
Mechanics

Bolton's Engineering

Lecture 7

22/10/25



Summary of linearized equilibrium theory for linear elasticity (beam)

$$\begin{cases} M' + T + m = 0 \\ N' + f_1 = 0 \\ T' + f_2 = 0 \end{cases}$$

$$\begin{cases} T = -M' - m \\ M'' = -m' + f_2 \\ N' = -f_1 \end{cases}$$

$$N = EA u'$$

$$M = EI w''$$

+ static BCs (M, N, T) + kinematic BCs (u, w, w')

5 linear (!) ODEs in the 5 unknown functions M, N, T, u, w .

More compact formulation.

$$T = -M' - m = -EI w''' - m$$

$$\begin{cases} EA u'' = -f_1 \\ EI w''' = -m' + f_2 \end{cases} \quad (*) \quad + \text{ static \& kinematic BC's on } \begin{matrix} u, u' \\ w, w', w'', w''' \end{matrix}$$

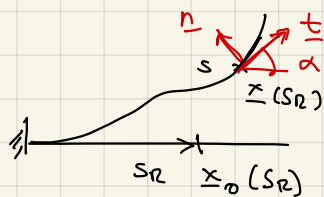
2 linear (!) ODEs in the unknown functions u, w .

(*) is known as "equazione delle linee elastiche"

Important Remark

In the linear theory, elastic displacements are small perturbations of the reference configuration. Equilibrium equations are written in the reference configuration.

General scheme of equilibrium problem



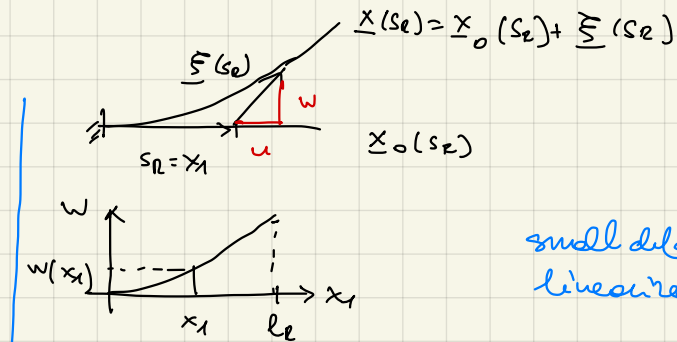
Kinematics

$$\alpha'(s_R) = k(s_R) + \frac{c_0}{\epsilon} \quad \text{In this case (straight ref. conf.)}$$

$$\underline{x}'(s_R) = (1 + \epsilon(s_R)) (\cos \alpha(s_R) \underline{e}_1 + \sin \alpha(s_R) \underline{e}_2) \quad \underline{t}(s_R)$$

+ kinematic BC's on $\alpha(0)$ and $\underline{x}(0)$

→ recover α, \underline{x} by integration from ϵ, k
 ↳ placement of the rod's axis



small deformation
linearized theory

$$\begin{aligned} \epsilon &= u' \\ \alpha &= w' \\ k &= w'' \end{aligned}$$

+ kinematic BC's on $u(0), w(0), w'(0)$

Equilibrium

$$\begin{cases} M_R' + (1 + \epsilon) T_R + m_R = 0 \\ N_R' - (c_0 + k) T_R + f_{Rt} = 0 \\ T_R' + (c_0 + k) N_R + f_{Rn} = 0 \end{cases}$$

where

$$\underline{R}_R = N_R \underline{t} + T_R \underline{n}$$

$$\underline{f}_R = f_{Rt} \underline{t} + f_{Rn} \underline{n}$$

+ static BC's $M(l), T(l), N(l)$

$$\begin{cases} M' + T + m = 0 \\ N' + f_1 = 0 \\ T' + f_2 = 0 \end{cases}$$

can rewrite as
 ↓

where

$$\underline{R}_R = N_R \underline{e}_1 + T_R \underline{e}_2$$

$$\underline{f}_R = f_1 \underline{e}_1 + f_2 \underline{e}_2$$

$$\begin{cases} T = -M' - m \\ M'' = -m' - T' = -m' + f_2 \\ N' = -f_1 \end{cases}$$

+ static BC's $M(l), N(l), T(l), \dots$

$$N_R = \tilde{N}(\epsilon, k), M_R = \tilde{M}(\epsilon, k)$$

Use for non linear dependence of N_R on ϵ (resp. M_R on k) as in linear theory. More later.

Constitutive equations

$$N_R = EA \epsilon$$

$$M_R = EI k$$

$$N = EA u'$$

$$M = EI w''$$

In summary

$$8 = \underbrace{2}_{x'} + \underbrace{1}_{\alpha'} + \underbrace{3}_{\text{const.}} + \underbrace{2}_{\text{equal}}$$

$\underbrace{x_1, x_2, \alpha}_{3 \text{ geom.}}$ $\underbrace{M_e, N_e, T_e}_{3 \text{ int. forces}}$ $\underbrace{\epsilon, \kappa}_{2 \text{ strains}}$

[Non-linear problem]

ODEs
in two unknown functions

$$5 = \underbrace{3}_{\text{const.}} + \underbrace{2}_{\text{equal}}$$

$\underbrace{M, N, T}_{3 \text{ int. forces}}$ $\underbrace{u, w}_{2 \text{ perturbation displacements}}$

[linear equations]

More compact formulation

$$T = -M' - m = -EI w''' - m$$

$$\begin{cases} EA u'' = -f_1 \\ EI w''' = -m' + f_2 \end{cases}$$

2 linear ODEs in two unknown functions u, w
Once u, w are known can recover M, N from const. equations and T from equilibrium.

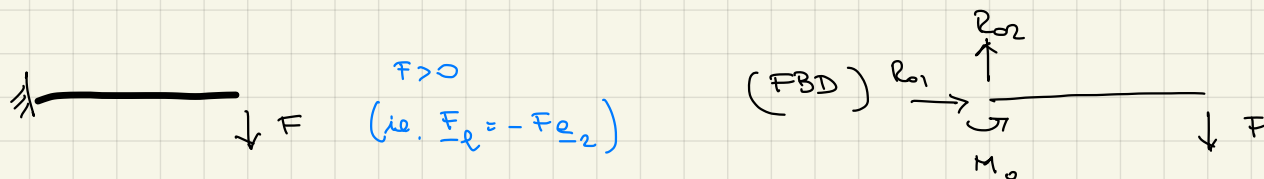
Solving equilibrium problems in the linear theory (HPP of the initial/reference configuration)

The BCs for u, N, T come from statics. Depending on whether one end is free or constrained we know u, N, T at that end or not. For example, at $s_R = 0$

clump	hinge	roller	free-end
$u(0) = w(0) = 0$	$u(0) = w(0) = 0$	$u(0) ? w(0) = 0$	$u(0) ? w(0) ?$
$\alpha(0) = w'(0) = 0$	$\alpha(0) = w'(0) ?$	$\alpha(0) = w'(0) ?$	$\alpha(0) = w'(0) ?$
$\underline{F}_0 \cdot \underline{e}_1 = -N(0) ?$	$\underline{F}_0 \cdot \underline{e}_1 ?$	$\underline{F}_0 \cdot \underline{e}_1 = -N(0) = 0$	$\underline{F}_0 \cdot \underline{e}_1 = -N(0)$
$\underline{F}_0 \cdot \underline{e}_2 = -T(0) ?$	$\underline{F}_0 \cdot \underline{e}_2 ?$	$\underline{F}_0 \cdot \underline{e}_2 = -T(0) ?$	$\underline{F}_0 \cdot \underline{e}_2 = -T(0)$
$M_0 = -M(0) ?$	$M(0) = 0$	$M(0) = 0$	$M_0 = -M(0)$

Remark Observe the "complementarity" between kinematic and static conditions in the prescription of constraints. For non-dissipative constraints, this is always the case.

Isostatic case (Free Body Diagrams FBD)



3 scalar constraints suppress all rigid DoFs, 3 scalar unknown reactions

$$3 \text{ global equil. equations } \begin{cases} \underline{F} = 0 \\ M_0 = 0 \end{cases} \Rightarrow \begin{cases} R_1 = -N(0) = 0 \\ R_2 - F = 0 \Rightarrow R_2 = -T(0) = -F \\ M_0 - Fl = 0 \Rightarrow -M_0 = M(0) = -Fl \end{cases}$$

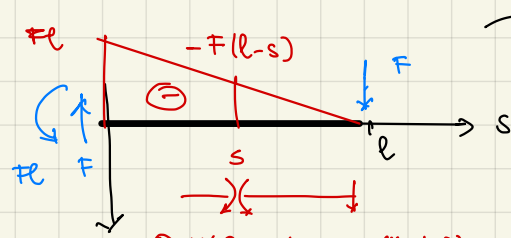
$\Rightarrow M, N, T$ can be determined by statics alone. Can integrate equil. eqn's directly, using $N(l) = 0, T(l) = -F, M(l) = 0$.

$$\begin{cases} N'(s) = 0 \\ T'(s) = 0 \\ M'(s) = -T(s) = +F \end{cases} \rightarrow \begin{cases} 0 = \int_s^l N'(\sigma) d\sigma = N(l) - N(s) \rightarrow N(s) \equiv N(l) = 0 \\ \rightarrow T(s) \equiv T(l) = -F \\ \rightarrow M(l) - M(s) = F(l-s) \rightarrow M(s) = -F(l-s) \end{cases}$$

$[T(0) = -F \checkmark]$
 $[M(0) = -Fl \checkmark]$

Diagrams of internal forces (convention for $s \rightarrow M(s)$ diagram) ① → ② → ③

$$T = -M' \quad \text{or} \quad T = -M'$$



① $M(s) = -M'(s) = M'_s(s, l)$
by equilibrium of (s, l)



$$T(s) \equiv -F$$

①

② $M' = -T - \gamma \Rightarrow M(s)$ affine

①+② can be replaced by $M'' = -\gamma' + \gamma_l = 0 \Rightarrow M(s)$ affine

② positive moment put bottom!
fibers in tension

Assuming linear constitutive equations hold, we integrate them to recover the small elastic perturbations of the equilibrium configuration (the initial or reference configuration) caused by the loads

$$u' = \frac{N}{EA} \rightarrow u(s) = u(0) + 0 = 0$$

$$w'' = \frac{M}{EI} \rightarrow w'(s) = \underbrace{w'(0)}_{=0} + \int_0^s \frac{F(\sigma-l)}{EI} d\sigma = \frac{F}{EI} \left(\frac{(\sigma-l)^2}{2} \right) \Big|_0^s$$

$$= \frac{F}{EI} \left(\frac{s^2}{2} - \cancel{\frac{2sl}{2}} + \frac{l^2}{2} - \cancel{\frac{l^2}{2}} \right)$$

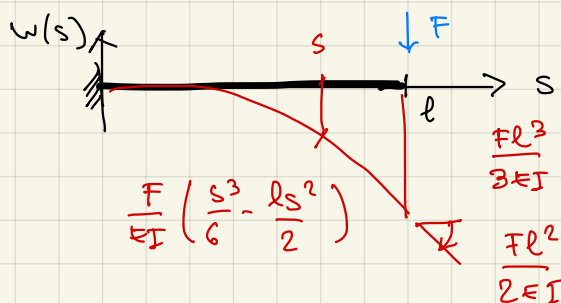
$$w(s) = \underbrace{w(0)}_{=0} + \frac{F}{EI} \int_0^s \left(\frac{\sigma^2}{2} - l\sigma \right) d\sigma = \frac{F}{EI} \left(\frac{s^3}{6} - \frac{l s^2}{2} \right)$$

$$w(l) = \frac{F}{EI} \left(\frac{l^3}{6} - \frac{l^3}{2} \right) = -\frac{1}{3} \frac{Fl^3}{EI}$$

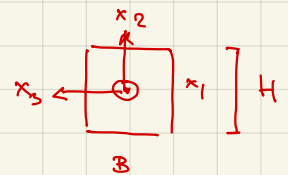
$$w'(l) \stackrel{w(l)}{=} \alpha(l) = \frac{F}{EI} \left(\frac{l^2}{2} - l^2 \right) = -\frac{Fl^2}{2EI}$$

Important values, check dimensions
why $[EI] = FL^2L^4 = FL^2$

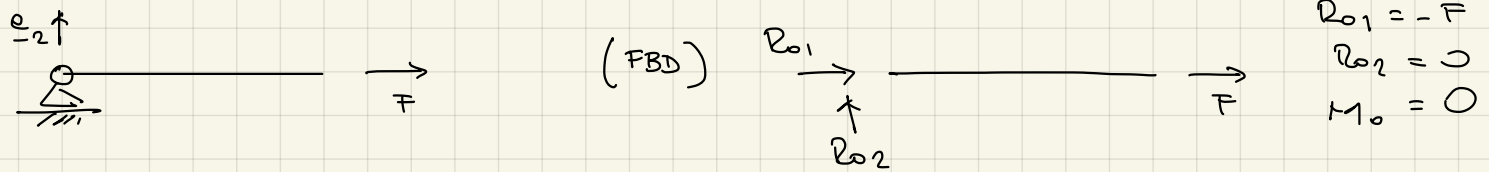
Graph of the transversal displacements (a cubic)



$$I = \int_{-B/2}^{B/2} \int_{-H/2}^{H/2} x_2^2 dx_3 dx_2 = \frac{BH^3}{12}$$



Exercise 2 Force $\underline{F} = F \underline{e}_1$ at the end l of a beam supported at $s = 0$



$$\begin{aligned} R_{01} &= -F \\ R_{02} &= 0 \\ M_0 &= 0 \end{aligned}$$

Equilibrium $\underline{\dot{z}} = 0$, $M_0 = 0$ is solved in ref. config. and we have enough BC's to integrate equl. equations. We have a free end: $N(l) = F$, $T(l) = 0$, $M(l) = 0$

$$\int_s^l N' = 0 \Rightarrow N(l) - N(s) = 0 \Rightarrow N(s) \equiv N(l) = F$$

$$\int_s^l T' = 0 \Rightarrow T(l) - T(s) = 0 \Rightarrow T(s) \equiv T(l) = 0$$

$$\int_s^l M' = 0 \Rightarrow M(l) - M(s) = 0 \Rightarrow M(s) \equiv M(l) = 0$$

(special affine function, $\equiv 0$)

With linear constitutive equations we can calculate u , w

$$u' = \frac{N}{EA} = \frac{F}{EA} \rightarrow u(s) = \underbrace{u(0)}_{=0} + \frac{F}{EA} s$$

$$u(l) = \frac{F}{EA} l$$

$$w'' = \frac{M}{EI} = 0 \rightarrow w(s) = a + bs$$

$$w(0) = a = 0$$

$$w(s) = bs, \quad b \text{ arbitrary}$$

$$b = w'(0) \quad \underline{\text{arbitrary}}$$

arbitrary small rotation allowed (!)

Exercise 3 Force $\underline{F}_e = -F \underline{e}_2$ at the end l of a beam supported at $s=0$



(FBD)



$$\begin{aligned} R_{01} &= 0 \\ R_{02} &= F \\ M_0 &= 0 \end{aligned}$$

$$[\underline{x}=0, m_0=0 \checkmark]$$

$$N' = 0 \Rightarrow N(s) = N(l) = F$$

$$\left(\text{since } N(l) = \underline{R}_e(l) \cdot \underline{t}_0 = \underline{F}_e \cdot \underline{t}_0 = F \right)$$

$$(-F \underline{e}_2) (-\underline{e}_2)$$

$$T' = 0 \Rightarrow T(l) = T(s) = 0$$

$$M' = -T = 0 \Rightarrow M(l) = M(s) = 0$$

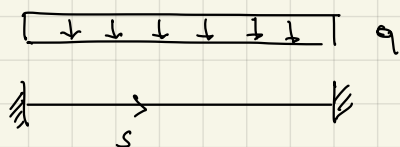
Calculate $u, w \dots$

$$u(s) = \frac{F}{EA} s$$

$$w(s) = bs \quad b \text{ arbitrary} \quad (\text{arbitrary small rotation allowed})$$

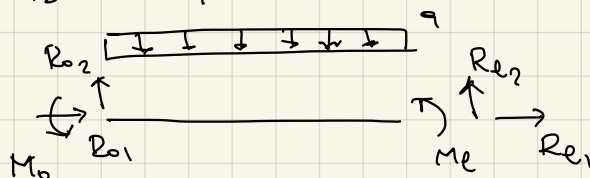
Exercise 4

$$f_1(s) = -q \cdot s^2, \quad f_2(s) \equiv -q$$



$$e_1 \uparrow$$

(FBD)



There exist (s^3) values of the reaction parameters that guarantee equilibrium in the ref. configuration, but cannot integrate the equil. eqn's because static BCs involve unknown reactions. Using constitutive equations we can integrate the reduced system in u, w

$$EA u'' = 0$$

(1)

$$EI w'''' = -q$$

(2)

2 eqn's in 2 unknowns u, w

6 BC's of kinematic / static nature

$$(1) \Rightarrow u(s) = A + Bs$$

$$u(0) = 0 = A$$

$$u(l) = Bl = 0 \Rightarrow B = 0$$

$$\Rightarrow u(s) \equiv 0$$

$$(2) w'''' = -\frac{q}{EI} \Rightarrow w(s) = a + bs + cs^2 + ds^3 - \frac{q}{24EI} s^4$$

$$\left. \begin{array}{l} w(0) = 0 \\ w'(0) = 0 \\ w(l) = 0 \\ w'(l) = 0 \end{array} \right\} \rightarrow \left\{ \begin{array}{l} a = 0 \\ b = 0 \end{array} \right.$$

$w(s)$ = 4th order polynomial

$$\left\{ \begin{array}{l} cl^2 + dl^3 - \frac{q}{24EI} l^4 = 0 \\ 2cl + 3dl^2 - \frac{q}{6EI} l^3 = 0 \end{array} \right\} \Rightarrow \left\{ \begin{array}{l} c = -\frac{1}{24} \frac{q}{EI} l^2 \\ d = \frac{1}{12} \frac{q}{EI} l \end{array} \right.$$

$$w''(s) = 2c + 6ds - \frac{q}{24EI} s^2 = \frac{M(s)}{EI}; \quad \frac{T(s)}{EI} = -\frac{M'(s)}{EI} = -6d + \frac{q}{EI} s$$

$M(s)$ quadratic

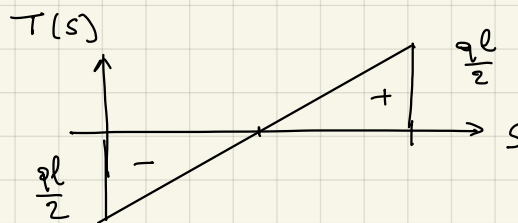
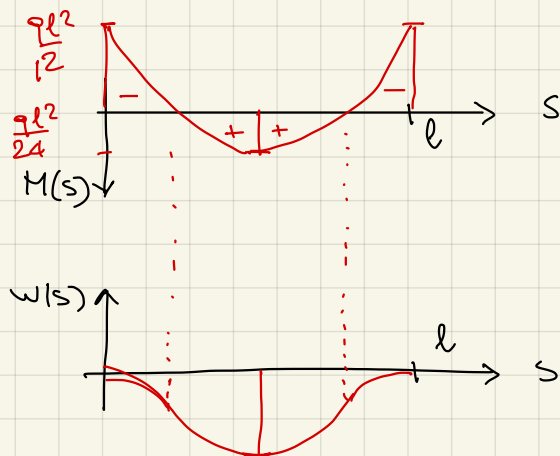
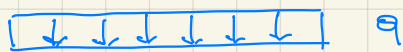
$T(s)$ affine

$$M(0) = 2EIc = -\frac{ql^2}{12} \stackrel{\text{check (symmetry)}}{=} M(l)$$

$$M\left(\frac{l}{2}\right) = \frac{ql^2}{24}$$

$$T(0) = -6EI d = -\frac{ql}{2}, \quad T(l) = \frac{ql}{2}$$

Diagrams



Remark

Isostatic: ① Determine M, N, T

② Determine u, w by integrating constitutive equations

Hyperstatic: ① Determine u, w

② Determine M, N, T by computing derivatives in the constitutive equations

Nonlinear theory: need to solve for the equilibrium configuration.

Accept constitutive relations

$$N_R = EA \varepsilon$$

$$M_R = EI \kappa$$

Look at general scheme for equilibrium problems again.