The BAR Element

I. Stiffness Matrix

The equations for the BAR element are

Define:
$$e_a = AE$$
, $k_t = AT$, $B = 12ET_{12}$

$$k_t = \frac{1}{4}R_1 + \frac{ET}{L}$$
, $k_t = \frac{1}{4}R_2 + \frac{ET}{L}$, $k_t = \frac{1}{4}R_1 - \frac{ET}{L}$, $k_t = \frac{1}{4}R_2 - \frac{ET}{L}$

When

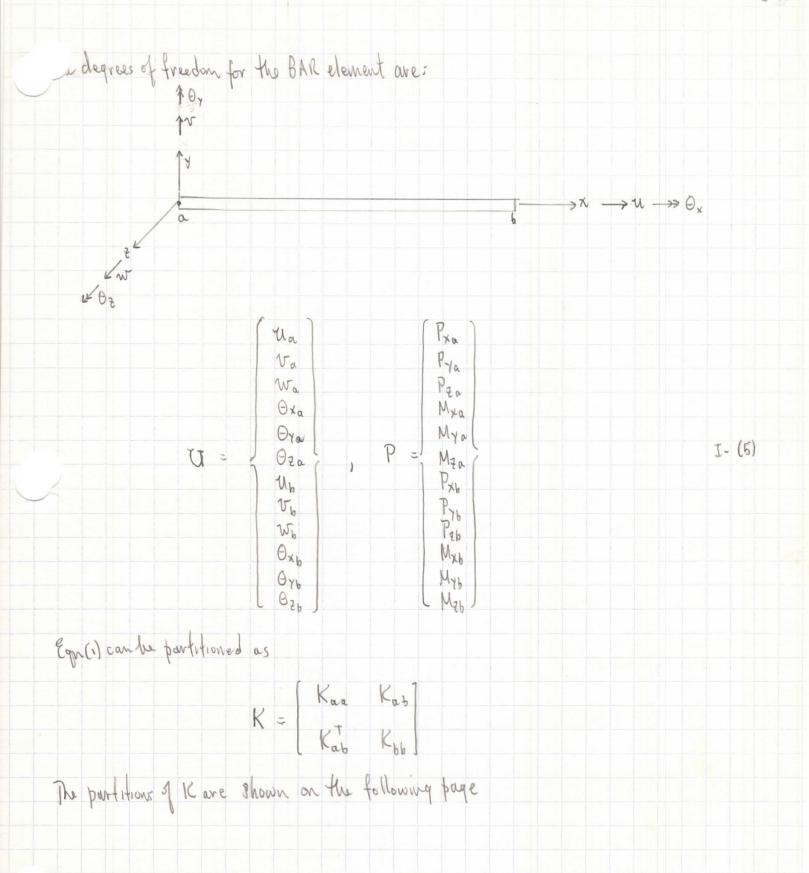
$$R_{2} = \frac{12EI_{1}}{2^{3}} \left(1 + V_{1}\right)^{-1}$$

$$R_{2} = \frac{12EI_{2}}{2^{3}} \left(1 + V_{2}\right)^{-1}$$

and

$$V_2 = \begin{cases} \frac{12EI_2}{K_2AEL^2} & \text{if } I_{12} = 0 \text{ and } K_2 \neq 0 \\ 0 & \text{if } I_{12} \neq 0 \text{ or } K_2 = 0 \end{cases}$$

Oarl K, Kz aro the area factors for shear (a zero value is assigned to Y, Yz when K, at Kz are zero which is interpreted to be the case of no shear flexibility (which is actually K_1 , $K_2 \rightarrow \infty$).



	Ца	Va	Wa	Oxa	Gra	O 2a	Ub	Vb	Who	Oxb	Оуь	Oah	
	ka	-0	<u> </u>	0	0	0	-ka	0	0	0	()	6	J.
	0	R	β	2	- E (b	& Ry		-12,	- B	0	-1B	ER,	Va
	0	P	R2	0	- LR2	\$ B		-6	-R2	- 6	- 2 Rz	£ 6	Wa
	Q	0	-0	ke	0	- 0	0		0	-kt	Ù	. 0	O+a
	()	-8 B	- \$ 12 -	9	ks	-2- h		2 B	2 82	0	ky	- l2 6	Bya
	9	&R,	1 p	0	- lz h	Ĵe,		- l R,	- l h	9	- l2 (h	kz	Kaa Kab
K = .	-ka	0		- 0			ka	0		- 0		U	Ub Kab Kbb
	0	- R,	-6	- 10	& A	- gr.		R.	ß	0	& h	-12,	V6 I- (6)
	0	- îp	-Rx	0	& Rz	-1 6		6	Ra		2 R2	- lg fs	Wb
		0	0	- let	. 0	0	0	(6)		kt			θ_{χ_b}
	0	-26	-1 Rz	0	Tex	-l2 6		\$ 12	2 82		ha	- l2 h	Θ _{Yb}
	<u>Q</u>	& R.	2 fs	Ci-	- 82 B	kz		- g R,	-2B	(i	-27 6	k,	026

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