Latitude	φ	=	50°00'00.000"N
	Longitude	λ	=	5°00'00.000"E

Further examples of input and output may be found in test procedure 5110 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

### **Polar aspect**

For the polar aspect of the Lambert Azimuthal Equal Area projection, some of the above equations are indeterminate. Instead, for the forward case from latitude and longitude ( $\phi$ ,  $\lambda$ ) to Easting (E) and Northing (N):

For the north polar case:

$$\begin{aligned}\text{Easting, } E &= FE + [\rho \sin(\lambda - \lambda_o)] \\ \text{Northing, } N &= FN - [\rho \cos(\lambda - \lambda_o)]\end{aligned}$$

where

$$\rho = a (q_P - q)^{0.5}$$

and  $q_P$  and  $q$  are found as for the general case above.

For the south polar case:

$$\begin{aligned}\text{Easting, } E &= FE + [\rho \sin(\lambda - \lambda_o)] \\ \text{Northing, } N &= FN + [\rho \cos(\lambda - \lambda_o)]\end{aligned}$$

where

$$\rho = a (q_P + q)^{0.5}$$

and  $q_P$  and  $q$  are found as for the general case above.

For the reverse formulas to derive the geodetic latitude and longitude of a point from its Easting and Northing:

$$\phi = \beta' + [(e^2/3 + 31e^4/180 + 517e^6/5040) \sin 2\beta'] + [(23e^4/360 + 251e^6/3780) \sin 4\beta'] + [(761e^6/45360) \sin 6\beta']$$

as for the oblique case, but where

$$\beta' = \pm a \sin [1 - \rho^2 / (a^2 \{1 - [(1 - e^2)/2e] \ln[(1 - e)/(1 + e)]\})], \text{ taking the sign of } \phi_o$$

$$\text{and } \rho = \{[(E - FE)]^2 + [(N - FN)]^2\}^{0.5}$$

Then

$$\lambda = \lambda_o + \text{atan2} [(E - FE), (N - FN)] \text{ for the south pole case}$$

and

$$\lambda = \lambda_o + \text{atan2} [(E - FE), -(N - FN)] = \lambda_o + \text{atan2} [(E - FE), (FN - N)] \text{ for the north pole case.}$$

#### **3.3.2.1 Lambert Azimuthal Equal Area (Spherical)**

(EPSG Dataset coordinate operation method code 1027)

The US National Atlas uses the spherical form of the oblique case, so exceptionally IOGP includes this method in the EPSG Dataset. See USGS Professional Paper 1395, "Map Projections - A Working Manual" by John P. Snyder for formulas and example.

$R$  is the radius of the sphere and will normally be one of the CRS parameters. If the figure of the earth used is an ellipsoid rather than a sphere then  $R$  should be calculated as the radius of the authalic sphere using the formula for  $R_A$  given in the implementation notes given at the beginning of this Guidance Note. Note however that if applying spherical formula to ellipsoidal coordinates, the authalic projection properties are not preserved.

### 3.3.3 Azimuthal Equidistant

#### 3.3.3.1 Modified Azimuthal Equidistant

(EPSG Dataset coordinate operation method code 9832)

For various islands in Micronesia the US National Geodetic Survey has developed formulae for the oblique form of the ellipsoidal projection which calculates distance from the origin along a normal section rather than the geodesic. For the distances over which these projections are used (under 800km) this modification introduces no significant error.

First calculate a constant for the projection:

$$v_0 = a / (1 - e^2 \sin^2 \phi_0)^{1/2}$$

Then the forward conversion from latitude and longitude is given by:

$$v = a / (1 - e^2 \sin^2 \phi)^{1/2}$$

$$\psi = \text{atan} [(1 - e^2) \tan \phi + e^2 v_0 \sin \phi_0 / (v \cos \phi)]$$

$$\alpha = \text{atan2} \{ \sin (\lambda - \lambda_0), [\cos \phi_0 \tan \psi - \sin \phi_0 \cos (\lambda - \lambda_0)] \}$$

$$G = e \sin \phi_0 / (1 - e^2)^{1/2}$$

$$H = e \cos \phi_0 \cos \alpha / (1 - e^2)^{1/2}$$

Then

$$\text{if } (\sin \alpha) = 0, s = \text{asin} (\cos \phi_0 \sin \psi - \sin \phi_0 \cos \psi) * \text{SIGN}(\cos \alpha)$$

$$\text{if } (\sin \alpha) \neq 0, s = \text{asin} [\sin (\lambda - \lambda_0) \cos \psi / \sin \alpha]$$

and in either case

$$c = v_0 s \{ [1 - s^2 H^2 (1 - H^2) / 6] + [(s^3/8)GH(1-2H^2)] + (s^4/120)[H^2(4-7H^2) - 3G^2(1-7H^2)] - [(s^5/48)GH] \}$$

Then

$$E = FE + c \sin \alpha$$

$$N = FN + c \cos \alpha$$

For the reverse conversion from easting and northing to latitude and longitude:

$$c' = [(E - FE)^2 + (N - FN)^2]^{0.5}$$

$$\alpha' = \text{atan2} [(E - FE), (N - FN)]$$

$$A = -e^2 \cos^2 \phi_0 \cos^2 \alpha' / (1 - e^2)$$

$$B = 3e^2 (1-A) \sin \phi_0 \cos \phi_0 \cos \alpha' / (1 - e^2)$$

$$D = c' / v_0$$

$$J = D - [A (1 + A) D^3 / 6] - [B (1 + 3A) D^4 / 24]$$

$$K = 1 - (A J^2 / 2) - (B J^3 / 6)$$

$$\Psi' = \text{asin} (\sin \phi_0 \cos J + \cos \phi_0 \sin J \cos \alpha')$$

Then

$$\phi = \text{atan} [(1 - e^2 K \sin \phi_0 / \sin \Psi') \tan \Psi' / (1 - e^2)]$$

$$\lambda = \lambda_0 + \text{asin} (\sin \alpha' \sin J / \cos \Psi')$$

**Example:**

For Projected Coordinate Reference System: Guam 1963 / Yap Islands

Parameters:

Ellipsoid:	Clarke 1866	a = 6378206.400 metres	1/f = 294.97870
	then	e = 0.08227185	e <sup>2</sup> = 0.00676866
Latitude of natural origin	φ <sub>0</sub>	9°32'48.15"N	= 0.166621493 rad
Longitude of natural origin	λ <sub>0</sub>	138°10'07.48"E	= 2.411499514 rad
False easting	FE	40000.00 metres	
False northing	FN	60000.00 metres	

Forward calculation for:

Latitude	φ	=	9°35'47.493"N	=	0.167490973 rad
Longitude	λ	=	138°11'34.908"E	=	2.411923377 rad

first gives

v <sub>0</sub> =	6378800.24	G =	0.013691332
v =	6378806.40	H =	0.073281276
ψ =	0.167485249	s =	0.000959566
α =	0.450640866	c =	6120.88

whence

E	=	42665.90
N	=	65509.82

Reverse calculation for the same Easting and Northing (42665.90 m E, 65509.82 m N) first gives:

c' =	6120.88	D =	0.000959566
α' =	0.450640866	J =	0.000959566
A =	-0.005370145	K =	1.000000002
B =	0.003026119	Ψ' =	0.167485249

whence

φ	=	0.167490973 rad	=	9°35'47.493"N
λ	=	2.411923377 rad	=	138°11'34.908"E

### 3.3.3.2 Guam Projection

(EPSG Dataset coordinate operation method code 9831)

The Guam projection is a simplified form of the oblique case of the azimuthal equidistant projection. For the Guam projection the forward conversion from latitude and longitude is given by:

$$x = a (\lambda - \lambda_0) \cos \varphi / [(1 - e^2 \sin^2 \varphi)^{(1/2)}]$$

$$E = FE + x$$

$$N = FN + M - M_0 + \{x^2 \tan \varphi [(1 - e^2 \sin^2 \varphi)^{(1/2)}] / (2a)\}$$

where

$$M = a[(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots)\varphi - (3e^2/8 + 3e^4/32 + 45e^6/1024 + \dots)\sin 2\varphi + (15e^4/256 + 45e^6/1024 + \dots)\sin 4\varphi - (35e^6/3072 + \dots)\sin 6\varphi + \dots]$$

with φ in radians and M<sub>0</sub> for φ<sub>0</sub>, the latitude of the natural origin, derived in the same way.

The reverse conversion from easting and northing to latitude and longitude requires iteration of three equations. The Guam projection uses three iterations, which is satisfactory over the small area of application. First M<sub>0</sub> for the latitude of the origin φ<sub>0</sub> is derived as for the forward conversion. Then:

$$e_1 = [1 - (1 - e^2)^{0.5}] / [1 + (1 - e^2)^{0.5}]$$

and

$$\begin{aligned} M' &= M_0 + (N - FN) - \{(E - FE)^2 \tan \phi_0 [(1 - e^2 \sin^2 \phi_0)^{(1/2)}] / (2a)\} \\ \mu' &= M' / a(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots) \\ \phi' &= \mu' + (3e_1/2 - 27e_1^3/32)\sin(2\mu') + (21e_1^2/16 - 55e_1^4/32)\sin(4\mu') + (151e_1^3/96)\sin(6\mu') \\ &\quad + (1097e_1^4/512)\sin(8\mu') \end{aligned}$$

$$\begin{aligned} M'' &= M_0 + (N - FN) - \{(E - FE)^2 \tan \phi' [(1 - e^2 \sin^2 \phi')^{(1/2)}] / (2a)\} \\ \mu'' &= M'' / a(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots) \\ \phi'' &= \mu'' + (3e_1/2 - 27e_1^3/32)\sin(2\mu'') + (21e_1^2/16 - 55e_1^4/32)\sin(4\mu'') + (151e_1^3/96)\sin(6\mu'') \\ &\quad + (1097e_1^4/512)\sin(8\mu'') \end{aligned}$$

$$\begin{aligned} M''' &= M_0 + (N - FN) - \{(E - FE)^2 \tan \phi'' [(1 - e^2 \sin^2 \phi'')^{(1/2)}] / (2a)\} \\ \mu''' &= M''' / a(1 - e^2/4 - 3e^4/64 - 5e^6/256 - \dots) \\ \phi''' &= \mu''' + (3e_1/2 - 27e_1^3/32)\sin(2\mu''') + (21e_1^2/16 - 55e_1^4/32)\sin(4\mu''') + (151e_1^3/96)\sin(6\mu''') \\ &\quad + (1097e_1^4/512)\sin(8\mu''') \end{aligned}$$

Then

$$\lambda = \lambda_0 + \{(E - FE) [(1 - e^2 \sin^2 \phi''')^{(1/2)}] / (a \cos \phi''')\}$$

### Example:

For Projected Coordinate Reference System: Guam 1963 / Guam SPCS

Parameters:

Ellipsoid:	Clarke 1866	a = 6378206.400 metres	1/f = 294.97870
	then	e = 0.08227185	e <sup>2</sup> = 0.00676866
Latitude of natural origin	$\phi_0$	13°28'20.87887"N	= 0.235138896 rad
Longitude of natural origin	$\lambda_0$	144°44'55.50254"E	= 2.526342288 rad
False easting	FE	50000.00 metres	
False northing	FN	50000.00 metres	

Forward calculation for:

Latitude	$\phi$	=	13°20'20.53846"N	=	0.232810140 rad
Longitude	$\lambda$	=	144°38'07.19265"E	=	2.524362746 rad

x	=	-12287.52 m
M <sub>0</sub>	=	1489888.76 m
M	=	1475127.96 m

whence

E	=	37,712.48 m
N	=	35,242.00 m

Reverse calculation for the same Easting and Northing (37,712.48 m E, 35,242.00 m N) first gives:

M <sub>0</sub>	=	1489888.76 m
e <sub>1</sub>	=	0.001697916

and

	<u>M (metres)</u>	<u><math>\mu</math> (radians)</u>	<u><math>\phi</math> (radians)</u>	
First iteration:	1475127.93	0.231668814	0.232810136	
Second iteration:	1475127.96	0.231668819	0.232810140	
Third iteration:	1475127.96	0.231668819	0.232810140	= 13°20'20.538"N

Then

$$\lambda = 2.524362746 \text{ rad} = 144°38'07.193"E$$



perspectives are the view of the Earth from a point on the perpendicular through a mapping plane which is either the tangent plane or a plane parallel to the tangent plane. Tilted perspectives are the view from a point which is not on the perpendicular. Tilted perspectives are not considered further in this guidance note.

In addition to vertical and tilted, perspectives can be classified as positive or negative. Perspectives with a positive viewing height  $h_v$  are the view of the Earth from above, as from a satellite or from another celestial body (and as shown in figure 11). Perspectives with a negative viewing height  $h_v$  are the “view” of the Earth from below, which is mathematically but not physically possible. The mapping equations, however, are identical; only the sign of one term (the viewing height,  $h_v$ ) differs. The viewing point cannot be on the mapping plane.

In this development vertical perspectives are based upon topocentric coordinates that are valid for an ellipsoidal Earth. The introduction of an intermediate topocentric coordinate system simplifies the mathematical exposition of vertical perspectives. In such a topocentric Cartesian coordinate system, two of the three axes represent the horizontal plane. A change of perspective (zooming in and out) is achieved by moving the viewing point along the perpendicular. The mapping plane is the plane parallel to the tangent plane which passes through the topocentric origin (rather than the tangent plane itself). In the special case of the topocentric origin being on the ellipsoid surface then the mapping plane will be the tangent plane.

### 3.3.4.2 Vertical Perspective

(EPSG Dataset coordinate operation method code 9838)

This general case deals with a viewing point at a finite height above the origin. If the viewing point is at infinity ( $h_v = \infty$ ), the formulas for the orthographic case given in the next section should be used.

The forward equations for the Vertical Perspective to convert geographic 3D coordinates ( $\phi$ ,  $\lambda$ ,  $h$ ) to Easting (E) and Northing (N) begin with the methods of Section 4.1.3 to convert the geographic coordinates to topocentric coordinates U, V, W. The perspective projection origin is coincident with the topographic origin and has coordinates ( $\phi_0$ ,  $\lambda_0$ ,  $h_0$ ). As in Section 4.1.3:

$$\begin{aligned} U &= (v + h) \cos \phi \sin (\lambda - \lambda_0) \\ V &= (v + h) [\sin \phi \cos \phi_0 - \cos \phi \sin \phi_0 \cos (\lambda - \lambda_0)] + e^2 (v_0 \sin \phi_0 - v \sin \phi) \cos \phi_0 \\ W &= (v + h) [\sin \phi \sin \phi_0 + \cos \phi \cos \phi_0 \cos (\lambda - \lambda_0)] + e^2 (v_0 \sin \phi_0 - v \sin \phi) \sin \phi_0 - (v_0 + h_0) \end{aligned}$$

Then, given the height  $h_v$  of the perspective viewing point above the origin, the perspective coordinates (E, N) are calculated from topocentric coordinates (U, V, W) as:

$$\begin{aligned} E &= U h_v / (h_v - W) \\ N &= V h_v / (h_v - W) \end{aligned}$$

The reverse calculation from E,N to U,V,W and  $\phi, \lambda, h$  is indeterminate.

#### Example:

For Projected Coordinate Reference System: WGS 84 / Vertical Perspective example

Parameters:

Ellipsoid:	WGS 84	$a = 6378137.0$ metres	$1/f = 298.2572236$
	then	$e = 0.081819191$	

Topographic origin latitude	$\varphi_0$	=	55°00'00.000"N	=	0.95993109 rad
Topographic origin longitude	$\lambda_0$	=	5°00'00.000"E	=	0.08726646 rad
Topographic origin ellipsoidal height	$h_0$	=	200 metres		
Height of viewpoint	$h_v$	=	5 900 kilometers		

Forward calculation for:

Latitude	$\varphi$	=	53°48'33.82"N	=	0.939151101 rad
Longitude	$\lambda$	=	2°07'46.38"E	=	0.037167659 rad
Ellipsoidal height	$h$	=	73 metres		

$$e^2 = 0.006694380$$

$$v_0 = 6392\,510.73 \text{ m}$$

$$v = 6392\,088.02 \text{ m}$$

$$U = -189\,013.869 \text{ m}$$

$$V = -128\,642.040 \text{ m}$$

$$W = -4\,220.171 \text{ m}$$

Then,

$$E = -188\,878.767 \text{ m}$$

$$N = -128\,550.090 \text{ m}$$

### 3.3.4.3 Vertical Perspective (orthographic case)

(EPSG Dataset coordinate operation method code 9839)

The orthographic vertical perspective is a special case of the vertical perspective with the viewing point at infinity ( $h_v = \infty$ ). Therefore, all projection "rays" are parallel to one another and all are perpendicular to the tangent plane. Since the rays are parallel, coordinates in the tangent-plane are the same in any other parallel mapping plane, i.e. are consistent for any value of  $h_0$ , which therefore becomes irrelevant to the forward formulas.

The orthographic vertical perspective forward conversion from 3D geographic coordinates latitude, longitude and ellipsoidal height ( $\varphi, \lambda, h$ ) to Easting (E) and Northing (N) is given by:

$$E = U = \lim (U h_v / (h_v - W), h_v \rightarrow \infty)$$

$$N = V = \lim (V h_v / (h_v - W), h_v \rightarrow \infty)$$

where, as in Sections 4.1.3 and 3.3.4.2:

$$U = (v + h) \cos \varphi \sin (\lambda - \lambda_0)$$

$$V = (v + h) [\sin \varphi \cos \varphi_0 - \cos \varphi \sin \varphi_0 \cos (\lambda - \lambda_0)] + e^2 (v_0 \sin \varphi_0 - v \sin \varphi) \cos \varphi_0$$

The reverse calculation from E,N to U,V,W and  $\varphi, \lambda, h$  is indeterminate.

#### **Example:**

For Projected Coordinate Reference System: WGS 84 / Vertical Perspective (Orthographic case) example

Parameters:

Ellipsoid:	WGS 84	$a = 6378137.0$ metres	$1/f = 298.2572236$
	then	$e = 0.081819191$	



Topographic origin latitude	$\varphi_0$	=	55°00'00.000"N	=	0.95993109 rad
Topographic origin longitude	$\lambda_0$	=	5°00'00.000"E	=	0.08726646 rad
Topographic origin ellipsoidal height	$h_0$	=	200 metres		

Forward calculation for:

Latitude	$\varphi$	=	53°48'33.82"N	=	0.939151101 rad
Longitude	$\lambda$	=	2°07'46.38"E	=	0.037167659 rad
Ellipsoidal height	$h$	=	73 metres		

The projection origin and example point are the same as those used in the general case of the Vertical Perspective in the previous section. Note that the ellipsoidal height at the point to be converted ( $h$ ) is 73 metres. The ellipsoidal height at the topocentric center ( $h_0$ ) is not used in any of the equations for the numerical examples that follow. But  $h_0$  will be used for the reverse case if  $W$  is known (for which a numerical example can be found in section 4.1.3).

$$\begin{aligned}e^2 &= 0.006694380 \\v_0 &= 6392\,510.727 \text{ m} \\v &= 6392\,088.017 \text{ m}\end{aligned}$$

Then,

$$\begin{aligned}E &= -189013.869 \text{ m} \\N &= -128642.040 \text{ m}\end{aligned}$$

### 3.3.5 **Orthographic**

(EPSG Dataset coordinate operation method code 9840)

Most cartographic texts which describe the orthographic projection do so using a spherical development. This section describes an ellipsoidal development. This allows the projected coordinates to be consistent with those for the vertical perspectives described in the previous section. If the projection origin is at the topocentric origin, the ellipsoidal Orthographic Projection is a special case of the orthographic vertical perspective in which the ellipsoid height of all mapped points is zero ( $h = 0$ ). The projection is neither conformal nor equal-area, but near the point of tangency there is no significant distortion. Within 90km of the origin the scale change is less than 1 part in 10,000.

The Orthographic projection forward conversion from 2D geographic coordinates latitude and longitude ( $\varphi$ ,  $\lambda$ ) and the origin on the ellipsoid ( $\varphi_0$ ,  $\lambda_0$ ) is given by:

$$\begin{aligned}E &= FE + v \cos \varphi \sin (\lambda - \lambda_0) \\N &= FN + v [\sin \varphi \cos \varphi_0 - \cos \varphi \sin \varphi_0 \cos (\lambda - \lambda_0)] + e^2 (v_0 \sin \varphi_0 - v \sin \varphi) \cos \varphi_0\end{aligned}$$

where

$v$  is the prime vertical radius of curvature at latitude  $\varphi$ ;  $v = a / (1 - e^2 \sin^2 \varphi)^{1/2}$ ,  
 $v_0$  is the prime vertical radius of curvature at latitude of origin  $\varphi_0$ ;  $v_0 = a / (1 - e^2 \sin^2 \varphi_0)^{1/2}$ ,  
 $e$  is the eccentricity of the ellipsoid and  $e^2 = (a^2 - b^2) / a^2 = 2f - f^2$   
 $a$  and  $b$  are the ellipsoidal semi-major and semi-minor axes,  
 $1/f$  is the inverse flattening, and  
the latitude and longitude of the projection origin are  $\varphi_0$  and  $\lambda_0$ .

These formulas are similar to those for the orthographic case of the vertical perspective except that, for the Orthographic projection given here,  $h = 0$  and the term  $(v + h)$  reduces to  $v$ . The projection origin is at the topocentric system origin  $\varphi_0$ ,  $\lambda_0$  with false origin coordinates  $FE$  and  $FN$ .

For the reverse formulas for latitude and longitude corresponding to a given Easting (E) and Northing (N), iteration is required as the prime vertical radius ( $v$ ) is a function of latitude.

Begin by seeding the iteration with the center of projection (or some better guess):

$$\varphi = \varphi_0$$

$$\lambda = \lambda_0$$

Enter the iteration here with the (next) best estimates of  $\varphi$  and  $\lambda$ . Then solve for the radii of curvature in the prime vertical ( $v$ ) and meridian ( $\rho$ ):

$$v = a / (1 - e^2 \sin^2 \varphi)^{0.5}$$

$$\rho = a (1 - e^2) / (1 - e^2 \sin^2 \varphi)^{1.5}$$

Compute test values of E and N ( $E'$  and  $N'$ ) using the forward equations:

$$E' = FE + v \cos \varphi \sin (\lambda - \lambda_0)$$

$$N' = FN + v [\sin \varphi \cos \varphi_0 - \cos \varphi \sin \varphi_0 \cos (\lambda - \lambda_0)] + e^2 (v_0 \sin \varphi_0 - v \sin \varphi) \cos \varphi_0$$

Partially differentiate the forward equations to solve for the elements of the Jacobian matrix:

$$J_{11} = \partial E / \partial \varphi = -\rho \sin \varphi \sin (\lambda - \lambda_0)$$

$$J_{12} = \partial E / \partial \lambda = v \cos \varphi \cos (\lambda - \lambda_0)$$

$$J_{21} = \partial N / \partial \varphi = \rho (\cos \varphi \cos \varphi_0 + \sin \varphi \sin \varphi_0 \cos (\lambda - \lambda_0))$$

$$J_{22} = \partial N / \partial \lambda = v \sin \varphi_0 \cos \varphi \sin (\lambda - \lambda_0)$$

Solve for the determinant of the Jacobian:

$$D = J_{11} J_{22} - J_{12} J_{21}$$

Solve the northerly and easterly differences this iteration:

$$\Delta E = E - E'$$

$$\Delta N = N - N'$$

Adjust the latitude and longitude for the next iteration by inverting the Jacobian and multiplying by the differences:

$$\varphi = \varphi + (J_{22} \Delta E - J_{12} \Delta N) / D$$

$$\lambda = \lambda + (-J_{21} \Delta E + J_{11} \Delta N) / D$$

Return to the entry point with new estimates of latitude and longitude and iterate until the change in  $\varphi$  and  $\lambda$  is not significant.

### Example:

For Projected Coordinate Reference System: WGS 84 / Orthographic projection example

Parameters:

Ellipsoid: WGS 84     $a = 6378137.0$  metres     $1/f = 298.2572236$   
                                  then     $e = 0.081819191$

Latitude of natural origin	$\varphi_0$	55°00'00.000"N	=	0.95993109 rad
Longitude of natural origin	$\lambda_0$	5°00'00.000"E	=	0.08726646 rad
False easting	FE	0 metres		
False northing	FN	0 metres		

Forward calculation for:

Latitude	$\varphi$	=	53°48'33.82"N	=	0.939151101 rad
Longitude	$\lambda$	=	2°07'46.38"E	=	0.037167659 rad

Note that ellipsoidal heights at the topocentric center ( $h_0$ ) and at the point to be converted ( $h$ ) may be the

same as in the Vertical Perspective examples in the previous section. Neither enter the computations that follow.

$$\begin{aligned}e^2 &= 0.006694380 \\v_0 &= 6392\,510.73 \text{ m} \\v &= 6392\,088.02 \text{ m}\end{aligned}$$

Then,

$$\begin{aligned}\text{Easting, } E &= -189\,011.711 \text{ m} \\ \text{Northing, } N &= -128\,640.567 \text{ m}\end{aligned}$$

Reverse calculation for these E, N coordinates into latitude ( $\phi$ ) and longitude ( $\lambda$ ) is iterative. The following values are constant every iteration.

$$\begin{aligned}e^2 &= 0.006694380 \\v_0 &= 6392\,510.73 \text{ m} \\\phi_0 &= 0.95993109 \text{ rad} \\\lambda_0 &= 0.08726646 \text{ rad}\end{aligned}$$

The following values change during 4 iterations to convergence:

	1	2	3	4
Latitude	0.95993109	0.9397628327	0.9391516179	0.9391511016
Longitude	0.08726646	0.0357167858	0.0371688977	0.0371676590
v	6392510.727	6392100.544	6392088.028	6392088.017
$\rho$	6378368.440	6377140.690	6377103.229	6377103.198
E'	0	-194318.490	-189006.908	-189011.711
N'	0	-124515.840	-128637.469	-128640.567
J <sub>11</sub>	0	265312.746	257728.730	257734.999
J <sub>12</sub>	3666593.522	3766198.868	3769619.566	3769621.986
J <sub>21</sub>	6378368.440	6370240.831	6370437.125	6370436.766
J <sub>22</sub>	0	-159176.388	-154825.395	-154829.329
D	-2338688440386	-24033825331760	-24054027385585	-24054043431047
$\Delta N$	-128640.567	-4124.727	-3.098	0
$\Delta E$	-189011.711	5306.779	-4.803	0
Latitude	0.9397628327	0.9391516179	0.9391511016	0.9391511016
Longitude	0.0357167858	0.0371688977	0.0371676590	0.0371676590

which results in:

$$\begin{array}{llllll} \text{Latitude} & \phi & = & 0.939151102 \text{ rad} & = & 53^\circ 48' 33.82'' \text{N} \\ \text{Longitude} & \lambda & = & 0.037167659 \text{ rad} & = & 2^\circ 07' 46.38'' \text{E} \end{array}$$

### 3.4 **Miscellaneous Map Projections**

#### 3.4.1 **New Zealand Map Grid**

(EPSG Dataset coordinate operation method code 9811)

This projection system typifies the recent development in the design and formulation of map projections where, by more complex mathematics yielding formulas readily handled by modern computers, it is possible to maintain the conformal property and minimise scale distortion over the total extent of a country area regardless of shape. Thus both North and South Islands of New Zealand, previously treated not very satisfactorily in two zones of a Transverse Mercator projection, can now be projected as one zone of what resembles most closely a curved version Oblique Mercator but which, instead of being based on a minimum scale factor straight central line, has a central line which is a complex curve roughly following the trend of both North and South Islands. The projected coordinate reference system achieves this by a form of double projection where a conformal projection of the ellipsoid is first made to say an oblique Stereographic projection and then the Cauchy-Riemann equations are invoked in order to further project the rectangular coordinates on this to a modification in which lines of constant scale can be made to follow other than the normal great or small circles of Central meridians or standard parallels. The mathematical treatment of the New Zealand Map Grid is covered by a publication by New Zealand Department of Lands and Survey Technical Circular 1973/32 by I.F.Stirling.

#### 3.4.2 **Tunisia Mining Grid**

(EPSG Dataset coordinate operation method code 9816)

This grid is used as the basis for mineral leasing in Tunisia. Lease areas are approximately 2 x 2 km or 400 hectares. The southwest corner of these blocks is defined through a six figure grid reference where the first three digits are an easting in kilometres and the last three digits are a northing. The origin of the grid is Djebel Kebar at 38.81973gN, 7.83445gE (of Paris) at which the grid reference is 270582. The latitudes and longitudes for block corners at 2 km intervals are tabulated in a mining decree dated 1<sup>st</sup> January 1953. From this tabulation in which geographic coordinates are given to 5 decimal places it can be seen that:

- a) the minimum easting is 94 km, on which the longitude is 5.68989 grads east of Paris.
- b) the maximum easting is 490 km, on which the longitude is 10.51515 grads east of Paris.
- c) each 2 km grid easting interval equals 0.02437 grads.
- d) the minimum northing is 40 km, on which the latitude is 33.39 grads.
- e) the maximum northing is 860 km, on which the latitude is 41.6039 grads.
- f) between 40 km N and 360 km N, each 2 km grid northing interval equals 0.02004 grads.
- g) between 360 km N and 860 km N, each 2 km grid northing interval equals 0.02003 grads.

From the above the latitude of N=360 can be determined to be 36.5964gN.

This grid could be considered to be two equidistant cylindrical projection zones, north and south of the 360 km northing line. However this would require the introduction of two spheres of unique dimensions. OGP has therefore implemented the Tunisia mining grid as a coordinate conversion method in its own right. Formulas are:

#### **Grads from Paris**

$$\begin{aligned}\varphi &= \varphi_0 + [(N - N_F) * A] \\ &= 36.5964 + [(N - 360) * A]\end{aligned}$$

where  $\varphi$  and  $\varphi_0$  are in grads,  $N$  and  $N_F$  are in kilometres and  $A = 0.010015$  if  $N > N_F = 360$ , else  $A = 0.01002$ .

$$\begin{aligned}\lambda &= \lambda_0 + [(E - E_F) * 0.012185] \\ &= 7.83445 + [(E - 270) * 0.012185]\end{aligned}$$

where  $\varphi$  and  $\varphi_0$  are in grads relative to the Paris meridian and E and  $E_F$  are in kilometres.

The reverse formulas are:

$$\begin{aligned} E \text{ (km)} &= E_F + [(\lambda - \lambda_0) / 0.012185] \\ &= 270 + [(\lambda - 7.83445) / 0.012185]. \end{aligned}$$

$$\begin{aligned} N \text{ (km)} &= N_F + [(\varphi - \varphi_0) / A] \\ &= 360 + [(\varphi - 36.5964) / A] \end{aligned}$$

where  $A = 0.010015$  if  $\varphi > \varphi_0 = 36.5964$ , else  $A = 0.01002$ .

### Degrees from Greenwich

Modern practice in Tunisia is to quote latitude and longitude in degrees with longitudes referenced to the Greenwich meridian. The formulas required in addition to the above are:

$$\varphi_d \text{ (degrees)} = (\varphi_g * 0.9) \text{ where } \varphi_g \text{ is in grads.}$$

$$\lambda_{\text{Greenwich}} \text{ (degrees)} = [(\lambda_{\text{Paris}} + 2.5969213) * 0.9] \text{ where } \lambda_{\text{Paris}} \text{ is in grads.}$$

$$\varphi_g \text{ (grads)} = (\varphi_d / 0.9) \text{ where } \varphi_d \text{ is in decimal degrees.}$$

$$\lambda_{\text{Paris}} \text{ (grads)} = [(\lambda_{\text{Greenwich}} / 0.9) - 2.5969213] \text{ where } \lambda_{\text{Greenwich}} \text{ is in decimal degrees.}$$

### **Example:**

Defining parameters are:

$$\varphi_0 = 36.5964 \text{gN}$$

$$\lambda_0 = 7.83445 \text{gE of Paris}$$

$$E_F = 270 \text{ km}$$

$$N_F = 360 \text{ km}$$

For grid location 302598, first derive the easting (302 km) and northing (598 km) from the grid reference. Then:

$$\begin{aligned} \text{Latitude } \varphi &= 36.5964 + [(598 - 360) * A]. \text{ As } N > 360, A = 0.010015. \\ &= 38.97997 \text{ grads North} = 35.08197 \text{ degrees North.} \end{aligned}$$

$$\begin{aligned} \text{Longitude } \lambda &= 7.83445 + [(302 - 270) * 0.012185] \\ &= 8.22437 \text{ grads East of Paris} = 9.73916 \text{ degrees East of Greenwich.} \end{aligned}$$

### **3.4.3 Colombia Urban**

(EPSG Dataset coordinate operation method code 1052)

The capital cities of each department in Colombia use an urban projection for large scale topographic mapping of the urban areas. It is based on a plane through the origin at an average height for the area, eliminating the need for corrections to engineering survey observations.

The forward conversion from latitude and longitude to easting and northing is:

$$E = FE + A \vee \cos \varphi (\lambda - \lambda_0)$$

$$N = FN + G \rho_0 [(\varphi - \varphi_0) + \{B (\lambda - \lambda_0)^2 \vee^2 \cos^2 \varphi\}]$$

where

$\varphi_0$ ,  $\lambda_0$  and  $h_0$  are the latitude, longitude and height of the projection origin,

$\rho$  is the radius of curvature of the ellipsoid in the plane of the meridian at latitude  $\varphi$ ,  $\rho_0$  at latitude  $\varphi_0$ , and  $\rho_M$  at mid-latitude  $(\varphi_0 + \varphi)/2$ ;  $\rho = a(1 - e^2)/(1 - e^2 \sin^2 \varphi)^{3/2}$  for  $\varphi$ ,  $\varphi_0$  and  $\varphi_M$

$v$  is the radius of curvature of the ellipsoid perpendicular to the meridian at latitude  $\varphi$  with  $v_0$  at latitude  $\varphi_0$ ;  $v = a/(1 - e^2 \sin^2 \varphi)^{0.5}$  for  $\varphi$  and  $\varphi_0$

$e$  is the eccentricity of the ellipsoid and  $e^2 = (a^2 - b^2)/a^2 = 2f - f^2$   
 $a$  is the ellipsoidal semi-major and  $1/f$  is the inverse flattening

$$A = 1 + h_0/v_0$$

$$B = \tan \varphi_0 / 2\rho_0 v_0$$

$$G = 1 + h_0/\rho_M$$

The reverse conversion from easting and northing to latitude and longitude is:

$$\varphi = \varphi_0 + [(N - FN) / D] - B [(E - FE) / C]^2$$

$$\lambda = \lambda_0 + (E - FE) / C v \cos(\varphi)$$

where

$B$  is as in the forward conversion,

$$C = 1 + h_0/a$$

$$D = \rho_0 (1 + h_0/a(1 - e^2))$$

### Example

For Projected Coordinate Reference System: MAGNA-SIRGAS / Bogota urban grid

Parameters:

Ellipsoid:	GRS 1980	$a = 6378137.0$ metres	$1/f = 298.2572221$
	then	$e^2 = 0.006694380$	

Latitude of natural origin	$\varphi_0$	$4^\circ 40' 49.75''\text{N}$	=	$0.081689893$ rad
Longitude of natural origin	$\lambda_0$	$74^\circ 08' 47.73''\text{W}$	=	$-1.294102154$ rad
False easting	FE	$92334.879$	metres	
False northing	FN	$109320.965$	metres	
Height of projection origin	$h_0$	$2550.000$	metres	

Forward calculation for:

Latitude	$\varphi$	=	$4^\circ 48' 00.000''\text{N}$	=	$0.083775804$ rad
Longitude	$\lambda$	=	$74^\circ 15' 00.000''\text{W}$	=	$-1.295906970$ rad

First calculate constants for the projection:

$\rho_0 =$	$6335862.944$	$v_0 =$	$6378279.154$
$A =$	$1.000399794$	$B =$	$1.012970\text{E-}15$
$C =$	$1.000399803$	$D =$	$6338413.114$

Then

$$v = 6378286.489$$

$$G = 1.000402470$$

and

$$E = 80859.033\text{m}$$

$$N = 122543.174\text{m}$$

Reverse calculation for the same Easting and Northing (80859.033E, 122543.174N) first gives:

$$\begin{array}{llll} \text{Latitude} & \varphi & = & 4^{\circ}48'00.000''\text{N} \\ \text{then} & v & = & 6378286.489 \\ \text{and} & \text{Longitude } \lambda & = & 74^{\circ}15'00.000''\text{W} \end{array}$$

### 3.4.4 **Equal Earth**

(EPSG Dataset coordinate operation method code 1078)

The Equal Earth projection is an equal-area pseudocylindrical projection. It is appropriate for mapping global phenomena or for any other thematic world map that requires areas at their correct relative sizes. Its key features are its resemblance to the Robinson projection and continents with a visually pleasing appearance similar to those found on a globe.

#### **Forward:**

To derive the projected coordinates of a point, geodetic latitude ( $\varphi$ ) is converted to authalic latitude ( $\beta$ ). The formulae to convert geodetic latitude and longitude ( $\varphi, \lambda$ ) to Easting (E) and Northing (N) are:

$$\begin{aligned} \text{Easting, } E &= FE + R_q 2 (\lambda - \lambda_0) \cos(\theta) / \{ \sqrt{3} [1.340264 - 0.243318 \theta^2 + \theta^6 (0.006251 + 0.034164 \theta^2)] \} \\ \text{Northing, } N &= FN + R_q \theta [1.340264 - 0.081106 \theta^2 + \theta^6 (0.000893 + 0.003796 \theta^2)] \end{aligned}$$

where

$$\begin{aligned} \theta &= \text{asin} [\sin(\beta) \sqrt{3} / 2] \\ R_q &= a (q_p / 2)^{0.5} \\ \beta &= \text{asin}(q / q_p) \\ q &= (1 - e^2) (\{ \sin(\varphi) / [1 - e^2 \sin^2(\varphi)] \} - 1/(2e) \ln \{ [1 - e \sin(\varphi)] / [1 + e \sin(\varphi)] \}) \\ q_p &= (1 - e^2) (\{ \sin(\varphi_p) / [1 - e^2 \sin^2(\varphi_p)] \} - 1/(2e) \ln \{ [1 - e \sin(\varphi_p)] / [1 + e \sin(\varphi_p)] \}) \\ &\quad \text{where } \varphi_p = \pi/2 \text{ radians, thus} \\ q_p &= (1 - e^2) ([1 / (1 - e^2)] - \{ 1/(2e) \} \ln [(1 - e) / (1 + e)]) \end{aligned}$$

#### **Reverse:**

The reverse conversion from easting and northing to latitude and longitude requires iteration of easting (E) equation to obtain  $\theta$ .  $\theta_0 = (E - FE) / R_q$  is used as the first trial  $\theta$ . A correction  $\Delta\theta_n$  is calculated and subtracted from the trial  $\theta_n$  value to obtain the next trial  $\theta_{n+1}$ .  $n$  is the suffix number of iteration.

$$\begin{aligned} \theta_0 &= (E - FE) / R_q \\ \Delta\theta_0 &= \{ \theta_0 [1.340264 - 0.081106 \theta_0^2 + \theta_0^6 (0.000893 + 0.003796 \theta_0^2)] - [E - FE] / R_q \} / \\ &\quad \{ 1.340264 - 0.243318 \theta_0^2 + \theta_0^6 [0.006251 + 0.034164 \theta_0^2] \} \\ \theta_1 &= \theta_0 - \Delta\theta_0 \end{aligned}$$

$$\begin{aligned} \Delta\theta_1 &= \{ \theta_1 [1.340264 - 0.081106 \theta_1^2 + \theta_1^6 (0.000893 + 0.003796 \theta_1^2)] - [E - FE] / R_q \} / \\ &\quad \{ 1.340264 - 0.243318 \theta_1^2 + \theta_1^6 [0.006251 + 0.034164 \theta_1^2] \} \\ \theta_2 &= \theta_1 - \Delta\theta_1 \end{aligned}$$

etc.

The calculation is repeated until  $\Delta\theta_n$  is less than a predetermined convergence value. Then, using the final  $\theta_{n+1}$  as  $\theta$ , the geodetic latitude and longitude of a point are determined as follows:

$$\varphi = \beta + \{[(e^2/3 + 31e^4/180 + 517e^6/5040) \sin(2\beta)] + [(23e^4/360 + 251e^6/3780) \sin(4\beta)] + [(761e^6/45360) \sin(6\beta)]\}$$

$$\lambda = \lambda_0 + \sqrt{3} (N-FN) \{1.340264 - 0.243318 \theta^2 + \theta^6 (0.006251 + 0.034164 \theta^2)\} / \{2 R_q \cos(\theta)\}$$

where  $\beta = \arcsin\{2 \sin(\theta) / \sqrt{3}\}$  and  $R_q$  is defined as in the forward equations.

### Sphere:

For the spherical form of the projection,  $\beta = \varphi$  and  $R_q = R$ , where  $R$  is the radius of the sphere.

### Example

For Projected Coordinate Reference System: WGS 84 / Equal Earth Americas (EPSG CRS code 8858)

Parameters:

Ellipsoid: WGS 1984  $a = 6378137.0$  metres  $1/f = 298.257223563$   
 then  $e = 0.08181919084262$

Longitude of natural origin  $\lambda_0 = 90^\circ 00' 00.00'' W = -1.570796327$  rad  
 False easting FE = 0.000 m  
 False northing FN = 0.000 m

Forward calculation for:

Latitude  $\varphi = 34^\circ 03' 27.169'' N = 0.594416330$  rad  
 Longitude  $\lambda = 117^\circ 11' 48.349'' W = -2.045469387$  rad

First gives

$\beta = 0.592339965$   $R_q = 6371007.181$   
 $\theta = 0.504654839$

whence

$E = -2390748.988$  m  
 $N = 4242849.764$  m

Reverse calculation for the same Easting and Northing (-2390748.988 E, 4242849.764 N) starts with an iteration to obtain  $\theta$ :

$(E-FE) / R_q = 0.665962169$   
 Step 1:  $\theta_0 = 0.665962169$   $\Delta\theta_0 = 0.164312383$   
 Step 2:  $\theta_1 = 0.501649786$   $\Delta\theta_1 = -0.003004201$   
 Step 3:  $\theta_2 = 0.504653987$   $\Delta\theta_2 = -8.512742e-07$   
 Step 4:  $\theta_3 = 0.504654838$   $\Delta\theta_3 = -6.859963e-14$   
 $\theta = \theta_4 = 0.504654838$

This gives:

$\beta = 0.592339965$

Then Latitude  $\varphi = 34^\circ 03' 27.169'' N$   
 Longitude  $\lambda = 117^\circ 11' 48.349'' W$

.



## 4 Formulas for Coordinate Operations other than Map Projections

### 4.1 Conversions between geodetic Coordinate Reference Systems

#### 4.1.1 Geographic/Geocentric conversions

(EPSG dataset coordinate operation method code 9602)

Latitude,  $\phi$ , and Longitude,  $\lambda$ , and ellipsoidal height,  $h$ , in terms of a 3-dimensional geographic coordinate reference system may be expressed in terms of a geocentric (earth centred) Cartesian coordinate reference system X, Y, Z with the Z axis corresponding with the earth's rotation axis positive northwards, the X axis through the intersection of the prime meridian and equator, and the Y axis through the intersection of the equator with longitude 90°E. The geographic and geocentric systems are based on the same geodetic datum.

Geocentric coordinate reference systems are conventionally taken to be defined with the X axis through the intersection of the Greenwich meridian and equator. This requires that the equivalent geographic coordinate reference system be based on the Greenwich meridian. In application of the formulas below, geographic coordinate reference systems based on a non-Greenwich prime meridian should first be transformed to their Greenwich equivalent. Geocentric coordinates X, Y and Z take their units from the units for the ellipsoid axes (a and b). As it is conventional for X, Y and Z to be in metres, if the ellipsoid axis dimensions are given in another linear unit they should first be converted to metres.

If the ellipsoidal semi major axis is **a**, semi minor axis **b**, and inverse flattening **1/f**, then

$$\begin{aligned} X &= (v + h) \cos \phi \cos \lambda \\ Y &= (v + h) \cos \phi \sin \lambda \\ Z &= [(1 - e^2) v + h] \sin \phi \end{aligned}$$

where  $v$  is the prime vertical radius of curvature at latitude  $\phi$  and is equal to

$$v = a / (1 - e^2 \sin^2 \phi)^{0.5},$$

$\phi$  and  $\lambda$  are respectively the latitude and longitude (related to the prime meridian) of the point,

$h$  is height above the ellipsoid, (see note below), and

$e$  is the eccentricity of the ellipsoid where  $e^2 = (a^2 - b^2)/a^2 = 2f - f^2$

*(Note that  $h$  is the height above the ellipsoid. This is the height value that is delivered by GPS satellite observations but is not the gravity-related height value which is normally used for national mapping and levelling operations. The gravity-related height ( $H$ ) is usually the height above mean sea level or an alternative level reference for the country. If one starts with a gravity-related height  $H$ , it will be necessary to convert it to an ellipsoid height ( $h$ ) before using the above transformation formulas. See section 4.11.1. For the WGS 84 ellipsoid the difference between ellipsoid and mean sea level can vary between values of -100m in the Sri Lanka area to +80m in the North Atlantic.)*

For the reverse conversion, Cartesian coordinates in the geocentric coordinate reference system may be converted to geographic coordinates in terms of the geographic 3D coordinate reference system by:

$$\begin{aligned} \phi &= \text{atan2} [(Z + e^2 v \sin \phi), (X^2 + Y^2)^{0.5}] \text{ by iteration} \\ \lambda &= \text{atan2} (Y, X) \\ h &= X \sec \lambda \sec \phi - v \end{aligned}$$

where  $\lambda$  is relative to the Greenwich prime meridian.

To avoid iteration for  $\phi$  it may alternatively be found from:

$$\phi = \text{atan2} [(Z + \varepsilon b \sin^3 \phi), (p - e^2 a \cos^3 \phi)]$$

where

$$\begin{aligned} \varepsilon &= e^2 / (1 - e^2) \\ b &= a(1 - f) \\ p &= (X^2 + Y^2)^{0.5} \end{aligned}$$

$$q = \text{atan2}[(Z/a), (p/b)]$$

Then  $h$  may more conveniently be found from

$$h = (p / \cos \varphi) - v$$

**Example:**

Consider a North Sea point with position derived by GPS satellite in the WGS 84 coordinate reference system. The WGS 84 ellipsoid parameters are:

$$\begin{aligned} a &= 6378\,137.000\text{m} \\ 1/f &= 298.2572236 \end{aligned}$$

then

$$\begin{aligned} e^2 &= 0.006694380 \\ \varepsilon &= 0.006739497 \\ b &= 6356\,752.314\text{ m} \end{aligned}$$

Using the reverse direction direct formulas above, the conversion of WGS 84 geocentric coordinates of

$$\begin{aligned} X &= 3771\,793.968\text{ m} \\ Y &= 140\,253.342\text{ m} \\ Z &= 5124\,304.349\text{ m} \end{aligned}$$

is:

$$\begin{aligned} p &= 3774400.712 \\ q &= 0.937546077 \\ \varphi &= 0.939151101\text{ rad} \\ v &= 6392088.017 \end{aligned}$$

Then WGS 84 geographic 3D coordinates are:

$$\begin{aligned} \text{latitude } \varphi &= 53^\circ 48' 33.820''\text{N} \\ \text{longitude } \lambda &= 2^\circ 07' 46.380''\text{E} \\ \text{and ellipsoidal height } h &= 73.0\text{m} \end{aligned}$$

Further examples of input and output may be found in test procedure 5201 of the Test Dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

#### 4.1.2 Geocentric/topocentric conversions

(EPSG Dataset coordinate operation method code 9836)

A topocentric coordinate system is a 3-D Cartesian system having mutually perpendicular axes  $U$ ,  $V$ ,  $W$  with an origin on or near the surface of the Earth. The  $U$ -axis is locally east, the  $V$ -axis is locally north and the  $W$ -axis is up forming a right-handed coordinate system. It is applied in two particular settings:

(i) the height axis  $W$  is chosen to be along the direction of gravity at the topocentric origin. The other two axes are then in the horizontal plane. A special case of this, often applied in engineering applications, is when the topocentric origin is on the vertical datum surface; then topocentric height  $W$  approximates to gravity-related height  $H$ .

(ii) the topocentric height axis  $W$  is chosen to be the direction through the topocentric origin and perpendicular to the surface of the ellipsoid. The other two topocentric axes ( $U$  and  $V$ ) are in the "topocentric plane", a plane parallel to the tangent to the ellipsoid surface at the topocentric origin and passing through the topocentric origin (see figures 12 and 13 below). The coordinates defining the topocentric origin will usually

be expressed in ellipsoidal terms as latitude  $\phi_0$ , longitude  $\lambda_0$  and ellipsoidal height  $h_0$  but may alternatively be expressed as geocentric Cartesian coordinates  $X_0, Y_0, Z_0$ . In this context the geocentric coordinates of the topocentric origin should not be confused with those of the geocentric origin where  $X=Y=Z=0$ .

A special case of this is when the topocentric origin is chosen to be exactly on the ellipsoid surface and  $h_0 = 0$ . Then the topocentric  $U$  and  $V$  axes are in the ellipsoid tangent plane and at (and only at) the topocentric origin topocentric height  $W = \text{ellipsoidal height } h$ .

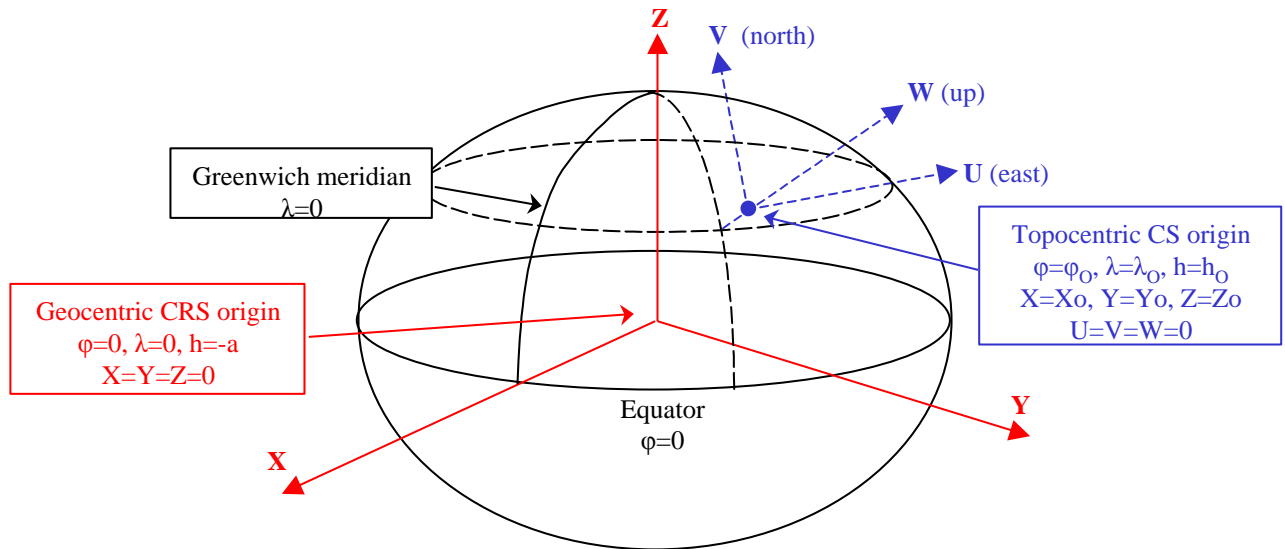


Figure 12 – Topocentric and geocentric systems

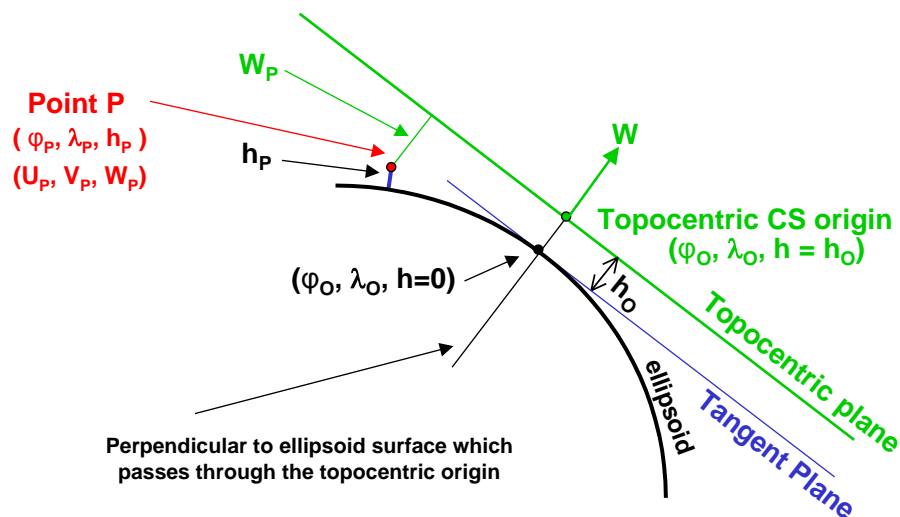


Figure 13 – Topocentric and ellipsoidal heights

In this and the following section we are concerned with the second of the two settings for topocentric coordinate systems where the system is associated with the ellipsoid and a particular geodetic datum. The application of such topocentric coordinates includes scalable mapping and visualisation systems as described in section 3.3.4. The following section covers the conversion between ellipsoidal coordinates and topocentric coordinates. The remainder of this section describes how geocentric coordinates  $X, Y, Z$  may be converted

into topocentric coordinates U, V, W given the geocentric coordinates of the topocentric CS origin (X<sub>0</sub>, Y<sub>0</sub>, Z<sub>0</sub>).

First it is necessary to derive ellipsoidal values φ<sub>0</sub>, λ<sub>0</sub> of the topocentric origin from their geocentric values X<sub>0</sub>, Y<sub>0</sub>, Z<sub>0</sub> through the reverse formulas given in Section 4.1.1. (The value h<sub>0</sub> for the ellipsoidal height of the topocentric origin is not required in what follows.)

Then topocentric coordinates [U, V, W] are computed as follows:

$$\begin{pmatrix} U \\ V \\ W \end{pmatrix} = \mathbf{R} * \begin{pmatrix} X \\ Y \\ Z \end{pmatrix} - \begin{pmatrix} X_0 \\ Y_0 \\ Z_0 \end{pmatrix}$$

where,

$$\mathbf{R} = \begin{pmatrix} -\sin \lambda_0 & \cos \lambda_0 & 0 \\ -\sin \varphi_0 \cos \lambda_0 & -\sin \varphi_0 \sin \lambda_0 & \cos \varphi_0 \\ \cos \varphi_0 \cos \lambda_0 & \cos \varphi_0 \sin \lambda_0 & \sin \varphi_0 \end{pmatrix}$$

Or, expressed as scalar equations:

$$\begin{aligned} U &= -(X-X_0) \sin \lambda_0 + (Y-Y_0) \cos \lambda_0 \\ V &= -(X-X_0) \sin \varphi_0 \cos \lambda_0 - (Y-Y_0) \sin \varphi_0 \sin \lambda_0 + (Z-Z_0) \cos \varphi_0 \\ W &= (X-X_0) \cos \varphi_0 \cos \lambda_0 + (Y-Y_0) \cos \varphi_0 \sin \lambda_0 + (Z-Z_0) \sin \varphi_0 \end{aligned}$$

The reverse formulas to calculate geocentric coordinates from topocentric coordinates are:

$$\begin{pmatrix} X \\ Y \\ Z \end{pmatrix} = \mathbf{R}^T * \begin{pmatrix} U \\ V \\ W \end{pmatrix} + \begin{pmatrix} X_0 \\ Y_0 \\ Z_0 \end{pmatrix}$$

where,

$$\mathbf{R}^T = \mathbf{R}^T = \begin{pmatrix} -\sin \lambda_0 & -\sin \varphi_0 \cos \lambda_0 & \cos \varphi_0 \cos \lambda_0 \\ \cos \lambda_0 & -\sin \varphi_0 \sin \lambda_0 & \cos \varphi_0 \sin \lambda_0 \\ 0 & \cos \varphi_0 & \sin \varphi_0 \end{pmatrix}$$

and, as for the forward case, φ<sub>0</sub> and λ<sub>0</sub> are calculated through the formulas in Section 4.1.1.

Or, expressed as scalar equations:

$$\begin{aligned} X &= X_0 - U \sin \lambda_0 - V \sin \varphi_0 \cos \lambda_0 + W \cos \varphi_0 \cos \lambda_0 \\ Y &= Y_0 + U \cos \lambda_0 - V \sin \varphi_0 \sin \lambda_0 + W \cos \varphi_0 \sin \lambda_0 \\ Z &= Z_0 + V \cos \varphi_0 + W \sin \varphi_0 \end{aligned}$$

**Example:**

$$\begin{aligned} \text{For Geocentric CRS} &= \text{WGS 84 (EPSG CRS code 4978)} \\ \text{and} & \\ \text{Topocentric origin } X_0 &= 3652\,755.3058 \text{ m} \\ \text{Topocentric origin } Y_0 &= 319\,574.6799 \text{ m} \\ \text{Topocentric origin } Z_0 &= 5201\,547.3536 \text{ m} \end{aligned}$$

$$\text{Ellipsoid parameters: } a = 6378137.0 \text{ metres} \quad 1/f = 298.2572236$$

First calculate additional ellipsoid parameters:

$$e^2 = 0.006694380 \quad \varepsilon = 0.006739497 \quad b = 6356752.314$$

Next, derive  $\varphi_0, \lambda_0$  from  $X_0, Y_0, Z_0$  by the formulas given in Section 4.1.1:

$$\begin{aligned} p &= 3666708.2376 \\ q &= 0.9583523313 \\ \varphi_0 &= 0.9599310885 \text{ rad} \\ \lambda_0 &= 0.0872664625 \text{ rad} \end{aligned}$$

Forward calculation for point with geocentric coordinates:

$$X = 3771\,793.968 \text{ m} \quad Y = 140\,253.342 \text{ m} \quad Z = 5124\,304.349 \text{ m}$$

gives topocentric coordinates

$$U = -189\,013.869 \text{ m} \quad V = -128\,642.040 \text{ m} \quad W = -4\,220.171 \text{ m}$$

The reverse calculation contains no intermediate terms other than those solved for above and is a trivial reversal of the forward.

#### 4.1.3 Geographic/topocentric conversions

(EPSG Dataset coordinate operation method code 9837)

Topocentric coordinates may be derived from geographic coordinates indirectly by concatenating the geographic/geocentric conversion described in 4.1.1 above with the geocentric/topocentric conversion described in 4.1.2 above. Alternatively the conversion may be made directly:

To convert latitude  $\varphi$ , longitude  $\lambda$  and ellipsoidal height  $h$  into topocentric coordinates  $U, V, W$ :

$$\begin{aligned} U &= (v + h) \cos(\varphi) \sin(\lambda - \lambda_0) \\ V &= (v + h) [\sin(\varphi) \cos(\varphi_0) - \cos(\varphi) \sin(\varphi_0) \cos(\lambda - \lambda_0)] + e^2 (v_0 \sin(\varphi_0) - v \sin(\varphi)) \cos(\varphi_0) \\ W &= (v + h) [\sin(\varphi) \sin(\varphi_0) + \cos(\varphi) \cos(\varphi_0) \cos(\lambda - \lambda_0)] + e^2 (v_0 \sin(\varphi_0) - v \sin(\varphi)) \sin(\varphi_0) - (v_0 + h_0) \end{aligned}$$

where  $\varphi_0, \lambda_0, h_0$  are the ellipsoidal coordinates of the topocentric origin

and  $v$  is the radius of curvature in the prime vertical at latitude  $\varphi = a / (1 - e^2 \sin^2(\varphi))^{0.5}$   
 $v_0$  is the radius of curvature in the prime vertical at latitude  $\varphi_0 = a / (1 - e^2 \sin^2(\varphi_0))^{0.5}$   
 $e$  is the eccentricity of the ellipsoid where  $e^2 = (a^2 - b^2)/a^2 = 2f - f^2$

The reverse formulae to convert topocentric coordinates (U, V, W) into latitude, longitude and ellipsoidal height ( $\phi$ ,  $\lambda$ , h) first draws on the reverse case of section 4.1.2 to derive geocentric coordinates X, Y, Z and then on the reverse case in section 4.1.1 to derive latitude, longitude and height.

First,

$$\begin{aligned} X &= X_0 - U \sin(\lambda_0) - V \sin(\phi_0) \cos(\lambda_0) + W \cos(\phi_0) \cos(\lambda_0) \\ Y &= Y_0 + U \cos(\lambda_0) - V \sin(\phi_0) \sin(\lambda_0) + W \cos(\phi_0) \sin(\lambda_0) \\ Z &= Z_0 + V \cos(\phi_0) + W \sin(\phi_0) \end{aligned}$$

where,

$$\begin{aligned} X_0 &= (v_0 + h_0) \cos(\phi_0) \cos(\lambda_0) \\ Y_0 &= (v_0 + h_0) \cos(\phi_0) \sin(\lambda_0) \\ Z_0 &= [(1 - e^2) v_0 + h_0] \sin(\phi_0) \\ \phi_0, \lambda_0, h_0 &\text{ are the ellipsoidal coordinates of the topocentric origin,} \\ v_0 &\text{ is the radius of curvature in the prime vertical at latitude } \phi_0 = a / (1 - e^2 \sin^2(\phi_0))^{0.5}, \text{ and} \\ e &\text{ is the eccentricity of the ellipsoid where } e^2 = (a^2 - b^2) / a^2 = 2f - f^2. \end{aligned}$$

Then,

$$\begin{aligned} \phi &= \text{atan2}[(Z + \varepsilon b \sin^3(q)), (p - e^2 a \cos^3(q))] \\ \lambda &= \text{atan2}(Y, X) \end{aligned}$$

where

$$\begin{aligned} \varepsilon &= e^2 / (1 - e^2) \\ b &= a(1 - f) \\ p &= (X^2 + Y^2)^{0.5} \\ q &= \text{atan2}[(Z a), (p b)] \\ \lambda &\text{ is relative to the Greenwich prime meridian.} \end{aligned}$$

and

$$h = (p / \cos(\phi)) - v$$

where

v is the radius of curvature in the prime vertical at latitude  $\phi = a / (1 - e^2 \sin^2(\phi))^{0.5}$

### Example:

For Geographic 3D CRS = WGS 84 (EPSG CRS code 4979)  
 and

Topocentric origin latitude	$\phi_0$	55°00'00.000"N	=	0.95993109 rad
Topocentric origin longitude	$\lambda_0$	5°00'00.000"E	=	0.08726646 rad
Topocentric origin ellipsoidal height	$h_0$	200 metres		

Ellipsoid parameters:  $a = 6378137.0$  metres  $1/f = 298.2572236$

First calculate additional ellipsoid parameter  $e^2$  and radius of curvature  $v_0$  at the topocentric origin:

$$e^2 = 0.006694380 \quad v_0 = 6392510.727$$

Forward calculation for:

Latitude	$\phi$	=	53°48'33.82"N	=	0.93915110 rad
Longitude	$\lambda$	=	2°07'46.38"E	=	0.03716765 rad
Height	h	=	73.0 metres		

$$v = 6392088.017$$

then

$$\begin{aligned} U &= -189\,013.869 \text{ m} \\ V &= -128\,642.040 \text{ m} \\ W &= -4\,220.171 \text{ m} \end{aligned}$$

Reverse calculation for:

$$\begin{aligned} U &= -189\,013.869 \text{ m} \\ V &= -128\,642.040 \text{ m} \\ W &= -4\,220.171 \text{ m} \end{aligned}$$

First calculate additional ellipsoid parameter  $e^2$  and radius of curvature  $v_0$  at the topocentric origin:

$$e^2 = 0.006694380 \quad v_0 = 6392510.727$$

then the following intermediate terms:

$$\begin{aligned} X_0 &= 3652\,755.306 & \varepsilon &= 0.0067394967 \\ Y_0 &= 319\,574.680 & b &= 6356\,752.314 \\ Z_0 &= 5201\,547.353 & p &= 3774\,400.712 \\ X &= 3771\,793.968 & q &= 0.937549875 \\ Y &= 140\,253.342 & \varphi &= 0.9391511015 \text{ rad} \\ Z &= 5124\,304.349 & v &= 6392\,088.017 \\ & & \lambda &= 0.03716765908 \text{ rad} \end{aligned}$$

for a final result of:

$$\begin{aligned} \text{Latitude } \varphi &= 53^\circ 48' 33.820'' \text{N} \\ \text{Longitude } \lambda &= 2^\circ 07' 46.380'' \text{E} \\ \text{Height } h &= 73.0 \text{ metres} \end{aligned}$$

#### 4.1.4 **Geographic 3D to 2D conversions**

(EPSG Dataset coordinate operation method code 9659)

The forward case is trivial. A geographic 2D CRS is derived from a 3-dimensional geographic coordinate reference system comprising of geodetic latitude, geodetic longitude and ellipsoidal height by the simple expedient of dropping the height. The change is described by the following matrix operation:

$$\begin{pmatrix} \text{Derived CRS 1st coordinate} \\ \text{Derived CRS 2nd coordinate} \\ \text{Derived CRS 3rd coordinate} \end{pmatrix} = \begin{pmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 0 \end{pmatrix} * \begin{pmatrix} \text{Base CRS 1st coordinate} \\ \text{Base CRS 2nd coordinate} \\ \text{Base CRS 3rd coordinate} \end{pmatrix}$$

The reverse conversion, from 2D to 3D, is indeterminate. It is however a requirement when a geographic 2D coordinate reference system is to be transformed using a geocentric method which is 3-dimensional (see section 4.3.1). In practice an artificial ellipsoidal height is created and appended to the geographic 2D coordinate reference system to create a geographic 3D coordinate reference system referenced to the same geodetic datum. The assumed ellipsoidal height is usually either set to the gravity-related height of a position in a compound coordinate reference system, or set to zero. As long as the height chosen is within a few kilometres of sea level, no error will be induced into the horizontal position resulting from the later geocentric transformation; the vertical coordinate will however be meaningless.

**Example:**

A location in the ETRS89 Geographic 3D coordinate reference system

$$\begin{aligned} \text{latitude } \varphi_s &= 53^\circ 48' 33.82'' \text{N} \\ \text{longitude } \lambda_s &= 2^\circ 07' 46.38'' \text{E} \\ \text{and ellipsoidal height } h_s &= 73.0 \text{m} \end{aligned}$$

is converted to the ETRS89 Geographic 2D coordinate reference system as

$$\begin{aligned} \text{latitude } \varphi_s &= 53^\circ 48' 33.82'' \text{N} \\ \text{longitude } \lambda_s &= 2^\circ 07' 46.38'' \text{E} \end{aligned}$$

For the reverse conversion of the same point in the ETRS89 Geographic 2D coordinate reference system with horizontal coordinates of

$$\begin{aligned} \text{latitude } \varphi_s &= 53^\circ 48' 33.82'' \text{N} \\ \text{longitude } \lambda_s &= 2^\circ 07' 46.38'' \text{E} \end{aligned}$$

an arbitrary value is given to the ellipsoidal height resulting in coordinates in the ETRS89 Geographic 3D coordinate reference system of

$$\begin{aligned} \text{latitude } \varphi_s &= 53^\circ 48' 33.82'' \text{N} \\ \text{longitude } \lambda_s &= 2^\circ 07' 46.38'' \text{E} \\ \text{and ellipsoidal height } h_s &= 0.0 \text{m} \end{aligned}$$

#### 4.1.5 **Horizontal axis order reversal**

a) **Horizontal axis reversal – Geographic 3D CRS**  
 (EPSG Dataset coordinate operation method code 9844)

A 3-dimensional geographic coordinate reference system comprising of geodetic latitude, geodetic longitude and ellipsoidal height is converted to one comprising of geodetic longitude, geodetic latitude and ellipsoidal height. This is a parameter-less conversion. The axis order change operates on coordinates of a point whose order change is described by the following matrix operation:

$$\begin{pmatrix} \text{Derived CRS 1st coordinate} \\ \text{Derived CRS 2nd coordinate} \\ \text{Derived CRS 3rd coordinate} \end{pmatrix} = \begin{pmatrix} 0 & 1 & 0 \\ 1 & 0 & 0 \\ 0 & 0 & 1 \end{pmatrix} * \begin{pmatrix} \text{Base CRS 1st coordinate} \\ \text{Base CRS 2nd coordinate} \\ \text{Base CRS 3rd coordinate} \end{pmatrix}$$

b) **Horizontal axis reversal – 2D CRS**  
 (EPSG Dataset coordinate operation method code 9843)

This is a parameter-less conversion. It is used when GIS systems require ordinate before abscissa, for example:

- a 2-dimensional geographic coordinate reference system comprising of geodetic latitude and geodetic longitude is converted to one comprising of geodetic longitude and geodetic latitude.
- a projected coordinate reference system comprising of northing and easting is converted to one comprising of easting and northing.

The axis order change operates on coordinates of a point whose order change is described by the following matrix operation:

$$\begin{pmatrix} \text{Derived CRS 1st coordinate} \\ \text{Derived CRS 2nd coordinate} \end{pmatrix} = \begin{pmatrix} 0 & 1 \\ 1 & 0 \end{pmatrix} * \begin{pmatrix} \text{Base CRS 1st coordinate} \\ \text{Base CRS 2nd coordinate} \end{pmatrix}$$



## 4.2 **Transformations between Geocentric Coordinate Reference Systems**

### 4.2.1 **Overview**

The methods in this section operate in the geocentric Cartesian coordinate domain. However they are most frequently used as the middle part of a transformation of coordinates from one geographic coordinate reference system into another forming a concatenated operation of:

[(geographic to geocentric) + (geocentric to geocentric) + (geocentric to geographic)]

in terms of coordinate reference systems or:

[ (ellipoidal to Cartesian) + (Cartesian to Cartesian) + (Cartesian to ellipoidal) ]

in terms of their coordinates.

The formulae given in the remainder of this section are for the transformation in the geocentric Cartesian coordinate domain forming the middle step. See section 4.3.1 below for a fuller description of these concatenated operations and Guidance Note 7 part 1 for a general discussion of implicit concatenated operations created by application software.

The coordinate transformation methods in this section consist of a family which progresses to a more extensive modelling of the CRS relationships:

- 3 translations model the offset of the source and target geocentric Cartesian coordinate reference system origins near the centre of the Earth. They assume that the two systems are orientated such that the axes are exactly parallel and have exactly the same scale. These conditions are rarely encountered in reality, but the modelling has been widely adopted in part because of its simplicity and in part because there may be insufficient data to develop a more rigorous transformation.
- 7-parameter Helmert transformations add three rotations and a scale change to the three translations. These better represent geodetic reality, especially over large areas. A variation used for modelling tectonic plate motion is to have the values of the seven parameters change with time.
- 10-parameter transformations add the three Cartesian coordinates of a non-geocentric point about which the rotations of the Helmert transformation are applied. These reduce correlations between the seven Helmert parameters when the transformation derivation is poorly conditioned, for example derived over small surface areas (subtending less than 30° at the centre of the Earth).

In implementation, the 3-parameter geocentric translation method can be treated as a special case of the 7-parameter Helmert transformation which in turn is a special case of the 10-parameter method. However issues connected with reversibility described below should be noted.

In the formulas in this section of the Guidance Note the parameters used in geocentric Cartesian coordinate transformation methods are represented by symbols as follows:

TABLE 5  
**Parameters used in geocentric coordinate transformations**

<b>a) Parameters used in geocentric transformations (including Helmert transformations)</b>		
<u>Parameter Name</u>	<u>Symbol</u>	<u>Description</u>
	$X_S Y_S Z_S$	Coordinates in the source coordinate reference system.
	$X_T Y_T Z_T$	Coordinates in the target coordinate reference system.
	$X_P Y_P Z_P$	Coordinates of a pivot point in the source coordinate reference system.
X-axis translation	$tX$	Translation vector, to be added to the point's position vector in the source CRS in order to transform from source CRS to target CRS; also: the coordinates of the origin of source CRS in the target CRS.
Y-axis translation	$tY$	
Z-axis translation	$tZ$	
X-axis rotation	$rX$	Rotations to be applied about the axes of the source CRS. There are two opposing conventions for defining the positive rotation direction, described below.
Y-axis rotation	$rY$	
Z-axis rotation	$rZ$	
Scale factor for source CRS axes	$M$	Multiplication factor to be applied to coordinates in the source CRS to obtain the correct scale of the target CRS. $M = (1 + dS)$ .
Scale difference	$dS$	Scale factor for source CRS axes minus one. $dS = (M - 1)$ . If a distance of 100 km in the source coordinate reference system translates into a distance of 100.0001 km in the target coordinate reference system, the scale difference is 1 ppm (the ratio being 1.000001).  When the scale difference $dS$ is expressed in parts per million (ppm), $M = (1 + dS \cdot 10^{-6})$ . When the scale difference $dS$ is expressed in parts per billion (ppb), $M = (1 + dS \cdot 10^{-9})$ .

<b>b) Additional parameters used only in time-dependent Helmert transformations</b>		
<u>Parameter Name</u>	<u>Symbol</u>	<u>Description</u>
Rate of change of X-axis translation	$\delta tX$	First derivative of X-axis translation with respect to time.
Rate of change of Y-axis translation	$\delta tY$	First derivative of Y-axis translation with respect to time.
Rate of change of Z-axis translation	$\delta tZ$	First derivative of Z-axis translation with respect to time.
Rate of change of X-axis rotation	$\delta rX$	First derivative of X-axis rotation with respect to time.
Rate of change of Y-axis rotation	$\delta rY$	First derivative of Y-axis rotation with respect to time.
Rate of change of Z-axis rotation	$\delta rZ$	First derivative of Z-axis rotation with respect to time.
Rate of change of scale difference	$\delta dS$	First derivative of scale difference with respect to time.
Parameter reference epoch	$t_0$	The reference epoch for the parameter values of a time-dependent transformation.

<b>c) Additional parameter used only in time-specific Helmert transformations</b>		
Transformation reference epoch	$t_{trans}$	The reference epoch for a time-specific transformation.

$R$  is a rotation matrix combining the three rotations about the X, Y and Z axes respectively. There are two issues with this:

- i) The sign convention used
- ii) The magnitude of the rotations

### **Rotation sign convention**

Two opposing conventions are in use for the rotations: they may be applied to the position vector in the source CRS or to the coordinate frame of the source CRS.

#### **a) The Coordinate Frame rotation convention.**

Rotations are applied to the coordinate frame (the set of coordinate axes). The sign convention is such that a positive rotation of the frame about an axis is defined as a clockwise rotation of the coordinate frame when viewed from the origin of the Cartesian CRS in the positive direction of that axis. A positive rotation about the Z-axis only from source CRS to target CRS will result in a smaller longitude value for the point in the target coordinate reference system.

$$R_Z = \begin{pmatrix} \cos(rZ) & \sin(rZ) & 0 \\ -\sin(rZ) & \cos(rZ) & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad R_Y = \begin{pmatrix} \cos(rY) & 0 & -\sin(rY) \\ 0 & 1 & 0 \\ \sin(rY) & 0 & \cos(rY) \end{pmatrix} \quad R_X = \begin{pmatrix} 1 & 0 & 0 \\ 0 & \cos(rX) & \sin(rX) \\ 0 & -\sin(rX) & \cos(rX) \end{pmatrix}$$

#### **b) The Position Vector rotation convention.**

Rotations are applied to the point's vector. The sign convention is such that a positive rotation about an axis is defined as a clockwise rotation of the position vector when viewed from the origin of the Cartesian CRS in the positive direction of that axis; e.g. a positive rotation about the Z-axis only from source system to target system will result in a larger longitude value for the point in the target system.

$$R_Z = \begin{pmatrix} \cos(rZ) & -\sin(rZ) & 0 \\ \sin(rZ) & \cos(rZ) & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad R_Y = \begin{pmatrix} \cos(rY) & 0 & \sin(rY) \\ 0 & 1 & 0 \\ -\sin(rY) & 0 & \cos(rY) \end{pmatrix} \quad R_X = \begin{pmatrix} 1 & 0 & 0 \\ 0 & \cos(rX) & -\sin(rX) \\ 0 & \sin(rX) & \cos(rX) \end{pmatrix}$$

In some literature, such as that from the International Earth Rotation Service (IERS), this rotation convention is described as an *anticlockwise (counter-clockwise)* rotation of the coordinate frame. This is the opposite of the Coordinate Frame convention in (a).

Note that the same rotation that is defined as positive in the Position Vector method is consequently negative in the Coordinate Frame method and vice versa:  $r_{PV} = -r_{CF}$ . It is therefore crucial that the convention underlying the definition of the rotation parameters is clearly understood and is communicated when exchanging transformation parameter values, so that the parameter values may be associated with the correct coordinate transformation method (algorithm) implemented in software.

### **Magnitude of rotations**

In general, with three rotations about the X, Y and Z axes respectively, the order in which the rotations are applied is significant. By convention, the rotations are first about the X-axis, then the Y-axis, then the Z-axis. This leads to the following compounded rotation matrix for the Coordinate Frame rotation convention:

$$R = \begin{pmatrix} \cos(rY) \cos(rZ) & \cos(rX) \sin(rZ) + \sin(rX) \sin(rY) \cos(rZ) & \sin(rX) \sin(rZ) - \cos(rX) \sin(rY) \cos(rZ) \\ -\cos(rY) \sin(rZ) & \cos(rX) \cos(rZ) - \sin(rX) \sin(rY) \sin(rZ) & \sin(rX) \cos(rZ) + \cos(rX) \sin(rY) \sin(rZ) \\ \sin(rY) & -\sin(rX) \cos(rY) & \cos(rX) \cos(rY) \end{pmatrix}$$

However, if the rotation angles are very small,  $\cos(r) \approx 1$ ,  $\sin(r) \approx r$  in radians and the product of multiple small angles can be considered to be insignificant and ignored. Then:

for the Position Vector transformation convention:  $\mathbf{R}_{PV} = \begin{pmatrix} 1 & -rZ & rY \\ rZ & 1 & -rX \\ -rY & rX & 1 \end{pmatrix}$

and for the Coordinate Frame rotation convention:  $\mathbf{R}_{CF} = \begin{pmatrix} 1 & rZ & -rY \\ -rZ & 1 & rX \\ rY & -rX & 1 \end{pmatrix}$

For this small angle approximation to be valid to better than 3mm at the Earth's surface, the rotations must be under 10 arc-seconds. If the rotations are less than 3 arc-seconds, the approximation is valid to better than 0.3mm at the Earth's surface. The approximation error occurs only when the transformation parameter values have been derived with an algorithm that includes the full rotation matrix and the application of the transformation uses the approximation, or vice versa. When the parameter values for a coordinate transformation have been derived using the small angle approximations in the rotation matrix described above and the same formula with approximated rotation matrix is used to apply the coordinate transformation, no error occurs.

#### 4.2.2 **Molodensky-Badekas 10-parameter transformations**

Several different formulas are documented in the literature. Each may be encountered using the Position Vector or Coordinate Frame rotation convention.

$$\text{Variant A: } \mathbf{V}_T = (1 + dS) * \mathbf{R}_A * (\mathbf{V}_S - \mathbf{V}_P) + \mathbf{V}_P + \mathbf{T}$$

$$\text{Variant B: } \mathbf{V}_T = \mathbf{R}_B * (\mathbf{V}_S - \mathbf{V}_P) + dS * (\mathbf{V}_S - \mathbf{V}_P) + \mathbf{V}_S + \mathbf{T}$$

$$\text{Variant C: } \mathbf{V}_T = \mathbf{R}_C * (\mathbf{V}_S - \mathbf{V}_P) + \mathbf{V}_S + \mathbf{T}$$

where

$$\mathbf{V}_T = \begin{pmatrix} X_T \\ Y_T \\ Z_T \end{pmatrix} \quad \mathbf{V}_S = \begin{pmatrix} X_S \\ Y_S \\ Z_S \end{pmatrix} \quad \mathbf{V}_P = \begin{pmatrix} X_P \\ Y_P \\ Z_P \end{pmatrix} \quad \mathbf{T} = \begin{pmatrix} tX \\ tY \\ tZ \end{pmatrix}$$

and, using the Coordinate Frame rotation convention,

$$\mathbf{R}_A = \begin{pmatrix} 1 & rZ & -rY \\ -rZ & 1 & rX \\ rY & -rX & 1 \end{pmatrix} \quad \mathbf{R}_B = \begin{pmatrix} 0 & rZ & -rY \\ -rZ & 0 & rX \\ rY & -rX & 0 \end{pmatrix} \quad \mathbf{R}_C = \begin{pmatrix} dS & rZ & -rY \\ -rZ & dS & rX \\ rY & -rX & dS \end{pmatrix}$$

The parameters in the matrices are defined in Table 5.

Variants B and C can be shown to be approximations to variant A: for rotation angles of under 10 arc-seconds the approximation is good to better than 0.1mm. In implementation, the 7-parameter Helmert transformation is a special case of variant A, but not of variants B or C. For both of these reasons EPSG documents only variant A. These formulas are commonly referred to by the name Molodensky-Badekas, but the formulation is not exactly due to either Molodensky or Badekas.

#### **Reversibility**

Variant A is

$$\mathbf{V}_T = (1 + dS) * \mathbf{R}_A * (\mathbf{V}_S - \mathbf{V}_P) + \mathbf{V}_P + \mathbf{T}$$

the exact reverse of which is

$$\mathbf{V}_S = [1/(1 + dS)] * \mathbf{R}_A^{-1} * (\mathbf{V}_T - \mathbf{V}_P) + \mathbf{V}_P - [1/(1 + dS)] * \mathbf{R}_A^{-1} * \mathbf{T}$$

This may be approximated by

$$\mathbf{V}_S = (1 - dS) * \mathbf{R}_A^{-1} * (\mathbf{V}_T - \mathbf{V}_P) + \mathbf{V}_P - \mathbf{T}$$

This approximation uses the same formula as in the forward calculation but with the signs of the translation, rotation and scale parameters changed. (The signs of the coordinates of the pivot point are unchanged). The approximation is valid only when the transformation parameter values are small compared to the magnitude of the geocentric coordinates. Under this condition the methods are considered to be reversible for practical purposes. The misclosure in a round trip calculation depends on the magnitude of the translation, rotation and scale parameter values. The two approximation errors are:

$$\begin{aligned}\Delta_1 &= [1/(1 + dS)] * \mathbf{R}_A^{-1} * \mathbf{T} - \mathbf{T} \\ \Delta_2 &= [dS^2/(1 + dS)] * \mathbf{R}_A^{-1} * (\mathbf{V}_T - \mathbf{V}_P)\end{aligned}$$

In practical circumstances these two approximation errors when combined do not exceed 2cm and are usually much less.

(Note: The EPSG Dataset documents reversibility using the approximation, that is using the same formula as for the forward calculation but with the signs of the parameter values changed. The alternative of using the exact reverse formula should be treated as a different method in which the reverse uses a different formula from for the forward calculation and in this the signs of the parameter values are not changed).

Molodensky-Badekas transformation (PV geocentric domain)  
 (EPSG Dataset coordinate operation method code 1061)

$$\begin{pmatrix} X_T \\ Y_T \\ Z_T \end{pmatrix} = \mathbf{M} * \begin{pmatrix} 1 & -rZ & +rY \\ +rZ & 1 & -rX \\ -rY & +rX & 1 \end{pmatrix} * \begin{pmatrix} X_S - X_P \\ Y_S - Y_P \\ Z_S - Z_P \end{pmatrix} + \begin{pmatrix} X_P + tX \\ Y_P + tY \\ Z_P + tZ \end{pmatrix}$$

where  $\mathbf{M} = 1 + dS$ .

Molodensky-Badekas transformation (CF geocentric domain)  
 (EPSG Dataset coordinate operation method code 1034)

$$\begin{pmatrix} X_T \\ Y_T \\ Z_T \end{pmatrix} = \mathbf{M} * \begin{pmatrix} 1 & +rZ & -rY \\ -rZ & 1 & +rX \\ +rY & -rX & 1 \end{pmatrix} * \begin{pmatrix} X_S - X_P \\ Y_S - Y_P \\ Z_S - Z_P \end{pmatrix} + \begin{pmatrix} X_P + tX \\ Y_P + tY \\ Z_P + tZ \end{pmatrix}$$

where  $\mathbf{M} = 1 + dS$ .

See *Transformations using geocentric methods* below for an example. Further examples of input and output may be found in test procedure 5205 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

#### 4.2.3 Helmert 7-parameter transformations

If the coordinates of pivot point P in the Molodensky-Badekas transformation methods are constrained to be zero, i.e.  $X_P = Y_P = Z_P = 0$ , then the formulas in the previous section simplify to a 7-parameter Helmert transformation, expressed in matrix form in what is sometimes known as the "Bursa-Wolf" formula.

##### Position Vector transformation (geocentric domain)

(EPSG Dataset coordinate operation method code 1033)

$$\begin{pmatrix} X_T \\ Y_T \\ Z_T \end{pmatrix} = M * \begin{pmatrix} 1 & -rZ & +rY \\ +rZ & 1 & -rX \\ -rY & +rX & 1 \end{pmatrix} * \begin{pmatrix} X_S \\ Y_S \\ Z_S \end{pmatrix} + \begin{pmatrix} tX \\ tY \\ tZ \end{pmatrix}$$

where  $M = 1 + dS$ .

##### **Example:**

Transformation from WGS 72 to WGS 84 (EPSG Dataset transformation code 1238). Transformation parameter values:

$$\begin{aligned} tX &= 0.000 \text{ m} \\ tY &= 0.000 \text{ m} \\ tZ &= +4.5 \text{ m} \\ rX &= 0.000 \text{ sec} \\ rY &= 0.000 \text{ sec} \\ rZ &= +0.554 \text{ sec} = 0.000002685868 \text{ radians} \\ dS &= +0.219 \text{ ppm} \quad \text{from which } M = 1.000000219 \end{aligned}$$

Input point coordinate reference system: WGS 72 (geocentric Cartesian coordinates):

$$\begin{aligned} X_S &= 3\,657\,660.66 \text{ m} \\ Y_S &= 255\,768.55 \text{ m} \\ Z_S &= 5\,201\,382.11 \text{ m} \end{aligned}$$

Application of the 7 parameter Position Vector transformation results in:

$$\begin{aligned} X_T &= 3\,657\,660.78 \text{ m} \\ Y_T &= 255\,778.43 \text{ m} \\ Z_T &= 5\,201\,387.75 \text{ m} \end{aligned}$$

on the WGS 84 geocentric coordinate reference system.

Further examples of input and output may be found in test procedure 5203 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

##### Coordinate Frame rotation (geocentric domain)

(EPSG Dataset coordinate operation method code 1032)

$$\begin{pmatrix} X_T \\ Y_T \\ Z_T \end{pmatrix} = M * \begin{pmatrix} 1 & +rZ & -rY \\ -rZ & 1 & +rX \\ +rY & -rX & 1 \end{pmatrix} * \begin{pmatrix} X_S \\ Y_S \\ Z_S \end{pmatrix} + \begin{pmatrix} tX \\ tY \\ tZ \end{pmatrix}$$

where  $M = 1 + dS$ .

The same example as for the Position Vector transformation can be calculated, however the following transformation parameters have to be applied to achieve the same input and output in terms of coordinate values:

Transformation parameters Coordinate Frame rotation convention:

tX	=	0.000 m	
tY	=	0.000 m	
tZ	=	+4.5 m	
rX	=	-0.000 sec	
rY	=	-0.000 sec	
rZ	=	-0.554 sec	= -0.000002685868 radians
dS	=	+0.219 ppm	from which M = 1.000000219

Note that only the rotation has changed sign as compared to the Position Vector transformation.

#### Reversibility

Using vector notation the forward formula is

$$\mathbf{V}_T = (1 + dS) * \mathbf{R} * \mathbf{V}_S + \mathbf{T}$$

the exact reverse of which is

$$\mathbf{V}_S = [1/(1 + dS)] * \mathbf{R}^{-1} * (\mathbf{V}_T - \mathbf{T})$$

This may be approximated by

$$\mathbf{V}_S = (1 - dS) * \mathbf{R}^{-1} * \mathbf{V}_T - \mathbf{T}$$

This approximation uses the same formula as in the forward calculation but with the signs of the translation, rotation and scale parameters changed. The approximation is valid only when the transformation parameter values are small compared to the magnitude of the geocentric coordinates. Under this condition the methods are considered to be reversible for practical purposes. The misclosure in a round trip calculation depends on the magnitude of the translation, rotation and scale parameter values. The two approximation errors are:

$$\mathbf{A}_1 = [1/(1 + dS)] * \mathbf{R}^{-1} * \mathbf{T} - \mathbf{T}$$

$$\mathbf{A}_2 = [dS^2/(1 + dS)] * \mathbf{R}^{-1} * \mathbf{V}_T$$

In practical circumstances these two approximation errors when combined do not exceed 2cm and are usually much less.

Note: The EPSG Dataset documents reversibility using the approximation, that is using the same formula as for the forward calculation but with the signs of the parameter values changed. The alternative of using the exact reverse formula should be treated as a different method in which the reverse uses a different formula from the forward calculation and in this the signs of the parameter values are not changed.

Further examples of input and output may be found in test procedure 5204 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

#### **4.2.4 3-parameter geocentric translations**

(EPSG Dataset coordinate operation method code 1031)

If we assume that the axes of the ellipsoids are parallel, that the prime meridian is Greenwich, and that there is no scale difference between the source and target coordinate reference system, then geocentric coordinate reference systems may be related to each other through three translations (colloquially known as shifts) **tX**, **tY**, **tZ** in the sense from source geocentric coordinate reference system to target geocentric coordinate reference system. They may then be applied as

$$\begin{aligned} X_T &= X_S + tX \\ Y_T &= Y_S + tY \\ Z_T &= Z_S + tZ \end{aligned}$$

This is a specific case of the Helmert 7-parameter transformation methods in section 4.2.3 above in which  $rX = rY = rZ = 0$  and  $dS = 0$  so  $M = 1$ .

### Reversibility

The geocentric translations method is exactly reversible: the same formula is used as for the forward transformation but in the reverse case the signs of the three translations are changed.

### **Example:**

Consider a North Sea point with coordinates derived by GPS satellite in the WGS84 geocentric coordinate reference system, with coordinates of:

$$\begin{aligned} X_S &= 3771\,793.97 \text{ m} \\ Y_S &= 140\,253.34 \text{ m} \\ Z_S &= 5124\,304.35 \text{ m} \end{aligned}$$

whose coordinates are required in terms of the ED50 coordinate reference system which takes the International 1924 ellipsoid. The three parameter geocentric translations method's parameter values from WGS84 to ED50 for this North Sea area are given as  $tX = +84.87\text{m}$ ,  $tY = +96.49\text{m}$ ,  $tZ = +116.95\text{m}$ .

Applying the quoted geocentric translations to these, we obtain new geocentric values now related to ED50:

$$\begin{aligned} X_T &= 3771\,793.97 + 84.87 = 3771\,878.84 \text{ m} \\ Y_T &= 140\,253.34 + 96.49 = 140\,349.83 \text{ m} \\ Z_T &= 5124\,304.35 + 116.95 = 5124\,421.30 \text{ m} \end{aligned}$$

Further examples of input and output may be found in test procedure 5211 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

## **4.2.5 Time-dependent Helmert 7-parameter transformations**

To account for tectonic plate motion (but not deformation) the rate of change of each of the seven parameters may be included in a time-dependent variation of the Helmert 7-parameter transformation. This leads to a 15-parameter transformation: the three translations, three rotations and the scale change, the rates for these seven parameters and the parameter reference epoch for these parameters. For a time-dependent transformation first an adjustment to the parameter values is calculated, and then the adjusted values used for the Helmert coordinate transformation. See the example below for the process to be followed when applying a time-dependent transformation.

### Time-dependent Position Vector transformation (geocentric domain)

(EPSG Dataset coordinate operation method code 1053)

$$\begin{pmatrix} X_T \\ Y_T \\ Z_T \end{pmatrix} = M' * \begin{pmatrix} 1 & -rZ' & +rY' \\ +rZ' & 1 & -rX' \\ -rY' & +rX' & 1 \end{pmatrix} * \begin{pmatrix} X_S \\ Y_S \\ Z_S \end{pmatrix} + \begin{pmatrix} tX' \\ tY' \\ tZ' \end{pmatrix}$$

where  $tX'$ ,  $tY'$ ,  $tZ'$ ,  $rX'$ ,  $rY'$ ,  $rZ'$  and  $M'$  are time-adjusted parameter values:

$$\begin{aligned} tX' &= tX + \delta tX (t - t_0) \\ tY' &= tY + \delta tY (t - t_0) \\ tZ' &= tZ + \delta tZ (t - t_0) \\ rX' &= rX + \delta rX (t - t_0) \\ rY' &= rY + \delta rY (t - t_0) \\ rZ' &= rZ + \delta rZ (t - t_0) \\ dS' &= dS + \delta dS (t - t_0) \\ M' &= 1 + dS' \end{aligned}$$

and  $t$  is the coordinate epoch of the dynamic coordinates and  $t_0$  the parameter reference epoch for the transformation parameters.



### Reverse transformation

For the reverse transformation the signs of all parameters need to be reversed except for that of the parameter reference epoch  $t_0$ .

Time-dependent Coordinate Frame rotation (geocentric domain)  
 (EPSG Dataset coordinate operation method code 1056)

$$\begin{pmatrix} X_T \\ Y_T \\ Z_T \end{pmatrix} = M' * \begin{pmatrix} 1 & +rZ' & -rY' \\ -rZ' & 1 & +rX' \\ +rY' & -rX' & 1 \end{pmatrix} * \begin{pmatrix} X_S \\ Y_S \\ Z_S \end{pmatrix} + \begin{pmatrix} tX' \\ tY' \\ tZ' \end{pmatrix}$$

where  $tX'$ ,  $tY'$ ,  $tZ'$ ,  $rX'$ ,  $rY'$ ,  $rZ'$  and  $M'$  are as in the Time-dependent Position Vector method in the previous section.

### Example:

Transformation (EPSG code 6276) from ITRF2008 to GDA94 at coordinate epoch 2013.90.

Source ITRF2008 geocentric Cartesian coordinates at coordinate epoch 2013.90 are:

$$\begin{aligned} X_S &= -3789470.710 \text{ m} \\ Y_S &= 4841770.404 \text{ m} \\ Z_S &= -1690893.952 \text{ m} \\ t &= 2013.90 \text{ years} \end{aligned}$$

Transformation parameter values:

$$\begin{aligned} tX &= -84.68 \text{ mm} & \delta tX &= +1.42 \text{ mm/yr} \\ tY &= -19.42 \text{ mm} & \delta tY &= +1.34 \text{ mm/yr} \\ tZ &= +32.01 \text{ mm} & \delta tZ &= +0.90 \text{ mm/yr} \\ rX &= -0.4254 \text{ msec} & \delta rX &= +1.5461 \text{ msec/yr} \\ rY &= +2.2578 \text{ msec} & \delta rY &= +1.1820 \text{ msec/yr} \\ rZ &= +2.4015 \text{ msec} & \delta rZ &= +1.1551 \text{ msec/yr} \\ dS &= +0.00971 \text{ ppm} & \delta dS &= +0.000109 \text{ ppm/yr} \\ t_0 &= 1994.00 \end{aligned}$$

First apply the correction due to rate of change to each of the 7 transformation parameters for the period ( $t - t_0$ ), taking care to convert the translations to the same units as the source CRS (in this case metres) and the rotations to radians:

$$\begin{aligned} tX' &= -84.68 + [(+1.42 * (2013.90 - 1994.00))] = -56.42 \text{ mm} = -0.056 \text{ m} \\ tY' &= -19.42 + [(+1.34 * (2013.90 - 1994.00))] = +7.25 \text{ mm} = +0.007 \text{ m} \\ tZ' &= +32.01 + [(+0.90 * (2013.90 - 1994.00))] = +49.92 \text{ mm} = +0.050 \text{ m} \\ rX' &= -0.4254 + [(+1.5461 * (2013.90 - 1994.00))] = +30.3420 \text{ msec} = +1.471021\text{E-}07 \text{ rad} \\ rY' &= +2.2578 + [(+1.1820 * (2013.90 - 1994.00))] = +25.7796 \text{ msec} = +1.249830\text{E-}07 \text{ rad} \\ rZ' &= +2.4015 + [(+1.1551 * (2013.90 - 1994.00))] = +25.3880 \text{ msec} = +1.230844\text{E-}07 \text{ rad} \\ dS' &= +0.00971 + [(+0.000109 * (2013.90 - 1994.00))] = +0.01188 \text{ ppm} \\ &\text{from which } M' = 1.00000001188 \end{aligned}$$

Using these seven time-adjusted parameter values, application of the 7 parameter Coordinate Frame rotation formula to the given (source) ITRF2008 coordinates results in GDA94 (target) geocentric coordinates of:

$$\begin{aligned} X_T &= -3789470.004 \text{ m} \\ Y_T &= 4841770.686 \text{ m} \\ Z_T &= -1690895.108 \text{ m} \end{aligned}$$

(In practice these coordinates often would be further converted to geographic 3D or compound geographic 2D + gravity-related height values. See sections 4.3.1 and 4.11.2 respectively).

For the reverse transformation from GDA94 coordinates to ITRF08 coordinates at coordinate epoch 2013.90, the signs of all parameters need to be reversed except for the parameter reference epoch. Then:

$tX' = +84.68 + [(-1.42 * (2013.90 - 1994.00))] = +56.42 \text{ mm} = +0.056 \text{ m}$   
 and similar for the other six parameters. Hence  $tY' = -0.007 \text{ m}$ ,  $tZ' = -0.050 \text{ m}$ ,  $rX' = -1.471021\text{E-}07 \text{ rad}$ ,  $rY' = -1.249830\text{E-}07 \text{ rad}$ ,  $rZ' = -1.230844\text{E-}07 \text{ rad}$  and  $dS' = -0.01188 \text{ ppm}$ ,  
 from which  $M' = 0.99999998812$ .

Using these time-adjusted parameters values, applying the 7 parameter Coordinate Frame rotation formula to the GDA94 coordinates of

$$\begin{aligned} X_S &= -3789470.004 \text{ m} \\ Y_S &= 4841770.686 \text{ m} \\ Z_S &= -1690895.108 \text{ m} \end{aligned}$$

results in coordinates referenced to ITRF08 at coordinate epoch 2013.90 of:

$$\begin{aligned} X_T &= -3789470.710 \text{ m} \\ Y_T &= 4841770.404 \text{ m} \\ Z_T &= -1690893.952 \text{ m} \end{aligned}$$

#### 4.2.6 **Time-specific Helmert 7-parameter transformations**

The time-specific coordinate transformation is an alternative approach to the time-dependent coordinate transformation. In this method the transformation parameter values are declared for a specified *transformation reference epoch* (an additional parameter for the transformation) and coordinates are adjusted to this epoch before the transformation is applied. This requires that the velocities for points whose coordinates are to be transformed are available. The time-specific transformation may then be applied as part of a concatenated coordinate operation in conjunction with one or two point motion operations (see section 4.4) in two or three steps:

- i) change the source CRS Cartesian coordinates from their dataset coordinate epoch to the transformation reference epoch at which the time-specific transformation is valid using a point motion operation;
- ii) apply the Helmert 7-parameter coordinate transformation;
- iii) change the target CRS Cartesian coordinates from the transformation reference epoch at which the time-specific transformation is valid to any other desired coordinate epoch using a point motion operation.

The transformation reference epoch of the Helmert 7-parameter transformation is used as the target epoch ( $t_2$ ) in step (i) and as the source epoch ( $t_1$ ) in step (iii). It is not used in step (ii).

The time-specific Helmert transformation may use either the Position Vector or Coordinate Frame rotation convention:

- Time-specific Position Vector transform (geocen)  
(EPSG Dataset coordinate operation method code 1065).
- Time-specific Coordinate Frame rotation (geocen)  
(EPSG Dataset coordinate operation method code 1066)

**Example:**

Initial coordinates are referenced to the ITRF2008 CRS (EPSG code 5332) at coordinate epoch 2005.00. (Note: this coordinate epoch happens to be the frame reference epoch for ITRF2008 but in other circumstances the coordinate set could be referenced to a coordinate epoch different from the CRS's frame reference epoch).

Coordinates are required to be referenced to the PZ-90.11 CRS (EPSG code 7679) at coordinate epoch 2013.90.

Input point geocentric Cartesian coordinates and linear velocities:

$$\begin{array}{llll} X_S & = & 2845\,456.0813 \text{ m} & V_X & = & -0.0212 \text{ m/yr} \\ Y_S & = & 2160\,954.2453 \text{ m} & V_Y & = & +0.0124 \text{ m/yr} \\ Z_S & = & 5265\,993.2296 \text{ m} & V_Z & = & +0.0072 \text{ m/yr} \\ t & = & 2005.0 & & & \end{array}$$

These are referenced to ITRF2008 at coordinate epoch 2005.00

Step 1: convert the coordinates in the source CRS from the dataset coordinate epoch (2005.00) to the epoch of validity of the time-specific transformation (2010.00) using the Point Motion (geocentric Cartesian) method described in section 4.4.1. The formulas are repeated here for convenience:

$$\begin{pmatrix} X_{S(t_2)} \\ Y_{S(t_2)} \\ Z_{S(t_2)} \end{pmatrix} = \begin{pmatrix} X_{S(t_1)} \\ Y_{S(t_1)} \\ Z_{S(t_1)} \end{pmatrix} + (t_2 - t_1) * \begin{pmatrix} V_X \\ V_Y \\ V_Z \end{pmatrix}$$

$$\begin{array}{llllll} X_{S(t=2010.0)} & = & 2845\,456.0813 & + & (-0.0212) & * & (2010.0 - 2005.0) & = & 2845\,455.9753 \text{ m} \\ Y_{S(t=2010.0)} & = & 2160\,954.2453 & + & (+0.0124) & * & (2010.0 - 2005.0) & = & 2160\,954.3073 \text{ m} \\ Z_{S(t=2010.0)} & = & 5265\,993.2296 & + & (+0.0072) & * & (2010.0 - 2005.0) & = & 5265\,993.2656 \text{ m} \end{array}$$

Step 2: transform the coordinates in the source CRS at coordinate epoch 2010.00 to the target CRS at coordinate epoch 2010.00 using the Coordinate Frame rotation (geocentric domain) method described in section 4.2.3. The time-specific transformation from PZ-90.11 to ITRF2008 (EPSG Dataset transformation code 7960) parameter values are:

$$\begin{array}{ll} tX & = -0.003 \text{ m} \\ tY & = -0.001 \text{ m} \\ tZ & = 0.000 \text{ m} \\ rX & = 0.019 \text{ msec} \\ rY & = -0.042 \text{ msec} \\ rZ & = 0.002 \text{ msec} \\ dS & = 0.000 \text{ ppm} \\ t & = 2010.0 \end{array} \quad \text{from which } M = 1.0$$

This uses the Coordinate Frame rotation convention described in section 4.2.3.

Because the transformation is required in the reverse direction to that defined, the relevant parameters need to have their signs reversed. Transformation parameter values (from ITRF2008 to PZ-90.11) then are:

$$\begin{array}{llll} tX & = & 0.003 \text{ m} \\ tY & = & 0.001 \text{ m} \\ tZ & = & 0.000 \text{ m} \\ rX & = & -0.019 \text{ msec} & = & -9.21145994108117\text{E-}11 \text{ radians} \\ rY & = & 0.042 \text{ msec} & = & 2.03621746066005\text{E-}10 \text{ radians} \\ rZ & = & -0.002 \text{ msec} & = & -9.69627362219071\text{E-}12 \text{ radians} \\ dS & = & 0.000 \text{ ppm} & M = 1.0 \\ t & = & 2010.0 \end{array}$$

Then:

$$\begin{aligned} X_{T(t=2010.0)} &= 2845\,455.9772 \\ Y_{T(t=2010.0)} &= 2160\,954.3078 \\ Z_{T(t=2010.0)} &= 5265\,993.2652 \end{aligned}$$

Step 3: convert the coordinates in the target CRS from epoch 2010.00 to the required coordinate epoch 2013.90 using the Point Motion (geocentric Cartesian) method:

$$\begin{aligned} X_{T(t=2013.9)} &= 2845\,455.9772 + [-0.0212 * (2013.9 - 2010.0)] = 2845\,455.8945 \text{ m} \\ Y_{T(t=2013.9)} &= 2160\,954.3078 + [+0.0124 * (2013.9 - 2010.0)] = 2160\,954.3562 \text{ m} \\ Z_{T(t=2013.9)} &= 5265\,993.2652 + [+0.0072 * (2013.9 - 2010.0)] = 5265\,993.2945 \text{ m} \end{aligned}$$

## 4.3 Transformations between Geographic Coordinate Reference Systems

### 4.3.1 Transformations using geocentric methods

Transformation of coordinates from one geographic coordinate reference system into another is often carried out as a concatenation of the following operations:

(geographic to geocentric) + (geocentric to geocentric) + (geocentric to geographic)

See section 4.4 of Guidance Note 7 part 1 for a fuller description of the concatenation technique.

The middle step of the concatenated transformation, from geocentric to geocentric, may be through any of the methods described in section 4.2 above: 10-parameter Molodensky-Badekas transformations, 7-parameter Helmert or Bursa-Wolf transformations (static or dynamic (time-dependent)), or 3-parameter geocentric translations. The geographic 3D to/from geocentric steps of the concatenated transformation are described in section 4.1.1 above. When involving geographic 2D coordinates, the techniques described in the Geographic 3D to 2D conversions section 4.1.4 above are also used as additional steps at each end of the concatenation.

The concatenated geodetic transformations are:

(a) Source and target coordinates referenced to CRSs in the geographic 3D domain:

Molodensky-Badekas (PV geog3D domain), EPSG method code 1062

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 3D to Geocentric	9602	4.1.1
2	Molodensky-Badekas (PV geocentric domain)	1061	4.2.2
3	Geocentric to Geographic 3D	9602	4.1.1

Molodensky-Badekas (CF geog3D domain), EPSG method code 1039

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 3D to Geocentric	9602	4.1.1
2	Molodensky-Badekas (CF geocentric domain)	1034	4.2.2
3	Geocentric to Geographic 3D	9602	4.1.1

Position Vector transformation (geog3D domain), EPSG method code 1037

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 3D to Geocentric	9602	4.1.1
2	Position Vector transformation (geocentric domain)	1033	4.2.3
3	Geocentric to Geographic 3D	9602	4.1.1

Coordinate Frame rotation (geog3D domain), EPSG method code 1038

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 3D to Geocentric	9602	4.1.1
2	Coordinate Frame rotation (geocentric domain)	1032	4.2.3
3	Geocentric to Geographic 3D	9602	4.1.1

Time-dependent Position Vector transformation (geog3D domain), EPSG method code 1055

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 3D to Geocentric	9602	4.1.1
2	Time-dependent Position Vector transformation (geocentric domain)	1053	4.2.5
3	Geocentric to Geographic 3D	9602	4.1.1

Time-dependent Coordinate Frame rotation (geog3D domain), EPSG method code 1058

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 3D to Geocentric	9602	4.1.1
2	Time-dependent Coordinate Frame rotation (geocentric domain)	1056	4.2.5
3	Geocentric to Geographic 3D	9602	4.1.1

Geocentric Translations (geog3D domain), EPSG method code 1035

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 3D to Geocentric	9602	4.1.1
2	Geocentric Translations (geocentric domain)	1031	4.2.4
3	Geocentric to Geographic 3D	9602	4.1.1

(b) The concatenated geodetic transformations for source and target coordinates referenced to CRSs in the geographic 2D domain are:

Molodensky-Badekas (PV geog2D domain), EPSG method code 1063

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 2D to Geographic 3D	9659	4.1.4
2	Geographic 3D to Geocentric	9602	4.1.1
3	Molodensky-Badekas (PV geocentric domain)	1061	4.2.2
4	Geocentric to Geographic 3D	9602	4.1.1
5	Geographic 3D to Geographic 2D	9659	4.1.4

Molodensky-Badekas (CF geog2D domain), EPSG method code 9636

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 2D to Geographic 3D	9659	4.1.4
2	Geographic 3D to Geocentric	9602	4.1.1
3	Molodensky-Badekas (CF geocentric domain)	1034	4.2.2
4	Geocentric to Geographic 3D	9602	4.1.1
5	Geographic 3D to Geographic 2D	9659	4.1.4

See below for an example using this concatenated method.

Position Vector transformation (geog2D domain), EPSG method code 9606

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 2D to Geographic 3D	9659	4.1.4
2	Geographic 3D to Geocentric	9602	4.1.1
3	Position Vector transformation (geocentric domain)	1033	4.2.3
4	Geocentric to Geographic 3D	9602	4.1.1
5	Geographic 3D to Geographic 2D	9659	4.1.4

Coordinate Frame rotation (geog2D domain), EPSG method code 9607

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 2D to Geographic 3D	9659	4.1.4
2	Geographic 3D to Geocentric	9602	4.1.1
3	Coordinate Frame rotation (geocentric domain)	1032	4.2.3
4	Geocentric to Geographic 3D	9602	4.1.1
5	Geographic 3D to Geographic 2D	9659	4.1.4

Time-dependent Position Vector transformation (geog2D domain), EPSG method code 1054

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 2D to Geographic 3D	9659	4.1.4
2	Geographic 3D to Geocentric	9602	4.1.1
3	Time-dependent Position Vector transformation (geocentric domain)	1053	4.2.5
4	Geocentric to Geographic 3D	9602	4.1.1
5	Geographic 3D to Geographic 2D	9659	4.1.4

Time-dependent Coordinate Frame rotation (geog2D domain), EPSG method code 1057

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 2D to Geographic 3D	9659	4.1.4
2	Geographic 3D to Geocentric	9602	4.1.1
3	Time-dependent Coordinate Frame rotation (geocentric domain)	1056	4.2.5
4	Geocentric to Geographic 3D	9602	4.1.1
5	Geographic 3D to Geographic 2D	9659	4.1.4

Geocentric Translations (geog2D domain), EPSG method code 9603

<u>Step #</u>	<u>Step Method Name</u>	<u>EPSG Method Code</u>	<u>GN7-2 section</u>
1	Geographic 2D to Geographic 3D	9659	4.1.4
2	Geographic 3D to Geocentric	9602	4.1.1
3	Geocentric Translations (geocentric domain)	1031	4.2.4
4	Geocentric to Geographic 3D	9602	4.1.1
5	Geographic 3D to Geographic 2D	9659	4.1.4

**Example**

(using Molodensky-Badekas (CF geog2D domain)\_method as the middle step)

Transformation from La Canoa to REGVEN between geographic 2D coordinate reference systems (EPSG Dataset transformation code 1771).

The ten Molodensky-Badekas (CF) transformation parameter values for this transformation are:

tX	=	-270.933 m	
tY	=	+115.599 m	
tZ	=	-360.226 m	
rX	=	-5.266 sec	= -0.000025530288 radians
rY	=	-1.238 sec	= -0.000006001993 radians
rZ	=	+2.381 sec	= +0.000011543414 radians
dS	=	-5.109 ppm	
Ordinate 1 of evaluation point	=	2464351.59 m	
Ordinate 2 of evaluation point	=	-5783466.61 m	
Ordinate 3 of evaluation point	=	974809.81 m	

$$M = 1 + dS = 0.999994891$$

Ellipsoid Parameters for the source and target coordinate reference systems are:

<u>CRS name</u>	<u>Ellipsoid name</u>	<u>Semi-major axis (a)</u>	<u>Inverse flattening (1/f)</u>
La Canoa	International 1924	6378388.0 metres	1/f = 297.0
REGVEN	WGS 84	6378137.0 metres	1/f = 298.2572236

Input point coordinate system: La Canoa (geographic 2D)

$$\begin{aligned}\text{Latitude } \varphi_s &= 9^\circ 35' 00.386'' \text{N} \\ \text{Longitude } \lambda_s &= 66^\circ 04' 48.091'' \text{W}\end{aligned}$$

Step 1: Using the technique described in section 4.1.4 above, this is taken to be geographic 3D with an assumed ellipsoidal height  $h_s = 201.46$  m

Step 2: Using the geographic (3D) to geocentric conversion method given in section 4.1.1, these three coordinates convert to Cartesian geocentric coordinates:

$$\begin{aligned}X_s &= 2\,550\,408.96 \text{ m} \\ Y_s &= -5\,749\,912.26 \text{ m} \\ Z_s &= 1\,054\,891.11 \text{ m}\end{aligned}$$

Step 3: Application of the Molodensky-Badekas (CF geocentric domain) Transformation (section 4.2.2) results in:

$$\begin{aligned}X_T &= 2\,550\,138.46 \text{ m} \\ Y_T &= -5\,749\,799.87 \text{ m} \\ Z_T &= 1\,054\,530.82 \text{ m}\end{aligned}$$

on the REGVEN geocentric coordinate reference system (CRS code 4962)

Step 4: Using the reverse formulas for the geographic/geocentric conversion method given in section 4.1.1 on the REGVEN geographic 3D coordinate reference system (CRS code 4963) this converts into:

$$\begin{aligned}\text{Latitude } \varphi_T &= 9^\circ 34' 49.001'' \text{N} \\ \text{Longitude } \lambda_T &= 66^\circ 04' 54.705'' \text{W} \\ \text{Ellipsoidal height } h_T &= 180.51 \text{ m}\end{aligned}$$

Step 5: Because the source coordinates were 2D, using the geographic 3D to 2D method (section 4.1.4) the target system ellipsoidal height is dropped and the results treated as a geographic 2D coordinate reference system (CRS code 4189):

$$\begin{aligned}\text{Latitude } \varphi_T &= 9^\circ 34' 49.001'' \text{N} \\ \text{Longitude } \lambda_T &= 66^\circ 04' 54.705'' \text{W}\end{aligned}$$

Further examples of input and output for the Geocentric Translation (geographic domain) methods 9603 and 1035 may be found in test procedures 5212 and 5213 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

#### **4.3.1.1 France geocentric interpolation**

(EPSG Dataset coordinate operation method code 9655)

In France the national mapping agency (IGN) have promulgated a transformation between the classical geographic 2D coordinate reference system NTF and the modern 3-dimensional system RGF93 which uses geocentric translations interpolated from a grid file. The method is described in IGN document NTG-88. In summary:

- The grid file nodes are given in RGF93 geographic 2D coordinates.
- Within the grid file the sense of the parameter values is *from* NTF *to* RGF93.



For NTF to RGF93 transformations an iteration to obtain coordinates in the appropriate system for interpolation within the grid is required. The steps are:

- Convert NTF geographic 2D coordinates to geographic 3D by assuming a height and then to NTF geocentric coordinates.
- Transform NTF geocentric coordinates to approximate RGF93 coordinates using an average value for all France (EPSG Dataset coordinate operation code 1651):

$$\begin{aligned} X_{\text{NTF}} &= X_{\text{RGF93}} - 168 \text{ m} \\ Y_{\text{NTF}} &= Y_{\text{RGF93}} - 60 \text{ m} \\ Z_{\text{NTF}} &= Z_{\text{RGF93}} + 320 \text{ m} \end{aligned}$$

- Convert the approximate RGF93 geocentric coordinates to approximate RGF93 geographic coordinates.
- Using the approximate RGF93 geographic coordinates, interpolate within the grid file to obtain the three geocentric translations (tX, tY, tZ) applicable at the point.
- Apply these geocentric translations to the NTF geocentric coordinates to obtain RGF93 geocentric coordinates:
- Transform RGF93 geocentric coordinates to NTF geocentric coordinates, taking account of the sense of the parameter values.

$$\begin{aligned} X_{\text{RGF93}} &= X_{\text{NTF}} + tX \\ Y_{\text{RGF93}} &= Y_{\text{NTF}} + tY \\ Z_{\text{RGF93}} &= Z_{\text{NTF}} + tZ \end{aligned}$$

- Convert RGF93 geocentric coordinates to RGF93 geographic 3D coordinates. Because the original input NTF coordinates were geographic 2D, the RGF93 ellipsoidal height is meaningless so it is dropped to give RGF93 geographic 3D coordinates.

For RGF93 to NTF transformations the steps are:

- Using the RGF93 geographic coordinates, interpolate within the grid file to obtain the three geocentric translations (tX, tY, tZ) applicable at the point.
- Convert RGF93 geographic coordinates to RGF93 geocentric coordinates.
- Transform RGF93 geocentric coordinates to NTF geocentric coordinates, taking into account the sense of the parameter values:

$$\begin{aligned} X_{\text{NTF}} &= X_{\text{RGF93}} + (-tX) \\ Y_{\text{NTF}} &= Y_{\text{RGF93}} + (-tY) \\ Z_{\text{NTF}} &= Z_{\text{RGF93}} + (-tZ) \end{aligned}$$

- Convert NTF geocentric coordinates to geographic 3D coordinates.
- Drop the ellipsoid height to give NTF geographic 2D coordinates.

#### 4.3.2 **Abridged Molodensky transformation**

(EPSG Dataset coordinate operation method code 9605)

As an alternative to the computation of the new latitude, longitude and ellipsoid height by concatenation of three operations (geographic 3D to geocentric + geocentric to geocentric + geocentric to geographic 3D), the changes in these coordinates may be derived directly as geographic coordinate offsets through formulas derived by Molodensky (EPSG Dataset coordinate operation method code 9604, not detailed in this Guidance Note). Abridged versions of these formulas, which quite satisfactory for most practical purposes, are as follows:

$$\begin{aligned} \varphi_t &= \varphi_s + d\varphi \\ \lambda_t &= \lambda_s + d\lambda \\ h_t &= h_s + dh \end{aligned}$$

where

$$\begin{aligned} d\varphi &= (-tX \sin\varphi_s \cos\lambda_s - tY \sin\varphi_s \sin\lambda_s + tZ \cos\varphi_s + [a_s df + f_s da] \sin 2\varphi_s) / (\rho_s \sin 1") \\ d\lambda &= (-tX \sin\lambda_s + tY \cos\lambda_s) / (\nu_s \cos\varphi_s \sin 1") \\ dh &= tX \cos\varphi_s \cos\lambda_s + tY \cos\varphi_s \sin\lambda_s + tZ \sin\varphi_s + (a_s df + f_s da) \sin^2\varphi_s - da \end{aligned}$$



and where  $tX$ ,  $tY$  and  $tZ$  are the geocentric translation parameters,  $\rho_s$  and  $v_s$  are the meridian and prime vertical radii of curvature at the given latitude  $\phi_s$  on the first ellipsoid,  $da$  is the difference in the semi-major axes of the target and source ellipsoids and  $df$  is the difference in the flattening of the two ellipsoids:

$$\rho_s = a_s (1 - e_s^2) / (1 - e_s^2 \sin^2 \phi_s)^{3/2}$$

$$v_s = a_s / (1 - e_s^2 \sin^2 \phi_s)^{1/2}$$

$$da = a_t - a_s$$

$$df = f_t - f_s = 1/(1/f_t) - 1/(1/f_s).$$

The formulas for  $d\phi$  and  $d\lambda$  indicate changes in  $\phi$  and  $\lambda$  in arc-seconds.

### Example:

For a North Sea point with coordinates derived by GPS satellite in the WGS84 geographic coordinate reference system, with coordinates of:

$$\begin{aligned} \text{latitude } \phi_s &= 53^\circ 48' 33.82'' \text{N}, \\ \text{longitude } \lambda_s &= 2^\circ 07' 46.38'' \text{E}, \\ \text{and ellipsoidal height } h_s &= 73.0 \text{m}, \end{aligned}$$

whose coordinates are required in terms of the ED50 geographic coordinate reference system which takes the International 1924 ellipsoid.

The three geocentric translations parameter values from WGS 84 to ED50 for this North Sea area are given as  $tX = +84.87\text{m}$ ,  $tY = +96.49\text{m}$ ,  $tZ = +116.95\text{m}$ .

Ellipsoid Parameters are:

WGS 84	$a = 6378137.0$ metres	$1/f = 298.2572236$
International 1924	$a = 6378388.0$ metres	$1/f = 297.0$

Then

$$da = 6378388 - 6378137 = 251$$

$$df = 0.003367003 - 0.003352811 = 1.41927\text{E-}05$$

whence

$$\begin{aligned} d\phi &= (-68.448 - 2.894 + 69.056 + 87.079) / 30.917 = 2.743'' \\ d\lambda &= (-3.154 + 96.423) / 18.299 = 5.097'' \\ dh &= 50.079 + 2.117 + 94.385 + 59.510 - 251.000 = -44.909 \text{ m} \end{aligned}$$

ED50 values on the International 1924 ellipsoid are then:

$$\begin{aligned} \text{latitude } \phi_t &= 53^\circ 48' 36.563'' \text{N} \\ \text{longitude } \lambda_t &= 2^\circ 07' 51.477'' \text{E} \\ \text{and ellipsoidal height } h_t &= 28.091 \text{ m} \end{aligned}$$

Because ED50 is a geographic 2D coordinate reference system the height is dropped to give:

$$\begin{aligned} \text{latitude } \phi_t &= 53^\circ 48' 36.56'' \text{N} \\ \text{longitude } \lambda_t &= 2^\circ 07' 51.48'' \text{E} \end{aligned}$$

For comparison, better values computed through the concatenation of the three operations (geographic to geocentric + geocentric to geocentric + geocentric to geographic) are:

$$\begin{aligned} \text{latitude } \phi_t &= 53^\circ 48' 36.565'' \text{N} \\ \text{longitude } \lambda_t &= 2^\circ 07' 51.477'' \text{E} \\ \text{and ellipsoidal height } h_t &= 28.02 \text{ m} \end{aligned}$$

Further examples of input and output may be found in test procedure 5213 of the Test Dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

#### 4.3.3 Geographic Offsets

This is the simplest of transformations between two geographic coordinate reference systems, but is normally used only for purposes where low accuracy can be tolerated. It is generally used for transformations in two dimensions, latitude and longitude, where:

$$\begin{aligned}\varphi_t &= \varphi_s + d\varphi \\ \lambda_t &= \lambda_s + d\lambda\end{aligned}$$

(EPSG Dataset coordinate operation method code 9619).

In very rare circumstances, a transformation in three dimensions additionally including ellipsoidal height may be encountered:

$$\begin{aligned}\varphi_t &= \varphi_s + d\varphi \\ \lambda_t &= \lambda_s + d\lambda \\ h_t &= h_s + dh\end{aligned}$$

(EPSG coordinate operation method code 9660)

This should not be confused with the Geographic2D with Height Offsets method used in Japan, where the height difference is between the ellipsoidal height component of a 3D geographic coordinate reference system and a gravity-related height system. This is discussed in section 4.11.3.

#### **Example:**

A position with coordinates of 38°08'36.565"N, 23°48'16.235"E referenced to the old Greek geographic 2D coordinate reference system (EPSG dataset CRS code 4120) is to be transformed to the newer GGRS87 system (EPSG Dataset CRS code 4121). Transformation parameters from Greek to GGRS87 are:

$$\begin{aligned}d\varphi &= -5.86'' \\ d\lambda &= +0.28''\end{aligned}$$

$$\begin{array}{llllll} \text{Then} & \varphi_{\text{GGRS87}} & = & 38^\circ 08' 36.565''\text{N} & + & (-5.86'') & = & 38^\circ 08' 30.705''\text{N} \\ \text{and} & \lambda_{\text{GGRS87}} & = & 23^\circ 48' 16.235''\text{E} & + & 0.28'' & = & 23^\circ 48' 16.515''\text{E} \end{array}$$

For the reverse transformation for the same point,

$$\begin{aligned}\varphi_{\text{GREEK}} &= 38^\circ 08' 30.705''\text{N} + 5.86'' = 38^\circ 08' 36.565''\text{N} \\ \lambda_{\text{GREEK}} &= 23^\circ 48' 16.515''\text{E} + (-0.28'') = 23^\circ 48' 16.235''\text{E}\end{aligned}$$

#### 4.3.4 Geographic Offset by Interpolation of Gridded Data

The relationship between some geographic 2D coordinate reference systems is available through gridded data sets of latitude and longitude offsets. This family of methods includes:

*NADCON* (EPSG Dataset coordinate operation method code 9613) which is used by the US National Geodetic Survey for transformation between US systems;

*NTv2* (EPSG Dataset coordinate operation method code 9615) which originated in the national mapping agency of Canada and was subsequently adopted in Australia, New Zealand and then several other countries; and

*OSTN* (EPSG Dataset coordinate operation method code 9633) used in Great Britain.

The offsets at a point are derived by interpolation within the gridded data. In some methods, separate grid files are given for latitude and longitude offsets whilst in other methods the offsets for both latitude and longitude are given within a single grid file. The EPSG Dataset differentiates methods by the format of the gridded data file(s). The grid file format is given in documentation available from the information source. Although the authors of some data sets suggest a particular interpolation method within the grid(s), generally the density of grid nodes should be such that any reasonable grid interpolation method will give the same

offset value. Bi-linear interpolation is the most usual grid interpolation mechanism. The interpolated value of the offset A is then added to the source CRS coordinate value to give the coordinates in the target CRS.

### Reversibility

The coordinate reference system for the coordinates of the grid nodes will be given either in the file itself or in accompanying documentation. This will normally be the source coordinate reference system for the forward transformation. Then in forward transformations the offset is obtained through straightforward interpolation of the grid file. But for the reverse transformation the first grid interpolation entry will be the value of the point in the second coordinate reference system, the offsets are interpolated and applied *with sign reversed*, and the result used in further iterations of interpolation and application of offset until the difference between results from successive iterations is insignificant.

### Example

Examples for the NADCON and NTV2 methods may be found in test procedures 5206 and 5207 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

## 4.4 Point Motion Operations

Classical coordinate reference systems are 'static', that is the spatial coordinates of a point do not change with time. Modern global geodetic reference frames have an Earth-centred Cartesian 3D coordinate system that co-rotates with the Earth as a whole. Points on the crust of the Earth are subject to tectonic motion and deformation and in a global geodetic reference frame their coordinates may change with time, either as the point on the tectonic plate moves (slowly) through the coordinate space or as a consequence of tectonic deformation caused by e.g. earthquake or post-glacial isostatic adjustment. Such systems are called 'dynamic'. Their coordinates are qualified by their *coordinate epoch*. By convention, coordinate epoch is given in decimal years referenced to the Gregorian calendar. Coordinates of a point at coordinate epoch  $t_1$  may be calculated at some other coordinate epoch  $t_2$  by one of the methods below.

It is stressed that the change of coordinates due to point motion is due entirely to the motion of the point and that there is *no change of coordinate reference system*. The coordinate change due to point motion happens *within* a coordinate reference system.

The velocities necessary for these conversions may come from the station solutions of the reference frame definition (datum parameters), or from a plate motion velocity grid. This motion compensation may then be concatenated with a 'static' transformation to form a time-specific transformation (refer to section 4.2.6).

### 4.4.1 Point motion (geocentric Cartesian)

(EPSG Dataset coordinate operation method code 1064)

The geocentric Cartesian 3D coordinates of a point at coordinate epoch  $t_1$  may be calculated at any other coordinate epoch  $t_2$  from:

$$\begin{pmatrix} X_{t2} \\ Y_{t2} \\ Z_{t2} \end{pmatrix} = \begin{pmatrix} X_{t1} \\ Y_{t1} \\ Z_{t1} \end{pmatrix} + (t_2 - t_1) * \begin{pmatrix} V_X \\ V_Y \\ V_Z \end{pmatrix}$$

where **V** describes the linear velocities of the Cartesian coordinates.

**Example:**

Given a point with geocentric Cartesian coordinates at coordinate epoch 2005.00 and having linear velocities:

$$\begin{array}{llll} X & = & 2845\,456.0813 \text{ m} & V_X = -0.0212 \text{ m/yr} \\ Y & = & 2160\,954.2453 \text{ m} & V_Y = +0.0124 \text{ m/yr} \\ Z & = & 5265\,993.2296 \text{ m} & V_Z = +0.0072 \text{ m/yr} \\ t_1 & = & 2005.00 & \end{array}$$

whose coordinates are required at coordinate epoch 2010.0.

$$\begin{array}{llll} X_{(t=2010.0)} & = & 2845\,456.0813 & + (2010.00 - 2005.00) * (-0.0212) \\ Y_{(t=2010.0)} & = & 2160\,954.2453 & + (2010.00 - 2005.00) * (+0.0124) \\ Z_{(t=2010.0)} & = & 5265\,993.2296 & + (2010.00 - 2005.00) * (+0.0072) \end{array}$$

Then

$$\begin{array}{ll} X & = 2845\,455.9753 \text{ m} \\ Y & = 2160\,954.3073 \text{ m} \\ Z & = 5265\,993.2656 \text{ m} \\ t_2 & = 2010.00 \end{array}$$

#### 4.4.2 Point motion (ellipsoidal)

(EPSG Dataset coordinate operation method code 1067)

The ellipsoidal coordinates of a point at coordinate epoch  $t_1$  may be calculated at any other coordinate epoch  $t_2$  from:

$$\begin{pmatrix} \phi_{(t_2)} \\ \lambda_{(t_2)} \\ h_{(t_2)} \end{pmatrix} = \begin{pmatrix} \phi_{(t_1)} \\ \lambda_{(t_1)} \\ h_{(t_1)} \end{pmatrix} + (t_2 - t_1) * \begin{pmatrix} V_\phi \\ V_\lambda \\ V_h \end{pmatrix}$$

where  $\mathbf{V}$  describes the velocities of the ellipsoidal coordinates. These are usually given as a rate in linear units (say millimetres per year) resolved into north, east and up components  $V_N$ ,  $V_E$  and  $V_h$ . The north and east components are converted into latitude and longitude components by:

$$\begin{aligned} V_\phi &= V_N / (\rho + h) \\ V_\lambda &= V_E / [(v + h) \cos \phi] \end{aligned}$$

where

$\rho$  is radius of curvature of the CRS's ellipsoid in the plane of the meridian at latitude  $\phi$

$$\rho = a(1 - e^2)/(1 - e^2 \sin^2 \phi)^{3/2}$$

$v$  is radius of curvature of the ellipsoid perpendicular to the meridian at latitude  $\phi$

$$v = a/(1 - e^2 \sin^2 \phi)^{1/2}$$

**Example:**

Given a point with ellipsoidal coordinates at coordinate epoch 2017.55 and having linear velocities:

$$\begin{array}{llll} \phi & = & 51.0^\circ \text{N} & V_N = +15.12 \text{ mm/yr} \\ \lambda & = & 141.0^\circ \text{W} & V_E = -2.86 \text{ mm/yr} \\ h & = & 1000 \text{ m} & V_{UP} = +1.10 \text{ mm/yr} \\ t & = & 2017.55 \text{ (years)} & \end{array}$$

whose coordinates are required at coordinate epoch 1997.00. The point is referenced to a CRS which uses the GRS 1980 ellipsoid for which  $a = 6378137.0\text{m}$  and  $e^2 = 0.006694380$ .

Then (after conversion to appropriate units):

$$\begin{aligned} V_\phi &= 2.37174\text{E-}09 \text{ rad/yr} \\ V_\lambda &= -7.10972\text{E-}07 \text{ rad/yr} \\ V_h &= 0.0011 \text{ m / yr} \end{aligned}$$

and

$$\begin{aligned}\varphi &= 0.890117919 + (1997.00-2017.55) * 2.37174\text{E-}09 = 0.890117870 \text{ radians} \\ \lambda &= 0.890117919 + (1997.00-2017.55) * -7.10972\text{E-}07 = -2.460914231 \text{ radians} \\ h &= 1000.0 + (1997.00-2017.55) * 0.0011 = 999.977\text{m}\end{aligned}$$

$$\begin{aligned}\varphi &= 50^{\circ}59'59.990''\text{N}, \quad \lambda = 140^{\circ}59'59.997''\text{W}, \quad h = 999.977\text{m} \\ t &= 1997.00\end{aligned}$$

#### 4.4.3 **Point motion using velocity grid**

The velocities of points in some geodetic coordinate reference systems are available through gridded data sets (sometimes called velocity grids). First the velocities at a point are obtained through interpolation within the grid; bi-linear interpolation is the most usual grid interpolation mechanism, although the providers of some gridded data sets may suggest an alternative interpolation method. Then the interpolated velocities are applied as for the point motion (geocentric Cartesian) or point motion (ellipsoidal) methods in the previous sections.

For the purposes of interpolation within the grid, horizontal coordinates of the point are required. The providers of gridded data sets define the horizontal CRS upon which the grid has been constructed and which is to be used for the grid interpolation; this is the geographical 2D subset of the geodetic CRS to which coordinates are referenced.

The EPSG Dataset differentiates methods by the format of the gridded data file. The grid file format is given in documentation available from the information source. An example is *NTv2\_Vel* (EPSG Dataset coordinate operation method code 1070) which is used by Natural Resources Canada for point motion in NAD83(CSRS) .

### 4.5 **Transformations between Projected Coordinate Reference Systems**

#### 4.5.1 **Cartesian Grid Offsets from Form Function**

(EPSG Dataset coordinate operation method code 1036)

In the German state of Schleswig-Holstein the Cartesian grid offsets to be applied are determined through interpolation within an irregular grid of points at which coordinates in both source and target coordinate reference systems are given. The interpolation uses a finite element method form function procedure described in papers by Joachim Boljen in *Zeitschrift für Vermessungswesen* (ZfV, the Journal of the German Association of Surveying) volume 128 of April 2003 pages 244-250 and volume 129 of April 2004 pages 258-260.

## 4.6 Polynomial transformations

*Note: In the sections that follow, the general mathematical symbols  $X$  and  $Y$  representing the axes of a coordinate reference system must not be confused with the specific axis abbreviations or axis order in particular coordinate reference systems.*

### 4.6.1 General case

Polynomial transformations between two coordinate reference systems are typically applied in cases where one or both of the coordinate reference systems exhibits lack of homogeneity in orientation and scale. The small distortions are then approximated by polynomial functions in latitude and longitude or in easting and northing. Depending on the degree of variability in the distortions, approximation may be carried out using polynomials of degree 2, 3, or higher. In the case of transformations between two projected coordinate reference systems, the additional distortions resulting from the application of two map projections and a datum transformation can be included in a single polynomial approximation function.

Polynomial approximation functions themselves are subject to variations, as different approximation characteristics may be achieved by different polynomial functions. The simplest of all polynomials is the general polynomial function. In order to avoid problems of numerical instability this type of polynomial should be used after reducing the coordinate values in both the source and the target coordinate reference system to ‘manageable’ numbers, between  $-10$  and  $+10$  at most. This is achieved by working with offsets relative to a central evaluation point, scaled to the desired number range by applying a scaling factor to the coordinate offsets.

Hence an evaluation point is chosen in the source coordinate reference system ( $X_{S0}$ ,  $Y_{S0}$ ) and in the target coordinate reference system ( $X_{T0}$ ,  $Y_{T0}$ ). Often these two sets of coordinates do not refer to the same physical point but two points are chosen that have the same coordinate values in both the source and the target coordinate reference system. (When the two points have identical coordinates, these parameters are conveniently eliminated from the formulas, but the general case where the coordinates differ is given here).

The selection of an evaluation point in each of the two coordinate reference systems allows the point coordinates in both to be reduced as follows:

$$\begin{aligned} X_S - X_{S0} \\ Y_S - Y_{S0} \end{aligned}$$

and

$$\begin{aligned} X_T - X_{T0} \\ Y_T - Y_{T0} \end{aligned}$$

These coordinate differences are expressed in their own unit of measure, which may not be the same as that of the corresponding coordinate reference system.<sup>5)</sup>

A further reduction step is usually necessary to bring these coordinate differences into the desired numerical range by applying a scaling factor to the coordinate differences in order to reduce them to a value range that may be applied to the polynomial formulae below without introducing numerical precision errors:

$$\begin{aligned} U &= m_S (X_S - X_{S0}) \\ V &= m_S (Y_S - Y_{S0}) \end{aligned}$$

where

$X_S$ ,  $Y_S$  are coordinates in the source coordinate reference system,  
 $X_{S0}$ ,  $Y_{S0}$  are coordinates of the evaluation point in the source coordinate reference system,  
 $m_S$  is the scaling factor applied the coordinate differences in the source coordinate reference system.

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<sup>5)</sup> If the source and/or the target coordinate reference system are geographic, the coordinates themselves may be expressed in sexagesimal degrees (degrees, minutes, seconds), which cannot be directly processed by a mathematical formula.

The normalised coordinates U and V of the point whose coordinates are to be transformed are used as input to the polynomial transformation formula. In order to control the numerical range of the polynomial coefficients  $A_n$  and  $B_n$  the output coordinate differences dX and dY are multiplied by a scaling factor,  $m_T$ .

$$\begin{aligned} m_T \cdot dX &= A_0 + A_1U + A_2V + A_3U^2 + A_4UV + A_5V^2 && \text{(to degree 2)} \\ &+ A_6U^3 + A_7U^2V + A_8UV^2 + A_9V^3 && \text{(degree 3 terms)} \\ &+ A_{10}U^4 + A_{11}U^3V + A_{12}U^2V^2 + A_{13}UV^3 + A_{14}V^4 && \text{(degree 4 terms)} \\ &+ A_{15}U^5 + A_{16}U^4V + A_{17}U^3V^2 + A_{18}U^2V^3 + A_{19}UV^4 + A_{20}V^5 && \text{(degree 5 terms)} \\ &+ A_{21}U^6 + A_{22}U^5V + A_{23}U^4V^2 + A_{24}U^3V^3 + A_{25}U^2V^4 + A_{26}UV^5 + A_{27}V^6 && \text{(degree 6 terms)} \\ &+ \dots + A_{104}V^{13} && \text{(degree 13 terms)} \end{aligned}$$

$$\begin{aligned} m_T \cdot dY &= B_0 + B_1U + B_2V + B_3U^2 + B_4UV + B_5V^2 && \text{(to degree 2)} \\ &+ B_6U^3 + B_7U^2V + B_8UV^2 + B_9V^3 && \text{(degree 3 terms)} \\ &+ B_{10}U^4 + B_{11}U^3V + B_{12}U^2V^2 + B_{13}UV^3 + B_{14}V^4 && \text{(degree 4 terms)} \\ &+ B_{15}U^5 + B_{16}U^4V + B_{17}U^3V^2 + B_{18}U^2V^3 + B_{19}UV^4 + B_{20}V^5 && \text{(degree 5 terms)} \\ &+ B_{21}U^6 + B_{22}U^5V + B_{23}U^4V^2 + B_{24}U^3V^3 + B_{25}U^2V^4 + B_{26}UV^5 + B_{27}V^6 && \text{(degree 6 terms)} \\ &+ \dots + B_{104}V^{13} && \text{(degree 13 terms)} \end{aligned}$$

from which dX and dY are evaluated. These will be in the units of the target coordinate reference system.

In the EPSG Dataset, the polynomial coefficients are given as parameters with names of the form *Aumvn* and *Bumvn*, where m is the power to which U is raised and n is the power to which V is raised. For example, polynomial coefficient  $A_{17}$  is represented as coordinate operation parameter named *Au3v2*.

*Note: for brevity, polynomial coefficients with a value of zero are omitted from the EPSG Dataset.*

The relationship between the two coordinate reference systems can now be written as follows:

$$\begin{aligned} (X_T - X_{TO}) &= (X_S - X_{SO}) + dX \\ (Y_T - Y_{TO}) &= (Y_S - Y_{SO}) + dY \end{aligned}$$

or

$$\begin{aligned} X_T &= X_S - X_{SO} + X_{TO} + dX \\ Y_T &= Y_S - Y_{SO} + Y_{TO} + dY \end{aligned}$$

where:

$X_T, Y_T$  are coordinates in the target coordinate reference system,  
 $X_S, Y_S$  are coordinates in the source coordinate reference system,  
 $X_{SO}, Y_{SO}$  are coordinates of the evaluation point in the source coordinate reference system,  
 $X_{TO}, Y_{TO}$  are coordinates of the evaluation point in the target coordinate reference system,  
dX, dY are derived through the scaled polynomial formulas.

Other (arguably better) approximating polynomials are described in mathematical textbooks such as “*Theory and applications of numerical analysis*”, by G.M. Phillips and P.J. Taylor (Academic Press, 1973).



**Example: General polynomial of degree 6** (EPSG Dataset coordinate operation method code 9648)

For coordinate transformation TM75 to ETRS89 (1)

Ordinate 1 of evaluation point  $X_O$  in source CRS:  $X_{SO} = \varphi_{SO} = 53^\circ 30' 00.000''N = +53.5$  degrees  
 Ordinate 2 of evaluation point  $Y_O$  in source CRS:  $Y_{SO} = \lambda_{SO} = 7^\circ 42' 00.000''W = -7.7$  degrees  
 Ordinate 1 of evaluation point  $X_O$  in target CRS :  $X_{TO} = \varphi_{TO} = 53^\circ 30' 00.000''N = +53.5$  degrees  
 Ordinate 2 of evaluation point  $Y_O$  in target CRS :  $Y_{SO} = \lambda_{TO} = 7^\circ 42' 00.000''W = -7.7$  degrees  
 Scaling factor for source CRS coordinate differences:  $m_S = 0.1$   
 Scaling factor for target CRS coordinate differences:  $m_T = 3600$

Polynomial coefficients (see EPSG Dataset transformation code 1041 for complete set of values):

$A_0 = 0.763$        $A_1 = -4.487$       ....       $A_{24} = -265.898$       ...       $A_{27} = 0$   
 $B_0 = -2.810$        $B_1 = -0.341$       ....       $B_{24} = -853.950$       ...       $B_{27} = 0$

*Note: for brevity, polynomial coefficients with a value of zero are omitted from the EPSG Dataset.*

Forward calculation for:

Latitude  $\varphi_{TM75} = X_s = 55^\circ 00' 00''N = +55.000$  degrees  
 Longitude  $\lambda_{TM75} = Y_s = 6^\circ 30' 00''W = -6.500$  degrees

$X_S - X_{SO} = \varphi_{TM75} - \varphi_{SO} = 55.0 - 53.5 = 1.5$  degrees  
 $Y_S - Y_{SO} = \lambda_{TM75} - \lambda_{SO} = -6.5 - (-7.7) = 1.2$  degrees

$U = m_S (X_S - X_{SO}) = m_S (\varphi_{TM75} - \varphi_{SO}) = 0.1 * (1.5) = 0.15$   
 $V = m_S (Y_S - Y_{SO}) = m_S (\lambda_{TM75} - \lambda_{SO}) = 0.1 * (1.2) = 0.12$

$dX = (A_0 + A_1 U + \dots + A_5 V^2 + \dots + A_{24} U^3 V^3) / m_T$   
 $= [0.763 + (-4.487 * 0.15) + \dots + (0.183 * 0.12^2) + \dots + (-265.898 * 0.15^3 * 0.12^3)] / 3600$

$dY = (B_0 + B_1 U + \dots + B_{24} U^3 V^3) / m_T$   
 $= [-2.81 + (-0.341 * 0.15) + \dots + (-853.95 * 0.15^3 * 0.12^3)] / 3600$

	to degree 2	degree 3	degree 4	degree 5	degree 6	Sum / $m_T$
$dX =$	0.1029127	-0.002185407	0.0064009440	0.0014247770	-0.0015507171	0.0000297229
$dY =$	-3.3955340	0.022364019	-0.0230149836	-0.0156886729	-0.0049802364	-0.0009491261

Then Latitude  $\varphi_{ETRS89} = X_T = X_S + dX = 55.0 + 0.00002972$  degrees =  $55^\circ 00' 00.107''N$   
 Longitude  $\lambda_{ETRS89} = Y_T = Y_S + dY = -6.5 - 0.00094913$  degrees =  $6^\circ 30' 03.417''W$

### Polynomial reversibility

Approximation polynomials are in a strict mathematical sense **not reversible**, i.e. the same polynomial coefficients cannot be used to execute the reverse transformation.

In principle two options are available to execute the reverse transformation:

1. By applying a similar polynomial transformation with a different set of polynomial coefficients for the reverse polynomial transformation. This would result in a separate forward and reverse transformation being stored in the EPSG Dataset (or any other geodetic data repository).
2. By applying the polynomial transformation with the same coefficients but with their signs reversed and then iterate to an acceptable solution, the number of iteration steps being dependent on the desired accuracy. (Note that only the signs of the polynomial coefficients should be reversed and not the coordinates of the evaluation points or the scaling factors!) The iteration procedure is usually described by the information source of the polynomial transformation.



However, under certain conditions, described below, a satisfactory solution for the reverse transformation may be obtained using the forward coefficient values in a single step, rather than multiple step iteration. If such a solution is possible, in the EPSG Dataset the polynomial coordinate transformation method is classified as a **reversible polynomial of degree  $n$** .

A (general) polynomial transformation is reversible only when the following conditions are met.

1. The coordinates of source and target evaluation point are (numerically) the same.
2. The unit of measure of the coordinate differences in source and target coordinate reference system are the same.
3. The scaling factors applied to source and target coordinate differences are the same.
4. The spatial variation of the differences between the coordinate reference systems around any given location is sufficiently small.

Clarification on conditions for polynomial reversibility:

Re 1 and 2 - In the reverse transformation the roles of the source and target coordinate reference systems are reversed. Consequently, the coordinates of the evaluation point in the *source* coordinate reference system become those in the *target* coordinate reference system in the reverse transformation. Usage of the same transformation parameters for the reverse transformation will therefore only be valid if the evaluation point coordinates are numerically the same in source and target coordinate reference system and in the same units. That is,  $X_{S0} = X_{T0} = X_0$  and  $Y_{S0} = Y_{T0} = Y_0$ .

Re 3 - The same holds for the scaling factors of the source and target coordinate differences and for the units of measure of the coordinate differences. That is,  $m_S = m_T = m$ .

Re 4 - If conditions 1, 2 and 3 are all satisfied it then may be possible to use the forward polynomial algorithm with the forward parameters for the reverse transformation. This is the case if the spatial variations in  $dX$  and  $dY$  around any given location are sufficiently constant. The signs of the polynomial coefficients are then reversed but the scaling factor and the evaluation point coordinates retain their signs. If these spatial variations in  $dX$  and  $dY$  are too large, for the reverse transformation iteration would be necessary. It is therefore not the algorithm that determines whether a single step solution is sufficient or whether iteration is required, but the desired accuracy combined with the degree of spatial variability of  $dX$  and  $dY$ .

An example of a reversible polynomial is transformation is ED50 to ED87 (1) for the North Sea. The suitability of this transformation to be described by a reversible polynomial can easily be explained. In the first place both source and target coordinate reference systems are of type geographic 2D. The typical difference in coordinate values between ED50 and ED87 is in the order of 2 metres ( $\approx 10^{-6}$  degrees) in the area of application. The polynomial functions are evaluated about central points with coordinates of  $55^\circ\text{N}$ ,  $0^\circ\text{E}$  in both coordinate reference systems. The reduced coordinate differences (*in degrees*) are used as input parameters to the polynomial functions. The output coordinate differences are corrections to the input coordinate offsets of about  $10^{-6}$  degrees. This difference of several orders of magnitude between input and output values is the property that makes a polynomial function reversible in practice (although not in a formal mathematical sense).

The error made by the polynomial approximation formulas in calculating the reverse correction is of the same order of magnitude as the ratio of output versus input:

$$\frac{\text{output error}}{\text{input error}} \approx \frac{\text{output value}}{\text{input value}} \quad (\approx 10^{-6})$$

As long as the input values (the coordinate offsets from the evaluation point) are orders of magnitude larger than the output (the corrections), and provided the coefficients are used with changed signs, the polynomial transformation may be considered to be reversible.

Hence the EPSG Dataset acknowledges two classes of general polynomial functions, reversible and non-reversible, as distinguished by whether or not the coefficients may be used in both forward and reverse transformations, i.e. are reversible. The EPSG Dataset does not describe the iterative solution as a separate algorithm. The iterative solution for the reverse transformation, when applicable, is deemed to be implied by the (forward) algorithm.

**Example: Reversible polynomial of degree 4** (EPSG Dataset coordinate operation method code 9651)  
 For coordinate transformation ED50 to ED87 (1)

Ordinate 1 of evaluation point:  $X_0 = \varphi_0 = 55^{\circ}00'00.000''\text{N} = +55 \text{ degrees}$   
 Ordinate 2 of evaluation point:  $Y_0 = \lambda_0 = 0^{\circ}00'00.000''\text{E} = +0 \text{ degrees}$

Scaling factor for coordinate differences:  $m = 1.0$

Parameters:

$A_0 = -5.56098\text{E-}06$        $A_1 = -1.55391\text{E-}06$     ...     $A_{14} = -4.01383\text{E-}09$   
 $B_0 = +1.48944\text{E-}05$        $B_2 = +2.68191\text{E-}05$     ...     $B_{14} = +7.62236\text{E-}09$

Forward calculation for:

Latitude  $\varphi_{\text{ED50}} = X_s = 52^{\circ}30'30''\text{N} = +52.508333333 \text{ degrees}$   
 Longitude  $\lambda_{\text{ED50}} = Y_s = 2^{\circ}\text{E} = +2.0 \text{ degrees}$

$U = m * (X_s - X_0) = m * (\varphi_{\text{ED50}} - \varphi_0) = 1.0 * (52.508333333 - 55.0) = -2.491666667 \text{ degrees}$   
 $V = m * (Y_s - Y_0) = m * (\lambda_{\text{ED50}} - \lambda_0) = 1.0 * (2.0 - 0.0) = 2.0 \text{ degrees}$

$dX = (A_0 + A_1U + \dots + A_{14}V^{14}) / k_{\text{CD}}$   
 $= [-5.56098\text{E-}06 + (-1.55391\text{E-}06 * -2.491666667) + \dots + (-4.01383\text{E-}09 * 2.0^{14})] / 1.0$   
 $= -3.12958\text{E-}06 \text{ degrees}$

$dY = (B_0 + B_1U + \dots + B_{14}V^{14}) / k_{\text{CD}}$   
 $= [+1.48944\text{E-}05 + (2.68191\text{E-}05 * -2.491666667) + \dots + (7.62236\text{E-}09 * 2.0^{14})] / 1.0$   
 $= +9.80126\text{E-}06 \text{ degrees}$

Then: Latitude  $\varphi_{\text{ED87}} = X_T = X_s + dX = 52.508333333 - 3.12958\text{E-}06 \text{ degrees} = 52^{\circ}30'29.9887''\text{N}$   
 Longitude  $\lambda_{\text{ED87}} = Y_T = Y_s + dY = 2^{\circ}00'00.0353''\text{E}$

Reverse calculation for coordinate transformation ED50 to ED87 (1).

The polynomial to degree 4 transformation for the ED50 to ED87 (1) coordinate transformation is reversible. The same formulas may be applied for the reverse calculation, but coefficients  $A_0$  through  $A_{14}$  and  $B_0$  through  $B_{14}$  are applied with reversal of their signs. Sign reversal is not applied to the coordinates of the evaluation point or scaling factor for coordinate differences. Thus:

Ordinate 1 of evaluation point:  $X_0 = \varphi_0 = 55^{\circ}00'00.000''\text{N} = +55 \text{ degrees}$   
 Ordinate 2 of evaluation point:  $Y_0 = \lambda_0 = 0^{\circ}00'00.000''\text{E} = +0 \text{ degrees}$   
 Scaling factor for coordinate differences:  $m = 1.0$

$A_0 = +5.56098\text{E-}06$        $A_1 = +1.55391\text{E-}06$     ...     $A_{14} = +4.01383\text{E-}09$   
 $B_0 = -1.48944\text{E-}05$        $B_1 = -2.68191\text{E-}05$     ...     $B_{14} = -7.62236\text{E-}09$

Reverse calculation for:

Latitude  $\varphi_{\text{ED87}} = X_s = 52^{\circ}30'29.9887''\text{N} = +52.5083301944 \text{ degrees}$   
 Longitude  $\lambda_{\text{ED87}} = Y_s = 2^{\circ}00'00.0353''\text{E} = +2.0000098055 \text{ degrees}$

$U = 1.0 * (52.5083301944 - 55.0) = -2.4916698056 \text{ degrees}$

$$V = 1.0 * (2.0000098055 - 0.0) = 2.0000098055 \text{ degrees}$$

$$\begin{aligned} dX &= (A_0 + A_1 U + \dots + A_{14} V^4) / k \\ &= [5.56098E-06 + (1.55391E-06 * -2.491666667) + \dots \\ &\quad \dots + (4.01383E-09 * 2.0000098055^4)] / 1.0 \\ &= +3.12957E-06 \text{ degrees} \end{aligned}$$

$$\begin{aligned} dY &= (B_0 + B_1 U + \dots + B_{14} V^4) / k \\ &= [-1.48944E-05 + (-2.68191E-05 * -2.491666667) + \dots \\ &\quad \dots + (-7.62236E-09 * 2.0000098055^4)] / 1.0 \\ &= -9.80124E-06 \text{ degrees} \end{aligned}$$

$$\begin{aligned} \text{Then: Latitude } \phi_{ED50} &= X_T = X_S + dX = 52.5083301944 + 3.12957E-06 \text{ degrees} = 52^\circ 30' 30.000'' \text{N} \\ \text{Longitude } \lambda_{ED50} &= Y_T = Y_S + dY = 2^\circ 00' 00.000'' \text{E} \end{aligned}$$

#### 4.6.2 Polynomial transformation with complex numbers

The relationship between two projected coordinate reference systems may be approximated more elegantly by a single polynomial regression formula written in terms of complex numbers. The advantage is that the dependence between the 'A' and 'B' coefficients (for U and V) is taken into account in the formula, resulting in fewer coefficients for the same order polynomial. A polynomial to degree 3 in complex numbers is used in Belgium. A polynomial to degree 4 in complex numbers is used in The Netherlands for transforming coordinates referenced to the Amersfoort / RD system to and from ED50 / UTM.

$$\begin{aligned} m_T (dX + i dY) &= (A_1 + i A_2) (U + i V) + (A_3 + i A_4) (U + i V)^2 \quad (\text{to degree 2}) \\ &\quad + (A_5 + i A_6) (U + i V)^3 \quad (\text{additional degree 3 terms}) \\ &\quad + (A_7 + i A_8) (U + i V)^4 \quad (\text{additional degree 4 terms}) \end{aligned}$$

$$\text{where } U = m_S (X_S - X_{S0})$$

$$V = m_S (Y_S - Y_{S0})$$

and  $m_S$ ,  $m_T$  are the scaling factors for the coordinate differences in the source and target coordinate reference systems.

The polynomial to degree 4 can alternatively be expressed in matrix form as

$$\begin{pmatrix} m_T \cdot dX \\ m_T \cdot dY \end{pmatrix} = \begin{pmatrix} +A_1 & -A_2 & +A_3 & -A_4 & +A_5 & -A_6 & +A_7 & -A_8 \\ +A_2 & +A_1 & +A_4 & +A_3 & +A_6 & +A_5 & +A_8 & +A_7 \end{pmatrix} * \begin{pmatrix} U \\ V \\ U^2 - V^2 \\ 2UV \\ U^3 - 3UV^2 \\ 3U^2V - V^3 \\ U^4 - 6U^2V^2 + V^4 \\ 4U^3V - 4UV^3 \end{pmatrix}$$

Then as for the general polynomial case above

$$X_T = X_S - X_{S0} + X_{T0} + dX$$

$$Y_T = Y_S - Y_{S0} + Y_{T0} + dY$$

where, as above,

$X_T$ ,  $Y_T$  are coordinates in the target coordinate system,

$X_S$ ,  $Y_S$  are coordinates in the source coordinate system,

$X_{SO}$ ,  $Y_{SO}$  are coordinates of the evaluation point in the source coordinate reference system,  
 $X_{TO}$ ,  $Y_{TO}$  are coordinates of the evaluation point in the target coordinate reference system.

Note that the zero order coefficients of the general polynomial,  $A_0$  and  $B_0$ , have apparently disappeared. In reality they are absorbed by the different coordinates of the source and of the target evaluation point, which in this case, are numerically very different because of the use of two different projected coordinate systems for source and target.

The transformation parameter values (the coefficients) are not reversible. For the reverse transformation a different set of parameter values are required, used within the same formulas as the forward direction.

**Example:** Complex polynomial of degree 4 (EPSG Dataset coordinate operation method code 9653)  
 Coordinate transformation: Amersfoort / RD New to ED50 / UTM zone 31N (1) (code 1044):

<u>Coordinate transformation parameter name</u>	<u>Formula symbol</u>	<u>Parameter value</u>	<u>Unit</u>
ordinate 1 of the evaluation point in the source CRS	$X_{SO}$	155,000.000	metre
ordinate 2 of the evaluation point in the source CRS	$Y_{SO}$	463,000.000	metre
ordinate 1 of the evaluation point in the target CRS	$X_{TO}$	663,395.607	metre
ordinate 2 of the evaluation point in the target CRS	$Y_{TO}$	5,781,194.380	metre
scaling factor for source CRS coordinate differences	$m_S$	$10^{-5}$	
scaling factor for target CRS coordinate differences	$m_T$	1.0	
A1	$A_1$	-51.681	coefficient
A2	$A_2$	+3,290.525	coefficient
A3	$A_3$	+20.172	coefficient
A4	$A_4$	+1.133	coefficient
A5	$A_5$	+2.075	coefficient
A6	$A_6$	+0.251	coefficient
A7	$A_7$	+0.075	coefficient
A8	$A_8$	-0.012	coefficient

For input point:

Easting,  $X_{AMERSFOORT/RD} = X_S = 200,000.00$  metres  
 Northing,  $Y_{AMERSFOORT/RD} = Y_S = 500,000.00$  metres

$$U = m_S (X_S - X_{SO}) = (200,000 - 155,000) 10^{-5} = 0.45$$

$$V = m_S (Y_S - Y_{SO}) = (500,000 - 463,000) 10^{-5} = 0.37$$

$$dX = (-1,240.050) / 1.0$$

$$dY = (1,468.748) / 1.0$$

Then: Easting,  $E_{ED50/UTM31} = X_T = X_S - X_{SO} + X_{TO} + dX$   
 $= 200,000 - 155,000 + 663,395.607 + (-1,240.050)$   
 $= 707,155.557$  metres

Northing,  $N_{ED50/UTM31N} = Y_T = Y_S - Y_{SO} + Y_{TO} + dY$   
 $= 500,000 - 463,000 + 5,781,194.380 + 1,468.748$   
 $= 5,819,663.128$  metres

This method is not reversible. For the reverse a different transformation is required, using the same formula.  
 Coordinate transformation: ED50 / UTM zone 31N to Amersfoort / RD New (1) (code 6303):

<u>Coordinate transformation parameter name</u>	<u>Formula symbol</u>	<u>Parameter value</u>	<u>Unit</u>
ordinate 1 of the evaluation point in the source CRS	$X_{S0}$	663,395.607	metre
ordinate 2 of the evaluation point in the source CRS	$Y_{S0}$	5,781,194.380	metre
ordinate 1 of the evaluation point in the target CRS	$X_{T0}$	155,000.000	metre
ordinate 2 of the evaluation point in the target CRS	$Y_{T0}$	463,000.000	metre
scaling factor for source CRS coordinate differences	$m_S$	$10^{-5}$	
scaling factor for target CRS coordinate differences	$m_T$	1.0	
A1	$A_1$	-56.619	coefficient
A2	$A_2$	-3,290.362	coefficient
A3	$A_3$	-20.184	coefficient
A4	$A_4$	+0.861	coefficient
A5	$A_5$	-2.082	coefficient
A6	$A_6$	+0.023	coefficient
A7	$A_7$	-0.070	coefficient
A8	$A_8$	+0.025	coefficient

For input point:

$$\begin{aligned} \text{Easting, } E_{ED50/UTM31} &= X_S &= 707,155.557 \text{ metres} \\ \text{Northing, } N_{ED50/UTM31} &= Y_S &= 5,819,663.128 \text{ metres} \end{aligned}$$

$$\begin{aligned} U &= m_S (X_S - X_{S0}) = (707,155.557 - 663,395.607) 10^{-5} = 0.437599497 \\ V &= m_S (Y_S - Y_{S0}) = (5,819,663.128 - 5,781,194.380) 10^{-5} = 0.384687482 \end{aligned}$$

$$\begin{aligned} dX &= (1240.0499) / 1.0 \\ dY &= (-1,468.7484) / 1.0 \end{aligned}$$

$$\begin{aligned} \text{Then: Easting, } X_{AMERSFOORT/RD} &= X_T = X_S - X_{S0} + X_{T0} + dX \\ &= 707,155.557 - 663,395.607 + 155,000.000 + 1,240.050 \\ &= 200,000.000 \text{ metres} \end{aligned}$$

$$\begin{aligned} \text{Northing, } Y_{AMERSFOORT/RD} &= Y_T = Y_S - Y_{S0} + Y_{T0} + dY \\ &= 5,819,663.128 - 5,781,194.380 + 463,000.000 + (-1,468.748) \\ &= 500,000.000 \text{ metres} \end{aligned}$$

#### 4.6.3 **Polynomial transformation for Spain** (EPSG Dataset coordinate operation method code 9617)

The original geographic coordinate reference system for the Spanish mainland was based on Madrid 1870 datum, Struve 1860 ellipsoid, with longitudes related to the Madrid meridian. Three second-order polynomial expressions have been empirically derived by El Servicio Geográfico del Ejército to transform geographic coordinates based on this system to equivalent values based on the European Datum of 1950 (ED50). The polynomial coefficients derived can be used to transform coordinates from the Madrid 1870 (Madrid) geographic coordinate reference system to the ED50 system. Three pairs of expressions have been derived: each pair is used to calculate the shift in latitude and longitude respectively for (i) a mean for all Spain, (ii) a better fit for the north of Spain, (iii) a better fit for the south of Spain.

The polynomial expressions are:

$$\begin{aligned} d\phi \text{ (arc sec)} &= A_0 + (A_1 * \phi_s) + (A_2 * \lambda_s) + (A_3 * H_s) \\ d\lambda \text{ (arc sec)} &= B_{00} + B_0 + (B_1 * \phi_s) + (B_2 * \lambda_s) + (B_3 * H_s) \end{aligned}$$

where latitude  $\varphi_s$  and longitude  $\lambda_s$  are in decimal degrees referred to the Madrid 1870 (Madrid) geographic coordinate reference system and  $H_s$  is gravity-related height in metres.  $B_{00}$  is the longitude (in seconds) of the Madrid meridian measured from the Greenwich meridian; it is the value to be applied to a longitude relative to the Madrid meridian to transform it to a longitude relative to the Greenwich meridian.

The results of these expressions are applied through the formulas:

$$\varphi_{ED50} = \varphi_{M1870(M)} + d\varphi$$

and  $\lambda_{ED50} = \lambda_{M1870(M)} + d\lambda.$

**Example:**

Input point coordinate reference system: Madrid 1870 (Madrid) (geographic 2D)

$$\begin{aligned}\text{Latitude } \varphi_s &= 42^\circ 38' 52.77'' \text{N} \\ &= +42.647992 \text{ degrees}\end{aligned}$$

$$\begin{aligned}\text{Longitude } \lambda_s &= 3^\circ 39' 34.57'' \text{E of Madrid} \\ &= +3.659603 \text{ degrees from the Madrid meridian.}\end{aligned}$$

$$\text{Gravity-related height } H_s = 0 \text{ m}$$

For the north zone transformation:

$$\begin{aligned}A_0 &= 11.328779 & B_{00} &= -13276.58 \\ A_1 &= -0.1674 & B_0 &= 2.5079425 \\ A_2 &= -0.03852 & B_1 &= 0.08352 \\ A_3 &= 0.0000379 & B_2 &= -0.00864 \\ & & B_3 &= -0.0000038\end{aligned}$$

$$d\varphi = +4.05 \text{ seconds}$$

$$\begin{aligned}\text{Then latitude } \varphi_{ED50} &= 42^\circ 38' 52.77'' \text{N} + 4.05'' \\ &= 42^\circ 38' 56.82'' \text{N}\end{aligned}$$

$$d\lambda = -13270.54 \text{ seconds} = -3^\circ 41' 10.54''$$

$$\begin{aligned}\text{Then longitude } \lambda_{ED50} &= 3^\circ 39' 34.57'' \text{E} - 3^\circ 41' 10.54'' \\ &= 0^\circ 01' 35.97'' \text{W of Greenwich.}\end{aligned}$$

## 4.7 Miscellaneous Linear Coordinate Operations (Affine Transformations)

An affine 2D transformation is used for converting or transforming a coordinate reference system possibly with non-orthogonal axes and possibly different units along the two axes to an isometric coordinate reference system (i.e. a system of which the axes are orthogonal and have equal scale units, for example a projected CRS). The transformation therefore involves a change of origin, differential change of axis orientation and a differential scale change. The EPSG Dataset distinguishes five methods to implement this class of coordinate operation:

- the parametric representation,
- the geometric representation,
- a simplified case of the geometric representation known as the Similarity Transformation in which the degrees of freedom are constrained.
- two variations of the geometric representation for seismic bin grids.

#### 4.7.1 **Affine Parametric Transformation**

(EPSG Dataset coordinate operation method code 9624)

Mathematical and survey literature usually provides the parametric representation of the affine transformation. The parametric algorithm is commonly used for rectification of digitised maps. It is often embedded in CAD software and Geographic Information Systems where it is frequently referred to as “rubber sheeting”.

The formula in matrix form is as follows:

$$\mathbf{V}_T = \mathbf{V}_{To} + \mathbf{R} * \mathbf{V}_S$$

where:

$$\mathbf{V}_T = \begin{pmatrix} X_T \\ Y_T \end{pmatrix} \quad \mathbf{V}_{To} = \begin{pmatrix} A_0 \\ B_0 \end{pmatrix} \quad \mathbf{R} = \begin{pmatrix} A_1 & A_2 \\ B_1 & B_2 \end{pmatrix} \quad \text{and} \quad \mathbf{V}_S = \begin{pmatrix} X_S \\ Y_S \end{pmatrix}$$

or using algebraic coefficients:

$$X_T = A_0 + (A_1 * X_S) + (A_2 * Y_S)$$

$$Y_T = B_0 + (B_1 * X_S) + (B_2 * Y_S)$$

where

$X_T, Y_T$  are the coordinates of a point P in the target coordinate reference system;

$X_S, Y_S$  are the coordinates of P in the source coordinate reference system.

This form of describing an affine transformation is analogous to the general polynomial transformation formulas (section 4.6.1 above). An affine transformation could be considered to be a first order general polynomial transformation but without the reduction to source and target evaluation points.

#### **Reversibility**

The reverse operation is another affine parametric transformation using the same formulas but with different parameter values. The reverse parameter values, indicated by a prime ('), can be calculated from those of the forward operation as follows:

$$D = (A_1 * B_2) - (A_2 * B_1)$$

$$A_0' = [(A_2 * B_0) - (B_2 * A_0)] / D$$

$$B_0' = [(B_1 * A_0) - (A_1 * B_0)] / D$$

$$A_1' = +B_2 / D$$

$$A_2' = -A_2 / D$$

$$B_1' = -B_1 / D$$

$$B_2' = +A_1 / D$$

Then

$$X_S = A_0' + (A_1' * X_T) + (A_2' * Y_T)$$

$$Y_S = B_0' + (B_1' * X_T) + (B_2' * Y_T)$$

Or in matrix form:

$$\mathbf{V}_S = \mathbf{R}^{-1} * (\mathbf{V}_T - \mathbf{V}_{To})$$

**Example:**

Coordinate transformation: Jamaica 1875 / Jamaica (Old Grid) to JAD69 / Jamaica National Grid:

<u>Coordinate transformation parameter</u>	<u>Parameter value</u>	<u>Unit</u>
$A_0$	82357.457	metre
$A_1$	0.304794369	coefficient
$A_2$	0.000015417425	coefficient
$B_0$	28091.324	metre
$B_1$	-0.000015417425	coefficient
$B_2$	0.304794369	coefficient

The units for the Jamaica 1875 / Jamaica (Old Grid) are feet, those for the JAD69 / Jamaica National Grid are metres. The foot-metre conversion factor is embedded within the transformation, together with the scale and orientation differences between the two coordinate reference systems.

For input point:

$$\begin{aligned} \text{Easting, } E_{\text{Jamaica 1875 / Jamaica (Old Grid)}} &= X_S = 553900.00 \text{ feet} \\ \text{Northing, } N_{\text{Jamaica 1875 / Jamaica (Old Grid)}} &= Y_S = 482500.00 \text{ feet} \end{aligned}$$

$$\begin{aligned} \text{Then: Easting, } E_{\text{JAD69 / Jamaica National Grid}} &= X_T = 251190.497 \text{ metres} \\ \text{Northing, } N_{\text{JAD69 / Jamaica National Grid}} &= Y_T = 175146.067 \text{ metres} \end{aligned}$$

For the reverse calculation, first calculate derived parameter values:

<u>Coordinate transformation parameter</u>	<u>Parameter value</u>	<u>Unit</u>
$D$	0.092899608	coefficient
$A_0'$	-270201.960	Clarke's foot
$A_1'$	3.280900499	coefficient
$A_2'$	-0.000165958	coefficient
$B_0'$	-92178.507	Clarke's foot
$B_1'$	0.000165958	coefficient
$B_2'$	3.280900499	coefficient

Then for the same point

$$\begin{aligned} \text{Easting, } E_{\text{JAD69 / Jamaica National Grid}} &= X_T = 251190.497 \text{ metres} \\ \text{Northing, } N_{\text{JAD69 / Jamaica National Grid}} &= Y_T = 175146.067 \text{ metres} \end{aligned}$$

the reverse transformation using the derived parameter values gives

$$\begin{aligned} \text{Easting, } E_{\text{Jamaica 1875 / Jamaica (Old Grid)}} &= X_S = 553900.000 \text{ feet} \\ \text{Northing, } N_{\text{Jamaica 1875 / Jamaica (Old Grid)}} &= Y_S = 482500.000 \text{ feet} \end{aligned}$$



#### 4.7.2 Affine Geometric Transformation

(EPSG Dataset coordinate operation method code 9623)

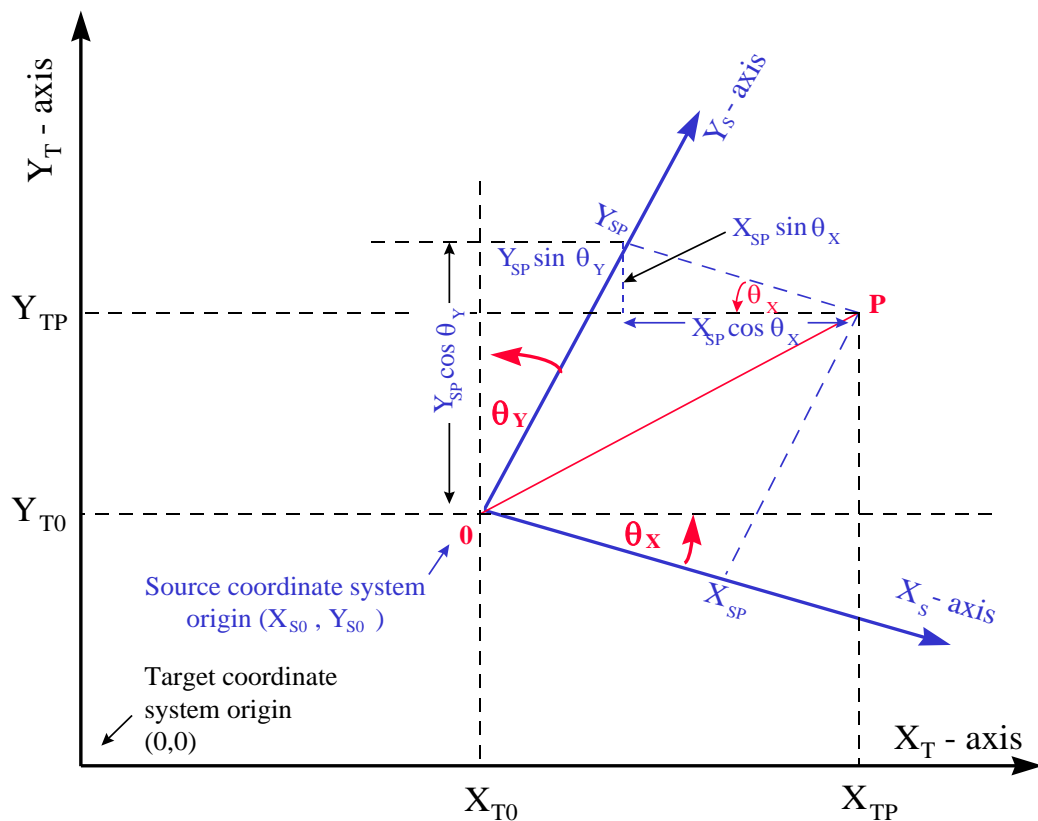


Figure 14 – Geometric representation of the affine coordinate transformation

(Note that the scale parameters of the  $X_s$  and  $Y_s$  axes have been omitted to prevent cluttering of the figure).

From the diagram above it can be seen that:

$$\begin{aligned} X_{TP} &= X_{TO} + Y_{SP} * \sin \theta_Y + X_{SP} * \cos \theta_X = X_{TO} + X_{SP} * \cos \theta_X + Y_{SP} * \sin \theta_Y \\ Y_{TP} &= Y_{TO} + Y_{SP} * \cos \theta_Y - X_{SP} * \sin \theta_X = Y_{TO} - X_{SP} * \sin \theta_X + Y_{SP} * \cos \theta_Y \end{aligned}$$

The scaling of both source and target coordinate reference systems adds some complexity to this formula. The operation will often be applied to transform an engineering coordinate reference system to a projected coordinate reference system. The orthogonal axes of the projected coordinate reference system have identical units. The engineering coordinate reference system may have different units of measure on its two axes: these have scale ratios of  $M_X$  and  $M_Y$  respective to the axes of the projected coordinate reference system.

The projected coordinate reference system is nominally defined to be in well-known units, e.g. metres. However, the distortion characteristics of the map projection only preserve true scale along certain defined lines or curves, hence the projected coordinate reference system's unit of measure is strictly speaking only valid along those lines or curves. Everywhere else its scale is distorted by the map projection. For conformal map projections the distortion at any point can be expressed by the point scale factor 'k' for that point. Note that this point scale factor 'k' should NOT be confused with the scale factor at the natural origin of the projection, denominated by 'k<sub>0</sub>'. (For non-conformal map projections the scale distortion at a point is bearing-dependent and is not described in this document).

It has developed as working practice to choose the origin of the source (engineering) coordinate reference system as the point in which to calculate this point scale factor 'k', although for engineering coordinate reference systems with a large coverage area a point in the middle of the area may be a better choice.

Adding the scaling between each pair of axes and dropping the suffix for point P, after rearranging the terms we have the geometric representation of the affine transformation:

$$\begin{aligned} X_T &= X_{TO} + X_S * k * M_X * \cos \theta_X + Y_S * k * M_Y * \sin \theta_Y \\ Y_T &= Y_{TO} - X_S * k * M_X * \sin \theta_X + Y_S * k * M_Y * \cos \theta_Y \end{aligned}$$

where:

- $X_{TO}, Y_{TO}$  = the coordinates of the origin point of the source coordinate reference system, expressed in the target coordinate reference system;
- $M_X, M_Y$  = multiplication factor applied to coordinates in the source coordinate reference system to obtain the correct scale of the equivalent axis of the target coordinate reference system, for the first and second source and target axes pairs respectively;
- $k$  = point scale factor of the target coordinate reference system at a chosen reference point;
- $\theta_X, \theta_Y$  = the angles about which the source coordinate reference system axes  $X_S$  and  $Y_S$  must be rotated to coincide with the target coordinate reference system axes  $X_T$  and  $Y_T$  respectively (counter-clockwise being positive).

Alternatively, in matrix form:

$$\mathbf{V}_T = \mathbf{V}_{TO} + \mathbf{R}_I * k * \mathbf{S}_I * \mathbf{V}_S$$

where:

$$\mathbf{V}_T = \begin{pmatrix} X_T \\ Y_T \end{pmatrix} \quad \mathbf{V}_{TO} = \begin{pmatrix} X_{TO} \\ Y_{TO} \end{pmatrix} \quad \mathbf{V}_S = \begin{pmatrix} X_S \\ Y_S \end{pmatrix}$$

and

$$\mathbf{R}_I = \begin{pmatrix} \cos \theta_X & \sin \theta_Y \\ -\sin \theta_X & \cos \theta_Y \end{pmatrix} \quad \mathbf{S}_I = \begin{pmatrix} M_X & 0 \\ 0 & M_Y \end{pmatrix}$$

or

$$\begin{pmatrix} X_T \\ Y_T \end{pmatrix} = \begin{pmatrix} X_{TO} \\ Y_{TO} \end{pmatrix} + \begin{pmatrix} \cos \theta_X & \sin \theta_Y \\ -\sin \theta_X & \cos \theta_Y \end{pmatrix} * k * \begin{pmatrix} M_X & 0 \\ 0 & M_Y \end{pmatrix} * \begin{pmatrix} X_S \\ Y_S \end{pmatrix}$$

Comparing the algebraic representation with the parameters of the parametric form in section 4.7.1 above it can be seen that the parametric and geometric forms of the affine coordinate transformation are related as follows:

$$\begin{aligned} A_0 &= X_{TO} \\ A_1 &= k * M_X * \cos \theta_X \\ A_2 &= k * M_Y * \sin \theta_Y \\ B_0 &= Y_{TO} \\ B_1 &= -k * M_X * \sin \theta_X \\ B_2 &= k * M_Y * \cos \theta_Y \end{aligned}$$

### Reversibility

For the Affine Geometric Transformation, the reverse operation can be described by a different formula, as shown below, in which the same parameter values as the forward transformation may be used. In matrix form:

$$\mathbf{V}_S = (1/k) * \mathbf{S}_I^{-1} * \mathbf{R}_1^{-1} * (\mathbf{V}_T - \mathbf{V}_{TO})$$

or

$$\begin{pmatrix} X_S \\ Y_S \end{pmatrix} = \frac{1}{k \cdot Z} * \begin{pmatrix} 1/M_X & 0 \\ 0 & 1/M_Y \end{pmatrix} * \begin{pmatrix} \cos \theta_Y & -\sin \theta_Y \\ \sin \theta_X & \cos \theta_X \end{pmatrix} * \begin{pmatrix} X_T - X_{TO} \\ Y_T - Y_{TO} \end{pmatrix}$$

where  $Z = \cos(\theta_X - \theta_Y)$ ;

Algebraically:

$$X_S = [(X_T - X_{TO}) * \cos \theta_Y - (Y_T - Y_{TO}) * \sin \theta_Y] / [k * M_X * \cos(\theta_X - \theta_Y)]$$

$$Y_S = [(X_T - X_{TO}) * \sin \theta_X + (Y_T - Y_{TO}) * \cos \theta_X] / [k * M_Y * \cos(\theta_X - \theta_Y)]$$

### Orthogonal case

If the source coordinate reference system happens to have orthogonal axes, that is both axes are rotated through the same angle to bring them into the direction of the orthogonal target coordinate reference system axes, i.e.  $\theta_X = \theta_Y = \theta$ , then the Affine Geometric Transformation can be simplified. In matrix form this is:

$$\mathbf{V}_T = \mathbf{V}_{TO} + \mathbf{R}_2 * k * \mathbf{S}_I * \mathbf{V}_S$$

where  $\mathbf{V}_T$ ,  $\mathbf{V}_{TO}$ ,  $\mathbf{S}_I$  and  $\mathbf{V}_S$  are as in the general case but

$$\mathbf{R}_2 = \begin{pmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{pmatrix}$$

Alternatively,

$$\begin{pmatrix} X_T \\ Y_T \end{pmatrix} = \begin{pmatrix} X_{TO} \\ Y_{TO} \end{pmatrix} + \begin{pmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{pmatrix} * k * \begin{pmatrix} M_X & 0 \\ 0 & M_Y \end{pmatrix} * \begin{pmatrix} X_S \\ Y_S \end{pmatrix}$$

Algebraically:

$$X_T = X_{TO} + X_S * k * M_X * \cos \theta + Y_S * k * M_Y * \sin \theta$$

$$Y_T = Y_{TO} - X_S * k * M_X * \sin \theta + Y_S * k * M_Y * \cos \theta$$

where:

$X_{TO}, Y_{TO}$  = the coordinates of the origin point of the source coordinate reference system, expressed in the target coordinate reference system;

$M_X, M_Y$  = multiplication factor applied to coordinates in the source coordinate reference system to obtain the correct scale of the equivalent axis in the target coordinate reference system, for the X axes and the Y axes respectively;

$k$  = the point scale factor of the target coordinate reference system at a chosen reference point;

$\theta$  = the angle through which the source coordinate reference system axes must be rotated to coincide with the target coordinate reference system axes (counter-clockwise is positive). Alternatively, the bearing (clockwise positive) of the source coordinate reference system  $Y_S$ -axis measured relative to target coordinate reference system north.

The reverse formulas of the general case can also be simplified by replacing  $\theta_X$  and  $\theta_Y$  with  $\theta$ . In matrix form:

$$\mathbf{V}_S = (1/k) * \mathbf{S}_I^{-1} * \mathbf{R}_2^{-1} * (\mathbf{V}_T - \mathbf{V}_{T0})$$

or

$$\begin{pmatrix} X_S \\ Y_S \end{pmatrix} = \frac{1}{k} * \begin{pmatrix} 1/M_X & 0 \\ 0 & 1/M_Y \end{pmatrix} * \begin{pmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{pmatrix} * \begin{pmatrix} X_T - X_{T0} \\ Y_T - Y_{T0} \end{pmatrix}$$

Algebraically:

$$X_S = [(X_T - X_{T0}) * \cos \theta - (Y_T - Y_{T0}) * \sin \theta] / [k * M_X]$$

$$Y_S = [(X_T - X_{T0}) * \sin \theta + (Y_T - Y_{T0}) * \cos \theta] / [k * M_Y]$$

In the EPSG Dataset this orthogonal case has been deprecated. The formulas for the general case should be used, inserting  $\theta$  for both  $\theta_X$  and  $\theta_Y$ . The orthogonal case has been documented here to show the progression through increasing constraints on the degrees of freedom between the general case and the Similarity Transformation.

#### 4.7.3 **Similarity Transformation**

(EPSG Dataset coordinate operation method code 9621)

If the source coordinate reference system has orthogonal axes and also has axes of the same scale, that is both axes are scaled by the same factor to bring them into the scale of the target coordinate reference system axes (i.e.  $M_X = M_Y = M$ ), then the orthogonal case of the Affine Geometric Transformation can be simplified further to a Similarity Transformation.

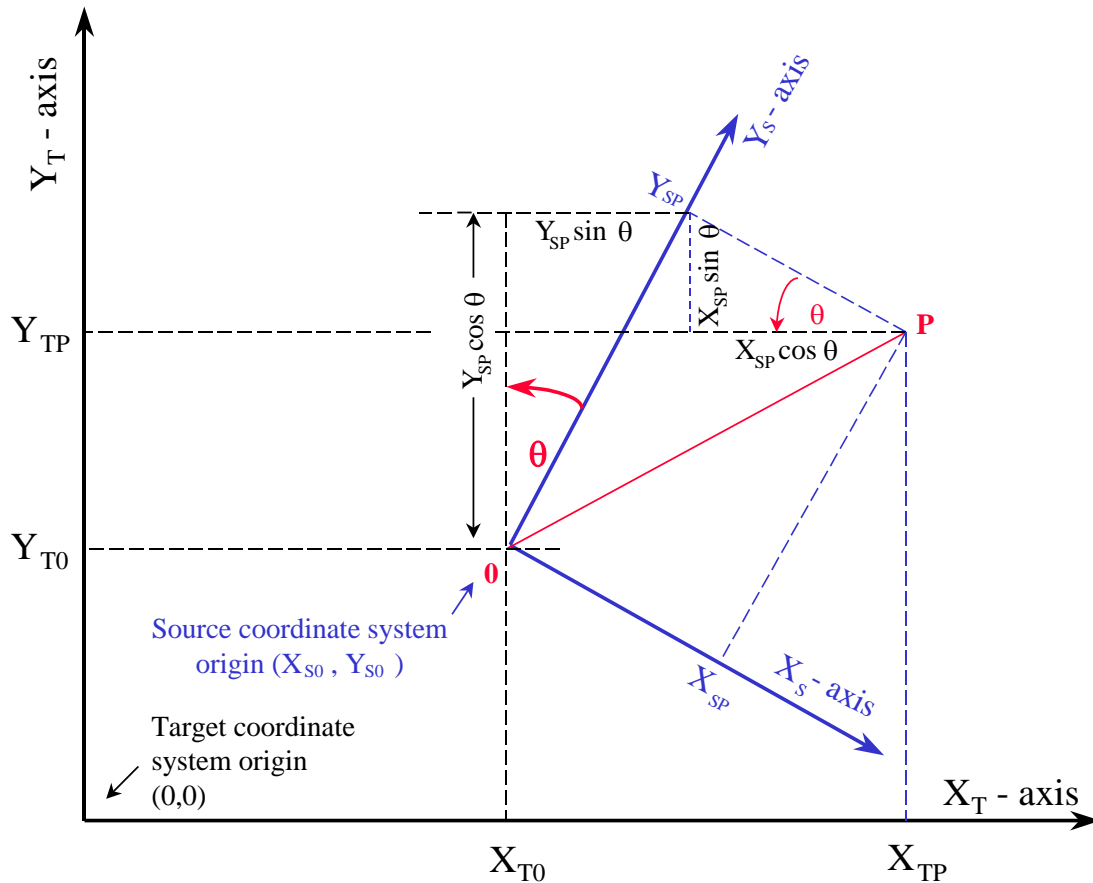


Figure 15 – Similarity Transformation

From the above diagram the Similarity Transformation in algebraic form is:

$$\begin{aligned} X_{TP} &= X_{T0} + Y_{SP} * M * \sin \theta + X_{SP} * M * \cos \theta \\ Y_{TP} &= Y_{T0} + Y_{SP} * M * \cos \theta - X_{SP} * M * \sin \theta \end{aligned}$$

Dropping the suffix for point P and rearranging the terms

$$\begin{aligned} X_T &= X_{T0} + X_S * M * \cos \theta + Y_S * M * \sin \theta \\ Y_T &= Y_{T0} - X_S * M * \sin \theta + Y_S * M * \cos \theta \end{aligned}$$

where:

- $X_{T0}, Y_{T0}$  = the coordinates of the origin point of the source coordinate reference system expressed in the target coordinate reference system;
- $M$  = multiplication factor applied to coordinates in the source coordinate reference system to obtain the correct scale of the target coordinate reference system;
- $\theta$  = the angle about which the axes of the source coordinate reference system need to be rotated to coincide with the axes of the target coordinate reference system, counter-clockwise being positive. Alternatively, the bearing of the source coordinate reference system  $Y_S$ -axis measured relative to target coordinate reference system north.

The Similarity Transformation can also be described as a special case of the Affine Parametric Transformation where coefficients  $A_1 = B_2$  and  $A_2 = -B_1$ .

In matrix form:

$$V_T = V_{T0} + M * R_2 * V_S$$

where  $V_T$ ,  $V_{TO}$ ,  $R_2$  and  $V_S$  are as in the Affine Orthogonal Geometric Transformation method, or

$$\begin{pmatrix} X_T \\ Y_T \end{pmatrix} = \begin{pmatrix} X_{TO} \\ Y_{TO} \end{pmatrix} + M * \begin{pmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{pmatrix} * \begin{pmatrix} X_S \\ Y_S \end{pmatrix}$$

### Reversibility

The reverse formula for the Similarity Transformation, in matrix form, is:

$$V_S = (1/M) * R_2^{-1} * (V_T - V_{TO})$$

or

$$\begin{pmatrix} X_S \\ Y_S \end{pmatrix} = \frac{1}{M} * \begin{pmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{pmatrix} * \begin{pmatrix} X_T - X_{TO} \\ Y_T - Y_{TO} \end{pmatrix}$$

Algebraically:

$$X_S = [(X_T - X_{TO}) * \cos \theta - (Y_T - Y_{TO}) * \sin \theta] / M$$

$$Y_S = [(X_T - X_{TO}) * \sin \theta + (Y_T - Y_{TO}) * \cos \theta] / M$$

An alternative approach for the reverse operation is to use the same formula as for the forward computation but with different parameter values:

$$X_T = X_{TO}' + X_S * M' * \cos \theta' + Y_S * M' * \sin \theta'$$

$$Y_T = Y_{TO}' - X_S * M' * \sin \theta' + Y_S * M' * \cos \theta'$$

The reverse parameter values, indicated by a prime ('), can be calculated from those of the forward operation as follows:

$$X_{TO}' = (Y_{TO} \sin \theta - X_{TO} \cos \theta) / M$$

$$Y_{TO}' = -(Y_{TO} \cos \theta + X_{TO} \sin \theta) / M$$

$$M' = 1/M$$

$$\theta' = -\theta$$

### **Example**

ED50 / UTM zone 31N to ETRS89 / UTM zone 31N

Parameters of the Similarity Transformation:

$$\text{Ordinate 1 of evaluation point in target CRS: } X_{TO} = -129.549 \text{ metres}$$

$$\text{Ordinate 2 of evaluation point in target CRS: } Y_{TO} = -208.185 \text{ metres}$$

$$\text{Scale factor for source CRS axes: } M = 1.00000155$$

$$\text{Rotation angle of source CRS axes: } \theta = 1.56504'' = 0.000007588 \text{ rad}$$

Forward computation of ED50 / UTM zone 31N (source) coordinates 300000m E, 4500000m N:

$$\begin{aligned} E_{ETRS89} &= -129.549 + 300000.465 + 34.144 \\ &= 299905.060 \text{ m E} \end{aligned}$$

$$\begin{aligned} N_{ETRS89} &= -208.185 - 2.276 + 4500006.977 \\ &= 4499796.515 \text{ m N} \end{aligned}$$

Reverse computation of ETRS89 / UTM zone 31N coordinates 299905.060m E, 4499796.515m N:

$$\begin{aligned}E_{ED50} &= (300034.609 - 34.144) / 1.00000155 \\ &= 300000.000\text{m E}\end{aligned}$$

$$\begin{aligned}N_{ED50} &= (2.276 + 4500004.700) / 1.00000155 \\ &= 4500000.000\text{m N}\end{aligned}$$

Alternative reverse computation:

First calculate new parameter values:

$$\begin{aligned}X_{TO}' &= 129.5472 \text{ m} \\ Y_{TO}' &= 208.1857 \text{ m} \\ M' &= 0.99999845 \\ \theta' &= -0.000007588 \text{ rad}\end{aligned}$$

Then apply these values to forward formula:

$$\begin{aligned}E_{ED50} &= 129.547 + 299904.595 + (-34.142) = 300000.000 \text{ m} \\ N_{ED50} &= 208.186 - (-2.276) + 4499789.539 = 4500000.000 \text{ m}\end{aligned}$$

#### When to use the Similarity Transformation

Similarity Transformations can be used when source and target coordinate reference systems

- each have orthogonal axes,
  - each have the same scale along both axes,
- and
- both have the same units of measure,

for example between engineering plant grids and projected coordinate reference systems.

Coordinate Operations between two coordinate reference systems where in either system either the scale along the axes differ or the axes are not orthogonal should be defined as an Affine Transformation in either the parametric or geometric form. But for seismic bin grids see the following section.

## **4.8 Seismic Bin Grid Transformations**

The P6/98 seismic bin grid exchange format described a special case of the Affine Geometric Transformation. The method is also described in the SEG-Y revision 1 seismic data exchange format.

In practice bin grid definitions are based on an identified map grid. As such the example bin grids in the EPSG Dataset, coordinate transformation codes 6918 and 6919, are described with the map grid (projected CRS) as source CRS. However to retain consistency with the syntax used in the description of the affine general transformations in the previous sections of this document, in the formulas below the bin grid is the affine source CRS and the map grid is the affine target CRS.

For the P6 seismic bin grid case of the affine geometric transformation

- the base coordinate reference system is a map grid (projected CRS) which by definition has orthogonal axes with the same units;
- the bin grid axes are orthogonal;

and one or both of the following may apply:

- the origin of the bin grid may be assigned non-zero bin grid coordinates;
- the bin grid units may increase in increments other than 1, i.e.  $\text{Inc}_{\text{SX}}$  and  $\text{Inc}_{\text{SY}}$  may not be unity.

#### 4.8.1 **P6 Right-handed Seismic Bin Grid Transformation**

(EPSG Dataset coordinate operation method code 9666)

The P6/98 exchange format defined the bin grid axes to be named I and J, where the I-axis is rotated 90° clockwise from the J-axis (when viewed from above the plane containing the two axes). This is sometimes described as 'right-handed'.

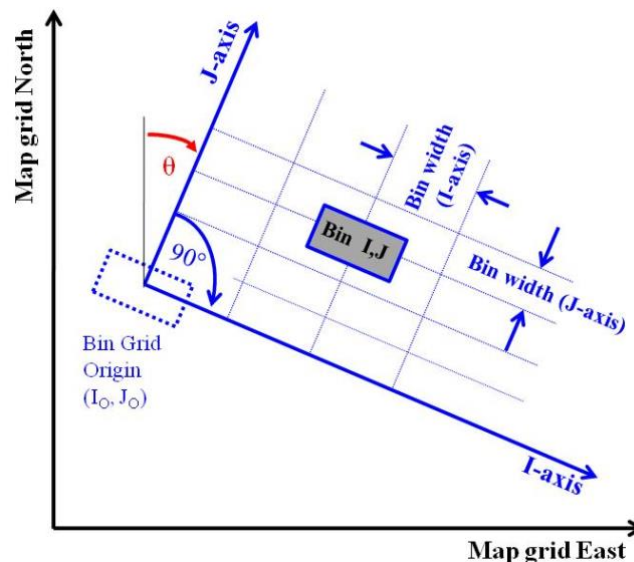


Figure 16 – Relationship of right-handed bin grid to map grid

The defining parameters are:

##### **P6/98 term**

Bin grid origin ( $I_0$ )  
 Bin grid origin ( $J_0$ )  
 Map grid easting of bin grid origin ( $E_0$ )  
 Map grid northing of bin grid origin ( $N_0$ )  
 Scale factor of bin grid ( $SF$ )  
 Nominal bin width along I axis ( $I\_bin\_width$ )  
 Nominal bin width along J axis ( $J\_bin\_width$ )  
 Grid bearing of bin grid J axis ( $\theta$ )  
 Bin node increment on I axis ( $I\_bin\_inc$ )  
 Bin node increment on J axis ( $J\_bin\_inc$ )

##### **Equivalent EPSG Dataset term**

Ordinate 1 of evaluation point in source CRS ( $X_{SO}$ )  
 Ordinate 2 of evaluation point in source CRS ( $Y_{SO}$ )  
 Ordinate 1 of evaluation point in target CRS ( $X_{TO}$ )  
 Ordinate 2 of evaluation point in target CRS ( $Y_{TO}$ )  
 Point scale factor ( $k$ )  
 Scale factor for source CRS first axis ( $M_X$ )  
 Scale factor for source CRS second axis ( $M_Y$ )  
 Rotation angle of source CRS axes ( $\theta$ )  
 Bin node increment on I-axis  
 Bin node increment on J-axis

In this method the terms  $X_s$ ,  $Y_s$ ,  $M_X$  and  $M_Y$  in the orthogonal case of the Affine Geometric Transformation formulas are replaced by  $(X_s - X_{SO})$ ,  $(Y_s - Y_{SO})$ ,  $(M_X / \text{Inc}_{\text{SX}})$  and  $(M_Y / \text{Inc}_{\text{SY}})$  respectively. Thus the



transformation from bin grid to map grid (source to target coordinate reference system in Affine Geometric Transformation formula nomenclature) is:

$$\mathbf{V}_T = \mathbf{V}_{To} + \mathbf{R}_2 * k * \mathbf{S}_2 * \mathbf{V}_2$$

where, as in the orthogonal case of the Affine Geometric Transformation method:

$$\mathbf{V}_T = \begin{pmatrix} X_T \\ Y_T \end{pmatrix} \quad \mathbf{V}_{To} = \begin{pmatrix} X_{To} \\ Y_{To} \end{pmatrix} \quad \text{and} \quad \mathbf{R}_2 = \begin{pmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{pmatrix}$$

but where

$$\mathbf{S}_2 = \begin{pmatrix} M_X / \text{Inc}_{SX} & 0 \\ 0 & M_Y / \text{Inc}_{SY} \end{pmatrix} \quad \text{and} \quad \mathbf{V}_2 = \begin{pmatrix} X_S - X_{So} \\ Y_S - Y_{So} \end{pmatrix}$$

That is,

$$\begin{pmatrix} X_T \\ Y_T \end{pmatrix} = \begin{pmatrix} X_{To} \\ Y_{To} \end{pmatrix} + \begin{pmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{pmatrix} * k * \begin{pmatrix} M_X / \text{Inc}_{SX} & 0 \\ 0 & M_Y / \text{Inc}_{SY} \end{pmatrix} * \begin{pmatrix} X_S - X_{So} \\ Y_S - Y_{So} \end{pmatrix}$$

Algebraically:

$$X_T = X_{To} + [(X_S - X_{So}) * \cos \theta * k * M_X / \text{Inc}_{SX}] + [(Y_S - Y_{So}) * \sin \theta * k * M_Y / \text{Inc}_{SY}]$$

$$Y_T = Y_{To} - [(X_S - X_{So}) * \sin \theta * k * M_X / \text{Inc}_{SX}] + [(Y_S - Y_{So}) * \cos \theta * k * M_Y / \text{Inc}_{SY}]$$

Using the symbol notation in the P6/98 document these expressions are:

$$\begin{pmatrix} E \\ N \end{pmatrix} = \begin{pmatrix} E_o \\ N_o \end{pmatrix} + \begin{pmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{pmatrix} * SF * \begin{pmatrix} I\_bin\_width / I\_bin\_inc & 0 \\ 0 & J\_bin\_width / J\_bin\_inc \end{pmatrix} * \begin{pmatrix} I - I_o \\ J - J_o \end{pmatrix}$$

or

$$E = E_o + [(I - I_o) * \cos \theta * SF * I\_bin\_width / I\_bin\_inc] + [(J - J_o) * \sin \theta * SF * J\_bin\_width / J\_bin\_inc]$$

$$N = N_o - [(I - I_o) * \sin \theta * SF * I\_bin\_width / I\_bin\_inc] + [(J - J_o) * \cos \theta * SF * J\_bin\_width / J\_bin\_inc]$$

For the reverse transformation (map grid to bin grid):

$$\mathbf{V}_S = (1/k) * \mathbf{S}_2^{-1} * \mathbf{R}_2^{-1} * (\mathbf{V}_T - \mathbf{V}_{To}) + \mathbf{V}_{So}$$

or

$$\begin{pmatrix} X_S \\ Y_S \end{pmatrix} = 1/k * \begin{pmatrix} \text{Inc}_{SX} / M_X & 0 \\ 0 & \text{Inc}_{SY} / M_Y \end{pmatrix} * \begin{pmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{pmatrix} * \begin{pmatrix} X_T - X_{To} \\ Y_T - Y_{To} \end{pmatrix} + \begin{pmatrix} X_{So} \\ Y_{So} \end{pmatrix}$$

or algebraically:

$$X_S = \{[(X_T - X_{TO}) * \cos \theta - (Y_T - Y_{TO}) * \sin \theta] * [\text{Inc}_{SX} / (k * M_X)]\} + X_{SO}$$

$$Y_S = \{[(X_T - X_{TO}) * \sin \theta + (Y_T - Y_{TO}) * \cos \theta] * [\text{Inc}_{SY} / (k * M_Y)]\} + Y_{SO}$$

Using the symbol notation in the P6/98 document these reverse expressions are:

$$\begin{pmatrix} I \\ J \end{pmatrix} = 1/\text{SF} * \begin{pmatrix} \text{I\_bin\_inc} / \text{I\_bin\_width} & 0 \\ 0 & \text{J\_bin\_inc} / \text{I\_bin\_width} \end{pmatrix} * \begin{pmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{pmatrix} * \begin{pmatrix} E - E_o \\ N - N_o \end{pmatrix} + \begin{pmatrix} I_o \\ J_o \end{pmatrix}$$

or

$$I = I_o + \{[(E - E_o) * \cos \theta - (N - N_o) * \sin \theta] * [\text{I\_bin\_inc} / (\text{SF} * \text{I\_bin\_width})]\}$$

$$J = J_o + \{[(E - E_o) * \sin \theta + (N - N_o) * \cos \theta] * [\text{J\_bin\_inc} / (\text{SF} * \text{J\_bin\_width})]\}$$

### Example:

This example is given in the P6/98 document. The bin grid is based on projected CRS WGS 84 / UTM Zone 31N. The origin of the bin grid is defined to be at E = 456781.0, N = 5836723.0. The point scale factor at this point is 0.99984.

The bin width on the I-axis ( $X_S$  axis) is 25 metres, whilst the bin width on the J-axis ( $Y_S$  axis) is 12.5 metres. The origin of the grid has bin values of 1,1.

In the map grid, the bearing of the bin grid I and J axes are 110° and 20° respectively. Thus the angle through which the bin grid axes need to be rotated to coincide with the map grid axes is +20 degrees.

The transformation parameter values are:

<u>Parameter</u>	<u>EPSG Affine Transformation symbol</u>	<u>P6 symbol</u>	<u>Parameter value</u>
Bin grid origin I	$X_{SO}$	$I_o$	1
Bin grid origin J	$Y_{SO}$	$J_o$	1
Bin grid origin Easting	$X_{TO}$	$E_o$	456781.00 m
Bin grid origin Northing	$Y_{TO}$	$N_o$	5836723.00 m
Scale factor of bin grid	$k$	$\text{SF}$	0.99984
Bin Width on I-axis	$M_X$	$\text{I\_bin\_width}$	25 m
Bin Width on J-axis	$M_Y$	$\text{J\_bin\_width}$	12.5 m
Map grid bearing of bin grid J-axis	$\theta$	$\theta$	20 deg
Bin node increment on I-axis	$\text{Inc}_{SX}$	$\text{I\_bin\_inc}$	1
Bin node increment on J-axis	$\text{Inc}_{SY}$	$\text{J\_bin\_inc}$	1

Calculation of map grid coordinates for centre of bin with coordinates: I = 300, J = 247:

$$\begin{aligned} \text{Easting} = X_T &= X_{TO} + [(X_S - X_{SO}) * \cos \theta * k * M_X / \text{Inc}_{SX}] + [(Y_S - Y_{SO}) * \sin \theta * k * M_Y / \text{Inc}_{SY}] \\ &= 456781.000 + 7023.078 + 1051.544 \\ &= 464855.62 \text{ m.} \end{aligned}$$

$$\begin{aligned} \text{Northing} = Y_T &= Y_{TO} - [(X_S - X_{SO}) * \sin \theta * k * M_X / \text{Inc}_{SX}] + [(Y_S - Y_{SO}) * \cos \theta * k * M_Y / \text{Inc}_{SY}] \\ &= 5836723.000 - 2556.192 + 2889.092 \\ &= 5837055.90 \text{ m.} \end{aligned}$$

Calculation of bin grid coordinates for this map grid location 464 855.62mE, 5 837 055.90mN:

$$\text{Bin grid I} = X_S = X_{SO} + \{[(X_T - X_{TO}) * \cos \theta - (Y_T - Y_{TO}) * \sin \theta] * [\text{Inc}_{SX} / (k * M_X)]\}$$

= 300 bins,

$$\text{Bin grid J} = Y_S = Y_{SO} + \{[(X_T - X_{TO}) * \sin \theta + (Y_T - Y_{TO}) * \cos \theta] * [\text{Inc}_{SY} / (k * M_Y)]\}$$

= 247 bins

Further examples of input and output may be found in test procedure 5209 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

#### 4.8.2 **P6 Left-handed Seismic Bin Grid Transformation** (EPSG Dataset coordinate operation method code 1049)

The P6/98 exchange format constrained the bin grid axes to be 'right-handed', i.e. by definition the bin grid I-axis orientation is 90° clockwise from the J-axis *when viewed from above the plane of the coordinate system*. The P6/11 format extends the P6/98 format to allow for a 'left-handed' bin grid in which the I-axis orientation is 90° counter-clockwise from the J-axis when viewed from above the plane of the coordinate system. (Using the P6/98 format this can be accomplished through giving the *Bin node increment on I-axis* parameter a negative value).

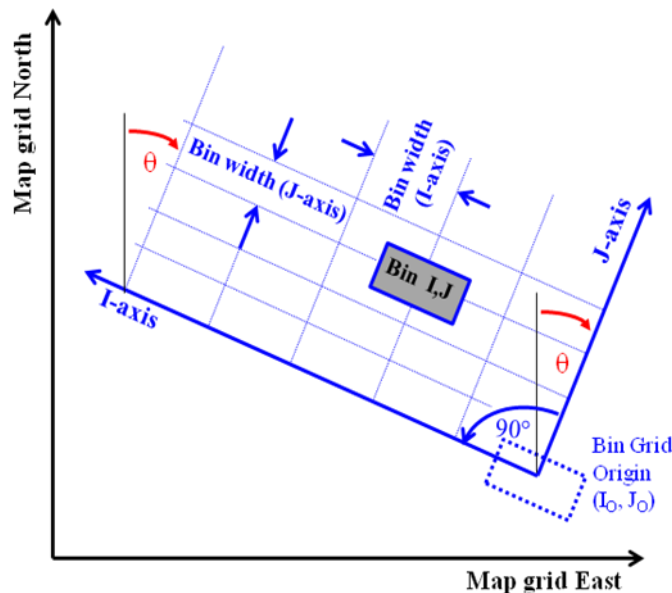


Figure 17 – Relationship of left-handed bin grid to map grid

The transformation from bin grid to map grid is:

$$\mathbf{V}_T = \mathbf{V}_{TO} + \mathbf{R}_2 * k * \mathbf{S}_3 * \mathbf{V}_2$$

where

$$\mathbf{S}_3 = \begin{pmatrix} M_X / \text{Inc}_{SX} * -1 & 0 \\ 0 & M_Y / \text{Inc}_{SY} \end{pmatrix}$$

and other matrices are as defined earlier,

or

$$\begin{pmatrix} E \\ N \end{pmatrix} = \begin{pmatrix} E_o \\ N_o \end{pmatrix} + \begin{pmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{pmatrix} * SF * \begin{pmatrix} I_{bin\_width} / -I_{bin\_inc} & 0 \\ 0 & J_{bin\_width} / J_{bin\_inc} \end{pmatrix} * \begin{pmatrix} I - I_o \\ J - J_o \end{pmatrix}$$

or

$$E = E_o - [(I - I_o) * \cos \theta * SF * I_{bin\_width} / I_{bin\_inc}] + [(J - J_o) * \sin \theta * SF * J_{bin\_width} / J_{bin\_inc}]$$

$$N = N_o + [(I - I_o) * \sin \theta * SF * I_{bin\_width} / I_{bin\_inc}] + [(J - J_o) * \cos \theta * SF * J_{bin\_width} / J_{bin\_inc}]$$

For the transformation from map grid to bin grid:

$$V_S = (1/k) * S_3^{-1} * R_2^{-1} * (V_T - V_{To}) + V_{So}$$

or

$$\begin{pmatrix} I \\ J \end{pmatrix} = 1/SF * \begin{pmatrix} -I_{bin\_inc} / I_{bin\_width} & 0 \\ 0 & J_{bin\_inc} / I_{bin\_width} \end{pmatrix} * \begin{pmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{pmatrix} * \begin{pmatrix} E - E_o \\ N - N_o \end{pmatrix} + \begin{pmatrix} I_o \\ J_o \end{pmatrix}$$

or

$$I = I_o - \{[(E - E_o) * \cos \theta - (N - N_o) * \sin \theta] * [I_{bin\_inc} / (SF * I_{bin\_width})]\}$$

$$J = J_o + \{[(E - E_o) * \sin \theta + (N - N_o) * \cos \theta] * [J_{bin\_inc} / (SF * J_{bin\_width})]\}$$

### Example:

This example is given in the OGP P6/11 user guide. The base map grid is a projected CRS NAD27 / BLM 16N (ftUS) upon which the origin of the bin grid is defined at E = 871200 ftUS, N = 10280160 ftUS. As the survey was acquired on the map grid, the bin grid scale factor at the bin grid origin is chosen to be 1.

The bin width on the I-axis ( $X_s$  axis) is 82.5 US survey feet, the bin width on the J-axis ( $Y_s$  axis) is 41.25 US survey feet. The origin of the bin grid has bin values of I=5000, J=0.

In the map grid, the bearing of the bin grid I and J axes are 250° and 340° respectively. Thus the angle through which the bin grid axes need to be rotated to coincide with the map grid axes is +340 degrees.

The transformation parameter values are:

<u>Parameter</u>	<u>EPSG Affine Transformation symbol</u>	<u>P6 symbol</u>	<u>Parameter value</u>
Bin grid origin I	$X_{SO}$	Io	5000
Bin grid origin J	$Y_{SO}$	Jo	0
Bin grid origin Easting	$X_{TO}$	Eo	871200 ftUS
Bin grid origin Northing	$Y_{TO}$	No	10280160 ftUS
Scale factor of bin grid	k	SF	1
Bin Width on I-axis	$M_X$	I_bin_width	82.5 ftUS
Bin Width on J-axis	$M_Y$	J_bin_width	41.25 ftUS
Map grid bearing of bin grid J-axis	$\theta$	$\theta$	340 deg
Bin node increment on I-axis	$Inc_{SX}$	I_bin_inc	1
Bin node increment on J-axis	$Inc_{SY}$	J_bin_inc	1

Calculation for centre of bin with coordinates: I = 4700, J = 247:

$$\text{Easting} = X_T = X_{TO} - [(X_S - X_{SO}) * \cos \theta * k * M_X / Inc_{SX}] + [(Y_S - Y_{SO}) * \sin \theta * k * M_Y / Inc_{SY}]$$

$$= 890\,972.63 \text{ ftUS.}$$

$$\text{Northing} = Y_T = Y_{TO} + [(X_S - X_{SO}) * \sin \theta * k * M_X / Inc_{SX}] + [(Y_S - Y_{SO}) * \cos \theta * k * M_Y / Inc_{SY}]$$

$$= 10\,298\,199.29 \text{ ftUS.}$$

Reverse calculation for this point 890 972.63 ftUS E, 10 298 199.29 ftUS N:

$$\text{Bin grid I} = X_S = X_{SO} - \{[(X_T - X_{TO}) * \cos \theta - (Y_T - Y_{TO}) * \sin \theta] * [Inc_{SX} / (k * M_X)]\}$$

$$= 4700 \text{ bins,}$$

$$\text{Bin grid J} = Y_S = Y_{SO} + \{[(X_T - X_{TO}) * \sin \theta + (Y_T - Y_{TO}) * \cos \theta] * [Inc_{SY} / (k * M_Y)]\}$$

$$= 247 \text{ bins}$$

## 4.9 Wellbore Local to Geodetic transformations

The P7/17 Wellbore Positional Data Exchange format describes methods to transform 3D ('cubic') coordinates of a wellbore to coordinates in geodetic space. Although considered 3D the wellbore local coordinates (n,e,d) may (and usually will) have separate horizontal and vertical origin points, the well WRP (well reference point) and ZDP (zero depth point) respectively. In the EPSG data model these local coordinates are referenced to a compound CRS consisting of a local horizontal CRS and a local vertical CRS. The local coordinates themselves are the output of a computation such as minimum curvature using the borehole survey observables measured depth, inclination and azimuth (MD, INC, AZ).

Local coordinates are unscaled and have the same unit of measure (metre or foot) on all three axes as MD. The local horizontal coordinates may be transformed to coordinates in either a geographic 2D or a projected CRS, and the local vertical coordinate may be transformed to coordinates in a geodetic vertical CRS. For more information on wellbore positioning the reader is referred to the IOGP P7 format documentation.

### 4.9.1 Local depth

The local depth (*d*) is given in a 1D vertical CRS with origin at the well Zero-Depth Point (ZDP) and with orientation “down”.

Local depth is transformed to depth in the target CRS by applying the vertical offset method (coordinate operation method code 9616):

$$D[i] = d[i] + D_{ZDP}$$

where:

$D_{ZDP}$  is the depth of the ZDP point below the vertical reference surface (VRS) of the target vertical CRS, negative if the ZDP is above the VRS);

$d[i]$  is the local depth at survey station  $i$ ;

$D[i]$  is the Depth below the VRS at survey station  $i$  (positive below VRS).

If the ZDP is referenced to a height vertical CRS, which often the case, its height must first be converted to a depth using a Height Depth Reversal (EPSG coordinate operation method code 1068, §2.4.2.4). Combining this with the vertical offset above:

$$D[i] = d[i] - H_{ZDP}$$

Before application of these vertical offset formulas care must be taken that the local depths are converted to the same unit as used by the target CRS, if necessary through using a Change of Vertical Unit (EPSG method code 1069, §2.4.2.5).

#### 4.9.2 **Local horizontal coordinates**

The local horizontal coordinates ( $n, e$ ) are given in a 2D Cartesian CRS with its origin at the Well Reference Point (WRP) at known location. An example of such local CRS is EPSG code 8377.

The local north axis aligns either with true north or with grid north, according to local practice. To transform local coordinates to 'global' coordinates the LMP method is used when the local north axis is aligned to true north and the GNL method when the local north axis is aligned to grid north. An advantage of the LMP method is that it eliminates the need to store the projected CRS grid convergence, which is a common source of error and confusion when managing wellbore survey data. The LMP method accounts for varying local grid scale factor and depth correction factor along the wellbore.

#### 4.9.3 **Wellbore local to geographic (LMP)**

(EPSG Dataset coordinate operation method code 1076)

The LMP method is clearly described by N.D. Zinn in SPE paper 96813 "*Accounting for Earth Curvature in Directional Drilling*", 2005. This paper includes formulas and pseudo-code that are not reproduced here. LMP requires that the input local horizontal coordinates are referenced to true north, and that depths (here referred to as  $D$ , but as  $TVD$  in the SPE paper) are referenced to the reference surface of the vertical CRS.

The output of LMP is ellipsoidal coordinates in a geographic 2D CRS. Projected coordinates of the wellbore may be calculated from the geographic coordinates using the relevant map projection formulas provided in this Guidance Note.

Required inputs are:

Latitude of the Well Reference Point in the target 2D geographic CRS;

Longitude of the Well Reference Point in the target 2D geographic CRS;

$dcf\_flag$  : 0 for none; 2 to apply a variable depth correction factor along the path;

$n[]$  : Array of length  $num\_points$  containing local northing coordinates, oriented to true north;

$e[]$  : Array of length  $num\_points$  containing local easting coordinates, oriented to true east;

$D[]$  : Array of length  $num\_points$  containing depths below the Vertical Reference Surface at survey stations down the wellbore (referred to as  $TVD$  in the SPE paper).

In this document (but not in the SPE paper) the method has an option for the application of the depth correction, to accomodate subsurface applications that do not correct for earth curvature:

**depth correction factor** (*dcf*) is used to scale horizontal offsets according to the depth of the point in the wellbore: (i) no dcf correction is applied, or (ii) a dcf correction is applied at each point along the wellbore.

For option (ii), depths  $D[i]$  are required as input but are not changed by the algorithm. The depths are used to compute a depth correction and therefore a depth of zero must be close to Mean Sea Level (strictly speaking to the reference ellipsoid, but that difference is insignificant and ignored for the computed depth correction). For option (i),  $D[i]$  is not required.

The following equations assume that the local coordinates ( $n, e$ ) and depth  $D$  are in metres. If these use a different unit of measure, then as a first step they should be converted to metres.

When  $dcf\_flag = 2$ , output arrays  $LAT, LON$  (in radians) are computed as:

```
for i=1:num_points
    ρ = a (1 - e2) / (1 - e2 sin2 (LAT[i-1]))3/2
    v = a / (1 - e2 sin2 (LAT[i-1]))1/2
    LAT[i] = LAT[i-1] + (n[i] - n[i-1]) / (ρ - D[i])
    LON[i] = LON[i-1] + (e[i] - e[i-1]) / (v - D[i]) / cos(LAT[i-1])
```

where for  $i=1$ , for the first station:  $LAT[0]$  and  $LON[0]$  are the geographic coordinates of the WRP in radians;  $n[0] = 0$ ,  $e[0] = 0$ ;

and (per the Implementation Notes in this document):

- $a$  is the semi-major axis of the reference ellipsoid;
- $e$  is the eccentricity of the reference ellipsoid (not to be confused with local easting);
- $\rho$  is the radius of curvature in the meridian;
- $v$  is the radius of curvature in the prime vertical.

Subtraction of  $D[i]$  from the radii of curvature in the above equations effectively accomplishes a depth correction as described in the SPE paper and may also be used in the GNL method. This correction should not be applied if the wellbore path is to be loaded into a subsurface application which is using a cubic model that does not apply a similar correction for its (seismic) data. When  $dcf\_flag = 0$ , the equations above are modified to:

$$\begin{aligned} LAT[i] &= LAT[i-1] + (n[i] - n[i-1]) / \rho \\ LON[i] &= LON[i-1] + (e[i] - e[i-1]) / \{v \cos(LAT[i-1])\} \end{aligned}$$

**Reverse case:** to compute ( $n, e$ ) from given ( $LAT, LON, D$ ) arrays:

```
For i=1:num_points
    ρ = a (1 - e2) / (1 - e2 sin2 (LAT[i-1]))3/2
    v = a / (1 - e2 sin2 (LAT[i-1]))1/2
    n[i] = n[i-1] + (LAT[i] - LAT[i-1]) * (ρ - D[i])
    e[i] = e[i-1] + (LON[i] - LON[i-1]) * (v - D[i]) * cos(LAT[i-1])
```

where for the first station  $i=0$  variables are initialised as in the forward case.

Note that  $LAT[i-1]$ , the latitude of the previous point, is used in the reverse formulas to compute the radii of curvature in order to be internally consistent in round trip calculations.

When  $dcf\_flag = 0$ , the equations above are modified to:

$$\begin{aligned} n[i] &= n[i-1] + (LAT[i] - LAT[i-1]) * \rho \\ e[i] &= e[i-1] + (LON[i] - LON[i-1]) * v * \cos(LAT[i-1]) \end{aligned}$$

### Example:

An example computation for the LMP method is provided below. This is not a realistic wellbore survey but demonstrates the computation steps. A straight path is modelled to a bottom hole at 5 km depth, approximately 14 km away from the surface location, using a constant inclination 70.54002°, azimuth 51.3178° from true north, and survey stations every 30.0167 metre.

#### Parameters:

Ellipsoid: WGS 84       $a = 6378137$  metres       $1/f = 298.257223563$   
then  $e = 0.0818191908$        $e^2 = 0.00669437999$

Forward calculation for a well with surface coordinates (origin of the local coordinates)

Latitude of Well Reference Point       $9.0364081^\circ = 0.1577150739$  rad  
Longitude of Well Reference Point       $-30.2713073^\circ = -0.5283339813$  rad  
Depth Correction       $dcf\_flag = 2$   
Zero Depth Point height       $ZDP\_height = 25$  m (not used in equations)

Local northing (n) and easting (e) given in the table are the unscaled output of an algorithm such as minimum curvature as commonly applied in wellbore survey (described as cubical coordinates in the SPE paper). Axes n and e are aligned with true north and true east, respectively.

Initialisation: LAT[0] = 0.1577150739 rad; LON[0] = -0.5283339813 rad.							
i	D (m)	n (m)	e (m)	p (m)	v (m)	LAT (rad)	LON (rad)
1	0.00	0.00	0.00	6,337,009.005	6,378,663.709	0.1577150739	-0.5283339813
2	10.00	15.00	24.00	6,337,009.005	6,378,663.709	0.1577174409	-0.5283301714
3	20.00	30.00	48.00	6,337,009.052	6,378,663.724	0.1577198080	-0.5283263616
4	30.00	45.00	72.00	6,337,009.099	6,378,663.740	0.1577221751	-0.5283225517
5	40.00	60.00	96.00	6,337,009.145	6,378,663.756	0.1577245421	-0.5283187419
...							
497	4960.00	7440.00	11904.00	6,337,032.231	6,378,671.502	0.1588895878	-0.5264433944
498	4970.00	7455.00	11928.00	6,337,032.278	6,378,671.517	0.1588919567	-0.5264395808
499	4980.00	7470.00	11952.00	6,337,032.325	6,378,671.533	0.1588943256	-0.5264357673
500	4990.00	7485.00	11976.00	6,337,032.373	6,378,671.549	0.1588966945	-0.5264319538
501	5000.00	7500.00	12000.00	6,337,032.420	6,378,671.565	0.1588990634	-0.5264281403

For convenience of interpretation the results are converted from radians to degrees, and to WGS 84 / UTM zone 25N, EPSG CRS code 32625:

i	Latitude	Longitude	Easting	Northing
1	9.0364081	-30.2713073	800000.00	1000000.00
2	9.0365437	-30.2710890	800023.91	1000015.19
3	9.0366793	-30.2708707	800047.81	1000030.38
4	9.0368150	-30.2706524	800071.71	1000045.57
5	9.0369506	-30.2704341	800095.62	1000060.76
...				
497	9.1037028	-30.1629846	811860.25	1007539.67
498	9.1038385	-30.1627662	811884.17	1007554.88
499	9.1039742	-30.1625477	811908.08	1007570.09
500	9.1041100	-30.1623292	811932.00	1007585.30
501	9.1042457	-30.1621107	811955.92	1007600.52

The normal use for this transformation method is the forward case: from survey measurements to calculated path in local coordinates to geographic 2D CRS. Reverse calculations are not expected to be needed. For completeness the reverse calculations using the above example are given below.



Initialise for i=1:

LAT[0] = 0.1577150739 rad  
 LON[0] = -0.5283339813 rad  
 e[0] = 0  
 n[0] = 0

Then with given:

LAT[1] = 9.03640810° = 0.1577150739 rad  
 LON[1] = -30.27130730° = -0.5283339813 rad  
 D[1] = 0 m

yields:

$\rho = 6,337,009.005$  m  
 $v = 6,378,663.709$  m  
 $n[1] = 0 + 0.0 * (6337009.005 - 0) = 0.0$   
 $e[1] = 0 + 0.0 * (6378663.709 - 0) * \cos(0.1577150739) = 0.0$

Next, for i=2 with given:

LAT[2] = 9.03654372° = 0.1577174409 rad  
 LON[2] = -30.27108901° = -0.5283301714 rad  
 D[2] = 10 m

yields:

$\rho = 6,337,009.005$  m  
 $v = 6,378,663.709$  m  
 $n[2] = 0.0 + (0.1577174409 - 0.1577150739) * (6337009.005 - 10) = 15.00$   
 $e[2] = 0.0 + (-0.5283301714 - -0.5283339813) * (6378663.709 - 10) * \cos(0.1577174409) = 24.00$

etc.

#### 4.9.4 **Wellbore local to projected (GNL)**

(EPSG Dataset coordinate operation method code 1077)

The GNL method requires as input (an array of) unscaled local coordinates with the local north axis aligned with grid north of the projected CRS.

If the input local coordinates are not aligned with grid north, but instead with magnetic north or true north, then they should be changed to grid north reference before input in the algorithm. In practice this is often done by applying a constant rotation using the map grid convergence and/or magnetic declination at the surface location to all stations. It is also possible to compute these corrections along-hole, e.g., iteratively or using the planned trajectory. However, the LMP method is recommended whenever the inputs are aligned with true north or corrections which vary along-hole (convergence or scale) are desired.

The method has options for the application of two corrections:

- i) **point scale factor** (*psf*) is used to scale horizontal offsets to the local scale for the projected CRS, resolved into east and north components (when a conformal map projection is used,  $psf_E = psf_N$ ): (i) no psf correction is applied, (ii) the point scale factor at the well reference point is applied to the complete wellbore, and (iii) the point scale factor at each point along the wellbore is applied.
- ii) **depth correction factor** (*dcf*) is used to scale horizontal offsets according to the depth of the point in the wellbore: (i) no dcf correction is applied, or (ii) a dcf correction is applied at each point along the wellbore.

The inputs are:

*WRP\_E* : Easting of the Well Reference Point in the target projected CRS;  
*WRP\_N* : Northing of the Well Reference Point in the target projected CRS;  
*psf\_flag* : 0 for none; 1 for constant at first point; 2 to apply a variable point scale factor;  
*dcf\_flag* : 0 for none; 2 to apply a variable depth correction factor along the path;  
*n[]* : Array of length *num\_points* containing local northing coordinates, oriented to Grid North of the target projected CRS;  
*e[]* : Array of length *num\_points* containing local easting coordinates, oriented to Grid East of the target projected CRS;  
*D[]* : Array of length *num\_points* containing depths below the Vertical Reference Surface at survey stations down the wellbore.

Care must be taken to convert local easting and northing to the same unit as used by the target projected CRS before applying the following equations.

The method is specified for three of the six possible combinations of *psf\_flag* and *dcf\_flag*. Formulas can readily be extended. However, it is considered not sensible to apply a variable point scale factor without depth correction factor, nor to apply depth correction factor without scale factor. The LMP method is recommended when it is desired to apply variable corrections along the wellbore.

If *psf\_flag*=0 and *dcf\_flag*=0 then the target coordinates are computed using a simple 2D translation:

$$\begin{aligned} E &= WRP\_E + e \\ N &= WRP\_N + n \end{aligned}$$

If *psf\_flag*=1 and *dcf\_flag*=0 then the target coordinates are computed using a scaled offset as:

$$\begin{aligned} E &= WRP\_E + e * psf\_E \\ N &= WRP\_N + n * psf\_N \end{aligned}$$

If *psf\_flag*=2 and *dcf\_flag*=2 then an iterative method is needed to compute the variable point scale factor and depth correction factor along the wellbore. The LMP method is recommended in this situation, but if the GNL method is used then map convergence should also be calculated variable along-hole to reference the local coordinates to grid north.

Output arrays *E* and *N* of length *num\_points* are computed as follows:

```
For i=1:num_points
    for psf_E and psf_N, compute_psf(E[i-1], N[i-1])
    dcf = R / (R - D[i])
    E[i] = E[i-1] + (e[i] - e[i-1]) * psf_E * dcf
    N[i] = N[i-1] + (n[i] - n[i-1]) * psf_N * dcf
```

where for i=1, for the first station: *E*[0] = *WRP\_E*; *N*[0] = *WRP\_N*; *e*[0] = 0; *n*[0] = 0, and:

*compute\_psf()* is a function that returns the local point scale factors in easting and northing directions at the given location;

*R* = 6371000 m. Note: Using a constant rather than the ellipsoidal local radii of curvature for the depth correction factors in easting and northing directions at the given location results in an insignificantly small error (e.g., for a lateral of 10 km at 10 km depth the worst case difference is 8 centimetres: 6371/(6371-10)=1.001572 vs. 6340/(6340-10)=1.001580).

**Reverse case:** to compute (*n,e*) from given (*E,N*):

For *psf\_flag*=0 and *dcf\_flag*=0:

$$\begin{aligned} n &= N - WRP\_N \\ e &= E - WRP\_E \end{aligned}$$

For *psf\_flag*=1 and *dcf\_flag*=0:

$$n = (N - \text{WRP\_N} / \text{psf\_N})$$

$$e = (E - \text{WRP\_E} / \text{psf\_E})$$

For  $\text{psf\_flag}=2$  and  $\text{dcf\_flag}=2$  (additionally requiring D as input):

```

For i=1:num_points
    for psf_E and psf_N, compute_psf(E[i-1], N[i-1])
        dcf = R / (R - D[i])
        n[i] = n[i-1] + (N[i] - N[i-1]) / (psf_N * dcf_N)
        e[i] = e[i-1] + (E[i] - E[i-1]) / (psf_E * dcf_E)
    
```

where for the first station  $i=0$  is initialised as in the forward case.

### Example:

An example of the GNL method is provided below, similar to the example given for the LMP method. This is not a realistic wellbore survey but demonstrates the magnitude of differences in computed coordinates when different corrections are applied. A straight path is modelled to a bottom hole at 5 km depth, approximately 14 km away from the surface location, using a constant inclination  $70.54002^\circ$ , bearing  $51.3178^\circ$  from grid north, and survey stations every 30.0167 metre.

Forward calculation for a well with surface coordinates in WGS 84 / UTM zone 25N, EPSG CRS code 32625. Since this is a conformal projection,  $\text{psf\_E} = \text{psf\_N}$ .

Easting of Well Reference Point	WRP_E	= 800,000 m
Northing of Well Reference Point	WRP_N	= 1,000,000 m
Point scale factor at WRP	$\text{psf\_E} = \text{psf\_N}$	= 1.00071
Zero Depth Point height	ZDP_height	= 25 m (not used in equations)

Local axes n and e are aligned with grid north and grid east, respectively.

Initialisation: E[0] = 800,000 m; N[0] = 1,000,000 m.											
						psf_flag=0; dcf_flag=0		psf_flag=1; dcf_flag=0		psf_flag=2; dcf_flag=2	
i	D (m)	n (m)	e (m)	psf_E = psf_N	dcf	E (m)	N (m)	E (m)	N (m)	E (m)	N (m)
1	0.00	0.00	0.00	1.000714	1.000000	800000.00	1000000.00	800000.00	1000000.00	800000.00	1000000.00
2	10.00	15.00	24.00	1.000714	1.000002	800000.00	1000000.00	800000.00	1000000.00	800000.00	1000000.00
3	20.00	30.00	48.00	1.000714	1.000003	800024.00	1000015.00	800024.02	1000015.01	800024.02	1000015.01
4	30.00	45.00	72.00	1.000714	1.000005	800048.00	1000030.00	800048.03	1000030.02	800048.03	1000030.02
5	40.00	60.00	96.00	1.000715	1.000006	800072.00	1000045.00	800072.05	1000045.03	800072.05	1000045.03
...											
497	4960.00	7440.00	11904.00	1.000804	1.000779	811904.00	1007440.00	811912.50	1007445.31	811917.68	1007448.55
498	4970.00	7455.00	11928.00	1.000804	1.000781	811928.00	1007455.00	811936.52	1007460.32	811941.72	1007463.57
499	4980.00	7470.00	11952.00	1.000804	1.000782	811952.00	1007470.00	811960.53	1007475.33	811965.76	1007478.60
500	4990.00	7485.00	11976.00	1.000804	1.000784	811976.00	1007485.00	811984.55	1007490.34	811989.79	1007493.62
501	5000.00	7500.00	12000.00	1.000804	1.000785	812000.00	1007500.00	812008.57	1007505.35	812013.83	1007508.65

## **4.10 Transformations between Vertical Coordinate Reference Systems**

The following coordinate transformations apply only when the positive direction of the axis of the source and target CRS is the same and where the two vertical CRSs share the same unit of measure:

4.10.1 Vertical Offset

4.10.2 Vertical Offset by Interpolation of Gridded Data

4.10.3 Vertical Offset and Slope

If either the positive axis direction or unit of measure are different, then a concatenated operation including one or more conversions through intermediate vertical CRSs is required. The additional operation methods required for such conversions:

4.10.4 Height Depth Reversal, and

4.10.5 Change of Vertical Unit

are covered later in this section. Their application is discussed in IOGP Guidance Note 323-24, Vertical data in oil and gas applications.

### **4.10.1 Vertical Offset**

(EPSG Dataset coordinate operation method code 9616)

A vertical offset allows calculation of coordinates in the target vertical coordinate reference system by adding a correction parameter A to the coordinate values of the point in the source system:

$$X_2 = X_1 + A_{1>2}$$

where

$X_2$  = value in the target vertical coordinate reference system.

$X_1$  = value in the source vertical coordinate reference system.

$A_{1>2}$  is the offset to be applied for the transformation from CRS 1 to CRS 2. Its value for the forward calculation is the value of the origin of the source CRS 1 in the target CRS 2.

Note: The sign convention used here is that which is applied in geodesy; this differs from that in general mathematics - see Offsets in Implementation Notes.

For the **reverse** transformation from CRS 2 to CRS 1 the same formula is used but with the sign of the offset  $A_{1>2}$  reversed:

$$X_1 = X_2 + (-A_{1>2})$$

#### **Example:**

For coordinate transformation: Baltic 1977 height to Black Sea height (1), code 5447:

Transformation Parameter: Vertical Offset  $A_{1>2}$  = 0.4m

Consider a point having a gravity-related height  $H_{\text{Baltic}}$  in the Baltic 1977 height system of 2.55m. Its value in the Black Sea height system is

$$H_{\text{BlackSea}} = 2.55\text{m} + 0.4\text{m} = 2.95\text{m}$$

For the reverse calculation to transform the Black Sea height of 2.95m to Baltic 1977 height:

$$H_{\text{Baltic}} = 2.95\text{m} + (-(0.4\text{m})) = 2.55\text{m}.$$

Further examples of input and output may be found in test procedure 5210 of the test dataset which accompanies OGP Publication 430, Geospatial Integrity in Geoscience Software (GIGS).

#### 4.10.2 Vertical Offset by Interpolation of Gridded Data

The relationship between some vertical coordinate reference systems is available through gridded data sets of offsets (sometimes called height differences). The vertical offset at a point is first interpolated within the grid of values. The interpolated offset is then applied as for the Vertical Offset method in the previous section.

Although the providers of some gridded data sets suggest a particular interpolation method within the grid, generally the density of grid nodes should be such that any reasonable grid interpolation method will give the same offset value within an appropriately small tolerance. Bi-linear interpolation is the most usual grid interpolation mechanism.

For the purposes of interpolation within the grid, horizontal coordinates of the point are required. The providers of gridded data sets define the horizontal CRS upon which the grid has been constructed and which is to be used for the grid interpolation, known as the *Interpolation CRS*; this is usually a geographic 2D CRS.

The EPSG Dataset differentiates methods by the format of the gridded data file. The grid file format is given in documentation available from the information source. An example method is *Vertcon* (EPSG Dataset coordinate operation method code 9658) which is used by the US National Geodetic Survey for transformation between the NGVD29 and NAVD88 gravity-related height systems. A further example is the *Vertical Offset by Grid Interpolation (NZLVD)* method (EPSG Dataset coordinate operation method code 1071) used in New Zealand where care is required in interpreting the direction of the transformation because of the file naming convention used: refer to the method formula and this document's Implementation Notes - Offsets. Sample test points are given in the Example field of each method in the EPSG Dataset.

#### 4.10.3 Vertical Offset and Slope

(EPSG Dataset coordinate operation method code 1046)

In Europe, national vertical systems are related to the pan-European vertical system through three transformation parameters and the formula:

$$X_2 = X_1 + \{A_{1>2} + [I_{\phi 1>2} * \rho_0 * (\phi - \phi_0)] + [I_{\lambda 1>2} * v_0 * (\lambda - \lambda_0) * \cos\phi]\} \text{ where}$$

$X_2$  = value in the target vertical coordinate reference system.

$X_1$  = value in the source vertical coordinate reference system.

$A_{1>2}$  is the offset to be applied for the transformation from CRS 1 to CRS 2. Its value is the value of the origin of the source CRS 1 in the target CRS 2.

$I_{\phi 1>2}$  is the value in radians of the slope parameter in the latitude domain, i.e. in the plane of the meridian, derived at an evaluation point with coordinates of  $\phi_0$ ,  $\lambda_0$ . When  $I_{\phi}$  is positive then to the north of the evaluation point latitude  $\phi_0$  the source and target CRS surfaces converge.

$I_{\lambda 1>2}$  is the value in radians of the slope parameter in the longitude domain, i.e. perpendicular to the plane of the meridian. When  $I_{\lambda}$  is positive then to the east of the evaluation point longitude  $\lambda_0$  the CRS surfaces converge.

$\rho_0$  is the radius of curvature of the meridian at latitude  $\phi_0$ ,

$$\text{where } \rho_0 = a(1 - e^2)/(1 - e^2 \sin^2 \phi_0)^{3/2}$$

$v_0$  is the radius of curvature on the prime vertical (i.e. perpendicular to the meridian) at latitude  $\phi_0$ ,

$$\text{where } v_0 = a/(1 - e^2 \sin^2 \phi_0)^{1/2}$$

$\phi$ ,  $\lambda$  are the horizontal coordinates of the point in the ETRS89 coordinate reference system, in radians.

$\phi_0$ ,  $\lambda_0$  are the coordinates of the evaluation point in the ETRS89 coordinate reference system, in radians.

The horizontal location of the point must always be given in ETRS89 terms. Care is required where compound coordinate reference systems are in use: if the horizontal coordinates of the point are known in the local CRS they must first be transformed to ETRS89 values.

### Reversibility

Similarly to the Vertical Offset method described in previous sections above, the Vertical Offset and Slope method is reversible using a slightly different formula to the forward formula and in which the signs of the parameters  $A$ ,  $I_\phi$  and  $I_\lambda$  from the forward transformation are reversed in the reverse transformation:

$$X_1 = \{X_2 + -A_{1>2} + [-I_{\phi 1>2} * \rho_0 * (\phi - \phi_0)] + [-I_{\lambda 1>2} * v_0 * (\lambda - \lambda_0) * \cos\phi]\}$$

### Example:

For coordinate transformation LN02 height to EVRF2000 height (1)

Transformation parameters:

Ordinate 1 of evaluation point:	$\phi_{S0} =$	46°55'N	=	0.818850307 rad
Ordinate 2 of evaluation point:	$\lambda_{S0} =$	8°11'E (of Greenwich)	=	0.142826110 rad
Vertical Offset:	$A =$	-0.245 m		
Inclination in latitude:	$I_\phi =$	-0.210"	=	-0.000001018 rad
Inclination in longitude:	$I_\lambda =$	-0.032"	=	-0.000000155 rad
Horizontal CRS code:			=	4258

Consider a point having a gravity-related height in the LN02 system ( $H_s$ ) of 473.0m and with horizontal coordinates in the ETRS89 geographic coordinate reference system of:

$$\text{Latitude } \phi_{\text{ETRS89}} = 47^\circ 20' 00.00'' \text{N} = 0.826122513 \text{ rad}$$

$$\text{Longitude } \lambda_{\text{ETRS89}} = 9^\circ 40' 00.00'' \text{E} = 0.168715161 \text{ rad}$$

ETRS89 uses the GRS1980 ellipsoid for which  $a = 6378137$  m and  $1/f = 298.25722221$

Then	$\rho_0 =$	6369526.88 m
	$I_\phi \text{ term} =$	-0.047 m
	$v_0 =$	6389555.64 m
	$I_\lambda \text{ term} =$	-0.017 m

$$\begin{aligned} \text{whence EVRF2000 height } X_2 = H_{\text{EVRF}} &= 473.0 + (-0.245) + (-0.047) + (-0.017) \\ &= 472.69 \text{ m.} \end{aligned}$$

For the reverse transformation from EVRF2000 height of 472.69 m to LN02 height:

$$\begin{aligned} X_1 = H_{\text{LN02}} &= \{472.69 + [ -(-0.245) ] + [ -(-0.047) ] + [ -(-0.017) ] \} \\ &= 473.00 \text{ m.} \end{aligned}$$

### 4.10.4 Height Depth Reversal

(EPSG Dataset coordinate operation method code 1068)

If there is a requirement to either (i) change a height in a source CRS to a depth in a target CRS referenced to the same vertical datum, or (ii) change a depth in the a source CRS to a height in a target CRS referenced to the same vertical datum, then the following generic formula can be used:

$$X_2 = -1 * X_1$$

### Examples:

1. Source CRS = MSL height, in which a height = -300m.

Then target CRS = MSL depth, and the depth =  $(-300 * -1) = 300\text{m}$ .

2. Source CRS = NAVD88 depth (ftUS), in which a depth = 500 ftUS.

Then target CRS = NAVD88 height (ftUS), and the height =  $(500 * -1) = -500$  ftUS.

#### 4.10.5 Change of Vertical Unit

(EPSG Dataset coordinate operation method code 1069)

If there is a requirement to change the linear unit of a vertical axis, the following formulae can be used:

$$\begin{array}{ll} \text{for the forward transformation:} & X_2 = (X_1 * U_1) / U_2 \\ \text{for the reverse transformation:} & X_1 = (X_2 * U_2) / U_1 \end{array}$$

where  $U_1$  and  $U_2$  are unit conversion ratios for the Unit of Measure of the coordinate system axis for  $CRS_1$  and  $CRS_2$  respectively.  $U = [(factor\ b) / (factor\ c)]$  from the EPSG Dataset Unit of Measure table, with respect to the SI base unit, for linear units the metre.

Then

$$\begin{array}{ll} \text{for the forward transformation:} & X_2 = [X_1 * (b_1 / c_1)] / (b_2 / c_2) = (X_1 * b_1 * c_2) / (b_2 * c_1) \\ \text{for the reverse transformation:} & X_1 = [X_2 * (b_2 / c_2)] / (b_1 / c_1) = (X_2 * b_2 * c_1) / (b_1 * c_2) \end{array}$$

#### **Example**

Conversion of a height of 25 metres into US survey feet:

For the metre (EPSG UoM code 9001) factor  $b = 1$  and factor  $c = 1$ , so  $U_1$  has a value of 1.

For the US survey foot (EPSG UoM code 9003) factor  $b = 12$  and factor  $c = 39.37$ , so  $U_2$  has a value of approximately 0.304800609601219 (one US survey foot  $\approx$  0.304800609601219 metres).

Then  $H_{ftUS} = (25 * (1 / 1)) / (12 / 39.37) = 82.02$  ftUS.

For the reverse conversion of 82.02 ftUS into metres:

$U_1$  and  $U_2$  are as for the forward case, with values of 1/1 and 12/39.37 respectively.

Then  $H_m = (82.02 * (12 / 39.37) / (1 / 1)) = 25.00$  m.

### 4.11 Geoid and Height Correction Models

#### 4.11.1 Geographic3D to GravityRelatedHeight

Although superficially involving a change of dimension from three to one, this transformation method is actually one-dimensional. The transformation applies an offset to the ellipsoidal height component of a geographic 3D coordinate reference system with the result being a gravity-related height in a vertical coordinate reference system. However the ellipsoidal height component of a geographic 3D coordinate reference system cannot exist without the horizontal components, i.e. it cannot exist as a one-dimensional coordinate reference system.

Geodetic science distinguishes between geoid-ellipsoid separation models ('geoid height models') and height correction models. Geoid separation models give the height difference between the ellipsoid surface and the geoid. Height correction models give height difference between the ellipsoid surface and a previously-adopted national vertical CRS. The mathematics of the application of these models is identical and for the purposes of the EPSG Dataset they are considered to be one method.



The correction value  $\zeta^6$  is interpolated from a grid of height differences and the interpolation requires the latitude and longitude components of the geographic 3D coordinate reference system as arguments. If  $\mathbf{h}$  is the ellipsoidal height (height of point above the ellipsoid, positive if up) in the geographic 3D CRS and  $\mathbf{H}$  is the gravity-related height in a vertical CRS, then

$$\mathbf{H} = \mathbf{h} - \zeta$$

Note that unlike the general convention adopted for offsets described in 2.4.1, geoid separation and height correction models conventionally use the true mathematical convention for sign.

The EPSG Dataset differentiates between the formats of the gridded height files and distinguishes separate coordinate operation methods for each file format. The coordinate operation method may also define the interpolation technique to be used. However the density of grid nodes is usually sufficient for any reasonable interpolation technique to be used, with bi-linear interpolation usually being applied.

### Reversibility

The reverse transformation, from gravity-related height in the vertical coordinate reference system to the ellipsoidal height component of the geographic3D coordinate reference system, requires that a horizontal position be associated with the gravity-related height. This is indeterminate unless a compound coordinate reference system is involved (see the Geographic3D to Geographic2D+GravityRelatedHeight method described below). Geographic3D to GravityRelatedHeight methods therefore are not reversible.

#### 4.11.2 Geographic3D to Geographic2D+GravityRelatedHeight

This method transforms coordinates between a geographic 3D coordinate reference system and a compound coordinate reference system consisting of separate geographic 2D and vertical coordinate reference systems. Separate operations are made between the horizontal and vertical components. In its simplest form it combines a Geographic 3D to 2D conversion and a Geographic3D to GravityRelatedHeight transformation (see sections 4.1.4 and 4.11.1). However, complexities arise (a) for the forward transformation if the source 3D and target 2D geographic coordinate reference systems are based on different geodetic datums, or (b) in the reverse transformation of height from compound to geographic 3D.

#### Horizontal component

If the horizontal component of the compound coordinate reference system and the geographic 3D coordinate reference system are based on the same geodetic datum, this operation is simply the Geographic 3D to 2D conversion described in section 4.1.4 except that for the reverse case (2D to 3D) no assumption is required for the ellipsoidal height as it will come from the operation for the vertical part.

If the horizontal component of the compound coordinate reference system and the geographic 3D coordinate reference system are based on different geodetic datums then any of the geographic to geographic transformations discussed in section 4.3, including those using geocentric methods (section 4.2), may be used.

#### Vertical component

The forward transformation from geographic 3D to vertical component of the compound system uses the Geographic3D to GravityRelatedHeight method described in section 4.11.1. Then:

$$\mathbf{H} = \mathbf{h} - \zeta$$

where, as before,  $\mathbf{h}$  is the ellipsoidal height (height of point above the ellipsoid, positive if up) in the

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<sup>6</sup> Geodetic science recognises several types of gravity-related height, differentiated by assumptions made about the gravitational field. A discussion of these types is beyond the scope of this document. In this document the symbol  $\zeta$  is used to indicate the correction to be applied to the ellipsoid height.



geographic 3D CRS,  $H$  is the gravity-related height in the vertical CRS part of the compound CRS and  $\zeta$  is the correction from ellipsoidal height to gravity-related height from the gridded data.

The reverse transformation, from vertical component of the compound system to geographic 3D system, requires interpolation within the grid of height differences. However the latitude and longitude arguments for this interpolation must be in the geographic 3D coordinate reference system, as the nodes for the gridded data will be in this system. Therefore the reverse operation on the horizontal component of the compound system must be executed before the reverse vertical transformation can be made. Then:

$$h = H - \zeta$$

#### 4.11.3 **Geographic2D with Height Offsets**

(EPSG Dataset coordinate operation method code 9618)

This method used in Japan is a simplified form of the general Geographic3D to Geographic2D+GravityRelatedHeight method described above. It combines the geographic 2D offset method described in section 4.3.3 above with an ellipsoidal height to gravity-related height value  $A$  applied as a vertical offset.

$$\begin{aligned}\phi_{\text{WGS84}} &= \phi_{\text{Tokyo}} + d\phi \\ \lambda_{\text{WGS84}} &= \lambda_{\text{Tokyo}} + d\lambda \\ h_{\text{WGS84}} &= H_{\text{JSLD}} + A\end{aligned}$$

## 5 Revision history

<u>Version</u>	<u>Date</u>	<u>Amendments</u>
1	December 1993	First release – POSC Epicentre
10	May 1998	Additionally issued as an EPSG guidance note.
11	November 1998	Polynomial for Spain and Tunisia Mining Grid methods added.
12	February 1999	Abridged Molodensky formulas corrected.
13	July 1999	Lambert Conic Near Conformal and American Polyconic methods added.
14	December 1999	Stereographic and Tunisia Mining Grid formulas corrected. Krovak method added.
15	June 2000	General Polynomial and Affine methods added
16	December 2000	Lambert Conformal (Belgium) remarks revised; Oblique Mercator methods consolidated and formulas added. Similarity Transformation reversibility remarks amended.
17	June 2001	Lambert Conformal, Mercator and Helmert formulas corrected.
18	August 2002	Revised to include ISO 19111 terminology. Section numbering revised. Added Preface. Lambert Conformal (West Orientated), Lambert Azimuthal Equal Area, Albers, Equidistant Cylindrical (Plate Carrée), TM zoned, Bonne, Molodensky-Badedas methods added. Errors in Transverse Mercator (South Orientated) formula corrected.
19	December 2002	Polynomial formulas amended. Formula for spherical radius in Equidistant Cylindrical projection amended. Formula for Krovak projection amended. Degree representation conversions added. Editorial amendments made to subscripts and superscripts.
20	May 2003	Font for Greek symbols in Albers section amended.
21	October 2003	Typographic errors in example for Lambert Conic (Belgium) corrected. Polar Stereographic formulae extended for secant variants. General polynomial extended to degree 13. Added Abridged Molodensky and Lambert Azimuthal Equal Area examples and Reversible polynomial formulae.
22	December 2003	Errors in FE and FN values in example for Lambert Azimuthal Equal Area corrected.
23	January 2004	Database codes for Polar Stereographic variants corrected. Degree representation conversions withdrawn.
24	October 2004  From this revision, published as part 2 of a two-part set.	Corrected equation for u in Oblique Mercator. Added Guam projection, Geographic 3D to 2D conversion, vertical offset and gradient method, geoid models, bilinear interpolation methods. Added tables giving projection parameter definitions. Amended Molodensky-Badekas method name and added example. Added section on reversibility to Helmert 7-parameter transformations. Transformation section 2 reordered. Section 3 (concatenated operations) added.
25	May 2005	Amended reverse formulas for Lambert Conic Near-Conformal. Corrected Lambert Azimuthal Equal Area formulae. Symbol for latitude of pseudo standard parallel parameter made consistent. Corrected Affine Orthogonal Geometric transformation reverse example. Added Modified Azimuthal Equidistant projection.
26	July 2005	Further correction to Lambert Azimuthal Equal Area formulae. Correction to Moldenski-Badekas example.
27	September 2005	Miscellaneous linear coordinate operations paragraphs re-written to include reversibility and UKOOA P6. Improved formula for r' in Lambert Conic Near-Conformal.
28	November 2005	Corrected error in formula for t and false grid coordinates of 2SP example in Mercator projection.
29	April 2006	Typographic errors corrected. (For oblique stereographic, corrected formula for w. For Lambert azimuthal equal area, changed example. For Albers equal area, corrected formulae for alpha. For modified azimuthal equidistant, corrected formula for c. For Krovak, corrected formula for theta', clarified formulae for t0 and lat. For Cassini, in example corrected radian value of longitude of natural origin). References to EPSG updated.

Version	Date	Amendments
30	June 2006	Added Hyperbolic Cassini-Soldner. Corrected FE and FN values in example for Modified Azimuthal Equidistant. Added note to Krovak. Amended Abridged Molodensky description, corrected example.
31	August 2006	Corrected sign of value for G in Modified Azimuthal Equidistant example.
32	February 2007	Descriptive text for Oblique Mercator amended; formula for Laborde projection for Madagascar added. Added polar aspect equations for Lambert Azimuthal Equal Area. Corrected example in polynomial transformation for Spain. For Lambert 1SP, corrected equation for $r'$ .
33	March 2007	For Krovak example, corrected axis names.
34	July 2007	Note on longitude wrap-around added prior to preamble to map projection formulas, section 1.4. For Laborde, corrected formula for $q'$ . For Albers Equal Area, corrected formulae for $\alpha$ and $\beta'$ .
35	April 2008	Longitude wrap-around note clarified. For Oblique Mercator, corrected symbol in formula for longitude. For Krovak, clarified defining parameters. Amended Vertical Offset description and formula. Added geographic/topocentric conversions, geocentric/topocentric conversions, Vertical Perspective, Orthographic, Lambert Cylindrical Equal Area, ellipsoidal development of Equidistant Cylindrical. Removed section on identification of map projection method.
36	July 2008	For Lambert Conic Near Conformal, corrected equations for $\phi'$ .
37	August 2008	Corrected general polynomial example.
38	January 2009	For Mercator (1SP), clarified use of $\phi_0$ . For Molodensky-Badekas, augmented example. Added Popular Visualisation Pseudo Mercator method, added formulas and examples for Mercator (Spherical) and formulas for American Polyconic.
39	April 2009	Preface revised to be consistent with other parts of GN7. For Lambert Azimuth Equal Area, in example corrected symbol for $\beta'$ . For Krovak, corrected formulas. For Equidistant Cylindrical (spherical) corrected formula for R; comments on R added to all spherical methods. For Equidistant Cylindrical updated formula to harmonise parameters and symbols with similar methods.
40	November 2009	For geographic/geocentric conversions, corrected equation for $\phi$ . For Transverse Mercator (South Oriented), added example. Corrected equation for computation of radius of authalic sphere (optionally referenced by equal area methods developed on a sphere). Augmented description of geocentric methods to clearly discriminate the coordinate domain to which they are applied.
41	October 2010	Augmented Transverse Mercator for wide area. Amended TM(SO) example. Amended Mercator and Oblique Mercator variant names, added Mercator variant C method. Corrected Oblique Mercator (Hotine variant B) formulae for azimuth = 90° case. Added Krovak variants. Corrected Polar Stereographic equations for calculation of longitude. Augmented reverse case for Similarity Transformation. References to OGP publication 430 (GIGS test data) added.
42	November 2010	Corrected formula for t in Mercator variant C reverse case.
43	July 2011	Corrected formulae for $\xi_0$ parameters in Transverse Mercator. Corrected coefficient B1 and longitude example in Polynomial transformation for Spain.
44	April 2012	Added missing equations for reverse case of American Polyconic. Amended Transverse Mercator JHS formula to allow for origin at pole. Corrected Modified Krovak formula and example. Corrected equation for $u'$ in Hotine Oblique Mercator.
45	July 2012	Added left-handed bin grid method.
46	June 2013	Added Lambert Conic Conformal 2SP (Michigan), Colombia Urban and time-dependent Helmert 7-parameter transformation methods and Affine Parametric Transformation example. Amended Tunisia Mining Grid description.
47	June 2014	Added Molodensky-Badekas (PV).
48	July 2014	Re-write of P6 Seismic Bin Grid transformation text for consistency with P6/11 exchange format v1.1.
49	February 2015	Helmert and Molodensky-Badekas descriptions (2.4.3) rewritten, corrected time-dependent Helmert example. Document sections reordered.

<u>Version</u>	<u>Date</u>	<u>Amendments</u>
50	April 2015	Complex polynomial reverse example added. Conformal sphere and Albers unbalanced brackets corrected. Minor editorial corrections made.
51	September 2016	Horizontal axis order reversal added. Pseudo Mercator section rewritten.
52	October 2017	Added time-specific transformations and point motion operations. Amended vertical transformation descriptions (splitting vertical offset into three methods). Note regarding coefficients with zero values added to Polynomial Transformations. Zoned Transverse Mercator formulae modified. Minor changes to Krovak examples.
53	April 2018	Added reverse example to Change of Vertical Unit method. Harmonised case for Helmert transformation names.
54	August 2018	Added implementation notes section to preface. Reviewed use of atan and atan2 functions throughout document. In point motion operation with velocity grid, removed reference to Interpolation CRS.
55	October 2018	Added Equal Earth projection.
56	February 2019	Added Wellbore and New Zealand vertical offset methods. Editorial amendments to Affine and Helmert transformation descriptions. Re-ordered sections.

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