3D Truss Analysis

CEE 421L. Matrix Structural Analysis

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1 Element Stiffness Matrix in Local Coordinates

Consider the relation between axial forces, $\{q_1, q_2\}$, and axial displacements, $\{u_1, u_2\}$, only (in local coordinates).

$$\mathbf{k} = \frac{EA}{L} \left[\begin{array}{cc} 1 & -1 \\ -1 & 1 \end{array} \right]$$

$$\mathbf{q} = \mathbf{k} \ \mathbf{u}$$

2 Coordinate Transformation

Global and local coordinates

$$L = \sqrt{(x_2 - x_1)^2 + (y_2 - y_1)^2 + (z_2 - z_1)^2}$$

$$\cos \theta_x = \frac{x_2 - x_1}{L} = c_x$$

$$\cos \theta_y = \frac{y_2 - y_1}{L} = c_y$$

$$\cos \theta_z = \frac{z_2 - z_1}{L} = c_z$$

Displacements

 $u_1 = v_1 \cos \theta_x + v_2 \cos \theta_y + v_3 \cos \theta_z$

 $u_2 = v_4 \cos \theta_x + v_5 \cos \theta_y + v_6 \cos \theta_z$

.

$$\begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = \begin{bmatrix} c_x & c_y & c_z & 0 & 0 & 0 \\ 0 & 0 & 0 & c_x & c_y & c_z \end{bmatrix} \begin{bmatrix} v_1 \\ v_2 \\ v_3 \\ v_4 \\ v_5 \\ v_6 \end{bmatrix}$$

$$\mathbf{u} = \mathbf{T} \ \mathbf{v}$$

Forces

.

$$\begin{bmatrix} f_1 \\ f_2 \\ f_3 \\ f_4 \\ f_5 \\ f_6 \end{bmatrix} = \begin{bmatrix} c_x & 0 \\ c_y & 0 \\ c_z & 0 \\ 0 & c_x \\ 0 & c_y \\ 0 & c_z \end{bmatrix} \begin{bmatrix} q_1 \\ q_2 \end{bmatrix}$$

$$\mathbf{f} = \mathbf{T}^\mathsf{T} \; \mathbf{q}$$

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3 Element Stiffness Matrix in Global Coordinates

$$\begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} \qquad \mathbf{f} = \mathbf{T}^\mathsf{T} \ \mathbf{q} \qquad \mathbf{u} = \mathbf{T} \ \mathbf{v}$$

$$\mathbf{q} = \mathbf{k} \ \mathbf{u}$$

$$\mathbf{q} = \mathbf{k} \ \mathbf{T} \ \mathbf{v}$$

$$\mathbf{T}^\mathsf{T} \mathbf{q} = \mathbf{T}^\mathsf{T} \ \mathbf{k} \ \mathbf{T} \ \mathbf{v}$$

$$\mathbf{f} = \mathbf{T}^\mathsf{T} \ \mathbf{k} \ \mathbf{T} \ \mathbf{v}$$

$$\mathbf{f} = \mathbf{K} \ \mathbf{v}$$

$$\mathbf{K} = \frac{EA}{L} \begin{bmatrix} c_x^2 & c_x c_y & c_x c_z & -c_x^2 & -c_x c_y & -c_x c_z \\ c_x c_y & c_y^2 & c_y c_z & -c_x c_y & -c_y^2 & -c_y c_z \\ c_x c_z & c_y c_z & c_z^2 & -c_x c_z & -c_x c_y & -c_z^2 \\ -c_x^2 & -c_x c_y & -c_x c_z & c_x^2 & c_x c_y & c_x c_z \\ -c_x c_y & -c_y^2 & -c_y c_z & c_x c_y & c_y^2 & c_y c_z \\ -c_x c_z & -c_y c_z & -c_z^2 & c_x c_z & c_y c_z & c_z^2 \end{bmatrix}$$

4 Numbering Convention for Degrees of Freedom

$$g = [3*n1-2; 3*n1-1; 3*n1; 3*n2-2; 3*n2-1; 3*n2];$$

5 Truss Bar Tensions, T

$$T = q_2 = (\mathbf{kTv})_2 = \frac{EA}{L} (c_x(v_4 - v_1) + c_y(v_5 - v_2) + c_z(v_6 - v_3))$$

6 Modifying truss_2d.m to truss_3d.m

• Copy truss_2d.m to truss_3d.m —

```
function [D,R,T,L,Ks] = truss_3d(XYZ,TEN,RCT,EA,P,D)
```

Modifications to the input arguments:

- the node location matrix XYZ has x, y, and z coordinates ... a 3 x nN matrix;
- the reaction matrix RCT has x, y, and z coordinates ... a 3 x nN matrix;
- the node load matrix P has x, y, and z coordinates ... a 3 x nN matrix;
- the prescribed displacement matrix D has x, y, and z coordinates . . . a 3 x nN matrix;

Modification to the computed output:

- the computed deflections D will be the x, y, z displacements at each node, returned as a 3 x nN matrix;
- the computed reactions R will be the x, y, z forces at each node with a reaction, returned as a 3 \times nN matrix;

Modifications to the program itself:

- Change how DoF is computed;
- Change [Ks,L] = truss_assemble_2d(XY,TEN,EA); to
 [Ks,L] = truss_assemble_3d(XYZ,TEN,EA);
- Change T = truss_forces_2d(XY,TEN,EA,Dv); to
 T = truss_forces_3d(XYZ,TEN,EA,D);
- Modify the section of code relating the node displacement vector Dv to the node displacement matrix D to account for the fact that there are three degrees of freedom per node.
- Change plot commands to plot3 commands and change XY to XYZ.
 For example, change ...

```
plot( XY(1,TEN(:,b)), XY(2,TEN(:,b)), '-g')
...to...
plot3( XYZ(1,TEN(:,b)), XYZ(2,TEN(:,b)), '-g')
Also change the ax variable to account for the Z dimension.
```

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```
• Copy truss_element_2d.m to truss_element_3d.m —
 function K = truss_element_3d(x1,y1,z1,x2,y2,z2,EA)
 L =
 cx =
 cy =
 cz =
 K =
• Copy truss_assemble_2d.m to truss_assemble_3d.m —
 function [Ks,L] = truss_assemble_3d(XYZ,TEN,EA)
 DoF =
 x1 =
 y1 =
 z1 =
 x2 =
 y2 =
 z2 =
  [K, L(b)] = truss_element_3d(x1,y1,z1,x2,y2,z2,EA(b));
 g =
• Copy truss_forces_2d.m to truss_forces_3d.m —
 function T = truss_forces_3d(XYZ,TEN,EA,D)
 x1 =
 y1 =
 z1 =
 x2 =
 y2 =
 z2 =
 L =
 cx =
  cy =
 cz =
 T(b) =
```