

UNIVERSIDADE FEDERAL DA PARAÍBA CENTRO DE TECNOLOGIA DEPARTAMENTO DE ENGENHARIA MECÂNICA DISCIPLINA DE ANÁLISE MATRICIAL E MODELAGEM DE ESTRUTURAS

 $1^{\underline{0}}$ Trabalho de Avaliação de Aprendizagem

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> João Pessoa, PB Novembro de 2019

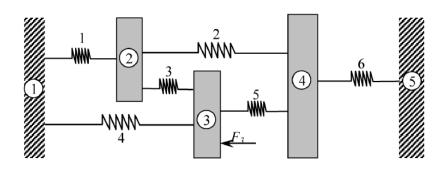


Figura 1: Sistema das questões 1 e 2

Matriz de Rigidez Global

Stiffiness 1							Stiffiness 2					
u1	u2	u3	u4	u5			u1	u2	u3	u4	u5	
k1	-k1	us	u-		u1		uı	u _z	45	u-	45	u1
-k1	k1				u2			k2		-k2		u2
	KI				u3			NZ.		NZ.		u3
					u4			-k2		k2		u4
					u5			N.E		I.E		u5
Stiffiness 3							Stiffiness 4					
u1	u2	u3	u4	u5			u1	u2	u3	u4	u5	
					u1		k4		-k4			u1
	k3	-k3			u2							u2
	-k3	k3			u3		-k4		k4			u3
					u4							u4
					u5							u5
		Stiffi	ness 5						Stiffi	ness 6		
u1	u2	u3	u4	u5			u1	u2	u3	u4	u5	
					u1							u1
					u2							u2
		k5	-k5		u3							u3
		-k5	k5		u4					k6	-k6	u4
					u5					-k6	k6	u5
	Global Matrix											
							2	u4	5			
		R12	R13	F1	uı	u2	u3	u4	u5	u1		
	R21	R23	R24	F1 F2						u1 u2		
	KZI	K23	K24	F2 F3						u2 u3		
		R42	D42									
		K4./	R43	F4						u4		

Figura 2: Obtenção da matriz de rigidez global da questão 1

```
ı clear
2 close all
3 clc
5 %% Inputs
7 \text{ F2} = 0; \% \text{ Force [N]}
8 \text{ F3} = -1000; \% \text{ Force [N]}
9 F4 = 0; % Force [N]
10 u1 = 0; % Displacement [mm]
u5 = 0; % Displacement [mm]
12 \text{ k1} = 500; % Spring Constant [N/mm]
13 k2 = 400; % Spring Constant [N/mm]
k3 = 600; % Spring Constant [N/mm]
15 k4 = 200; % Spring Constant [N/mm]
16 k5 = 400; % Spring Constant [N/mm]
17 k6 = 300; % Spring Constant [N/mm]
18 n = 5; % Degrees of Freedom
19
20 %% Characterization
21
22 A = [1 -1; -1 1];
^{23}
24 % Stiffness
25
26 K1 = k1 \star A;
_{27} K2 = k2*A;
28 \text{ K3} = \text{k3} \times \text{A};
29 K4 = k4 \star A;
30 \text{ K5} = \text{k5} \times \text{A};
31 K6 = k6*A;
32
33 % Global Matrix Equation
35 K = zeros(n);
36 \text{ K}(1:2,1:2) = \text{K}(1:2,1:2) + \text{K1};
K(2:2:4,2:2:4) = K(2:2:4,2:2:4) + K2;
38 \text{ K}(2:3,2:3) = \text{K}(2:3,2:3) + \text{K}3;
K(1:2:3,1:2:3) = K(1:2:3,1:2:3) + K4;
40 \text{ K}(3:4,3:4) = \text{K}(3:4,3:4) + \text{K5};
K(4:5,4:5) = K(4:5,4:5) + K6;
42
43 % Force Vector
44
45 syms F1 F5
46 \text{ F} = [\text{F1; F2; F3; F4; F5}];
```

```
% Displacement Vectors
49
  syms u2 u3 u4
  U = [u1; u2; u3; u4; u5];
52
  %% Calculations
53
54
55
  AN = solve(F-K*U);
56
  F = double([AN.F1; F2; F3; F4; AN.F5]);
57
  U = double([u1; AN.u2; AN.u3; AN.u4; u5]);
59
  f1 = k1*(U(1) - U(2));
  f2 = k2*(U(2) - U(4));
  f3 = k3*(U(2) - U(3));
  f4 = k4*(U(1) - U(3));
  f5 = k5*(U(3) - U(4));
  f6 = k6 * (U(4) - U(5));
66
  f = [f1; f2; f3; f4; f5; f6];
67
68
  clear A AN f1 f2 f3 f4 f5 f6 F1 F2 F3 F4 F5 K1 K2 K3 K4 K5 K6 n u1 u2 \dots
      u3 u4 u5
```

Obteve-se, a partir do script apresentado, os seguintes resultados:

$$[F] = \begin{bmatrix} R_1 \\ 0 \\ F_3 \\ 0 \\ R_5 \end{bmatrix} = \begin{bmatrix} 737.5 \\ 0 \\ -1000 \\ 0 \\ 262.5 \end{bmatrix} N$$

$$[K] = \begin{bmatrix} 700 & -500 & -200 & 0 & 0 \\ -500 & 1500 & -600 & -400 & 0 \\ -200 & -600 & 1200 & -400 & 0 \\ 0 & -400 & -400 & 1100 & -300 \\ 0 & 0 & 0 & -300 & 300 \end{bmatrix} N/mm$$

$$[U] = \begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \\ u_5 \end{bmatrix} = \begin{bmatrix} 0 \\ -0.8541667 \\ -1.55208333 \\ -0.875 \\ 0 \end{bmatrix} mm$$

$$[f] = \begin{vmatrix} 427.08333 \\ 8.333 \\ 418.75 \\ 310.41667 \\ -270.8333 \\ -262.5 \end{vmatrix} N$$

sendo [F] o vetor força (carregamento e reações nos nós), [K] a matriz de rigidez global, [U] o vetor deslocamento e [f] o vetor força (tração/compressão nos elementos de mola).

```
ı clear
2 close all
3 clc
5 %% Inputs
7 \text{ F3} = -1000; \% \text{ Force [N]}
8 \text{ F4} = 0; % \text{ Force [N]}
9 u1 = 0; % Displacement [mm]
u0 u2 = 0; % Displacement [mm]
u5 = 0; % Displacement [mm]
k1 = 500; % Spring Constant [N/mm]
k2 = 400; % Spring Constant [N/mm]
k3 = 600; % Spring Constant [N/mm]
15 k4 = 200; % Spring Constant [N/mm]
16 k5 = 400; % Spring Constant [N/mm]
17 k6 = 300; % Spring Constant [N/mm]
18 n = 5; % Degrees of Freedom
  %% Characterization
21
22 A = [1 -1; -1 1];
24 % Stiffness
_{26} K1 = k1 * A;
27 K2 = k2 \star A;
28 \text{ K3} = \text{k3} \times \text{A};
_{29} K4 = k4 * A;
30 \text{ K5} = \text{k5} \times \text{A};
31 K6 = k6*A;
33 % Global Matrix Equation
35 K = zeros(n);
36 \text{ K}(1:2,1:2) = \text{K}(1:2,1:2) + \text{K1};
K(2:2:4,2:2:4) = K(2:2:4,2:2:4) + K2;
K(2:3,2:3) = K(2:3,2:3) + K3;
K(1:2:3,1:2:3) = K(1:2:3,1:2:3) + K4;
40 \text{ K}(3:4,3:4) = \text{K}(3:4,3:4) + \text{K5};
K(4:5,4:5) = K(4:5,4:5) + K6;
```

```
% Force Vector
44
  syms F1 F2 F5
45
  F = [F1; F2; F3; F4; F5];
47
  % Displacement Vectors
49
  syms u3 u4
50
  U = [u1; u2; u3; u4; u5];
  %% Calculations
  AN = solve(F-K*U);
56
  F = double([AN.F1; AN.F2; F3; F4; AN.F5]);
  U = double([u1; u2; AN.u3; AN.u4; u5]);
59
  clear A AN F1 F2 F3 F4 F5 K1 K2 K3 K4 K5 K6 n u1 u2 u3 u4 u5
```

Obteve-se, a partir do script apresentado, os seguintes resultados:

$$[F] = \begin{bmatrix} R_1 \\ F_2 \\ F_3 \\ 0 \\ R_5 \end{bmatrix} = \begin{bmatrix} 189.65517 \\ 706.89655 \\ -1000 \\ 0 \\ 103.44828 \end{bmatrix} N$$
$$\begin{bmatrix} u_1 \\ 0 \\ 0 \\ 0 \end{bmatrix}$$

$$[U] = \begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \\ u_5 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ -0.94828 \\ -0.34483 \\ 0 \end{bmatrix} mm$$

sendo [F] o novo vetor força (carregamento e reações nos nós) e [U] o novo vetor deslocamento.

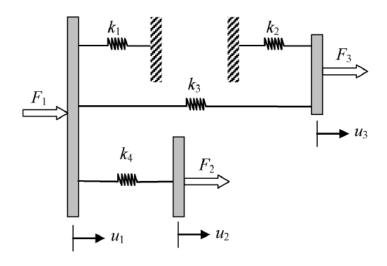


Figura 3: Sistema da questão 3

Matriz de Rigidez Global

Stiffiness 1							Stiffiness 2					
u1	u2	u3	u4	u5			u1	u2	u3	u4	u5	
k1			-k1		u1							u1
					u2							u2
					u3				k2		-k2	u3
-k1			k1		u4							u4
					u5				-k2		k2	u5
Stiffiness 3							Stiffiness 4					
u1	u2	u3	u4	u5			u1	u2	u3	u4	u5	
k3 -k3		-k3			u1		k4	-k4				u1
					u2		-k4	k4				u2
		k3			u3							u3
					u4							u4
					u5							u5
	Global Mat											
			u1	u2	u3	u4	u5					
		F1						u1				
		F2						u2				
		F3						u3				
		F4						u4				
		F5						u5				

Figura 4: Obtenção da matriz de rigidez global da questão 3

```
ı clear
2 close all
  clc
5 %% Inputs
7 syms k1 k2 k3 k4 F1 F2 F3 F4 F5 u1 u2 u3
  u4 = 0; % Displacement [mm]
10 u5 = 0; % Displacement [mm]
11
12 %% Characterization
14 % Stiffness
15
_{16} K = [k1+k3+k4 -k4 -k3 -k1 0; ...
17 -k4 k4 0 0 0; ...
18 -k3 0 k2+k3 0 -k2; ...
19 -k1 0 0 k1 0; ...
20 0 0 k2 0 -k2];
22 % Force Vector
F = [F1; F2; F3; F4; F5];
25
26 % Displacement Vectors
U = [u1; u2; u3; u4; u5];
29
30 clear F1 F2 F3 F4 F5 u1 u2 u3 u4 u5
```

Obteve-se, a partir do script apresentado, os seguintes resultados:

$$[F] = \begin{bmatrix} F_1 \\ F_2 \\ F_3 \\ R_4 \\ R_5 \end{bmatrix} N$$

$$[K] = \begin{bmatrix} k1 + k3 + k4 & -k4 & -k3 & -k1 & 0 \\ -k4 & k4 & 0 & 0 & 0 \\ -k3 & 0 & k2 + k3 & 0 & -k2 \\ -k1 & 0 & 0 & k1 & 0 \\ 0 & 0 & -k2 & 0 & k2 \end{bmatrix} N/mm$$

$$[U] = \begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ 0 \\ 0 \end{bmatrix} mm$$

sendo [F] o vetor força (carregamento e reações nos nós), [K] a matriz de rigidez global e [U] o vetor deslocamento.

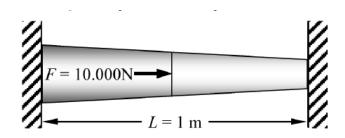


Figura 5: Sistema da questão 4

```
ı clear
2 close all
3 clc
  %% Inputs
  F2 = 0; % Force [N]
  F3 = 10000; % Force [N]
9 F4 = 0; % Force [N]
u1 = 0; % Displacement [m]
u2 = 0; % Displacement [m]
12 L = 1; % Lenght [m]
13 E = 100*10^6; % Young's Modulus [Pa]
n = 5; % Degrees of Freedom
  %% Characterization
16
17
  M = [1 -1; -1 1];
19
  % Stiffness
20
  x = linspace(0, L, n);
r = 0.05 - 0.04.*(x); % Radius [m]
A = @(r) pi*r(1:end-1).*r(2:end); % Area [m^2]
  A = A(r);
26
k = E.*A./(diff(x));
  K1 = k(1) *M;
30 K2 = k(2) *M;
```

```
31 \text{ K3} = \text{k(3)} \star \text{M};
32 \text{ K4} = \text{k(4)} \star \text{M};
33
34 % Global Matrix Equation
36 \text{ K} = zeros(n);
K(1:2,1:2) = K(1:2,1:2) + K1;
K(2:3,2:3) = K(2:3,2:3) + K2;
  K(3:4,3:4) = K(3:4,3:4) + K3;
40 \text{ K}(4:5,4:5) = \text{K}(4:5,4:5) + \text{K4};
41
   % Force Vector
43
44 syms F1 F5
   F = [F1; F2; F3; F4; F5];
   % Displacement Vectors
47
48
  syms u3 u4 u5
   U = [u1; u3; u4; u5; u2];
51
   %% Calculations
  AN = solve(F-K*U);
54
55
  F = double([AN.F1; F2; F3; F4; AN.F5]);
   U = double([u1; AN.u3; AN.u4; AN.u5; u2]);
   f = k'.*diff(U);
  clear AN F1 F2 F3 F4 F5 K1 K2 K3 K4 M n u1 u2 u3 u4 u5
```

Diante da característica do sistema, o mesmo foi discretizado a fim de simplificar a análise, obtendo-se:

$$[x] = \begin{bmatrix} x_1 \\ x_2 \\ x_3 \\ x_4 \\ x_5 \end{bmatrix} = \begin{bmatrix} 0 \\ 0.25 \\ 0.5 \\ 0.75 \\ 1 \end{bmatrix} m$$

$$[r] = \begin{bmatrix} r_1 \\ r_2 \\ r_3 \\ r_4 \\ r_5 \end{bmatrix} = \begin{bmatrix} 0.05 \\ 0.04 \\ 0.03 \\ 0.02 \\ 0.01 \end{bmatrix} m$$

$$[A] = \begin{bmatrix} A_1 \\ A_2 \\ A_3 \\ A_4 \end{bmatrix} = \begin{bmatrix} 0.0062832 \\ 0.0037699 \\ 0.001885 \\ 0.00062839 \end{bmatrix} m^2$$

$$[k] = \begin{bmatrix} k_1 \\ k_2 \\ k_3 \end{bmatrix} = 10^6 \times \begin{bmatrix} 2.5133 \\ 1.5080 \\ 0.7540 \end{bmatrix} N/m$$

sendo [X] o vetor comprimento (distância entre origem e nós), [r] o vetor raio (raio da seção transversal de cada nó), [A] o vetor área (área da seção transversal de cada nó) e [k] o vetor rigidez de cada elemento.

Obteve-se, a partir do script apresentado, os seguintes resultados:

$$[F] = \begin{bmatrix} R_1 \\ F_2 \\ F_3 \\ F_4 \\ R_5 \end{bmatrix} = 10^4 \times \begin{bmatrix} -0.8333 \\ 0 \\ 1.0000 \\ 0 \\ -0.1667 \end{bmatrix} N$$

$$[K] = 10^6 \times \begin{bmatrix} 2.51327 & -2.51327 & 0 & 0 & 0 \\ -2.51327 & 4.02124 & -1.50796 & 0 & 0 \\ 0 & -1.50796 & 2.26195 & -0.75398 & 0 \\ 0 & 0 & -0.75398 & 1.00531 & -0.25133 \\ 0 & 0 & 0 & -0.25133 & 0.25133 \end{bmatrix} N/m$$

$$[U] = \begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \\ u_5 \end{bmatrix} = \begin{bmatrix} 0 \\ 0.0033 \\ 0.0088 \\ 0.0066 \\ 0 \end{bmatrix} m$$

$$[f] = 10^3 \times \begin{bmatrix} 8.3333 \\ 8.3333 \\ -1.6667 \\ -1.6667 \end{bmatrix} N$$

sendo [F] o vetor força (carregamento e reações nos nós), [K] a matriz de rigidez global, [U] o vetor deslocamento e [f] o vetor força (tração/compressão nos elementos de mola).

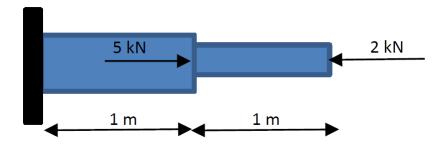


Figura 6: Sistema da questão 5

```
ı clear
2 close all
3 clc
  %% Inputs
7 \text{ F2} = 5000; \% \text{ Force [N]}
8 \text{ F3} = -2000; \% \text{ Force [N]}
9 u1 = 0; % Displacement [m]
10 L1 = 1; % Lenght [m]
11 L2 = 1; % Lenght [m]
12 E = 100*10^9; % Young's Modulus [Pa]
13 A1 = 1*10^{(-4)}; % Area [m<sup>2</sup>]
14 A2 = 2*10^{(-4)}; % Area [m^2]
n = 3; % Degrees of Freedom
   %% Characterization
18
19 M = [1 -1; -1 1];
   % Stiffness
21
22
23 A = [A1; A2];
   x = cumsum([0; L1; L2]);
k = E.*A./(diff(x));
28 \text{ K1} = \text{k(1)} *\text{M};
```

```
_{29} K2 = k(2) *M;
  % Global Matrix Equation
31
32
  K = zeros(n);
  K(1:2,1:2) = K(1:2,1:2) + K1;
  K(2:3,2:3) = K(2:3,2:3) + K2;
  % Force Vector
37
38
  syms F1
  F = [F1; F2; F3];
  % Displacement Vectors
44 syms u2 u3
  U = [u1; u2; u3];
  %% Calculations
48
  AN = solve(F-K*U);
49
  F = double([AN.F1; F2; F3]);
  U = double([u1; AN.u2; AN.u3]);
  f = k.*diff(U);
  clear AN A1 A2 F1 F2 F3 L1 L2 M n u1 u2 u3
```

Obteve-se, a partir do script apresentado, os seguintes resultados:

Item (a)

$$[K_1] = 10^7 \times \left[\begin{array}{cc} 1 & -1 \\ -1 & 1 \end{array} \right] N/m$$

$$[K_2] = 10^7 \times \left[\begin{array}{cc} 2 & -2 \\ -2 & 2 \end{array} \right] N/m$$

sendo $[K_1]$ a matriz de rigidez do elemento de mola 1 e $[K_2]$ a matriz de rigidez do elemento de mola 2.

Item (b)

$$\begin{bmatrix} R1 \\ F2 \\ F3 \end{bmatrix} = \begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix} \begin{bmatrix} u1 \\ u2 \\ u3 \end{bmatrix}$$
$$[F] = \begin{bmatrix} -3000 \\ 5000 \\ -2000 \end{bmatrix} N$$
$$[K] = 10^7 \times \begin{bmatrix} 1 & -1 & 0 \\ -1 & 3 & -2 \\ 0 & -2 & 2 \end{bmatrix} N/m$$
$$[U] = 10^{-3} \times \begin{bmatrix} 0 \\ 0.3000 \\ 0.2000 \end{bmatrix} m$$

sendo [F] o vetor força (carregamento e reações nos nós), [K] a matriz de rigidez global e [U] o vetor deslocamento.

Item (c)

$$[U] = 10^{-3} \times \begin{bmatrix} 0\\0.3000\\0.2000 \end{bmatrix} m$$

sendo [U] o vetor deslocamento nodal.

Item (d)

$$[f] = \begin{bmatrix} 3000 \\ -2000 \end{bmatrix} N$$

sendo [f] o vetor força (tração/compressão nos elementos de mola).

```
ı clear
2 close all
3 clc
5 %% Inputs
7 \text{ F2} = 0; \% \text{ Force [N]}
8 \text{ F3} = 5000; \% \text{ Force [N]}
9 \text{ F4} = 0; % \text{Force [N]}
10 F5 = -2000; % Force [N]
u1 u1 = 0; % Displacement [m]
12 L1 = 1; % Lenght [m]
13 L2 = 1; % Lenght [m]
14 E = 100 \times 10^9; % Young's Modulus [Pa]
15 A1 = 1*10^{(-4)}; % Area [m<sup>2</sup>]
16 \text{ A2} = 2 \times 10^{(-4)}; \% \text{ Area } [\text{m}^2]
n = 5; % Degrees of Freedom
18
19 %% Characterization
21 M = [1 -1; -1 1];
23 % Stiffness
24
25 A = [A1; A1; A2; A2];
  x = cumsum([0; L1/2; L2/2; L2/2]);
28 k = E.*A./(diff(x));
30 K1 = k(1) *M;
31 K2 = k(2) *M;
32 \text{ K3} = \text{k(3)} *\text{M};
33 K4 = k(4) *M;
35 % Global Matrix Equation
36
37 K = zeros(n);
38 K(1:2,1:2) = K(1:2,1:2) + K1;
39 \text{ K}(2:3,2:3) = \text{K}(2:3,2:3) + \text{K}2;
40 \text{ K}(3:4,3:4) = \text{K}(3:4,3:4) + \text{K3};
K(4:5,4:5) = K(4:5,4:5) + K4;
```

```
42
43 % Force Vector
44
45 syms F1
46 F = [F1; F2; F3; F4; F5];
47
48 % Displacement Vectors
49
50 syms u2 u3 u4 u5
51 U = [u1; u2; u3; u4; u5];
52
53 %% Calculations
54
55 AN = solve(F-K*U);
56
57 F = double([AN.F1; F2; F3; F4; F5]);
58 U = double([u1; AN.u2; AN.u3; AN.u4; AN.u5]);
59
60 f = k.*diff(U);
61
62 clear AN A1 A2 F1 F2 F3 F4 F5 L1 L2 M n u1 u2 u3 u4 u5
```

Obteve-se, a partir do script apresentado, os seguintes resultados:

Item (a)

$$[K_{1}] = 10^{7} \times \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix} N/m$$

$$[K_{2}] = 10^{7} \times \begin{bmatrix} 2 & -2 \\ -2 & 2 \end{bmatrix} N/m$$

$$[K_{3}] = 10^{7} \times \begin{bmatrix} 4 & -4 \\ -4 & 4 \end{bmatrix} N/m$$

$$[K_{4}] = 10^{7} \times \begin{bmatrix} 4 & -4 \\ -4 & 4 \end{bmatrix} N/m$$

sendo $[K_i]$ a matriz de rigidez do elemento de mola i.

Item (b)

$$\begin{bmatrix} R1 \\ F2 \\ F3 \\ F4 \\ F5 \end{bmatrix} = \begin{bmatrix} k_1 & -k_1 & 0 & 0 & 0 \\ -k_1 & k_1 + k_2 & -k_2 & 0 & 0 \\ 0 & -k_2 & k_2 + k_3 & -k3 & 0 \\ 0 & 0 & -k_3 & k_3 + k_4 & -k_4 \\ 0 & 0 & 0 & -k_4 & k_4 \end{bmatrix} \begin{bmatrix} u1 \\ u2 \\ u3 \\ u4 \\ u5 \end{bmatrix}$$

$$[F] = \begin{vmatrix} -3000 \\ 0 \\ 5000 \\ 0 \\ -2000 \end{vmatrix} N$$

$$[K] = 10^7 \times \begin{bmatrix} 2 & -2 & 0 & 0 & 0 \\ -2 & 4 & -2 & 0 & 0 \\ 0 & -2 & 6 & -4 & 0 \\ 0 & 0 & -4 & 8 & -4 \\ 0 & 0 & 0 & -4 & 4 \end{bmatrix} N/m$$

$$[U] = 10^{-3} \times \begin{vmatrix} 0 \\ 0.15 \\ 0.3 \\ 0.25 \\ 0.2 \end{vmatrix} m$$

sendo [F] o vetor força (carregamento e reações nos nós), [K] a matriz de rigidez global e [U] o vetor deslocamento.

Item (c)

$$[U] = 10^{-3} \times \begin{bmatrix} 0 \\ 0.15 \\ 0.3 \\ 0.25 \\ 0.2 \end{bmatrix} m$$

sendo [U] o vetor deslocamento nodal.

Item (d)

$$[f] = \begin{bmatrix} 3000 \\ 3000 \\ -2000 \\ -2000 \end{bmatrix} N$$

sendo [f]o vetor força (tração/compressão nos elementos de mola).

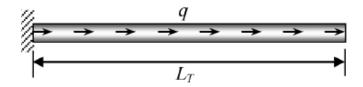


Figura 7: Sistema da questão 7

Script (MATLAB)

Item (a)

```
ı clear
2 close all
3 clc
5 %% Inputs
7 u1 = 0; % Boundary Condition
8 du15dx = 1; % Boundary Condition
p = 1000; % Load [N/m]
10 l = 1.5; % Length [m]
r = 0.1; % Ray [m]
12 E = 207*10^9; % Young's Modulus [Pa]
13 NE = 3; % Number of Elements
14
  %% FEM - Direct Method
15
_{17} ND = NE + 1;
18 M = [1 -1; -1 1];
   % Stiffness
21
22 A = pi*r^2;
X = linspace(0, 1, ND);
L = diff(X);
^{25}
26 k = E \star A./L;
28 \text{ K1} = \text{k(1)} \star \text{M};
29 K2 = k(2) *M;
30 \text{ K3} = \text{k(3)} *\text{M};
```

```
32 % Global Matrix Equation
33
_{34} K = zeros(ND);
35 K(1:2,1:2) = K(1:2,1:2) + K1;
36 \text{ K}(2:3,2:3) = \text{K}(2:3,2:3) + \text{K}2;
37 \text{ K}(3:4,3:4) = \text{K}(3:4,3:4) + \text{K}3;
39 % Force Vector
40
41 syms F1 F2 F3 F4
43 F = [F1; F2; F3; F4];
44 aux = p*X;
45 \text{ F } (2:4) = \text{aux}(2:4);
47 % Displacement Vectors
48
49 syms u2 u3 u4
50 U = [u1; u2; u3; u4];
51
52 % Calculations
AN = solve(F-K*U);
55
56 \text{ F (1)} = \text{AN.F1};
U(2:4) = [AN.u2; AN.u3; AN.u4];
F = double(F);
60 U = double(U);
61
62 % Plots
64 figure
plot(X,F/A,'-ob','MarkerFaceColor','b')
67 figure
68 plot(X,U,'-ob','MarkerFaceColor','b')
70 %% Garlekin's Method
71
72 % Characterization
73
74 syms x F1 u2 u3 u4 du0dx
75 \text{ ND} = \text{NE} + 1;
76
77 X = linspace(0, 1, ND);
```

```
78 L = diff(X);
so for i = 1:ND
81
82 if i == 1
83
84 phi(i,1) = (X(i+1) - x)/L(i);
85 \text{ phi}(i,2) = 0;
86 \text{ phi}(i,3) = 0;
87
88 elseif i == ND
90 phi(i,1) = 0;
91 phi(i,2) = 0;
92 phi(i,3) = (x - X(i-1))/L(i-1);
94 elseif i == 2
95
96 phi(i,1) = (x - X(i-1))/L(i-1);
97 phi(i,2) = (X(i+1) - x)/L(i);
98 phi(i,3) = 0;
100 else
101
102 \text{ phi}(i,1) = 0;
103 phi(i,2) = (x - X(i-1))/L(i-1);
phi(i,3) = (X(i+1) - x)/L(i);
105
106 end
107
108
   end
109
110 % Force
111
|_{112} P = p*X;
113 P = [P(2:4); P(2:4); P(2:4); P(2:4)];
114 f = P.*phi;
\lim_{x \to 0} F = \inf(f(:,1),x,0,0.5) + \inf(f(:,2),x,0.5,1) + \inf(f(:,3),x,1,1.5) + \dots
       du15dx*subs(phi(:,2),x,1.5) - du0dx*subs(phi(:,1),x,0);
_{116} F(1) = F1;
117
118 % Stiffness
119
|_{120} M = [1 -1; -1 1];
121
_{122} A = pi*r^2;
_{123} k = E*A./L;
```

```
124
_{125} K1 = k(1) \starM;
_{126} K2 = k(2) \starM;
_{127} K3 = k(3) \starM;
128
_{129} K = zeros(ND);
130 \text{ K}(1:2,1:2) = \text{K}(1:2,1:2) + \text{K1};
131 \text{ K}(2:3,2:3) = \text{K}(2:3,2:3) + \text{K2};
132 \text{ K}(3:4,3:4) = \text{K}(3:4,3:4) + \text{K3};
133
134 % Displacement
135
136 U = [u1; u2; u3; u4];
137
138 % Calculations
139
140 AN = solve(F - K*U);
_{141} F(1) = AN.F1;
142 \text{ U}(2:4) = [AN.u2; AN.u3; AN.u4];
143
144 % Plot
145
146 figure(1)
147 hold on
148 plot(X,F/A,'-or','MarkerFaceColor','r')
149 legend('Método Direto', 'Método de Garlekin', 'Location', 'southeast')
150 xlabel('Comprimento [m]')
151 ylabel('Tensão [Pa]')
152 grid minor
153
154 figure(2)
155 hold on
plot(X,U,'-or','MarkerFaceColor','r')
157 legend('Método Direto', 'Método de Garlekin','Location','southeast')
158 xlabel('Comprimento Inicial [m]')
ylabel('Deslocamento [m]')
160 grid minor
```

Item (b)

```
1 clear
2 close all
3 clc
4
5 %% Inputs
```

```
7 u1 = 0; % Boundary Condition
8 du15dx = 1; % Boundary Condition
p = 1000; % Load [N/m]
10 l = 1.5; % Length [m]
r = 0.1; % Ray [m]
12 E = 207 \times 10^9; % Young's Modulus [Pa]
14 %% Garlekin's Method (4 Elements)
15
16 % Characterization
18 syms x F1 u2 u3 u4 u5 du0dx
19 NE = 4; % Number of Elements
_{20} ND = NE + 1;
X = linspace(0, 1, ND);
L = diff(X);
25 for i = 1:ND
26
27 if i == 1
29 phi(i,1) = (X(i+1) - x)/L(i);
30 phi(i,2) = 0;
31 phi(i,3) = 0;
32 \text{ phi}(i, 4) = 0;
33
34 elseif i == 2
36 \text{ phi}(i,1) = (x - X(i-1))/L(i-1);
37 phi(i,2) = (X(i+1) - x)/L(i);
38 \text{ phi}(i,3) = 0;
39 phi(i, 4) = 0;
41 elseif i == 3
43 phi(i,1) = 0;
44 phi(i,2) = (x - X(i-1))/L(i-1);
45 phi(i,3) = (X(i+1) - x)/L(i);
46 phi(i, 4) = 0;
47
48 elseif i == 4
50 phi(i,1) = 0;
51 \text{ phi}(i,2) = 0;
52 \text{ phi}(i,3) = (x - X(i-1))/L(i-1);
```

```
53 phi(i,4) = (X(i+1) - x)/L(i);
55 else
56
57 \text{ phi}(i,1) = 0;
58 \text{ phi}(i,2) = 0;
59 phi(i,3) = 0;
60 phi(i,4) = (x - X(i-1))/L(i-1);
62 end
63
64 end
65
66 % Force
68 P = p \star X;
69 P = [P(2:5); P(2:5); P(2:5); P(2:5); P(2:5)];
70 f = P.*phi;
71 F = int(f(:,1),x,X(1),X(2)) + int(f(:,2),x,X(2),X(3)) + ...
       int(f(:,3),x,X(3),X(4)) ...
72 + int(f(:,4),x,X(4),X(5)) + du15dx*subs(phi(:,2),x,1.5) - ...
       du0dx*subs(phi(:,1),x,0);
73 F(1) = F1;
74
75 % Stiffness
77 M = [1 -1; -1 1];
78
79 A = pi * r^2;
80 k = E \star A \cdot / L;
81
82 \text{ K1} = \text{k(1)} *\text{M};
83 K2 = k(2) *M;
84 \text{ K3} = \text{k(3)} *\text{M};
85 \text{ K4} = \text{k(4)} *\text{M};
87 K = zeros(ND);
88 K(1:2,1:2) = K(1:2,1:2) + K1;
89 K(2:3,2:3) = K(2:3,2:3) + K2;
90 K(3:4,3:4) = K(3:4,3:4) + K3;
91 \text{ K}(4:5,4:5) = \text{K}(4:5,4:5) + \text{K4};
92
93 % Displacement
95 U = [u1; u2; u3; u4; u5];
96
97 % Calculations
```

```
98
99 AN = solve(F - K*U);
100 F(1) = AN.F1;
101 \text{ U}(2:5) = [AN.u2; AN.u3; AN.u4; AN.u5];
102
103 % Plot
104
105 figure
plot(X,F/A,'-ob','MarkerFaceColor','b')
107
108 figure
plot (X,U,'-ob','MarkerFaceColor','b')
110
111 %% Garlekin's Method (6 Elements)
112
113 % Characterization
114
115 syms x F1 u2 u3 u4 u5 u6 u7 du0dx
116 NE = 6; % Number of Elements
_{117} ND = NE + 1;
118
X = linspace(0, l, ND);
_{120} L = diff(X);
121
122 for i = 1:ND
123
124 if i == 1
125
phi(i,1) = (X(i+1) - x)/L(i);
_{127} phi(i,2) = 0;
_{128} phi(i,3) = 0;
_{129} phi(i,4) = 0;
130 \text{ phi}(i,5) = 0;
131 phi(i,6) = 0;
132
133 elseif i == 2
134
phi(i,1) = (x - X(i-1))/L(i-1);
phi(i,2) = (X(i+1) - x)/L(i);
137 phi(i,3) = 0;
138 \text{ phi}(i, 4) = 0;
_{139} phi(i,5) = 0;
_{140} phi(i,6) = 0;
141
_{142} elseif i == 3
143
_{144} phi(i,1) = 0;
```

```
phi(i,2) = (x - X(i-1))/L(i-1);
|_{146} \text{ phi}(i,3) = (X(i+1) - x)/L(i);
_{147} phi(i,4) = 0;
_{148} phi(i,5) = 0;
_{149} phi(i,6) = 0;
150
151 elseif i == 4
152
153 phi(i,1) = 0;
_{154} phi(i,2) = 0;
phi(i,3) = (x - X(i-1))/L(i-1);
156 phi(i,4) = (X(i+1) - x)/L(i);
157 \text{ phi}(i,5) = 0;
_{158} phi(i,6) = 0;
159
160 elseif i == 5
161
_{162} phi(i,1) = 0;
_{163} phi(i,2) = 0;
_{164} phi(i,3) = 0;
phi(i,4) = (x - X(i-1))/L(i-1);
_{166} phi(i,5) = (X(i+1) - x)/L(i);
_{167} phi(i,6) = 0;
168
170
171 phi(i,1) = 0;
_{172} phi(i,2) = 0;
_{173} phi(i,3) = 0;
174 \text{ phi}(i,4) = 0;
175 phi(i,5) = (x - X(i-1))/L(i-1);
phi(i,6) = (X(i+1) - x)/L(i);
177
178 else
179
_{180} phi(i,1) = 0;
181 phi(i,2) = 0;
_{182} phi(i,3) = 0;
183 \text{ phi}(i, 4) = 0;
184 phi(i, 5) = 0;
185 phi(i,6) = (x - X(i-1))/L(i-1);
186
187 end
188
189 end
190
191 % Force
```

```
192
_{193} P = p*X;
P = [P(2:7); P(2:7); P(2:7); P(2:7); P(2:7); P(2:7); P(2:7); P(2:7)];
195 f = P.*phi;
F = int(f(:,1),x,X(1),X(2)) + int(f(:,2),x,X(2),X(3)) + ...
        int(f(:,3),x,X(3),X(4)) ...
_{197} + int(f(:,4),x,X(4),X(5)) + int(f(:,5),x,X(5),X(6)) + ...
        int(f(:,6),x,X(6),X(7)) ...
198 + du15dx*subs(phi(:,2),x,1.5) - du0dx*subs(phi(:,1),x,0);
_{199} F(1) = F1;
200
201 % Stiffness
202
_{203} M = [1 -1; -1 1];
204
_{205} A = pi*r^2;
206 \text{ k} = \text{E} * \text{A./L};
207
_{208} K1 = k(1) \starM;
_{209} K2 = k(2) \starM;
_{210} K3 = k(3) \starM;
_{211} K4 = k(4) *M;
_{212} K5 = k(5) *M;
_{213} K6 = k(6) *M;
214
_{215} K = zeros(ND);
216 \text{ K}(1:2,1:2) = \text{K}(1:2,1:2) + \text{K1};
217 \text{ K}(2:3,2:3) = \text{K}(2:3,2:3) + \text{K}2;
_{218} K(3:4,3:4) = K(3:4,3:4) + K3;
219 \text{ K}(4:5,4:5) = \text{K}(4:5,4:5) + \text{K4};
220 \text{ K}(5:6,5:6) = \text{K}(5:6,5:6) + \text{K5};
221 \text{ K}(6:7,6:7) = \text{K}(6:7,6:7) + \text{K6};
222
223 % Displacement
224
225 U = [u1; u2; u3; u4; u5; u6; u7];
^{226}
227 % Calculations
228
229 AN = solve(F - K*U);
_{230} F(1) = AN.F1;
231 U(2:7) = [AN.u2; AN.u3; AN.u4; AN.u5; AN.u6; AN.u7];
232
233 % Plot
234
235 figure (1)
236 hold on
```

```
plot(X,F/A,'-or','MarkerFaceColor','r')
238
239 figure (2)
240 hold on
plot (X,U,'-or','MarkerFaceColor','r')
242
243 %% Garlekin's Method (8 Elements)
244
245 % Characterization
246
247 syms x F1 u2 u3 u4 u5 u6 u7 u8 u9 du0dx
248 NE = 8; % Number of Elements
_{249} ND = NE + 1;
250
X = linspace(0, 1, ND);
_{252} L = diff(X);
253
_{254} for i = 1:ND
255
256 if i == 1
257
258 phi(i,1) = (X(i+1) - x)/L(i);
_{259} phi(i,2) = 0;
_{260} phi(i,3) = 0;
_{261} phi(i,4) = 0;
_{262} phi(i,5) = 0;
_{263} phi(i,6) = 0;
_{264} phi(i,7) = 0;
_{265} phi(i,8) = 0;
266
267 elseif i == 2
268
269 phi(i,1) = (x - X(i-1))/L(i-1);
270 \text{ phi}(i,2) = (X(i+1) - x)/L(i);
_{271} phi(i,3) = 0;
_{272} phi(i,4) = 0;
_{273} phi(i,5) = 0;
_{274} phi(i,6) = 0;
_{275} phi(i,7) = 0;
_{276} phi(i,8) = 0;
277
278 elseif i == 3
279
_{280} phi(i,1) = 0;
281 phi(i,2) = (x - X(i-1))/L(i-1);
_{282} phi(i,3) = (X(i+1) - x)/L(i);
_{283} phi(i,4) = 0;
```

```
_{284} phi(i,5) = 0;
_{285} phi(i,6) = 0;
_{286} phi(i,7) = 0;
_{287} phi(i,8) = 0;
289 elseif i == 4
290
_{291} phi(i,1) = 0;
_{292} phi(i,2) = 0;
293 phi(i,3) = (x - X(i-1))/L(i-1);
_{294} phi(i,4) = (X(i+1) - x)/L(i);
_{295} phi(i,5) = 0;
_{296} phi(i,6) = 0;
_{297} phi(i,7) = 0;
_{298} phi(i,8) = 0;
300 elseif i == 5
301
302 \text{ phi}(i,1) = 0;
303 \text{ phi}(i,2) = 0;
304 \text{ phi}(i,3) = 0;
305 phi(i,4) = (x - X(i-1))/L(i-1);
306 phi(i,5) = (X(i+1) - x)/L(i);
307 \text{ phi}(i,6) = 0;
308 \text{ phi}(i,7) = 0;
309 \text{ phi}(i,8) = 0;
310
311 elseif i == 6
312
_{313} phi(i,1) = 0;
|_{314} phi(i,2) = 0;
_{315} phi(i,3) = 0;
_{316} phi(i,4) = 0;
_{317} phi(i,5) = (x - X(i-1))/L(i-1);
318 phi(i,6) = (X(i+1) - x)/L(i);
_{319} phi(i,7) = 0;
_{320} phi(i,8) = 0;
321
322 elseif i == 7
323
324 \text{ phi}(i,1) = 0;
|_{325} phi(i,2) = 0;
_{326} phi(i,3) = 0;
327 \text{ phi}(i,4) = 0;
|328 \text{ phi}(i,5) = 0;
|_{329} phi(i,6) = (x - X(i-1))/L(i-1);
330 phi(i,7) = (X(i+1) - x)/L(i);
```

```
_{331} phi(i,8) = 0;
332
333 elseif i == 8
334
335 \text{ phi}(i,1) = 0;
336 \text{ phi}(i,2) = 0;
337 \text{ phi}(i,3) = 0;
338 \text{ phi}(i,4) = 0;
339 phi(i,5) = 0;
_{340} phi(i,6) = 0;
phi(i,7) = (x - X(i-1))/L(i-1);
_{342} phi(i,8) = (X(i+1) - x)/L(i);
343
344 else
345
_{346} phi(i,1) = 0;
_{347} phi(i,2) = 0;
_{348} phi(i,3) = 0;
349 \text{ phi}(i,4) = 0;
350 \text{ phi}(i,5) = 0;
351 phi(i,6) = 0;
_{352} phi(i,7) = 0;
353 phi(i,8) = (x - X(i-1))/L(i-1);
354
355 end
356
357 end
358
359 % Force
360
_{361} P = p*X;
_{362} P = [P(2:9); P(2:9); P(2:9); P(2:9); P(2:9); P(2:9); P(2:9); P(2:9); ...
       P(2:9)];
363 f = P.*phi;
364 \text{ F} = \text{int}(f(:,1),x,X(1),X(2)) + \text{int}(f(:,2),x,X(2),X(3)) + \dots
       int(f(:,3),x,X(3),X(4)) ...
_{365} + int(f(:,4),x,X(4),X(5)) + int(f(:,5),x,X(5),X(6)) + ...
       int(f(:,6),x,X(6),X(7)) ...
366 + int(f(:,7),x,X(7),X(8)) + int(f(:,8),x,X(8),X(9)) + ...
       du15dx*subs(phi(:,2),x,1.5) ...
367 - du0dx*subs(phi(:,1),x,0);
_{368} F(1) = F1;
369
370 % Stiffness
371
_{372} M = [1 -1; -1 1];
373
```

```
_{374} A = pi * r^2;
375 \text{ k} = \text{E} * \text{A./L};
376
_{377} K1 = k(1) \starM;
378 \text{ K2} = \text{k(2)} \star \text{M};
_{379} K3 = k(3) \starM;
|_{380} K4 = k(4) \starM;
381 \text{ K5} = \text{k(5)} *\text{M};
_{382} K6 = k(6) *M;
383 \text{ K7} = \text{k}(7) *\text{M};
_{384} K8 = k(8) \starM;
385
386 K = zeros(ND);
387 \text{ K}(1:2,1:2) = \text{K}(1:2,1:2) + \text{K1};
388 \text{ K}(2:3,2:3) = \text{K}(2:3,2:3) + \text{K2};
389 \text{ K}(3:4,3:4) = \text{K}(3:4,3:4) + \text{K3};
390 \text{ K}(4:5,4:5) = \text{K}(4:5,4:5) + \text{K4};
_{391} K(5:6,5:6) = K(5:6,5:6) + K5;
_{392} K(6:7,6:7) = K(6:7,6:7) + K6;
393 K(7:8,7:8) = K(7:8,7:8) + K7;
_{394} K(8:9,8:9) = K(8:9,8:9) + K8;
395
396 % Displacement
397
398 U = [u1; u2; u3; u4; u5; u6; u7; u8; u9];
399
400 % Calculations
401
402 AN = solve(F - K*U);
403 \text{ F (1)} = \text{AN.F1};
404 U(2:9) = [AN.u2; AN.u3; AN.u4; AN.u5; AN.u6; AN.u7; AN.u8; AN.u9];
405
406 % Plot
407
408 figure(1)
409 hold on
410 plot(X,F/A,'-og','MarkerFaceColor','g')
411
412 figure(2)
413 hold on
414 plot(X,U,'-og','MarkerFaceColor','g')
415
416 %% Garlekin's Method (10 Elements)
417
418 % Characterization
419
420 syms x F1 u2 u3 u4 u5 u6 u7 u8 u9 u10 u11 du0dx
```

```
421 NE = 10; % Number of Elements
_{422} ND = NE + 1;
423
_{424} X = linspace(0,1,ND);
_{425} L = diff(X);
426
_{427} for i = 1:ND
428
429 if i == 1
430
431 phi(i,1) = (X(i+1) - x)/L(i);
432 \text{ phi}(i,2) = 0;
433 \text{ phi}(i,3) = 0;
434 \text{ phi}(i, 4) = 0;
435 \text{ phi}(i,5) = 0;
436 \text{ phi}(i,6) = 0;
437 \text{ phi}(i,7) = 0;
_{438} phi(i,8) = 0;
_{439} phi(i,9) = 0;
_{440} phi(i,10) = 0;
441
442 elseif i == 2
444 phi(i,1) = (x - X(i-1))/L(i-1);
_{445} phi(i,2) = (X(i+1) - x)/L(i);
_{446} phi(i,3) = 0;
_{447} phi(i,4) = 0;
|448 \text{ phi}(i,5) = 0;
_{449} phi(i,6) = 0;
_{450} phi(i,7) = 0;
_{451} phi(i,8) = 0;
_{452} phi(i,9) = 0;
_{453} phi(i,10) = 0;
454
455 elseif i == 3
456
_{457} phi(i,1) = 0;
458 phi(i,2) = (x - X(i-1))/L(i-1);
459 phi(i,3) = (X(i+1) - x)/L(i);
_{460} phi(i,4) = 0;
_{461} phi(i,5) = 0;
_{462} phi(i,6) = 0;
_{463} phi(i,7) = 0;
_{464} phi(i,8) = 0;
_{465} phi(i,9) = 0;
_{466} phi(i,10) = 0;
467
```

```
468 elseif i == 4
470 \text{ phi}(i,1) = 0;
_{471} phi(i,2) = 0;
472 \text{ phi}(i,3) = (x - X(i-1))/L(i-1);
473 phi(i,4) = (X(i+1) - x)/L(i);
474 \text{ phi}(i,5) = 0;
_{475} phi(i,6) = 0;
_{476} phi(i,7) = 0;
|477 \text{ phi}(i, 8) = 0;
_{478} phi(i,9) = 0;
_{479} phi(i,10) = 0;
480
481 elseif i == 5
482
|_{483} phi(i,1) = 0;
|484 \text{ phi}(i,2) = 0;
_{485} phi(i,3) = 0;
486 \text{ phi}(i,4) = (x - X(i-1))/L(i-1);
487 phi(i,5) = (X(i+1) - x)/L(i);
488 \text{ phi}(i,6) = 0;
_{489} phi(i,7) = 0;
_{490} phi(i,8) = 0;
_{491} phi(i,9) = 0;
|_{492} phi(i,10) = 0;
494 elseif i == 6
495
_{496} phi(i,1) = 0;
_{497} phi(i,2) = 0;
_{498} phi(i,3) = 0;
|_{499} phi(i,4) = 0;
500 phi(i,5) = (x - X(i-1))/L(i-1);
phi(i,6) = (X(i+1) - x)/L(i);
502 \text{ phi}(i,7) = 0;
503 \text{ phi}(i,8) = 0;
504 \text{ phi}(i,9) = 0;
505 \text{ phi}(i, 10) = 0;
506
507 elseif i == 7
508
509 \text{ phi}(i,1) = 0;
510 \text{ phi}(i,2) = 0;
_{511} phi(i,3) = 0;
|_{512} phi(i,4) = 0;
513 \text{ phi}(i,5) = 0;
phi(i,6) = (x - X(i-1))/L(i-1);
```

```
_{515} phi(i,7) = (X(i+1) - x)/L(i);
516 \text{ phi}(i,8) = 0;
|_{517} phi(i,9) = 0;
_{518} phi(i,10) = 0;
519
520 elseif i == 8
521
522 \text{ phi}(i,1) = 0;
523 phi(i,2) = 0;
524 \text{ phi}(i,3) = 0;
525 \text{ phi}(i, 4) = 0;
526 \text{ phi}(i,5) = 0;
527 phi(i,6) = 0;
528 \text{ phi}(i,7) = (x - X(i-1))/L(i-1);
_{529} phi(i,8) = (X(i+1) - x)/L(i);
530 \text{ phi}(i, 9) = 0;
531 \text{ phi}(i, 10) = 0;
532
533 elseif i == 9
534
535 \text{ phi}(i,1) = 0;
536 \text{ phi}(i,2) = 0;
537 phi(i,3) = 0;
538 \text{ phi}(i, 4) = 0;
_{539} phi(i,5) = 0;
540 \text{ phi}(i, 6) = 0;
541 \text{ phi}(i,7) = 0;
_{542} phi(i,8) = (x - X(i-1))/L(i-1);
_{543} phi(i,9) = (X(i+1) - x)/L(i);
|_{544} phi(i,10) = 0;
545
546 elseif i == 10
547
548 \text{ phi}(i,1) = 0;
549 phi(i,2) = 0;
550 \text{ phi}(i,3) = 0;
551 \text{ phi}(i,4) = 0;
552 \text{ phi}(i,5) = 0;
553 \text{ phi}(i,6) = 0;
554 \text{ phi}(i,7) = 0;
555 \text{ phi}(i,8) = 0;
phi(i,9) = (x - X(i-1))/L(i-1);
| \text{557 phi}(i,10) = (X(i+1) - x)/L(i);
558
559 else
560
561 \text{ phi}(i,1) = 0;
```

```
_{562} phi(i,2) = 0;
563 \text{ phi}(i,3) = 0;
_{564} phi(i,4) = 0;
_{565} phi(i,5) = 0;
566 \text{ phi}(i,6) = 0;
_{567} phi(i,7) = 0;
568 \text{ phi}(i, 8) = 0;
_{569} phi(i,9) = 0;
phi(i,10) = (x - X(i-1))/L(i-1);
571
572 end
573
574 end
575
576 % Force
577
578 P = p*X;
p_{79} P = [P(2:11); P(2:11); P(2:11); P(2:11); P(2:11); P(2:11); P(2:11); \dots]
                      P(2:11); ...
580 P(2:11); P(2:11); P(2:11)];
581 f = P.*phi;
F = int(f(:,1),x,X(1),X(2)) + int(f(:,2),x,X(2),X(3)) + ...
                       int(f(:,3),x,X(3),X(4)) ...
583 + int(f(:,4),x,X(4),X(5)) + int(f(:,5),x,X(5),X(6)) + ...
                       int(f(:,6),x,X(6),X(7)) ...
_{584} + int(f(:,7),x,X(7),X(8)) + int(f(:,8),x,X(8),X(9)) + ...
                       int(f(:,9),x,X(9),X(10)) ...
f(x) = f(x) + int(f(x), f(x), f(x), f(x)) + f(x) + int(f(x), f(x), f(x)) + f(x) + f(
                       du0dx*subs(phi(:,1),x,0);
586 \text{ F}(1) = \text{F1};
587
588 % Stiffness
589
_{590} M = [1 -1; -1 1];
591
592 A = pi * r^2;
593 k = E*A./L;
594
595 \text{ K1} = \text{k(1)} *\text{M};
596 \text{ K2} = \text{k(2)} *\text{M};
597 \text{ K3} = \text{k(3)} *\text{M};
598 \text{ K4} = \text{k(4)} * \text{M};
599 \text{ K5} = \text{k(5)} *\text{M};
600 \text{ K6} = \text{k(6)} *\text{M};
601 \text{ K7} = \text{k}(7) * \text{M};
_{602} K8 = k(8) \starM;
603 \text{ K9} = \text{k(9)} \star \text{M};
```

```
|_{604} K10 = k(10) *M;
605
_{606} K = zeros(ND);
607 \text{ K}(1:2,1:2) = \text{K}(1:2,1:2) + \text{K1};
608 \text{ K}(2:3,2:3) = \text{K}(2:3,2:3) + \text{K2};
609 \text{ K}(3:4,3:4) = \text{K}(3:4,3:4) + \text{K3};
610 \text{ K}(4:5,4:5) = \text{K}(4:5,4:5) + \text{K4};
K(5:6,5:6) = K(5:6,5:6) + K5;
\kappa_{612} \text{ K}(6:7,6:7) = \text{K}(6:7,6:7) + \text{K6};
613 \text{ K}(7:8,7:8) = \text{K}(7:8,7:8) + \text{K7};
614 \text{ K}(8:9,8:9) = \text{K}(8:9,8:9) + \text{K8};
615 \text{ K}(9:10,9:10) = \text{K}(9:10,9:10) + \text{K}9;
616 \text{ K}(10:11,10:11) = \text{K}(10:11,10:11) + \text{K}10;
617
618 % Displacement
619
620 U = [u1; u2; u3; u4; u5; u6; u7; u8; u9; u10; u11];
621
622 % Calculations
623
_{624} AN = solve(F - K*U);
625 \text{ F}(1) = \text{AN.F1};
626 U(2:11) = [AN.u2; AN.u3; AN.u4; AN.u5; AN.u6; AN.u7; AN.u8; AN.u9; ...
        AN.u10; AN.u11];
627
628 % Plot
629
630 figure(1)
631 hold on
632 plot(X,F/A,'-oc','MarkerFaceColor','c')
legend('4 Elementos', '6 Elementos', '8 Elementos', '10 ...
        Elementos','Location','southeast')
634 xlabel('Comprimento [m]')
635 ylabel('Tensão [Pa]')
636 grid minor
637
638 figure(2)
639 hold on
640 plot(X,U,'-oc','MarkerFaceColor','c')
641 legend('4 Elementos', '6 Elementos', '8 Elementos', '10 ...
        Elementos','Location','southeast')
642 xlabel('Comprimento Inicial [m]')
643 ylabel('Deslocamento [m]')
644 grid minor
```

Item (a)

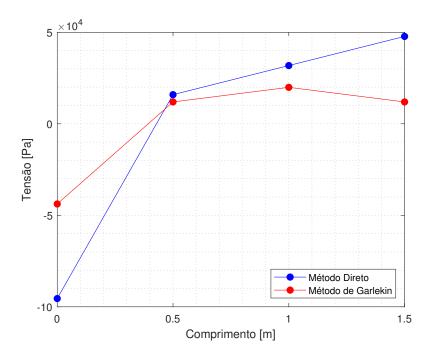


Figura 8: Tensões na barra

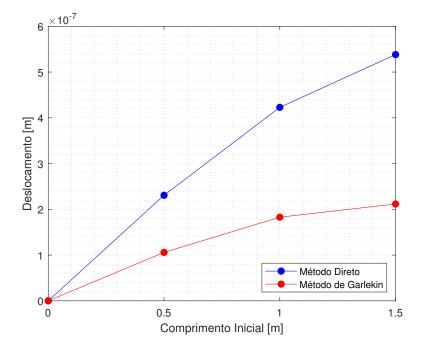


Figura 9: Deslocamentos dos nós

Item (b)

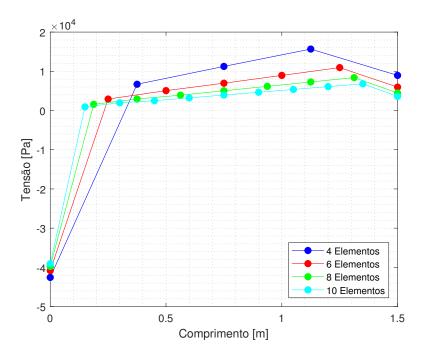


Figura 10: Comparativo de tensões na barra

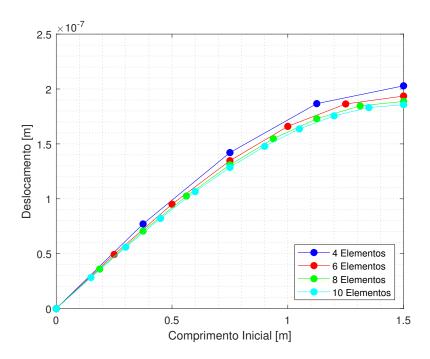


Figura 11: Comparativo de deslocamentos dos nós