

# Elasticity of one-dimensional continua and nanostructures

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## Extension, torsion and bending of rods/beams

$$F = E A \epsilon$$

$$M_t = G J \frac{\Theta}{L}$$

$$M = E I \kappa$$

## 3D elasticity (recap of some basics)

### deformation map $\underline{x} = \underline{f}(\underline{X})$

- takes a (Langrangian/material) vector/point  $\underline{X}$  in the undeformed configuration
- returns a (Eulerian/spatial) vector/point  $\underline{x}$  in the deformed configuration

### deformation gradient $\underline{\underline{F}}(\underline{X}) = \nabla_{\underline{X}} \underline{f}(\underline{X})$

- takes a (Langrangian/material) vector/point  $\underline{X}$  in the undeformed configuration  
→  $\underline{X}$  is material point where the gradient is evaluated
- returns a second order (two-point) tensor  $\underline{\underline{F}}$  that depends on  $\underline{X}$  (in the general case)
- $\underline{\underline{F}}$  contains (first order accurate) information about how line elements in the material configuration are transformed into deformed (stretched and rotated) line elements in the spatial configuration by  $\underline{f}(\cdot)$   
→  $d\underline{x} = \underline{\underline{F}}(\underline{X}) \cdot d\underline{X} = \nabla_{\underline{X}} \underline{f}(\underline{X}) \cdot d\underline{X}$

### equations of equilibrium

- material description:  $\nabla_{\underline{X}} \cdot \underline{\underline{P}} + \underline{\underline{B}} = \rho_0 \cdot \underline{\underline{A}}$
- spatial description:  $\nabla_{\underline{x}} \cdot \underline{\underline{\sigma}} + \underline{\underline{b}} = \rho \cdot \underline{\underline{a}}$

## 1D elasticity

we talk about **slender bodies** ( $\frac{\text{Length}}{\text{Diameter}} > 10$ )

we only have **one material coordinate**  $s$  along the axis of the beam

- $s$  uniquely identifies a particular cross section along the beam axis
- model equations will be a system of ODEs in  $s$  (instead of PDE)
- equations are obtained by averaging a PDE in 3 dimensions over the cross section

kinematic quantities in the model

- **position of centerline**  $\underline{r}(s)$
- **orientation/rotation of cross section**  $\underline{\underline{R}}(s)$
- more accuracy  $\rightarrow$  more unknowns

## Traditional beam models

- Euler-Bernoulli beam theory
- Timoshenko beam theory

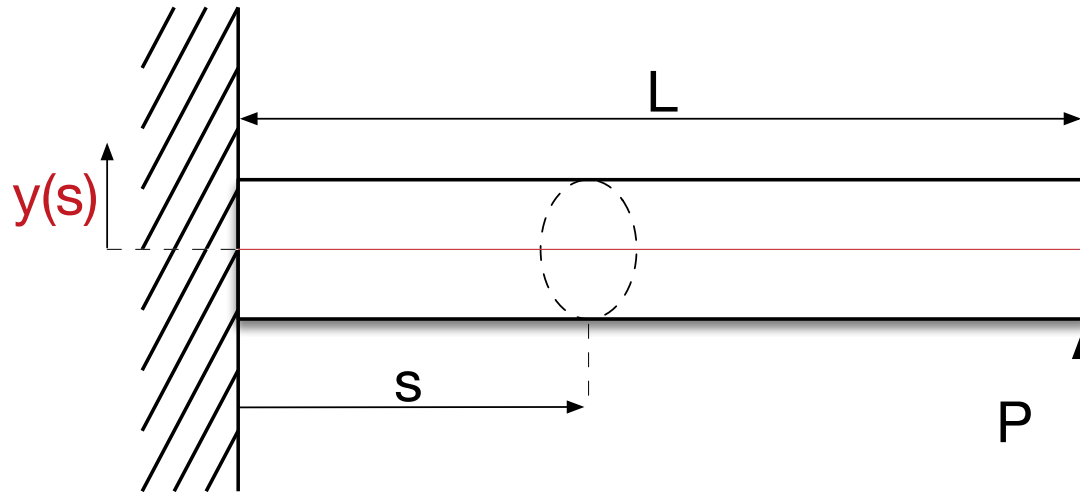
## Theory of Special Cosserat rods

- Kinematics of Cosserat rods
- Kinematics of Special Cosserat rods
- Balance equations
- Constitutive laws
- Combined model
- Constitutive laws (part 2)
- Relaxation of cross section
- Modeling of 1D nanostructures
- FEM discretization

## The Euler-Bernoulli beam (planar case)

- assumption of the model
  - all cross sections remain planar
  - cross section normal and centerline tangent are of same orientation  $\varphi$ 
    - model only allows for pure bending (no shear)
- bending moment  $M(s) = E I \kappa(s)$
- centerline of beam described by function  $y(s)$
- curvature of centerline given by  $\kappa(s) = \frac{\frac{d^2 y}{ds^2}(s)}{\left(1 + \left(\frac{dy}{ds}(s)\right)^2\right)^{\frac{3}{2}}}$ 
  - geometric nonlinearity → non-linear ODE
- linear approximation for small deflections:  $\frac{dy}{ds} = \tan(\varphi) \ll 1 \Rightarrow \frac{dy}{ds} \approx \varphi$
- Euler-Bernoulli beam equation:  $M(s) = E I \frac{d^2 y}{ds^2}$

## Example: Cantilever beam (part 1)



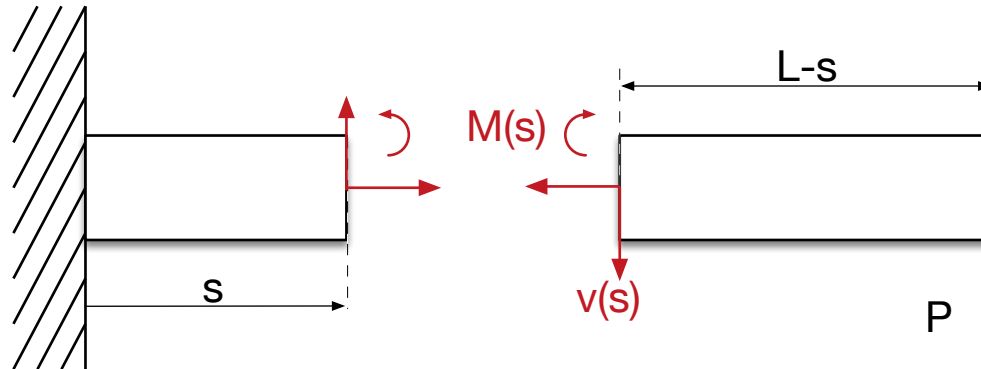
boundary conditions at  $s = 0$ :

- $y(0) = 0$
- $\frac{dy}{ds}(0) = 0$

boundary conditions at  $s = L$ :

- $v(L) = P$
- $M(L) = 0 \Rightarrow \frac{d^2y}{ds^2}(L) = 0$

## Example: Cantilever beam (part 2)



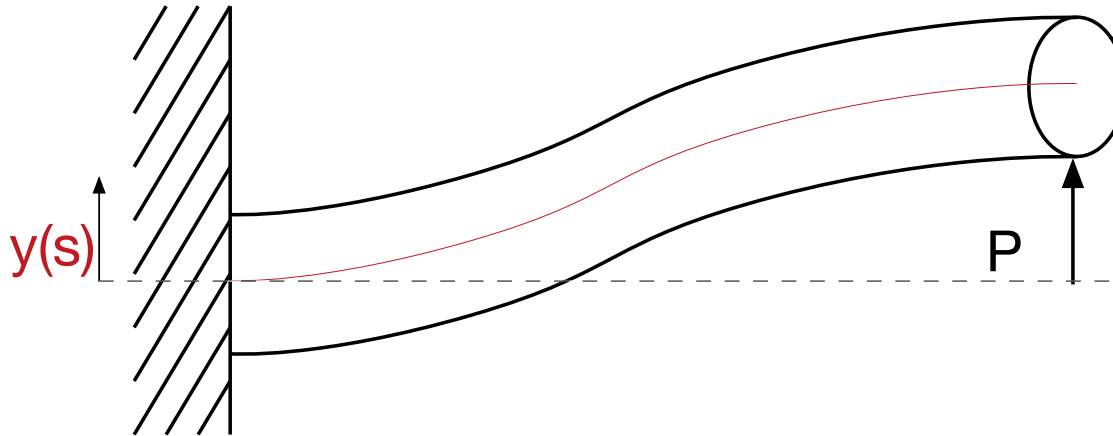
moment balance:  $-M(s) + P(L - s) = 0 \Rightarrow M(s) = P(L - s)$

$$M(x) = EI \frac{d^2 y}{ds^2} \Rightarrow \frac{d^2 y}{ds^2} = \frac{M(s)}{EI} = \frac{P(L - s)}{EI}$$

$$\Rightarrow y(s) = \frac{P}{EI} \left( -\frac{s^3}{6} + \frac{Ls^2}{2} \right) + \cancel{c_1} s + \cancel{c_2} = \frac{PL^3}{2EI} \left( \left( \frac{s}{L} \right)^2 - \frac{1}{3} \left( \frac{s}{L} \right)^3 \right)$$



## Another example of a cantilever beam



boundary conditions at  $s = 0$ :

- $y(0) = 0$
- $\frac{dy}{ds}(0) = 0$

boundary conditions at  $s = L$ :

- $v(L) = P$
- $\frac{dy}{ds}(L) = 0 \Rightarrow M(L) \neq 0$   
(rotation is restricted)

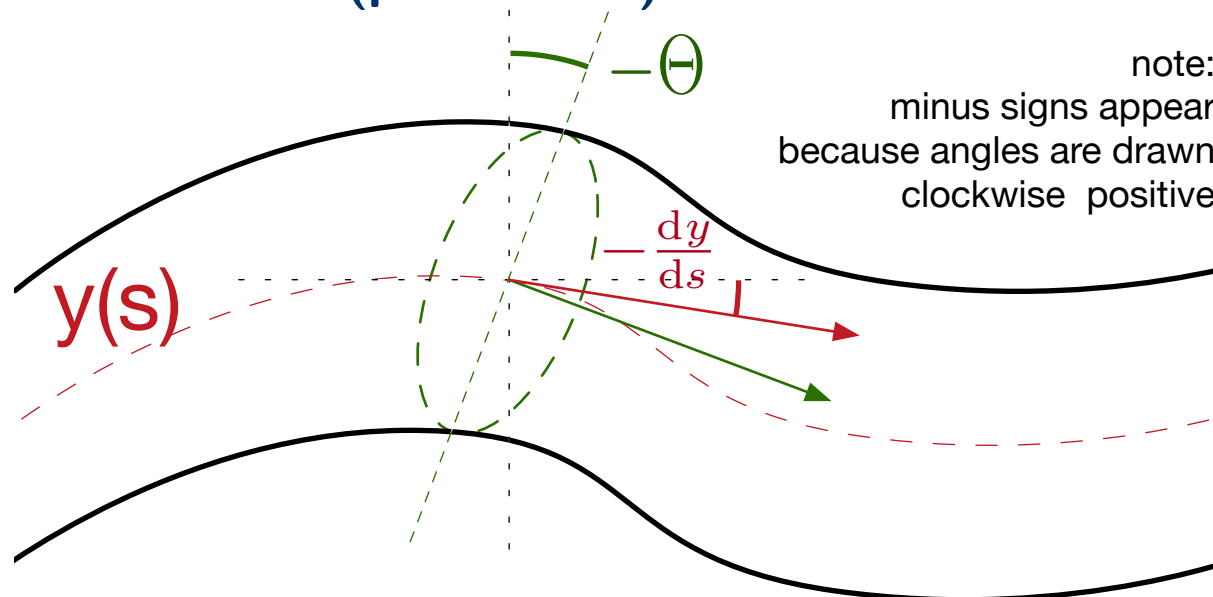
$$\Rightarrow M(s) = M(L) + P(L - s)$$

## Boundary conditions

displacement	force
prescribed $\Rightarrow$	unknown
unknown	$\Leftarrow$ prescribed

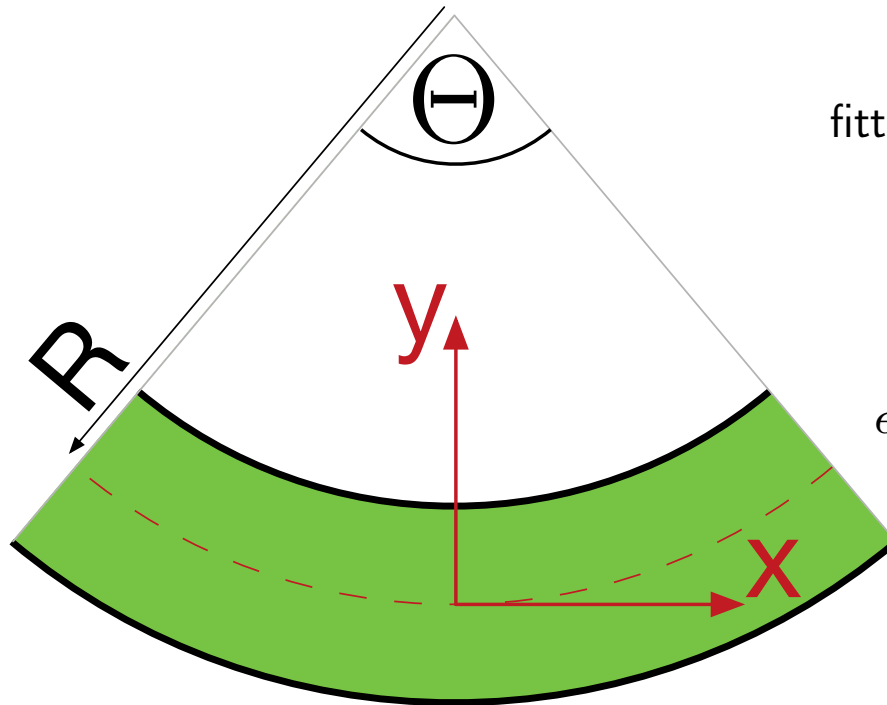
rotation	moment
prescribed $\Rightarrow$	unknown
unknown	$\Leftarrow$ prescribed

## The Timoshenko beam (planar case)



- individual cross sections still remain planar
- centerline tangent orientation (red)  $\neq$  cross section normal (green)
  - $\Theta$  measures orientation of cross section
  - centerline tangent  $\frac{dy}{ds}$  influenced by **bending and shearing**
- shear strain  $\gamma(s) = \frac{dy}{ds}(s) - \Theta(s) = \frac{v(s)}{kGA}$
- curvature  $\kappa(s) = \frac{d\Theta}{ds}(s) = \frac{M(s)}{EI}$  (see next slide)

## Connection between curvature and moment



$$L = R \Theta \quad \Rightarrow \quad \frac{1}{R} = \frac{\Theta}{L} = \frac{d\Theta}{ds}$$

fitting circles to the centerline locally:

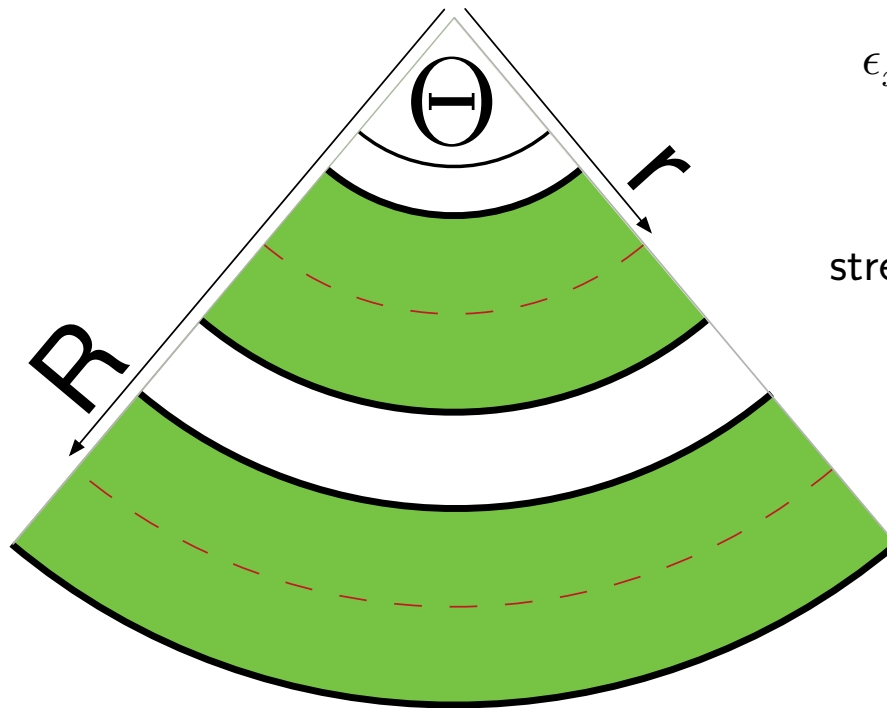
$$\kappa(s) = \frac{d\Theta}{ds}(s)$$

$$\epsilon_{xx} = \frac{\Delta L}{L} = \frac{(R - y) \Theta - R \Theta}{R \Theta} = \frac{-y}{R}$$

$$\sigma_{xx} = -E \frac{y}{R}$$

$$M = - \int_{\Omega} y \sigma_{xx} dA = E I \frac{1}{R} = E I \kappa$$

## Bending and axial compression together



because of axial forces the beam got shorter!

$$\begin{aligned}\epsilon_{xx} &= \frac{(r - y) \Theta - R \Theta}{R \Theta} = \frac{r - R}{R} + \frac{-y}{R} \\ &= \epsilon - \frac{y}{R}\end{aligned}$$

stress due to axial stretch + bending:

$$\sigma_{xx} = E \left( \epsilon + \frac{-y}{R} \right)$$

$$\begin{aligned}M &= - \int_{\Omega} y \sigma_{xx} \, dA \\ &= -E \epsilon \int_{\Omega} y \, dA + \frac{E}{R} \int_{\Omega} y^2 \, dA \\ &= E I \kappa\end{aligned}$$

→ bending and axial stretch are independent!

## Model linearity

$$\frac{dy}{ds} = \frac{1}{k G A} \cdot v(s) + \Theta$$

$$\frac{d\Theta}{ds} = \frac{1}{E I} \cdot M(s)$$

we obtained a linear model because we assumed...

- small slopes of the centerline (linear kinematics)

$$\frac{dy}{ds} = \tan \Theta \approx \Theta$$

approximation is not valid for larger deformations

- material linearity (linear constitutive law)

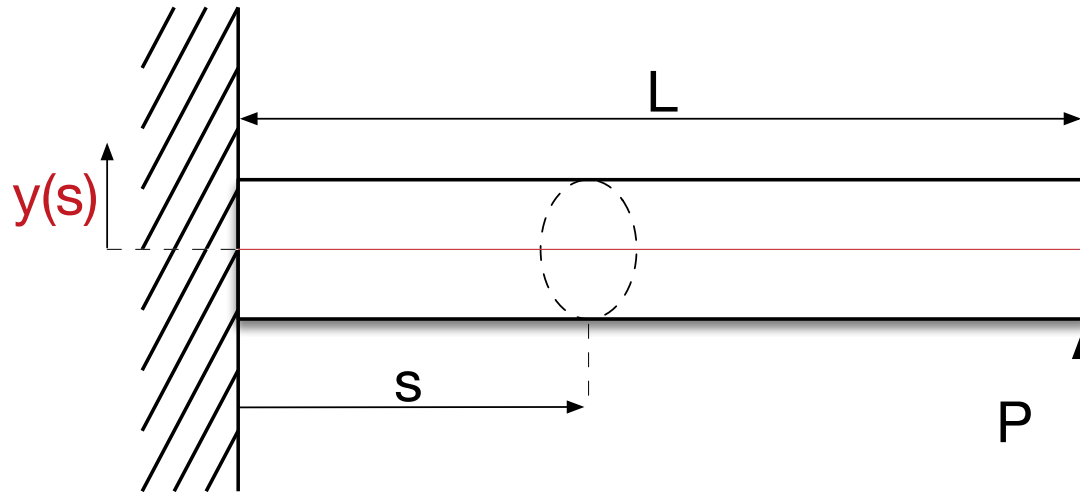
$v(s)$ ,  $M(s)$  get multiplied by constants

or by functions that depend on  $s$  but not on  $v(s)$ ,  $M(s)$

material may eg. get stiffer with increasing deformation energy

btw.  $k$  is a correction factor, that depends on the shape of the beam's cross section

## Example: Cantilever beam (part 1)



boundary conditions at  $s = 0$ :

- $y(0) = 0$
- $\frac{dy}{ds}(0) \neq 0$  (because of shear!)
- $\Theta(0) = 0$  (cross section fixed)

boundary conditions at  $s = L$ :

- $v(L) = P$
- $M(L) = 0$

## Example: Cantilever beam (part 2)

As before with the Euler-Bernoulli cantilever beam we have

$$v(s) = P \quad \text{and} \quad M(s) = P(L - s)$$

We integrate the Timoshenko beam equations ...

$$\frac{d\Theta}{ds}(s) = \frac{M(s)}{EI} = \frac{P(L - s)}{EI} \Rightarrow \Theta(s) = \frac{P}{EI} \left( Ls - \frac{s^2}{2} \right) + \varrho$$

$$\frac{dy}{ds}(s) = \frac{v(s)}{kGA} + \Theta(s) = \frac{P}{kGA} + \frac{P}{EI} \left( Ls - \frac{s^2}{2} \right)$$

$$\Rightarrow y(s) = \frac{Ps}{kGA} + \frac{P}{EI} \left( L\frac{s^2}{2} - \frac{s^3}{6} \right) + \varrho = \frac{Ps}{kGA} + \frac{PL^3}{2EI} \left( \left( \frac{s}{L} \right)^2 - \frac{1}{3} \left( \frac{s}{L} \right)^3 \right)$$



## Comparison of Euler-Bernoulli and Timoshenko cantilever beams

### Euler-Bernoulli beam

$$y^{(E)}(s) = \frac{P L^3}{2 E I} \left( \left( \frac{s}{L} \right)^2 - \frac{1}{3} \left( \frac{s}{L} \right)^3 \right) \Rightarrow y^{(E)}(L) = \frac{P L^3}{3 E I}$$

### Timoshenko beam

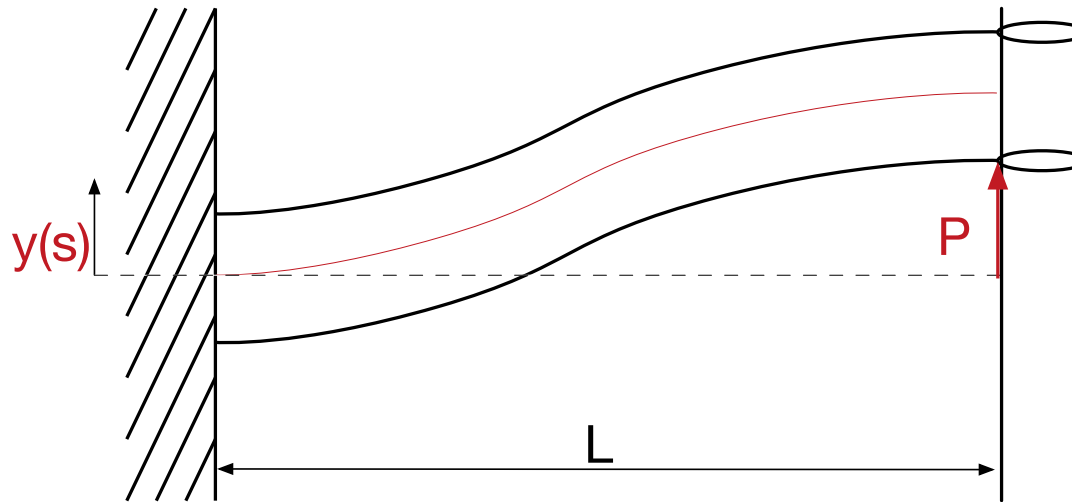
$$y^{(T)}(s) = \frac{P s}{k G A} + \frac{P L^3}{2 E I} \left( \left( \frac{s}{L} \right)^2 - \frac{1}{3} \left( \frac{s}{L} \right)^3 \right) \Rightarrow y^{(T)}(L) = \frac{P L}{k G A} + \frac{P L^3}{3 E I}$$

**relative error** (taking Euler-Bernoulli as the reference)

$$\begin{aligned} \epsilon_r &= \frac{y^{(T)}(L) - y^{(E)}(L)}{y^{(E)}(L)} = \frac{\frac{P L}{k G A}}{\frac{P L^3}{3 E I}} = \frac{3 E I}{k G A L^2} \\ &= \frac{3 E b h^3}{k G b h L^2 12} = \frac{1}{4} \frac{E}{k G} \left( \frac{h}{L} \right)^2 \end{aligned}$$

→ Euler-Bernoulli beam model performs equally good as the Timoshenko model  
when the beam is very slender ( $h/L$  small)

## Another example of a cantilever beam (part 1)



cross section at  $s = L$  can not rotate or displace horizontally

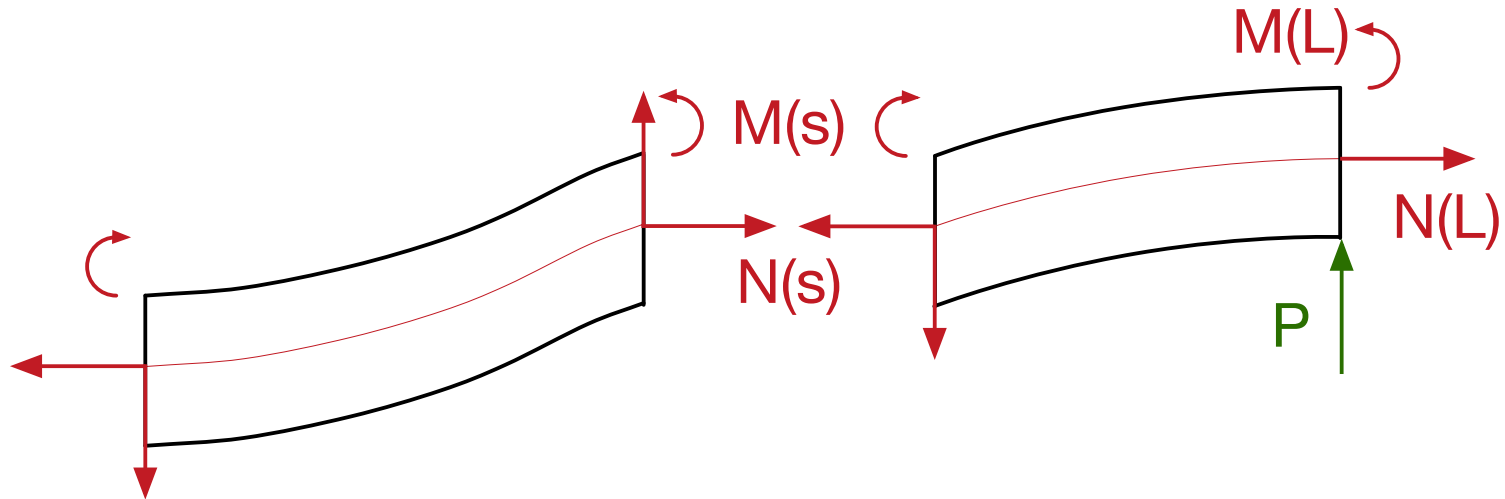
boundary conditions at  $s = 0$ :

- $y(0) = 0$
- $\Theta(0) = 0$

boundary conditions at  $s = L$ :

- $v(L) = P$
- $\Theta(L) = 0$

## Another example of a cantilever beam (part 2)



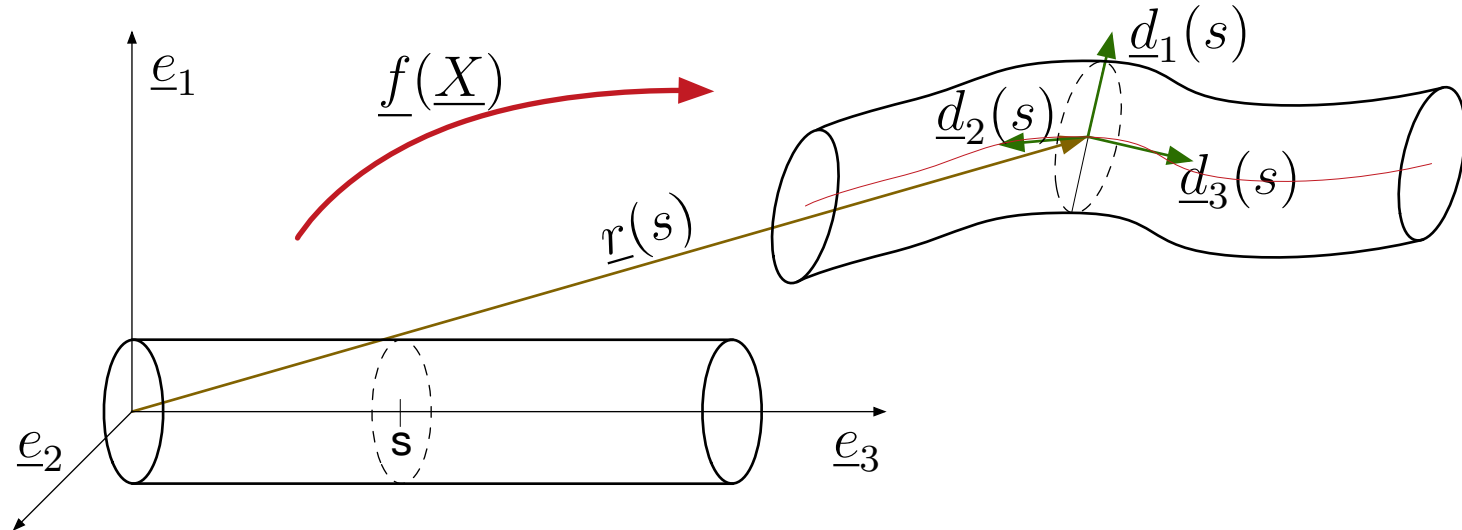
moment balance (in the deformed configuration):

$$-M(s) + M(L) + P(L - s) - N(y(L) - y(s))$$

→ the last term results in nonlinear behavior of the model

→  $M(L)$  and  $N(L)$  are two extra unknowns

## Introduction to special Cosserat rods (part 1)



remember:  $s$  is the unique identifier for a particular cross section of the beam

### kinematic quantities:

- position of the deformed cross section  $\underline{r}(s)$
- shearing of the deformed cross section  $\underline{d}_1(s)$ ,  $\underline{d}_2(s)$
- orientation of the deformed cross section  $\underline{d}_3(s)$   
(perpendicular to  $\underline{d}_1(s)$  and  $\underline{d}_2(s)$  → not independent)

## Introduction to ~~special~~ Cosserat rods (part 2)

### constrained deformation map

$$\underline{f}(\underline{X}) = \underline{f}(X_1, X_2, X_3 = s) = \underline{r}(s) + X_\alpha \underline{d}_\alpha \quad \text{with } \alpha \in \{1, 2\}$$

### consequences of the imposed kinematic constraints

- cross sections remain flat
- straight lines within the cross section remain straight lines
- boundary of the cross section: circles are mapped to ellipses  
(not to arbitrary curves in 2D)
- $\underline{d}_1(s)$  and  $\underline{d}_2(s)$  are not perpendicular in the general case (shearing)

## What makes the Special Cosserat rod special?

- $\underline{d}_1(s)$  and  $\underline{d}_2(s)$  are perpendicular and unit-normed
- hence  $(\underline{d}_1, \underline{d}_2, \underline{d}_3)$  form an orthonormal triad

### consequences

$$\exists \underline{\underline{R}}(s) \in SO(3) \forall s : \left( R : (\underline{e}_1, \underline{e}_2, \underline{e}_3) \mapsto (\underline{d}_1, \underline{d}_2, \underline{d}_3) \right)$$

- $(\underline{e}_1, \underline{e}_2, \underline{e}_3)$  is called the global basis
- $(\underline{d}_1, \underline{d}_2, \underline{d}_3)(s)$  is called the local basis or director basis at  $s$
- $\underline{d}_i(s) = \underline{\underline{R}}(s) \cdot \underline{e}_i$  ( $\underline{\underline{R}}$  : rotation matrix ;  $SO(3)$  : special orthogonal matrix group)

### constrained deformation map

$$\underline{f}(\underline{X}) = \underline{f}(X_1, X_2, X_3 = s) = \underline{r}(s) + \underline{\underline{R}}(s) \cdot (X_\alpha \underline{e}_\alpha) \quad \text{with } \alpha \in \{1, 2\}$$

→ but the rigidity of the cross section is stiffening the beam!

## Warping / relaxation of the cross section

we want to introduce warping of the cross section,  
while at the same time keeping the director basis  $\underline{d}_i$

### modified deformation map

$$\underline{f}(\underline{X}) = \underline{f}(X_1, X_2, X_3 = s) = \underline{r}(s) + \underline{R}(s) \cdot (X_\alpha \underline{e}_\alpha + \underline{u}) \quad \text{with } \alpha \in \{1, 2\}$$

### geometric meaning of $\underline{u}$

- $u_1, u_2$  : in-plane shrinking
- $u_3$  : out-of-plane warping  $\rightarrow$  cross section not planar anymore
- then  $\underline{d}_1, \underline{d}_2$  represent the average orientation of the warped cross section

### what are our kinematic unknowns?

- if  $\underline{u} = \underline{u}(X_1, X_2, s)$  then we would be dealing with a 3D elasticity problem
- here we have  $\underline{u}(X_1, X_2, \text{local strains})$ , not a function of  $s$   
(we will come back to this later)
- $\underline{r}(s)$  and  $\underline{R}(s)$  are the only kinematic unknowns! (6 scalar quantities)

## Rotations in 3D revisited

a **rotation about *one* axis** is determined by

- axis of rotation given by  $\underline{a}$  with  $\|\underline{a}\| = 1$
- angle of rotation  $\Theta$

## composition of rotations

$$(\underline{a}_1, \Theta_1) + (\underline{a}_2, \Theta_2) + (\underline{a}_3, \Theta_3) + \dots = (\underline{a}_{eff}, \Theta_{eff})$$

note: “+” here denotes the composition of two rotations

$$\underline{\underline{R}}_{eff} = \dots \cdot \underline{\underline{R}}_3 \cdot \underline{\underline{R}}_2 \cdot \underline{\underline{R}}_1$$

In the general case, i.e. when  $\underline{a}_i \neq \underline{a}_j$  if  $i \neq j$ ) rotations do not commute!

**example** (rotation about  $\underline{e}_3$ ):

$$\left( \underline{a} = \begin{bmatrix} 0 \\ 0 \\ 1 \end{bmatrix}, \Theta \right) \rightarrow \underline{\underline{R}} = \begin{bmatrix} +\cos(\Theta) & -\sin(\Theta) & 0 \\ +\sin(\Theta) & +\cos(\Theta) & 0 \\ 0 & 0 & 1 \end{bmatrix}$$



## Axis-angle representation of rotations in 3D

from the axis-angle representation

$$\left( \underline{a} = \begin{bmatrix} a_1 \\ a_2 \\ a_3 \end{bmatrix}, \Theta \right)$$

we obtain the corresponding rotation matrix

$$\underline{\underline{R}} = \exp(\Theta \cdot \underline{\underline{a}})$$

where

$$\Theta \cdot \underline{\underline{a}} = \Theta \cdot \begin{bmatrix} 0 & -a_3 & a_2 \\ a_3 & 0 & -a_1 \\ -a_2 & a_1 & 0 \end{bmatrix}$$

is a skew-symmetric matrix obtained from the definition

$$\begin{aligned} \underline{\underline{a}} &= \underline{\underline{a}} \cdot \underline{\underline{I}} = \begin{bmatrix} \underline{a} \times \underline{e}_1 & \underline{a} \times \underline{e}_2 & \underline{a} \times \underline{e}_3 \end{bmatrix} \\ \Rightarrow \quad \underline{\underline{a}} \cdot \underline{v} &= \underline{a} \times \underline{v} \quad \forall \underline{v} \in \mathbb{R}^3 \end{aligned}$$

Observe the relationship between cross products and skew-symmetric matrices:

$$\underline{\underline{a}} = \text{axial}(\underline{a}) = \text{axial}(\underline{a} \times \underline{\underline{I}}) = \text{axial}([\underline{a}]_{\times})$$

## Axis-angle representation of rotations in 3D: Rodrigues' formula

$$\underline{\underline{R}} = \cos(\Theta) \underline{\underline{I}} + \sin(\Theta) \underline{\underline{a}} + (1 - \cos(\Theta)) \underline{\underline{a}} \otimes \underline{\underline{a}}$$

the Rodrigues' rotation formula allows us to compute the rotation matrix  $\underline{\underline{R}}$ , that corresponds to a given axis-angle representation  $(\underline{\underline{a}}, \Theta)$ , without actually computing the matrix exponential

computing matrix exponentials (of non-diagonal matrices) is either expensive or not precise!

### example

$$\underline{v}_{\text{rotated}} = \underline{\underline{R}} \underline{v} = \cos(\Theta) \underline{v} + \sin(\Theta) \underline{\underline{a}} \times \underline{v} + (1 - \cos(\Theta)) (\underline{\underline{a}} \cdot \underline{v}) \underline{\underline{a}}$$

## 3D rotations expressed by unit quaternions

Quaternions are a number system that extends the complex numbers. A quaternion consists of one real part and three independent imaginary parts. A unit quaternion is a quaternion of norm one and therefore has three independent components.

There exists an isomorphism between unit quaternions and rotation matrices:

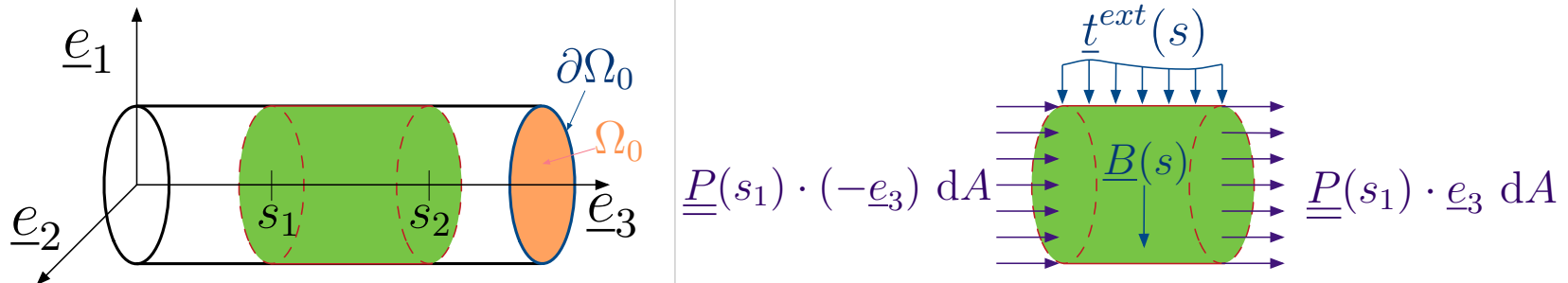
$$\text{given } \underline{q} = \begin{bmatrix} q_0 \\ q_1 \\ q_2 \\ q_3 \end{bmatrix} \rightarrow \underline{\underline{R}}(\underline{q}) = 2 \cdot \begin{bmatrix} \frac{1}{2} - (q_2^2 + q_3^2) & q_1 q_2 - q_0 q_3 & q_1 q_3 + q_0 q_2 \\ q_1 q_2 + q_0 q_3 & \frac{1}{2} - (q_1^2 + q_3^2) & q_2 q_3 - q_0 q_1 \\ q_1 q_3 - q_0 q_2 & q_2 q_3 + q_0 q_1 & \frac{1}{2} - (q_1^2 + q_2^2) \end{bmatrix}$$

$$\text{real part: } q_0 = \cos\left(\frac{\Theta}{2}\right) \text{ and imaginary part: } \begin{bmatrix} q_1 \\ q_2 \\ q_3 \end{bmatrix} = \sin\left(\frac{\Theta}{2}\right) \underline{a}$$

### advantages

- quadratic polynomials are faster for computation than  $\sin(\cdot)$  and  $\cos(\cdot)$
- numeric stability: when composing rotations rounding error makes unit quaternions not unit normed but normalizing is simple ; making a slightly non-orthogonal matrix orthogonal again is much harder

## Balance of forces (part 1)



to obtain the forces and moments we integrate the pullback of the tractions / tensions acting in the deformed configuration, hence using the reference configuration as the domain of integration ...

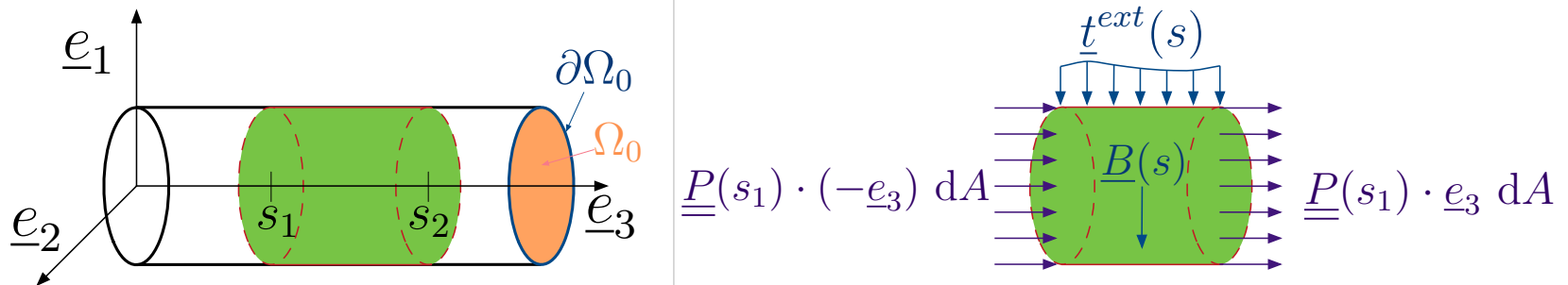
**external forces:** body force  $\underline{B}$  and external traction  $\underline{t}^{ext}$

$$\int_{s_1}^{s_2} \left( \iint_{\Omega_0(s)} \underline{B}(X_1, X_2, s) dA + \oint_{\partial\Omega_0(s)} \underline{t}^{ext}(l, s) dl \right) ds =:$$

$$=: \int_{s_1}^{s_2} \underline{\hat{n}}(s) ds$$

→ distributed external load  $\underline{\hat{n}}(s)$  (force per unit of undeformed length)

## Balance of forces (part 2)

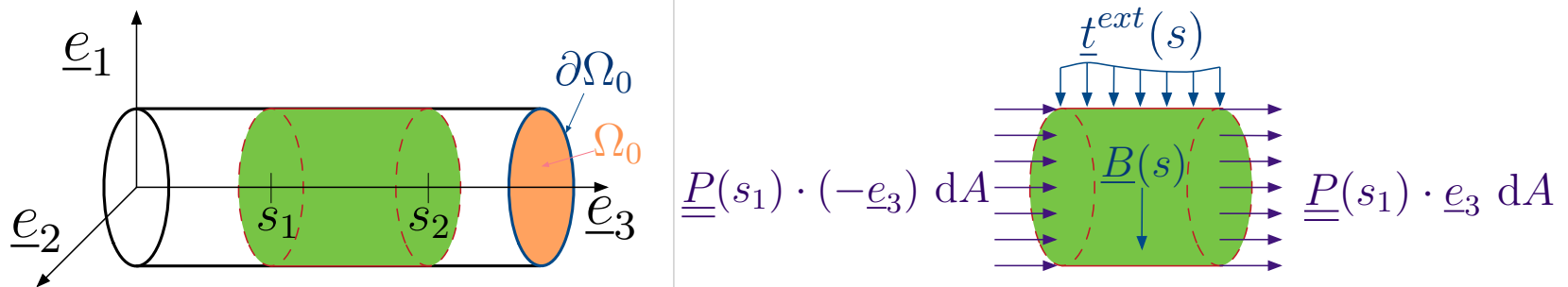


### internal forces

$$\begin{aligned}
 & \iint_{\Omega_0(s)} \underline{\underline{P}} \cdot \underline{n} \, dA = \\
 & = \iint_{\Omega_0(s)} \underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3 \, dX_1 \, dX_2 = \\
 & =: \underline{n}(s)
 \end{aligned}$$

→ internal contact force  $\underline{n}(s)$  (force per unit of undeformed length)

## Balance of forces (part 3)



### balance of forces (statics)

$$\int_{s_1}^{s_2} \hat{n}(s) \, ds + \underline{n}(s_2) - \underline{n}(s_1) = \int_{s_1}^{s_2} \hat{n}(s) + \underline{n}'(s) \, ds = \underline{0}$$

since  $s_1$  and  $s_2$  are arbitrary cross sections we let  $s_2 \rightarrow s_1$

and obtain the **local force balance**

$$\underline{n}'(s) + \hat{n}(s) = \underline{0}$$

### local balance of linear momentum (dynamics)

$$\underline{n}'(s) + \hat{n}(s) = \rho_0 \cdot A \cdot \ddot{\underline{r}}(s)$$

## Balance of moments (part 1)

we write the moment balance about the origin:  $\underline{M} = \underline{x} \times \underline{F}$   
with  $\underline{x}(X_1, X_2, s) = \underline{r}(s) + (\underline{x}(X_1, X_2, s) - \underline{r}(s))$

### moment due to external loads

$$\begin{aligned} & \iiint_{\text{reference volume}} \underline{x} \times \underline{B} \, dV + \iint_{\text{lateral surface of reference volume}} \underline{x} \times \underline{t}^{ext} \, dA = \\ &= \int_{s_1}^{s_2} \left( \underline{r}(s) \times \iint_{\Omega_0(s)} \underline{B}(X_1, X_2, s) \, dA \right) ds + \\ &+ \int_{s_1}^{s_2} \left( \iint_{\Omega_0(s)} (\underline{x}(X_1, X_2, s) - \underline{r}(s)) \times \underline{B}(X_1, X_2, s) \, dA \right) ds + \\ &+ \int_{s_1}^{s_2} \left( \underline{r}(s) \times \oint_{\partial\Omega_0(s)} \underline{t}^{ext}(l, s) \, dl \right) ds + \\ &+ \int_{s_1}^{s_2} \left( \oint_{\partial\Omega_0(s)} (\underline{x}(X_1, X_2, s) - \underline{r}(s)) \times \underline{t}^{ext}(l, s) \, dl \right) ds = \end{aligned}$$

## Balance of moments (part 2)

$$\begin{aligned} &= \int_{s_1}^{s_2} \left( \underline{r}(s) \times \left( \iint_{\Omega_0} \underline{B}(X_1, X_2, s) \, dA + \oint_{\partial\Omega_0(s)} \underline{t}^{ext}(s) \, dl \right) \right) ds + \\ &+ \int_{s_1}^{s_2} \left( \iint_{\Omega_0(s)} \left( \underline{x}(X_1, X_2, s) - \underline{r}(s) \right) \times \underline{B}(X_1, X_2, s) \, dA + \right. \\ &\quad \left. + \oint_{\partial\Omega_0} \left( \underline{x}(X_1, X_2, s) - \underline{r}(s) \right) \times \underline{t}^{ext}(l, s) \, dl \right) ds = \\ &=: \int_{s_1}^{s_2} \underline{r}(s) \times \hat{\underline{n}}(s) \, ds + \int_{s_1}^{s_2} \hat{\underline{m}}(s) \, ds \end{aligned}$$



## Balance of moments (part 3)

### moment due to internal forces

$$\begin{aligned} & \iint_{\Omega_0(s)} \underline{x}(X_1, X_2, s) \times (\underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3) \, dA = \\ &= \iint_{\Omega_0(s)} \underline{r}(s) \times (\underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3) \, dX_1 \, dX_2 + \\ &+ \iint_{\Omega_0(s)} (\underline{x}(X_1, X_2, s) - \underline{r}(s)) \times (\underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3) \, dX_1 \, dX_2 = \\ &= \underline{r}(s) \times \iint_{\Omega_0(s)} \underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3 \, dX_1 \, dX_2 + \dots = \\ &=: \underline{r}(s) \times \underline{n}(s) + \underline{m}(s) \end{aligned}$$

## Balance of moments (part 4)

### balance of moments (statics)

$$\begin{aligned} & \int_{s_1}^{s_2} \left( \underline{r}(s) \times \underline{\hat{n}}(s) + \underline{\hat{m}}(s) \right) ds + \\ & + \left( \underline{r}(s_2) \times \underline{n}(s_2) + \underline{m}(s_2) \right) - \left( \underline{r}(s_1) \times \underline{n}(s_1) + \underline{m}(s_1) \right) = \\ & = \int_{s_1}^{s_2} \left( \left( \cancel{\underline{r}(s)} \times \cancel{\underline{\hat{n}}(s)} + \underline{\hat{m}}(s) \right) + \left( \underline{r}'(s) \times \underline{n}(s) + \cancel{\underline{r}(s)} \times \cancel{\underline{n}'(s)} + \underline{m}'(s) \right) \right) ds = \\ & = \int_{s_1}^{s_2} \left( \underline{\hat{m}}(s) + \underline{r}'(s) \times \underline{n}(s) + \underline{m}'(s) \right) ds \end{aligned}$$

since  $s_1$  and  $s_2$  are arbitrary cross sections we let  $s_2 \rightarrow s_1$   
and obtain the **local moment balance**

$$\underline{m}'(s) + \underline{r}'(s) \times \underline{n}(s) + \underline{\hat{m}}(s) = \underline{0}$$

**local balance of angular momentum** (dynamics) (with  $\underline{I}_{=0}$  the moment of area tensor)

$$\underline{m}'(s) + \underline{r}'(s) \times \underline{n}(s) + \underline{\hat{m}}(s) = \rho_0 \cdot \frac{d}{dt} \left( \underline{I}_{=0} \cdot \underline{\omega} \right)$$

## Review of balance equations (statics)

**force balance** (3 equations)

$$\underline{n}'(s) + \underline{\hat{n}}(s) = \underline{0}$$

**moment balance** (3 equations)

$$\underline{m}'(s) + \underline{r}'(s) \times \underline{n}(s) + \underline{\hat{m}}(s) = \underline{0}$$

we have **6 kinematic unknowns**,  $\underline{r}(s)$  and  $\underline{\underline{R}}(s)$  with 3 unknowns each,  
and we have **6 kinetic unknowns**

$$\underline{n}(s) = \iint_{\Omega_0(s)} \underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3 \, dX_1 \, dX_2$$

$$\underline{m}(s) = \iint_{\Omega_0(s)} \left( \underline{x}(X_1, X_2, s) - \underline{r}(s) \right) \times \left( \underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3 \right) \, dX_1 \, dX_2$$

to close the system, we need a relationship between the kinematic and the kinetic quantities  
→ this relationship is called *constitutive law of the material*

## Motivation of constitutive law

let's recall the **constrained deformation map** (without warping)

$$\underline{x}(X_1, X_2, s) = \underline{f}(X_1, X_2, s) = \underline{r}(s) + X_\alpha \underline{d}_\alpha = \underline{r}(s) + \underline{\underline{R}}(s) \cdot (X_\alpha \underline{e}_\alpha)$$

we compute the **deformation gradient** and obtain

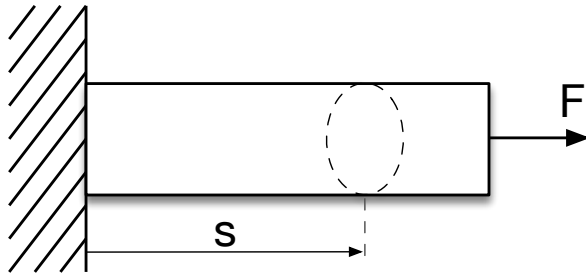
$$\begin{aligned} \underline{\underline{F}}(X_1, X_2, s) &= \underline{r}'(s) \otimes \underline{e}_3 + \underline{\underline{R}}(s) \cdot (\underline{e}_\alpha \otimes \underline{e}_\alpha) + \underline{\underline{R}}'(s) \cdot (X_\alpha \underline{e}_\alpha) \otimes \underline{e}_3 = \\ &= \underline{\underline{R}}(s) \cdot \left( \underline{\underline{R}}^T(s) \cdot \underline{r}'(s) \otimes \underline{e}_3 + \underline{e}_\alpha \otimes \underline{e}_\alpha + \underline{\underline{R}}^T(s) \cdot \underline{\underline{R}}'(s) \cdot (X_\alpha \underline{e}_\alpha) \otimes \underline{e}_3 \right) \end{aligned}$$

we then obtain an expression of the form

$$\underline{\underline{P}}(X_1, X_2, s) = \text{function}(\underline{\underline{F}}(X_1, X_2, s)),$$

from the 3-dimensional constitutive law of the material

## Example with constrained deformation map: stretching of a bar



we have the **deformed configuration**

$$\underline{r}(s) = \lambda s \underline{e}_3 \quad \text{with } \lambda \in \mathbb{R}^+$$

$$\underline{\underline{R}}(s) = \underline{\underline{I}}$$

therefore the **deformation map** is

$$\underline{x}(X_1, X_2, s) \underline{x}(s) = \lambda s \underline{e}_3 + \underline{\underline{I}} \cdot (X_\alpha \underline{e}_\alpha)$$

and its **deformation gradient** is given by

$$\underline{\underline{F}}(X_1, X_2, s) = \underline{\underline{F}}(s) = \lambda \underline{e}_3 \otimes \underline{e}_3 + \underline{e}_\alpha \otimes \underline{e}_\alpha = (\lambda - 1) \underline{e}_3 \otimes \underline{e}_3 + \underline{\underline{I}}$$

but the **axial force**

$$\underline{n}(s) = \iint_{\Omega_0(s)} \underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3 \, dX_1 \, dX_2$$

$$\neq E A (\lambda - 1)$$

because the deformation map violates the traction-free boundary condition  
→ the cross section must be able to shrink when the bar is stretched!

## Constitutive equation for a hyperelastic rod

- a hyperelastic material is a model for reversible elasticity → stress-strain relationship derives from a potential function, i.e. the strain energy density of the material
- in general a hyperelastic material has a nonlinear stress-strain relationship
- well known hyperelastic material models are the Neo-Hookean and Mooney-Rivlin solids

### strain energy per unit of undeformed length

$$\phi = \phi(\underline{r}(s), \underline{\underline{R}}(s), \underline{r}'(s), \underline{\underline{R}}'(s))$$

### strain energy per unit of undeformed volume

$$W(\cancel{\underline{f}(s)}, \underline{\underline{F}}(s), \cancel{\nabla \underline{\underline{F}}(s)}) = W(\underline{\underline{F}}(s))$$

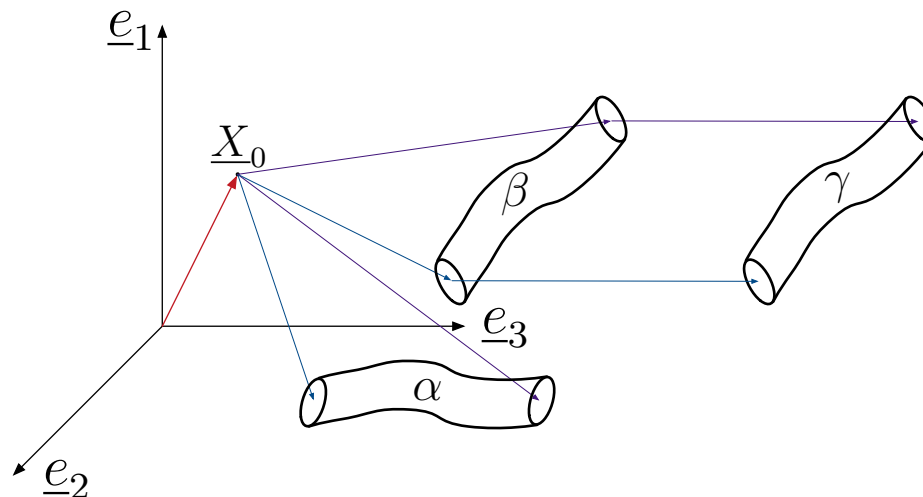
- $\cancel{\underline{f}(s)}$  canceled because of principle of frame-indifference
- $\cancel{\nabla \underline{\underline{F}}(s)}$  canceled because we will not consider higher order derivatives of the deformation map in classical elasticity (compared to higher gradient elasticity theory)

## Principle of material frame-indifference (part 1)

- constitutive laws should be invariant with regard to the external frame of reference, i.e. the coordinate system ( $\rightarrow$  observer objectivity)
- $\Rightarrow$  rigid body motions do not affect the internally stored energy in the system, (i.e. the mechanical/deformation energy)
- $\rightarrow$  we want to identify a set of strain measures which is invariant to rigid body motions!

to that end we set up a simple **thought experiment**

- perform two rigid body motions: one rotation followed by one translation
- equate the internal mechanical energy potentials:  $\phi|_{\alpha} = \phi|_{\beta} = \phi|_{\gamma}$



## Principle of material frame-indifference (part 2)

### 1. initial configuration $\alpha$

- centerline given by  $\underline{r}^\alpha(s)$
- cross section orientation given by  $\underline{\underline{R}}^\alpha(s)$
- energy potential given by  $\phi|_\beta(s) = \phi(\underline{r}^\alpha(s), \underline{\underline{R}}^\alpha(s), \underline{r}^{\alpha'}(s), \underline{\underline{R}}^{\alpha'}(s))$

### 2. configuration $\beta$ after rigid body rotation by some $\underline{\underline{Q}} \in SO(3)$ about some $\underline{X}_0 \in \mathbb{R}^3$

- $\underline{r}^\beta(s) = \underline{X}_0 + \underline{\underline{Q}}(\underline{r}^\alpha(s) - \underline{X}_0) \Rightarrow \underline{r}^{\beta'}(s) = \underline{\underline{Q}}\underline{r}^{\alpha'}(s)$   
(shift coordinate system s.t. rotation's fix-point  $\hat{=}$  origin; rotate; undo shift)
- $\underline{\underline{R}}^\beta(s) = \underline{\underline{Q}}\underline{\underline{R}}^\alpha(s) \Rightarrow \underline{\underline{R}}^{\beta'}(s) = \underline{\underline{Q}}\underline{\underline{R}}^{\alpha'}(s)$
- $\phi|_\beta(s) = \phi(\underline{X}_0 + \underline{\underline{Q}}(\underline{r}^\alpha(s) - \underline{X}_0), \underline{\underline{Q}}\underline{\underline{R}}^\alpha(s), \underline{\underline{Q}}\underline{r}^{\alpha'}(s), \underline{\underline{Q}}\underline{\underline{R}}^{\alpha'}(s))$

### 3. configuration $\gamma$ after rigid body translation by some $\underline{t} \in \mathbb{R}^3$

- $\underline{r}^\gamma(s) = \underline{X}_0 + \underline{\underline{Q}}(\underline{r}^\alpha(s) - \underline{X}_0) + \underline{t} \Rightarrow \underline{r}^{\gamma'}(s) = \underline{r}^{\beta'}(s) = \underline{\underline{Q}}\underline{r}^{\alpha'}(s)$
- $\underline{\underline{R}}^\gamma(s) = \underline{\underline{I}}\underline{\underline{R}}^\beta(s) = \underline{\underline{Q}}\underline{\underline{R}}^\alpha(s) \Rightarrow \underline{\underline{R}}^{\gamma'}(s) = \underline{\underline{R}}^{\beta'}(s) = \underline{\underline{Q}}\underline{\underline{R}}^{\alpha'}(s)$
- $\phi|_\gamma(s) = \phi(\underline{X}_0 + \underline{\underline{Q}}(\underline{r}^\alpha(s) - \underline{X}_0) + \underline{t}, \underline{\underline{Q}}\underline{\underline{R}}^\alpha(s), \underline{\underline{Q}}\underline{r}^{\alpha'}(s), \underline{\underline{Q}}\underline{\underline{R}}^{\alpha'}(s))$



## Principle of material frame-indifference (part 3)

comparing  $\phi|_{\beta}(s) \stackrel{!}{=} \phi|_{\gamma}(s)$  gives

$$\begin{aligned} \phi(\underline{X}_0 + \underline{Q}(\underline{r}^{\alpha}(s) - \underline{X}_0), \underline{Q}\underline{R}^{\alpha}(s), \underline{Q}\underline{r}^{\alpha'}(s), \underline{Q}\underline{R}^{\alpha'}(s)) = \\ \phi(\underline{X}_0 + \underline{Q}(\underline{r}^{\alpha}(s) - \underline{X}_0) + \underline{t}, \underline{Q}\underline{R}^{\alpha}(s), \underline{Q}\underline{r}^{\alpha'}(s), \underline{Q}\underline{R}^{\alpha'}(s)) \Rightarrow \end{aligned}$$

$\Rightarrow \phi$  is independent of  $\underline{r}(s)$ , i.e. the first argument can be omitted

without loss of generality we can set the arbitrarily chosen rotation matrix  $\underline{Q} := \underline{R}^T$

$$\phi(s) = \phi(\cdot, \underline{R}^T(s)\underline{R}(s), \underline{R}^T(s)\underline{r}'(s), \underline{R}^T(s)\underline{R}'(s))$$

we define a new version of  $\phi$  that depends only on the invariants

$$\psi(s) := \psi(\underline{R}^T(s)\underline{r}'(s), \underline{R}^T(s)\underline{R}'(s))$$

## Invariant strain measures (part 1)

we can verify that  $\underline{\underline{R}}^T \underline{r}'$  and  $\underline{\underline{R}}^T \underline{\underline{R}}'$  are truly invariant strain measures:

we get

$$\underline{\underline{R}}^{\gamma T} \underline{r}^{\gamma'} = (\underline{\underline{Q}} \underline{\underline{R}}^{\alpha})^T (\underline{\underline{Q}} \underline{r}^{\alpha'}) = (\underline{\underline{R}}^{\alpha T} \underline{\underline{Q}}^T) (\underline{\underline{Q}} \underline{r}^{\alpha'}) = \underline{\underline{R}}^{\alpha T} \underline{r}^{\alpha'}$$

and

$$\underline{\underline{R}}^{\gamma T} \underline{\underline{R}}^{\gamma'} = (\underline{\underline{Q}} \underline{\underline{R}}^{\alpha})^T (\underline{\underline{Q}} \underline{\underline{R}}^{\alpha'}) = (\underline{\underline{R}}^{\alpha T} \underline{\underline{Q}}^T) (\underline{\underline{Q}} \underline{\underline{R}}^{\alpha'}) = \underline{\underline{R}}^{\alpha T} \underline{\underline{R}}^{\alpha'}$$

## Invariant strain measures (part 2)

we define  $\underline{v}$  as the shorthand for the first invariant

$$\underline{v} = \begin{bmatrix} v_1 \\ v_2 \\ v_3 \end{bmatrix} = v_i \underline{e}_i := \underline{\underline{R}}^T \underline{r}' \quad \Rightarrow \quad \underline{r}' = \underline{\underline{R}} \underline{v} = v_i \underline{\underline{R}} \underline{e}_i = v_i \underline{d}_i$$

and from orthogonality of the director basis ( $\underline{d}_i \cdot \underline{d}_j = \delta_{ij}$ ) it follows that  $v_i = \underline{r}' \cdot \underline{d}_i$

$\rightarrow \underline{v} = \underline{\underline{R}}^T \underline{r}'$  is the centerline tangent vector resolved in the local director basis!

more specifically we have ...

- $v_1 = \underline{r}' \cdot \underline{d}_1$  : rate of transverse shift  $\hat{=}$  shear along  $\underline{d}_1$
- $v_2 = \underline{r}' \cdot \underline{d}_2$  : shear along  $\underline{d}_2$
- $v_3 = \underline{r}' \cdot \underline{d}_3$  : rate of axial shift  $\hat{=}$  axial stretch

## Invariant strain measures (part 3)

we define  $\underline{\underline{K}}$  as the shorthand for the second invariant

$$\underline{\underline{K}} = K_{ij} \underline{e}_i \otimes \underline{e}_j := \underline{\underline{R}}^T \underline{\underline{R}}'$$

**proof** that  $\underline{\underline{K}}$  is *skew-symmetric* ( $K_{ij} = -K_{ji}$ )

$$\underline{\underline{R}}^T \underline{\underline{R}} = \underline{\underline{I}} \quad \Rightarrow \quad \underline{\underline{R}}^T \underline{\underline{R}}' + \underline{\underline{R}}'^T \underline{\underline{R}} = \underline{\underline{0}} \quad \Rightarrow \quad \underline{\underline{K}} = -\underline{\underline{K}}^T$$

we therefore can write

$$\underline{\underline{K}} = \begin{bmatrix} 0 & -k_3 & k_2 \\ k_3 & 0 & -k_1 \\ -k_2 & k_1 & 0 \end{bmatrix}$$

and we recall that  $\underline{\underline{K}} \underline{a} = \underline{k} \times \underline{a} \quad \forall \underline{a} \in \mathbb{R}^3$  with  $\underline{k} = \text{axial}(\underline{\underline{K}}) = k_i \underline{e}_i$

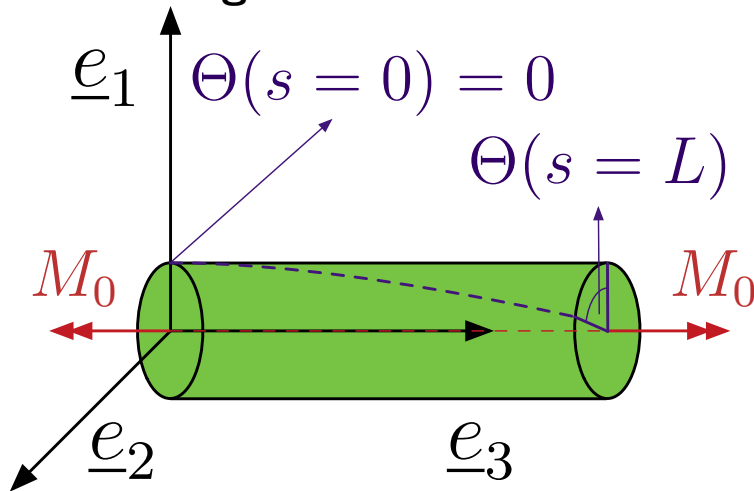
→ with  $(\underline{v}, \underline{k})$  we have the 6 strain measures given in the frame of the director basis!

→  $\psi = \psi(\underline{v}, \underline{\underline{K}}) =: \hat{\psi}(\underline{v}, \underline{k})$  (for easy notation we will drop the  $\hat{\cdot}$  in the sequel)

## Invariant strain measures (part 4)

to better understand the physical meaning of  $\underline{k}$  we have a look at two examples ...

### pure twisting of a rod



- rotation of cross sections around  $\underline{e}_3$ -axis
- angle of rotation for the cross section at  $s$  given by  $\Theta(s) = \Theta' \cdot s$  with  $\Theta' = \text{const.}$  because external moments act only at  $s = 0$  and  $s = L$

$$\underline{r}(s) = s \cdot \underline{e}_3 \Rightarrow \underline{r}'(s) = \underline{e}_3$$

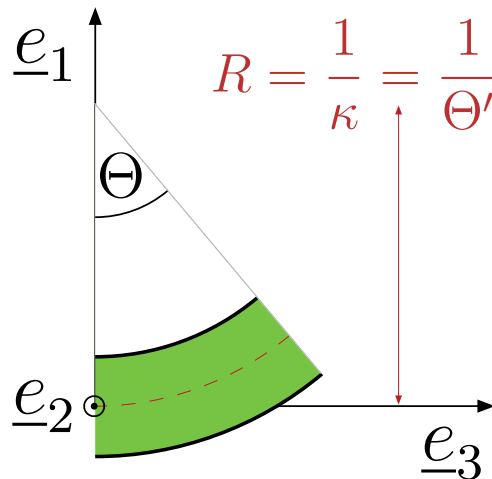
$$\Rightarrow \underline{v} = \underline{\underline{R}}^T \underline{r}' = \begin{bmatrix} 0 & 0 & 1 \end{bmatrix}^T \Rightarrow v_3 = 1$$

$$\underline{\underline{R}}(s) = \begin{bmatrix} +\cos(\Theta(s)) & -\sin(\Theta(s)) & 0 \\ +\sin(\Theta(s)) & +\cos(\Theta(s)) & 0 \\ 0 & 0 & 1 \end{bmatrix} \Rightarrow \underline{\underline{R}}'(s) = \Theta' \cdot \begin{bmatrix} -\sin(\Theta(s)) & -\cos(\Theta(s)) & 0 \\ +\cos(\Theta(s)) & -\sin(\Theta(s)) & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$\underline{\underline{K}}(s) = \underline{\underline{R}}^T(s) \underline{\underline{R}}'(s) = \begin{bmatrix} 0 & -\Theta' & 0 \\ +\Theta' & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \Rightarrow \underline{k} = \begin{bmatrix} 0 \\ 0 \\ \Theta' \end{bmatrix} \Rightarrow k_3 = \Theta'$$

## Invariant strain measures (part 5)

### pure bending of a rod



- rotation of cross sections around  $\underline{e}_2$ -axis
- angle of rotation for the cross section at  $s$  given by  $\Theta(s) = \Theta' \cdot s = \kappa \cdot s$  with  $\Theta' = \kappa = \text{const.}$  because external moments act only at  $s = 0$  and  $s = L$

$$\underline{r}(s) = s \cdot \underline{e}_3 \Rightarrow \underline{r}'(s) = \underline{e}_3$$

$$\Rightarrow \underline{v} = \underline{\underline{R}}^T \underline{r}' = \begin{bmatrix} 0 & 0 & 1 \end{bmatrix}^T \Rightarrow v_3 = 1$$

$$\underline{\underline{R}}(s) = \begin{bmatrix} +\cos(\Theta(s)) & 0 & +\sin(\Theta(s)) \\ 0 & 1 & 0 \\ -\sin(\Theta(s)) & 0 & +\cos(\Theta(s)) \end{bmatrix} \Rightarrow \underline{\underline{R}}'(s) = \Theta' \cdot \begin{bmatrix} -\sin(\Theta(s)) & 0 & +\cos(\Theta(s)) \\ 0 & 0 & 0 \\ -\cos(\Theta(s)) & 0 & -\sin(\Theta(s)) \end{bmatrix}$$

$$\underline{\underline{K}}(s) = \underline{\underline{R}}^T(s) \underline{\underline{R}}'(s) = \begin{bmatrix} 0 & 0 & +\Theta' \\ 0 & 0 & 0 \\ -\Theta' & 0 & 0 \end{bmatrix} \Rightarrow \underline{k} = \begin{bmatrix} 0 \\ \Theta' \\ 0 \end{bmatrix} \Rightarrow k_2 = \Theta'$$

## Recap of what we have so far

**balance equations** (in terms of nominal stresses in the reference configuration)

$$\underline{n}' + \underline{\hat{n}} = \underline{0} \quad \text{and} \quad \underline{m}' + \underline{r}' \times \underline{n} + \underline{\hat{m}} = \underline{0}$$

**kinematic quantities**  $\underline{v}, \underline{k}$

→ local strain measures

- $v_1, v_2, v_3$  : shear along  $d_1$ , shear along  $d_2$ , and stretch along  $d_3$ , respectively
- $k_1, k_2, k_3$  : curvature about  $d_1$ -, curvature about  $d_2$ -, and twist about  $d_3$ -axis,

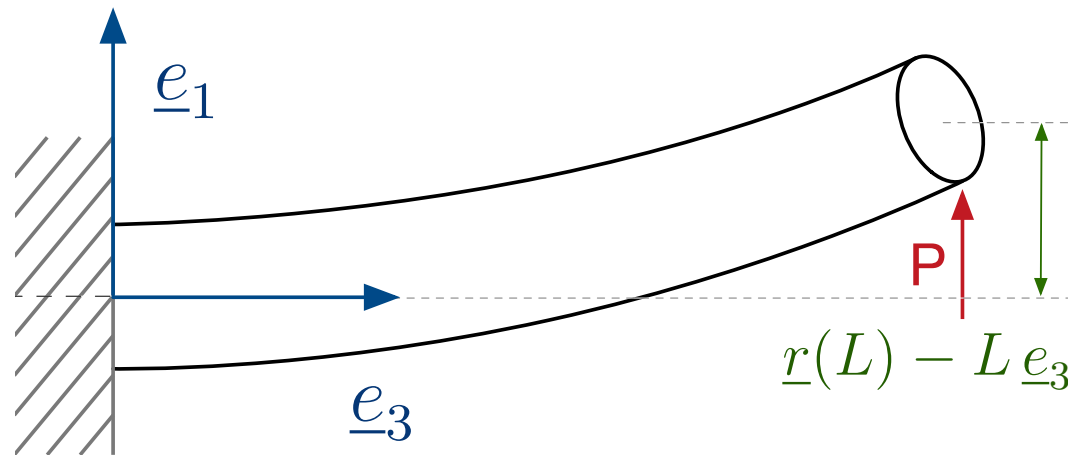
**deformation energy per unit of undeformed length**

$$\psi(\underline{\underline{R}}^T \underline{r}', \underline{\underline{R}}^T \underline{R}') = \psi(\underline{v}, \underline{\underline{K}}) = \psi(\underline{v}, \underline{k})$$

→ the derivatives of  $\psi$  with respect to the strains  $\underline{v}, \underline{k}$  are the stress measures resolved in the local director basis

## Minimum potential energy method (part 1)

we consider a system consisting of a beam with and an external load  $P$  at  $s = L$



the **total energy of the closed system** is given by

$$\Pi(\underline{r}, \underline{\underline{R}}) = \int_0^L \psi(\underline{v}, \underline{k}) \, ds - P \underline{e}_1 \cdot \underline{r}(L)$$

note regarding minus sign: load performs work on the beam,  
thereby lowering the load's potential and also the potential of the closed system

→ we now want to identify the **conditions for a minimum of  $\Pi$**



## Minimum potential energy method (part 2)

**perturbed versions of  $\underline{r}$  and  $\underline{R}$**

$$\underline{r}_\epsilon(s) = \underline{r}(s) + \epsilon \cdot \delta \underline{r}(s)$$

$$\underline{R}_\epsilon(s) = \underbrace{\underline{R}(s) + \epsilon \cdot \delta \underline{R}(s)}_{\notin SO(3)} = \underbrace{\exp(\epsilon \cdot \delta \underline{\Theta}(s))}_{\in SO(3)} \underline{R}(s) \quad \text{with} \quad \delta \underline{\Theta} := \text{axial}(\delta \underline{\Theta})$$

**perturbed version of total energy**

$$\Pi(\underline{r}_\epsilon, \underline{R}_\epsilon) = \int_0^L \psi(\underline{v}_\epsilon, \underline{k}_\epsilon) \, ds - P \underline{e}_1 \cdot \underline{r}_\epsilon(L) \stackrel{!}{=} \Pi(\underline{r}, \underline{R}) + \epsilon \cdot \delta \Pi(\underline{r}, \underline{R}) + o(\epsilon)$$

**first variation of total energy**

$$\delta \Pi = \left. \frac{d\Pi}{d\epsilon} \right|_{\epsilon=0} = \int_0^L \left( \frac{\partial \psi}{\partial \underline{v}}(s) \cdot \underbrace{\left. \frac{d\underline{v}_\epsilon}{d\epsilon}(s) \right|_{\epsilon=0}}_{\delta \underline{v}(s)} + \frac{\partial \psi}{\partial \underline{k}}(s) \cdot \underbrace{\left. \frac{d\underline{k}_\epsilon}{d\epsilon}(s) \right|_{\epsilon=0}}_{\delta \underline{k}(s)} \right) ds - P \underline{e}_1 \cdot \left. \frac{d\underline{r}_\epsilon}{d\epsilon}(L) \right|_{\epsilon=0}$$

in the sequel we will work on the terms  $\delta \underline{v}(s)$  and  $\delta \underline{k}(s)$

## Minimum potential energy method (part 3)

**perturbed version of  $\underline{v}$**

$$\underline{v}_\epsilon(s) = \underline{\underline{R}}_\epsilon^T(s) \cdot \underline{r}'_\epsilon(s) = \underline{\underline{R}}^T(s) \cdot \left( \exp(\epsilon \cdot \delta \underline{\underline{\Theta}}(s)) \right)^T \cdot \left( \underline{r}'(s) + \epsilon \cdot \delta \underline{r}'(s) \right)$$

**first variation of  $\underline{v}$**

$$\delta \underline{v}(s) = \left. \frac{d \underline{v}_\epsilon(s)}{d\epsilon} \right|_{\epsilon=0} = \underline{\underline{R}}^T(s) \cdot \left. \frac{d}{d\epsilon} \left( \exp(\epsilon \cdot \delta \underline{\underline{\Theta}}(s)) \right)^T \right|_{\epsilon=0} \cdot \underline{r}'(s) + \underline{\underline{R}}^T(s) \cdot \delta \underline{r}'(s)$$

$$\left. \frac{d}{d\epsilon} \left( \exp(\epsilon \cdot \delta \underline{\underline{\Theta}}(s)) \right)^T \right|_{\epsilon=0} = \left. \frac{d}{d\epsilon} \left( \underline{\underline{I}} + \epsilon \cdot \delta \underline{\underline{\Theta}} + \frac{\epsilon^2}{2} \cdot \delta \underline{\underline{\Theta}}^2 + \dots \right)^T \right|_{\epsilon=0} = \left( \delta \underline{\underline{\Theta}} \right)^T = -\delta \underline{\underline{\Theta}}$$

$$\Rightarrow \delta \underline{v}(s) = \underline{\underline{R}}^T(s) \cdot \left( -\delta \underline{\underline{\Theta}}(s) \cdot \underline{r}'(s) + \delta \underline{r}'(s) \right)$$

## Minimum potential energy method (part 4)

**perturbed version of  $\underline{\underline{K}}$ , respectively  $\underline{k}$**

$$\underline{\underline{K}}_{\epsilon}(s) = \underline{\underline{R}}_{\epsilon}^T(s) \cdot \underline{\underline{R}}'_{\epsilon}(s) \quad \text{and} \quad \underline{k}_{\epsilon}(s) = \text{axial}(\underline{\underline{K}}_{\epsilon}(s))$$

**first variation of  $\underline{k}$**

$$\begin{aligned} \delta \underline{k}(s) &= \left. \frac{d \underline{k}_{\epsilon}(s)}{d \epsilon} \right|_{\epsilon=0} = \left. \frac{d}{d \epsilon} \left( \text{axial}(\underline{\underline{R}}_{\epsilon}^T(s) \cdot \underline{\underline{R}}'_{\epsilon}(s)) \right) \right|_{\epsilon=0} = \\ &= \text{axial} \left( \left. \frac{d}{d \epsilon} (\underline{\underline{R}}_{\epsilon}^T(s) \cdot \underline{\underline{R}}'_{\epsilon}(s)) \right|_{\epsilon=0} \right) \stackrel{(1)}{=} \text{axial}(\underline{\underline{R}}^T(s) \cdot \delta \underline{\underline{\Theta}}'(s) \cdot \underline{\underline{R}}) \stackrel{(2)}{=} \underline{\underline{R}}^T(s) \cdot \delta \underline{\underline{\Theta}}'(s) \end{aligned}$$

regarding equality (2) we have ...

$$(\underline{\underline{R}}^T \cdot \delta \underline{\underline{\Theta}}' \cdot \underline{\underline{R}}) \cdot \underline{a} = \underline{\underline{R}}^T \cdot \delta \underline{\underline{\Theta}}' \cdot (\underline{\underline{R}} \cdot \underline{a}) = \underline{\underline{R}}^T \cdot \left( \delta \underline{\underline{\Theta}}' \times (\underline{\underline{R}} \cdot \underline{a}) \right) = (\underline{\underline{R}}^T \cdot \delta \underline{\underline{\Theta}}') \times \underline{a}$$

## Minimum potential energy method (part 5)

and regarding equality (1) we have ...

$$\begin{aligned} & \left. \frac{d}{d\epsilon} \left( \underline{\underline{R}}^\text{T}_\epsilon(s) \cdot \underline{\underline{R}}'_\epsilon(s) \right) \right|_{\epsilon=0} = \\ &= \frac{d}{d\epsilon} \left[ \underline{\underline{R}}^\text{T} \cdot \left( \exp(\epsilon \cdot \delta \underline{\underline{\Theta}}) \right)^\text{T} \cdot \left( \left( \exp(\epsilon \cdot \delta \underline{\underline{\Theta}}) \right)' \cdot \underline{\underline{R}} + \exp(\epsilon \cdot \delta \underline{\underline{\Theta}}) \cdot \underline{\underline{R}}' \right) \right] \Big|_{\epsilon=0} = \\ &= \left[ -\cancel{\underline{\underline{R}}^\text{T} \cdot \delta \underline{\underline{\Theta}} \cdot \underline{\underline{R}}'} + \underline{\underline{R}}^\text{T} \cdot \left( \delta \underline{\underline{\Theta}}' \cdot \underline{\underline{R}} + \cancel{\delta \underline{\underline{\Theta}} \cdot \underline{\underline{R}}'} \right) \right] = \underline{\underline{R}}^\text{T} \cdot \delta \underline{\underline{\Theta}}' \cdot \underline{\underline{R}} \end{aligned}$$

... where we used ...

$$\begin{aligned} \left. \frac{d}{d\epsilon} \left( \exp(\epsilon \cdot \delta \underline{\underline{\Theta}}(s)) \right)' \right|_{\epsilon=0} &= \left. \frac{d}{d\epsilon} \left( \underline{\underline{I}} + \epsilon \cdot \delta \underline{\underline{\Theta}} + \frac{\epsilon^2}{2} \cdot \delta \underline{\underline{\Theta}}^2 + \dots \right)' \right|_{\epsilon=0} = \\ &= \left. \frac{d}{d\epsilon} \left( \epsilon \cdot \delta \underline{\underline{\Theta}}' + \frac{\epsilon^2}{2} \cdot \delta \underline{\underline{\Theta}}'^2 + \dots \right)' \right|_{\epsilon=0} = \delta \underline{\underline{\Theta}}' \end{aligned}$$

## Minimum potential energy method (part 6)

plugging back into the **first variation of total energy** we get ...

$$\begin{aligned}
 \delta \Pi &= \int_0^L \left( \frac{\partial \psi}{\partial \underline{v}}(s) \cdot \left( \underline{\underline{R}}^T(s) \cdot \left( -\delta \underline{\underline{\Theta}}(s) \cdot \underline{r}'(s) + \delta \underline{r}'(s) \right) \right) + \right. \\
 &\quad \left. + \frac{\partial \psi}{\partial \underline{k}}(s) \cdot \left( \underline{\underline{R}}^T(s) \cdot \delta \underline{\underline{\Theta}}'(s) \right) \right) ds - P \underline{e}_1 \cdot \delta \underline{r}(L) = \\
 &= \int_0^L \left( \left( \underline{\underline{R}} \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right) \cdot \left( \delta \underline{r}'(s) + \underline{r}'(s) \times \delta \underline{\underline{\Theta}}(s) \right) + \right. \\
 &\quad \left. + \left( \underline{\underline{R}} \cdot \frac{\partial \psi}{\partial \underline{k}}(s) \right) \cdot \delta \underline{\underline{\Theta}}'(s) \right) ds - P \underline{e}_1 \cdot \delta \underline{r}(L) \stackrel{\text{IBP}}{=} \\
 &= \left[ \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \cdot \delta \underline{r}(s) \right]_0^L - \int_0^L \left( \left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right)' \cdot \delta \underline{r}(s) \right) ds - P \underline{e}_1 \cdot \delta \underline{r}(L) + \\
 &+ \left[ \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s) \cdot \delta \underline{\underline{\Theta}}(s) \right]_0^L - \int_0^L \left( \left( \left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s) \right)' + \underline{r}'(s) \times \left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right) \right) \cdot \delta \underline{\underline{\Theta}}(s) \right) ds
 \end{aligned}$$

IBP : integration by parts

## Minimum potential energy method (part 7)

to finally obtain the conditions for a minimum of the potential energy we set  $\delta\Pi = 0$

the integrals must each be 0 regardless of the variations  $\delta\underline{r}$  and  $\delta\underline{\Theta}$

this gives us the conditions

$$\left(\underline{\underline{R}}(s) \cdot \frac{\partial\psi}{\partial\underline{v}}(s)\right)' = \underline{0}$$

$$\left(\underline{\underline{R}}(s) \cdot \frac{\partial\psi}{\partial\underline{k}}(s)\right)' + \underline{r}'(s) \times \left(\underline{\underline{R}}(s) \cdot \frac{\partial\psi}{\partial\underline{v}}(s)\right) = \underline{0}$$

which must be satisfied for  $s \in [0, L]$  in a point-wise manner

at the boundary  $s = 0$  the cross section is fixed  $\Rightarrow \delta\underline{r}(0) \stackrel{!}{=} \underline{0}$  and  $\delta\underline{\Theta}(0) \stackrel{!}{=} \underline{0}$

at the boundary  $s = L$  we get the condition

$$\left(\underline{\underline{R}}(L) \cdot \frac{\partial\psi}{\partial\underline{v}}(L) - P \underline{e}_1\right) \cdot \delta\underline{r}(L) + \underline{\underline{R}}(L) \cdot \frac{\partial\psi}{\partial\underline{k}}(L) \cdot \delta\underline{\Theta}(L) = \underline{0}$$

because  $\delta\underline{r}(L)$  and  $\delta\underline{\Theta}(L)$  are independent we get

$$\underline{\underline{R}}(L) \cdot \frac{\partial\psi}{\partial\underline{v}}(L) = P \underline{e}_1 \quad \text{and} \quad \underline{\underline{R}}(L) \cdot \frac{\partial\psi}{\partial\underline{k}}(L) = \underline{0}$$

## Minimum potential energy method (part 8)

inside the domain  $s \in [0, L]$  we obtained the conditions

$$\left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right)' = \underline{0}$$

$$\left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s) \right)' + \underline{r}'(s) \times \left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right) = \underline{0}$$

balance of linear / angular momentum gives

$$\underline{n}'(s) + \cancel{\hat{n}(s)} = \underline{0}$$

$$\underline{m}'(s) + \underline{r}'(s) \times \underline{n}(s) + \cancel{\hat{m}(s)} = \underline{0}$$

note: there are no distributed loads in the example

we identify the relations

$$\underline{n}(s) = \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s)$$

$$\underline{m}(s) = \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s)$$

## Relationship between potential energy and kinetic quantities

$$\underline{n}(s) = \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) = n_i \underline{e}_i = N_i \underline{d}_i$$

$$\underline{m}(s) = \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s) = m_i \underline{e}_i = M_i \underline{d}_i$$

with  $\underline{d}_i(s) = \underline{\underline{R}}(s) \cdot \underline{e}_i$  ; it follows that

$$N_i(s) = \frac{\partial \psi}{\partial v_i}(s)$$

$$M_i(s) = \frac{\partial \psi}{\partial k_i}(s)$$

- $N_1, N_2$  are the shear forces in direction  $\underline{d}_1, \underline{d}_2$ , respectively
- $N_3$  is the axial force (in direction  $\underline{d}_3$ )
- $M_1, M_2$  are the bending moments about the  $\underline{d}_1$ -,  $\underline{d}_2$ -axis, respectively
- $M_3$  is the twisting moment about the  $\underline{d}_3$ -axis



## Energy potential for isotropic circular beams

$$\psi(\underline{v}, \underline{k}) = \frac{1}{2} \mathcal{C} v_1^2 + \frac{1}{2} \mathcal{C} v_2^2 + \frac{1}{2} \mathcal{D} (v_3 - 1)^2 + \frac{1}{2} \mathcal{A} k_1^2 + \frac{1}{2} \mathcal{A} k_2^2 + \frac{1}{2} \mathcal{B} k_3^2$$

- shearing stiffness

$$N_1(s) = \frac{\partial \psi}{\partial v_1}(s) = \mathcal{C} v_1(s) \stackrel{!}{=} k G A v_1(s) \Rightarrow \mathcal{C} = k G A$$

- stretching stiffness

$$N_3(s) = \frac{\partial \psi}{\partial v_3}(s) = \mathcal{D} (v_3(s) - 1) \stackrel{!}{=} E A (v_3(s) - 1) \Rightarrow \mathcal{D} = E A$$

- bending stiffness

$$M_1(s) = \frac{\partial \psi}{\partial k_1}(s) = \mathcal{A} k_1(s) \stackrel{!}{=} E I k_1(s) \Rightarrow \mathcal{A} = E I$$

- twisting stiffness

$$M_3(s) = \frac{\partial \psi}{\partial k_3}(s) = \mathcal{B} k_3(s) \stackrel{!}{=} G J k_3(s) \Rightarrow \mathcal{B} = G J$$

similarly in 3D-elasticity we have as a constitutive law

$$\underline{\underline{P}} = \frac{\partial W(\underline{\underline{F}})}{\partial \underline{\underline{F}}}$$

## Model equations

**force balance** with constitutive law

$$\left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right)' + \hat{n}(s) = \underline{\underline{R}}'(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) + \underline{\underline{R}}(s) \cdot \left( \frac{\partial \psi}{\partial \underline{v}}(s) \right)' + \hat{n}(s) = \underline{0}$$

**moment balance** with constitutive law

$$\begin{aligned} & \left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s) \right)' + \underline{r}'(s) \times \left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right) + \hat{m}(s) = \\ & = \underline{\underline{R}}'(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s) + \underline{\underline{R}}(s) \cdot \left( \frac{\partial \psi}{\partial \underline{k}}(s) \right)' + \underline{r}'(s) \times \left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right) + \hat{m}(s) = \underline{0} \end{aligned}$$

we now want to write the equations in terms of  $\underline{v}$  and  $\underline{k}$  ...

## Force balance

multiplying with  $\underline{\underline{R}}^T$  on the left we get ...

$$\begin{aligned} & \underline{\underline{R}}^T(s) \cdot \underline{\underline{R}}'(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) + \left( \frac{\partial \psi}{\partial \underline{v}}(s) \right)' + \underline{\underline{R}}^T(s) \cdot \hat{\underline{n}}(s) = \\ & = \underline{k}(s) \times \frac{\partial \psi}{\partial \underline{v}}(s) + \frac{\partial^2 \psi}{\partial \underline{v}^2}(s) \cdot \underline{v}'(s) + \frac{\partial^2 \psi}{\partial \underline{v} \partial \underline{k}}(s) \cdot \underline{k}'(s) + \tilde{\underline{n}}(s) = \underline{0} \end{aligned}$$

with

$$\tilde{\underline{n}}(s) := \underline{\underline{R}}^T(s) \cdot \hat{\underline{n}}(s)$$

## Moment balance

again multiplying with  $\underline{\underline{R}}^T$  on the left we get ...

$$\begin{aligned} & \underline{\underline{R}}^T(s) \cdot \underline{\underline{R}}'(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s) + \left( \frac{\partial \psi}{\partial \underline{k}}(s) \right)' + \\ & + \underline{\underline{R}}^T(s) \cdot \left( \underline{r}'(s) \times \left( \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \right) \right) + \underline{\underline{R}}^T(s) \cdot \underline{\hat{m}}(s) = \\ & = \underline{k}(s) \times \frac{\partial \psi}{\partial \underline{k}}(s) + \frac{\partial^2 \psi}{\partial \underline{v} \partial \underline{k}}(s) \cdot \underline{v}'(s) + \frac{\partial^2 \psi}{\partial \underline{k}^2}(s) \cdot \underline{k}'(s) + \\ & + \underline{v}(s) \times \frac{\partial \psi}{\partial \underline{v}}(s) + \underline{\tilde{m}}(s) = \underline{0} \end{aligned}$$

with

$$\underline{\tilde{m}}(s) := \underline{\underline{R}}^T(s) \cdot \underline{\hat{m}}(s)$$

## Model equations

### force balance

$$\underline{k}(s) \times \frac{\partial \psi}{\partial \underline{v}}(s) + \frac{\partial^2 \psi}{\partial \underline{v}^2}(s) \cdot \underline{v}'(s) + \frac{\partial^2 \psi}{\partial \underline{v} \partial \underline{k}}(s) \cdot \underline{k}'(s) + \underline{\tilde{n}}(s) = \underline{0}$$

### and moment balance

$$\begin{aligned} \underline{k}(s) \times \frac{\partial \psi}{\partial \underline{k}}(s) + \frac{\partial^2 \psi}{\partial \underline{v} \partial \underline{k}}(s) \cdot \underline{v}'(s) + \frac{\partial^2 \psi}{\partial \underline{k}^2}(s) \cdot \underline{k}'(s) + \\ + \underline{v}(s) \times \frac{\partial \psi}{\partial \underline{v}}(s) + \underline{\tilde{m}}(s) = \underline{0} \end{aligned}$$

expressed in terms of the **strains** (in the director basis)

$$\underline{v}(s) = \underline{\underline{R}}^T(s) \cdot \underline{r}'(s) \quad ; \quad \underline{k}(s) = \text{axial}(\underline{\underline{R}}^T(s) \cdot \underline{\underline{R}}'(s))$$

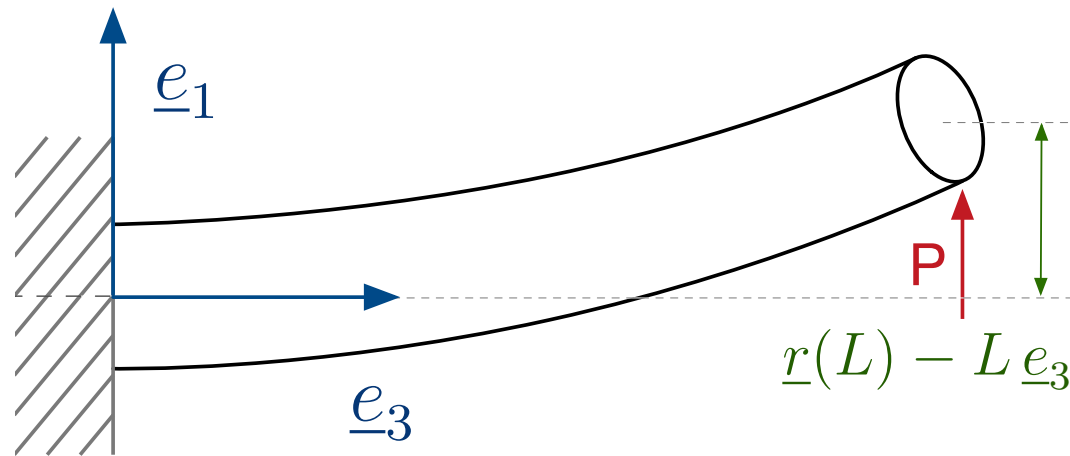
given the **distributed external loads** (resolved in the director basis)

$$\underline{\tilde{n}}(s) = \underline{\underline{R}}^T(s) \cdot \underline{\hat{n}}(s) \quad ; \quad \underline{\tilde{m}}(s) = \underline{\underline{R}}^T(s) \cdot \underline{\hat{m}}(s)$$

→ system of 6 equations ; 1<sup>st</sup> order in  $\underline{v}$ ,  $\underline{k}$  but 2<sup>nd</sup> order in  $\underline{r}$ ,  $\underline{\underline{R}}$

→ 12 boundary conditions required

## Example with boundary conditions



we have 12 boundary conditions:

boundary conditions at  $s = 0$ :

- $\underline{r}(0) = \underline{0}$
- $\underline{R}(0) = \underline{I} = \exp \underline{\Theta} \iff \underline{\Theta} = \underline{0}$

boundary conditions at  $s = L$ :

- $\underline{R} \cdot \frac{\partial \psi}{\partial \underline{v}}(L) = P \cdot \underline{e}_1$
- $\frac{\partial \psi}{\partial \underline{k}}(L) = \underline{0} \iff M_i = 0 \ (i \in 1, 2, 3)$

note that the boundary conditions are not stated in our unknown variables  $\underline{v}$ ,  $\underline{k}$

## Model as a system of first order equations (part 1)

boundary conditions for forces and moments can be written in terms of strains ; this is not possible with the boundary conditions for displacements and rotations ; therefore the system is not closed

to *close the system*, we will rewrite the model as 12 first order equations, by including 6 extra equations relating strain quantities to displacement quantities:

$$\underline{v} = \underline{\underline{R}}^T \cdot \underline{r}' \Rightarrow \underline{r}' = \underline{\underline{R}} \cdot \underline{v}$$
$$\underline{\underline{K}} = \underline{\underline{R}}^T \cdot \underline{\underline{R}}' \Rightarrow \underline{\underline{R}}' = \underline{\underline{R}} \cdot \underline{\underline{K}}$$

we already noticed, that using unit quaternions to encode  $SO(3)$  matrices has some great advantages for computation. therefore we chose to replace the matrix differential equation in  $\underline{\underline{R}}$  by a matrix differential equation in terms of unit quaternions  $\underline{q} \in \mathbb{R}^4$

$$\underline{q}' = \underline{\underline{E}}(\underline{q}) \cdot \underline{k} = \frac{1}{2} \cdot \begin{bmatrix} -q_1 & -q_2 & -q_3 \\ +q_0 & -q_3 & +q_2 \\ +q_3 & +q_0 & -q_1 \\ -q_2 & +q_1 & +q_0 \end{bmatrix} \cdot \underline{k}$$

## Model as a system of first order equations (part 2)

we have the same 6 equations as before here expressed in matrix form

$$\underbrace{\begin{bmatrix} \frac{\partial^2 \psi}{\partial v^2} & \frac{\partial^2 \psi}{\partial v \partial k} \\ \frac{\partial^2 \psi}{\partial k \partial v} & \frac{\partial^2 \psi}{\partial k^2} \end{bmatrix}}_{=: \underline{\underline{C}}} \cdot \begin{bmatrix} \underline{v}' \\ \underline{k}' \end{bmatrix} = \begin{bmatrix} -\underline{k} \times \frac{\partial \psi}{\partial v} \\ -\underline{k} \times \frac{\partial \psi}{\partial k} - \underline{v} \times \frac{\partial \psi}{\partial v} \end{bmatrix} - \begin{bmatrix} \underline{\underline{R}}^T \hat{n} \\ \underline{\underline{R}}^T \hat{m} \end{bmatrix}$$

$\underline{\underline{C}}$  is the (positive definite) elasticity tensor matrix

we close the system by adding the seven extra first order equations  
(relating derivatives of displacements to strains)

$$\underline{r}' = \underline{\underline{R}} \cdot \underline{v}$$

$$\underline{q}' = \underline{\underline{E}}(\underline{q}) \cdot \underline{k}$$

→ this gives us a system of 13 coupled nonlinear ODEs



## Extra constraint for unit quaternions

using unit quaternions to encode rotations leads to a system of 13 equations instead of just 12 ; this extra equation is contained within

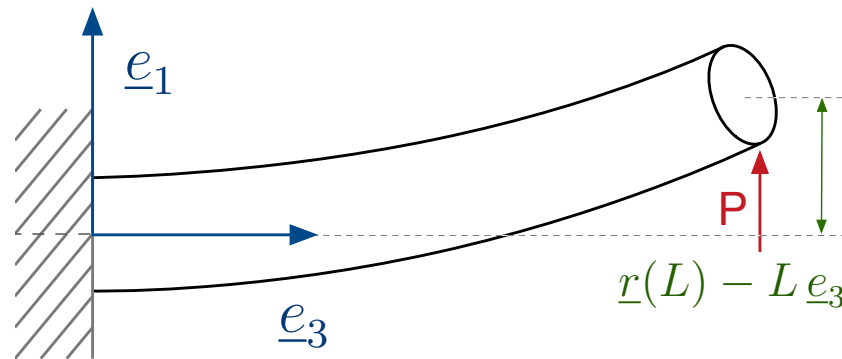
$$\underline{q}' = \underline{E}(\underline{q}) \cdot \underline{k} \quad (\star)$$

since rotations are encoded by *unit* quaternions the constraint  $\|\underline{q}\|_2 \stackrel{!}{=} 1$  must be satisfied from the constraint we obtain a necessary condition:  $\underline{q} \cdot \underline{q} \stackrel{!}{=} 1 \Rightarrow \underline{q} \cdot \underline{q}' \stackrel{!}{=} 0$ , which is automatically satisfied by  $(\star)$ : dotting  $(\star)$  with  $\underline{q}$  gives ...

$$\underline{q}' \cdot \underline{q} = (\underline{E}(\underline{q}) \cdot \underline{k}) \cdot \underline{q} = \underline{k} \cdot (\underline{E}^T \cdot \underline{q}) = \underline{k} \cdot \left( \begin{bmatrix} -q_1 & +q_0 & +q_3 & -q_2 \\ -q_2 & -q_3 & +q_0 & +q_1 \\ -q_3 & +q_2 & -q_1 & +q_0 \end{bmatrix} \cdot \begin{bmatrix} q_0 \\ q_1 \\ q_2 \\ q_3 \end{bmatrix} \right) = \underline{k} \cdot \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} = 0$$

adding one (13<sup>th</sup>) boundary condition for  $\underline{q}$  (either  $\|\underline{q}\|_2 \Big|_{s=0} = 1$  or  $\|\underline{q}\|_2 \Big|_{s=L} = 1$ ) is (in combination with the automatically satisfied necessary condition) sufficient to satisfy the constraint for all  $s$

## Example with boundary conditions (revisited)



we have 13 boundary conditions:

boundary conditions at  $s = 0$ :

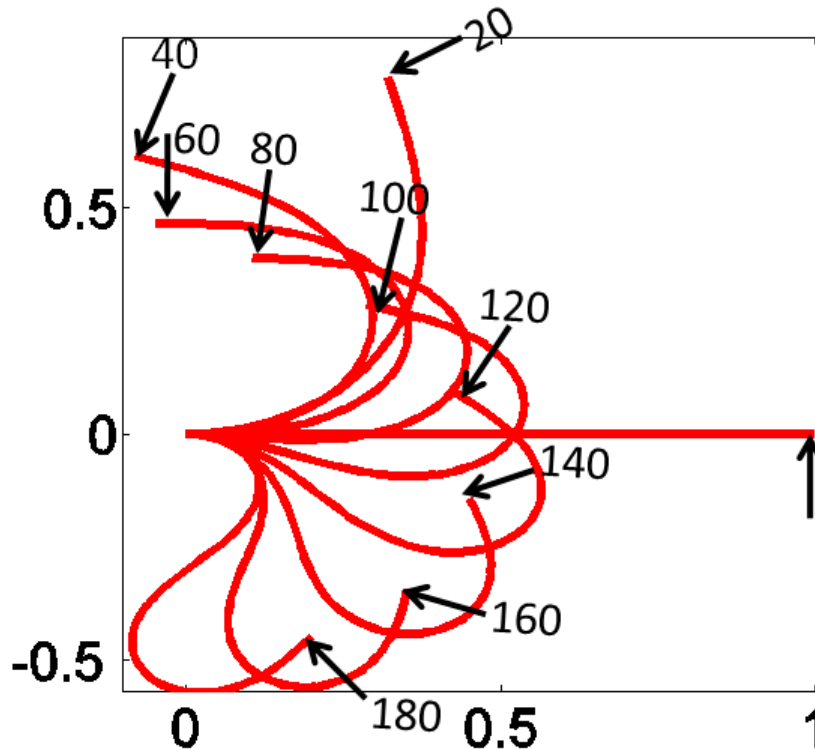
- $\underline{r}(0) = \underline{0}$
- $\underline{q}(0) = \begin{bmatrix} 1 & 0 & 0 & 0 \end{bmatrix}^T$  because  
 $q_0 = \cos\left(\frac{\Theta}{2}\right) = 1$  and

$$\begin{bmatrix} q_1 \\ q_2 \\ q_3 \end{bmatrix} = \sin\left(\frac{\Theta}{2}\right) \cdot \underline{a} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix}$$

boundary conditions at  $s = L$ :

- $\underline{\underline{R}} \cdot \frac{\partial \psi}{\partial \underline{v}}(L) = P \cdot \underline{e}_1$
- $\frac{\partial \psi}{\partial \underline{k}}(L) = \underline{0} \iff$   
 $M_i(L) = 0 \ (i \in 1, 2, 3)$

## Another example with boundary conditions: follower load problem



boundary conditions at  $s = 0$ :

- $\underline{r}(0) = \underline{0}$

- $\underline{q}(0) = [1 \ 0 \ 0 \ 0]^T$  because

$$q_0 = \cos\left(\frac{\Theta}{2}\right) = 1 \text{ and}$$

$$\begin{bmatrix} q_1 \\ q_2 \\ q_3 \end{bmatrix} = \sin\left(\frac{\Theta}{2}\right) \cdot \underline{a} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix}$$

boundary conditions at  $s = L$ :

- $\underline{R} \cdot \frac{\partial \psi}{\partial \underline{v}}(L) = P \cdot \underline{d}_1 = \underline{R} \cdot P \cdot \underline{e}_1 \Rightarrow$

$$\frac{\partial \psi}{\partial v_1}(L) = P$$

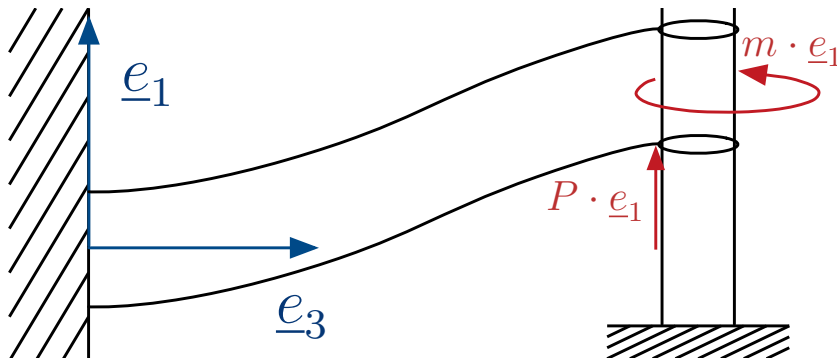
$$\frac{\partial \psi}{\partial v_2}(L) = 0$$

$$\frac{\partial \psi}{\partial v_3}(L) = 0$$

- $\frac{\partial \psi}{\partial \underline{k}}(L) = \underline{0} \iff$

$$M_i(L) = 0 \ (i \in 1, 2, 3)$$

## Yet another example with boundary conditions



boundary conditions at  $s = L$ :

- $n_1(L) = P$

$$\underline{\underline{R}} \cdot \frac{\partial \psi}{\partial \underline{v}}(L) = P \cdot \underline{e}_1 \Rightarrow$$

$$\left( \underline{\underline{R}} \cdot \frac{\partial \psi}{\partial \underline{v}}(L) \right) \cdot \underline{e}_1 = P \wedge \underline{e}_1 = \underline{d}_1(L) \Rightarrow$$

$$\frac{\partial \psi}{\partial v_1}(L) = P$$

boundary conditions at  $s = 0$ :

- $\underline{r}(0) = \underline{0}$

- $\underline{q}(0) = \begin{bmatrix} 1 & 0 & 0 & 0 \end{bmatrix}^T$  because

$$q_0 = \cos\left(\frac{\Theta}{2}\right) = 1 \text{ and}$$

$$\begin{bmatrix} q_1 \\ q_2 \\ q_3 \end{bmatrix} = \sin\left(\frac{\Theta}{2}\right) \cdot \underline{a} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix}$$

- $r_2(L) = 0$

- $r_3(L) = L$

- $\frac{\partial \psi}{\partial k_1}(L) = m$

- $\begin{bmatrix} q_1 \\ q_2 \\ q_3 \end{bmatrix} = \sin\left(\frac{\Theta}{2}\right) \cdot \begin{bmatrix} 1 \\ 0 \\ 0 \end{bmatrix} \Rightarrow q_2 = 0, q_3 = 0$

## Material symmetry in 3D elasticity

$$W(\underline{\underline{F}}) \stackrel{\star}{=} W(\underline{\underline{U}}) \stackrel{\diamond}{=} W(I_1, I_2, I_3)$$

- $\star$  : principle of frame-indifference
- $\underline{\underline{U}}$  : from polar decomposition  $\underline{\underline{F}} = \underline{\underline{\tilde{R}}} \cdot \underline{\underline{U}}$
- $\diamond$  : isotropy: material law is the same for all directions in the material
- $I_1, I_2, I_3$  : strain-invariants
- in case of an orthotropic material law, there would simply result a few more invariants ; orthotropy here means that the material behavior in  $s$  direction differs from the behavior in  $X_1, X_2$  directions

### how to obtain the specific form of the energy potential function?

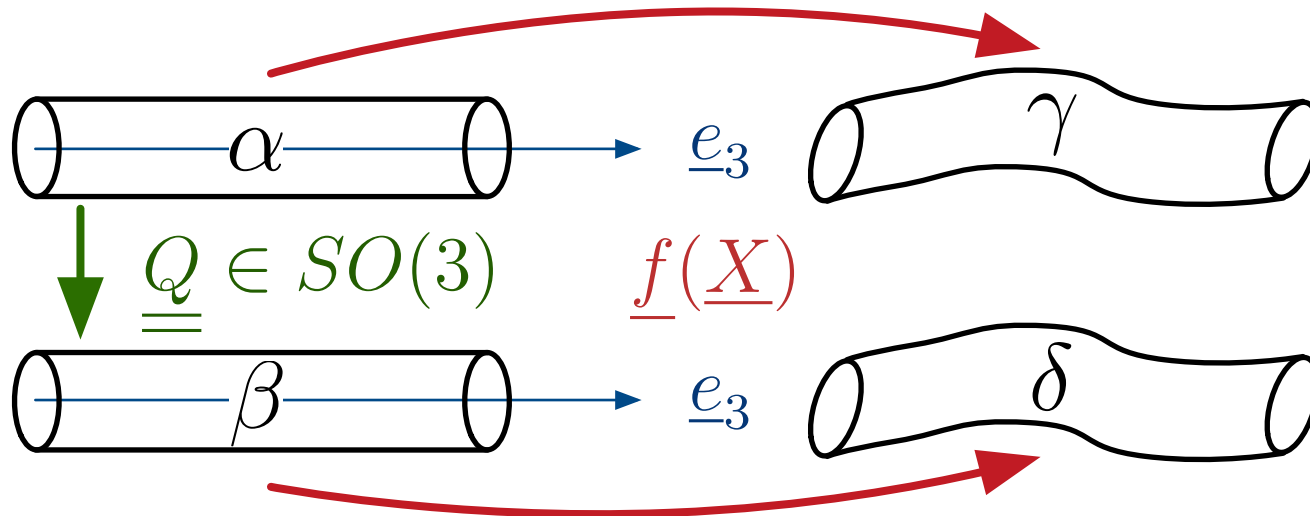
remember for example the already postulated function

$$\psi(\underline{v}, \underline{k}) = \frac{1}{2}\mathcal{C} v_1^2 + \frac{1}{2}\mathcal{C} v_2^2 + \frac{1}{2}\mathcal{D} (v_3 - 1)^2 + \frac{1}{2}\mathcal{A} k_1^2 + \frac{1}{2}\mathcal{A} k_2^2 + \frac{1}{2}\mathcal{B} k_3^2$$

→ how to identify the strain invariants?

## Material symmetry in elastic rods (part 1)

to get a better understanding of the concept of material symmetry,  
we set up **another thought experiment**



- we start with the reference configuration  $\alpha$
  - we rotate  $\alpha$  by  $\underline{\underline{Q}}$  to get  $\beta$  (a rotated reference configuration)
  - $\underline{\underline{f}}(\cdot)$  maps  $\alpha \mapsto \gamma$  and  $\beta \mapsto \delta$
- if  $\underline{\underline{Q}}$  is in the symmetry group  $\mathcal{G}$  then then we have  $\psi|_{\gamma} = \psi|_{\delta}$

## Material symmetry in elastic rods (part 2)

### 1. configuration $\gamma$ (image of reference configuration $\alpha$ )

- $\underline{r}^\gamma(s)$
- $\underline{\underline{R}}^\gamma(s)$

### 2. configuration $\delta$ (image of rotated reference configuration $\beta$ )

- $\underline{r}^\delta(s) = \underline{r}^\gamma(s)$
- $\underline{\underline{R}}^\delta(s) = \underline{\underline{R}}^\gamma(s) \cdot \underline{\underline{Q}}$
- $\underline{v}^\delta(s) = \underline{\underline{R}}^{\gamma T}(s) \cdot \underline{r}^{\gamma'}(s) = \underline{\underline{Q}}^T \cdot \underline{\underline{R}}^{\gamma T}(s) \cdot \underline{r}^{\gamma'}(s) = \underline{\underline{Q}}^T \cdot \underline{v}^\gamma(s)$
- $\underline{k}^\delta = \text{axial}(\underline{\underline{R}}^{\delta T} \cdot \underline{\underline{R}}^{\delta'}) = \text{axial}(\underline{\underline{Q}}^T \cdot \underline{\underline{R}}^{\gamma T} \cdot \underline{\underline{R}}^{\gamma'} \cdot \underline{\underline{Q}}) = \underline{\underline{Q}}^T \cdot \text{axial}(\underline{\underline{R}}^{\gamma T} \cdot \underline{\underline{R}}^{\gamma'}) = \underline{\underline{Q}}^T \cdot \underline{k}^\gamma$

$$\Rightarrow \psi(\underline{\underline{Q}}^T \cdot \underline{v}^\gamma, \underline{\underline{Q}}^T \cdot \underline{k}^\gamma) \stackrel{!}{=} \psi(\underline{v}^\gamma, \underline{k}^\gamma) \quad \forall \underline{\underline{Q}} \in \mathcal{G}$$

- the symmetry group  $\mathcal{G}$  depends on geometry and on the material of the beam
- $\mathcal{G} \equiv SO(2)$  (all rotations about  $\underline{e}_3$ -axis), if beam has circular cross section and isotropic material ; also the case for circular ropes with continuous helicity

## Material symmetry in elastic rods (part 3)

with  $\mathcal{G} \equiv SO(2)$  we had ...

$$\underline{\underline{Q}} = \begin{bmatrix} +\cos(\Theta) & -\sin(\Theta) & 0 \\ +\sin(\Theta) & +\cos(\Theta) & 0 \\ 0 & 0 & 1 \end{bmatrix} \quad \forall \Theta \in [0, 2\pi]$$

$$\underline{v}^\delta = \underline{\underline{Q}}^T \cdot \underline{v}^\gamma = \begin{bmatrix} \begin{bmatrix} +\cos(\Theta) & +\sin(\Theta) \\ -\sin(\Theta) & +\cos(\Theta) \end{bmatrix} \cdot \begin{bmatrix} v_1^\gamma \\ v_2^\gamma \end{bmatrix} \\ v_3^\gamma \end{bmatrix} ; \quad \underline{v}^\delta = \dots \text{ (replace } v \text{ by } k)$$

we introduce some abbreviations ...

$$\underline{v}^\delta = \begin{bmatrix} \underline{\hat{Q}}^T \cdot \underline{\hat{v}}^\gamma \\ v_3^\gamma \end{bmatrix} \quad \text{with } \underline{\hat{v}}^\gamma = \begin{bmatrix} v_1^\gamma \\ v_2^\gamma \end{bmatrix} \quad \text{and } \underline{\hat{Q}} = \begin{bmatrix} +\cos(\Theta) & -\sin(\Theta) \\ +\sin(\Theta) & +\cos(\Theta) \end{bmatrix}$$

we observe the following **invariants** under all such transformations  $\underline{\underline{Q}} \in \mathcal{G}$

- angle between and magnitude of  $\underline{\hat{v}}, \underline{\hat{k}} \rightarrow \|\underline{\hat{v}}\|, \|\underline{\hat{k}}\|, \underline{\hat{v}} \cdot \underline{\hat{k}}, (\underline{\hat{v}} \times \underline{\hat{k}}) \cdot \underline{e}_3$
- $v_3, k_3$

$$\Rightarrow \psi = \psi \left( \underbrace{v_1^2 + v_2^2}_{I_1}, \underbrace{k_1^2 + k_2^2}_{I_2}, \underbrace{v_1 \cdot k_1 + v_2 \cdot k_2}_{I_3}, \underbrace{v_1 \cdot k_2 - v_2 \cdot k_1}_{I_4}, \underbrace{v_3}_{I_5}, \underbrace{k_3}_{I_6} \right)$$



## Material symmetry in elastic rods (part 4)

to derive the concrete form of the energy potential function,

we do a Taylor-expansion of  $\psi$  about  $(\underline{v}_0, \underline{k}_0) = ([0 \ 0 \ 1]^T, [0 \ 0 \ 0]^T)$

$$\begin{aligned}\psi(\underline{v}, \underline{k}) = & \cancel{\psi(\underline{v}_0, \underline{k}_0)}^0 + \cancel{\frac{\partial \psi}{\partial \underline{v}}(\underline{v}_0, \underline{k}_0)}^0 \cdot (\underline{v} - \underline{v}_0) + \cancel{\frac{\partial \psi}{\partial \underline{k}}(\underline{v}_0, \underline{k}_0)}^0 \cdot (\underline{k} - \underline{k}_0) + \\ & + \frac{1}{2} \left( \left( \frac{\partial^2 \psi}{\partial \underline{v}^2} \cdot (\underline{v} - \underline{v}_0) \right) \cdot (\underline{v} - \underline{v}_0) + \left( \frac{\partial^2 \psi}{\partial \underline{k}^2} \cdot (\underline{k} - \underline{k}_0) \right) \cdot (\underline{k} - \underline{k}_0) + \right. \\ & \left. + 2 \cdot \left( \frac{\partial^2 \psi}{\partial \underline{v} \partial \underline{k}} \cdot (\underline{k} - \underline{k}_0) \right) \cdot (\underline{v} - \underline{v}_0) \right) + \text{HOT}\end{aligned}$$

from the Taylor expansion consider for example the term

$$\frac{\partial^2 \psi}{\partial \underline{v}^2} = \frac{\partial}{\partial \underline{v}} \left( \frac{\partial \psi}{\partial \underline{v}} \right) = \frac{\partial}{\partial \underline{v}} \left( \frac{\partial \psi}{\partial I_1} \cdot \frac{\partial I_1}{\partial \underline{v}} + \dots + \frac{\partial \psi}{\partial I_6} \cdot \frac{\partial I_6}{\partial \underline{v}} \right) = \dots$$

$\frac{\partial \psi}{\partial I_j}$  are unknowns that must be obtained from experiments / 3D elasticity, whereas  $\frac{\partial I_j}{\partial \underline{v}}$  can easily be computed from kinematics ( $j = 1, \dots, 6$ )

## Material symmetry in elastic rods (part 5)

doing the computations and grouping terms in a way to get a polynomial in the strains we obtain

$$\begin{aligned}\psi = & \frac{1}{2} \left( \mathcal{A} \cdot (k_1^2 + k_2^2) + \mathcal{B} \cdot k_3^2 + \mathcal{C} \cdot (v_1^2 + v_2^2) + \mathcal{D} \cdot (k_3 - 1)^2 \right) + \\ & + \cdot \mathcal{E} \cdot (v_3 - 1) \cdot k_3 + \cdot \mathcal{F} \cdot (v_1 \cdot k_1 + v_2 \cdot k_2) + \text{HOT}\end{aligned}$$

as **another experiment** consider now a reflection of the beam about the  $\underline{e}_1$ - $\underline{e}_2$ -plane

- reflections are in the group  $O(2)$
- reflections are not in the symmetry group  $\mathcal{G} \equiv SO(2)$  of a pre-twisted rod
- $O(2)$  is the symmetry group for rods without helicity of fibers in the undeformed configuration ; for such rods  $\mathcal{E} = \mathcal{F} = 0$

## Recap: kinematics

- $s$  identifies a certain cross section of the rod
- $\underline{r}(s)$  describes the position of the cross section centroid for the cross section at  $s$   
→ describes the centerline of the rod
- $\underline{\underline{R}}(s)$  describes the (average) orientation of the cross section at  $s$   
→  $\underline{d}_i(s) = \underline{\underline{R}}(s) \cdot \underline{e}_i$  is the local director basis

## deformation map with warping of the cross section

$$\underline{f}(\underline{X}) = \underline{f}(X_1, X_2, X_3 = s) = \underline{r}(s) + \underline{\underline{R}}(s) \cdot (X_\alpha \underline{e}_\alpha + \underline{u}) \quad \text{with } \alpha \in \{1, 2\}$$

note that so far we did not use any deformation map in the derivations!

## local strain measures

$$\circ \underline{v} = \underline{\underline{R}}^T \cdot \underline{r}' = \begin{bmatrix} v_1 & v_2 & v_3 \end{bmatrix}^T$$

$v_1$  : shear along  $\underline{d}_1$

$v_2$  : shear along  $\underline{d}_2$

$v_3$  : axial stretch (along  $\underline{d}_3$ )

$$\circ \underline{k} = \text{axial}(\underline{\underline{R}}^T \cdot \underline{\underline{R}}') = \begin{bmatrix} k_1 & k_2 & k_3 \end{bmatrix}^T$$

$k_1$  : curvature about  $\underline{d}_1$ -axis

$k_2$  : curvature about  $\underline{d}_2$ -axis

$k_3$  : twist about  $\underline{d}_3$ -axis

## Recap: balance laws

### balance of linear momentum

$$\underline{n}'(s) + \hat{\underline{n}}(s) = \rho_0 \cdot A \cdot \underline{\ddot{r}}(s)$$

### balance of angular momentum

$$\underline{m}'(s) + \underline{r}'(s) \times \underline{n}(s) + \hat{\underline{m}}(s) = \rho_0 \cdot \frac{d}{dt} (\underline{I}_{\underline{=0}} \cdot \underline{\omega})$$

equations are in terms of the **internal contact force** and **internal moment**

$$\underline{n}(s) = \iint_{\Omega_0(s)} \underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3 \, dX_1 \, dX_2$$

$$\underline{m}(s) = \iint_{\Omega_0(s)} \left( \underline{x}(X_1, X_2, s) - \underline{r}(s) \right) \times \left( \underline{\underline{P}}(X_1, X_2, s) \cdot \underline{e}_3 \right) \, dX_1 \, dX_2$$

with **external loads**

$$\hat{\underline{n}}(s) = \iint_{\Omega_0(s)} \underline{\underline{B}}(X_1, X_2, s) \, dA + \oint_{\partial\Omega_0(s)} \underline{t}^{ext}(l, s) \, dl$$

$$\hat{\underline{m}}(s) = \iint_{\Omega_0(s)} \left( \underline{x}(\cdot, \cdot, s) - \underline{r}(s) \right) \times \underline{\underline{B}}(s) \, dA + \oint_{\partial\Omega_0} \left( \underline{x}(\cdot, \cdot, s) - \underline{r}(s) \right) \times \underline{t}^{ext}(s) \, dl$$

## Recap: constitutive laws

### strain energy per unit of undeformed length

$$\phi(\underline{r}(s), \underline{\underline{R}}(s), \underline{r}'(s), \underline{\underline{R}}'(s)) \stackrel{\star}{=} \psi(\underline{\underline{R}}^T(s) \underline{r}'(s), \underline{\underline{R}}^T(s) \underline{\underline{R}}'(s)) = \psi(\underline{v}(s), \underline{k}(s))$$

★ : principle of frame-indifference

### relationship between potential energy and kinetic quantities

$$\underline{n}(s) = \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{v}}(s) \quad ; \quad \underline{m}(s) = \underline{\underline{R}}(s) \cdot \frac{\partial \psi}{\partial \underline{k}}(s)$$

### material symmetry in elastic rods

$$\begin{aligned} \psi(\underline{v}(s), \underline{k}(s)) &\stackrel{\mathcal{G} \equiv SO(2)}{=} \psi(v_1^2 + v_2^2, k_1^2 + k_2^2, v_1 \cdot k_1 + v_2 \cdot k_2, v_1 \cdot k_2 - v_2 \cdot k_1, v_3, k_3) \\ &\approx \frac{1}{2} \left( \mathcal{A}(k_1^2 + k_2^2) + \mathcal{B} \cdot k_3^2 + \mathcal{C} \cdot (v_1^2 + v_2^2) + \mathcal{D} \cdot (k_3 - 1)^2 \right) + \\ &\quad + \mathcal{E} \cdot (v_3 - 1) \cdot k_3 + \mathcal{F} \cdot (v_1 \cdot k_1 + v_2 \cdot k_2) \end{aligned}$$

if fibers of the rod are not pre-twisted we have  $\mathcal{G} \equiv O(2) \Rightarrow \mathcal{E} = \mathcal{F} = 0$

## Motivation for relaxation / warping of the cross section

the model that we have so far is too stiff because every cross section is assumed to be rigid!

we therefore already introduced a modified deformation map, that allows for warping / relaxation of the cross section:

$$\underline{f}(X_1, X_2, s) = \underline{r}(s) + \underline{\underline{R}}(s) \cdot (X_\alpha \underline{e}_\alpha + \underline{u})$$

it has already been stated that we do not want to have  $\underline{u}$  as a function of  $s$ , because that would imply solving the expensive 3D problem

### strategy for 1D theory

- we consider the rod in a configuration where  $(\underline{v}(s), \underline{k}(s)) = (\underline{v}^*, \underline{k}^*)$  constant  $\forall s$
- in the sequel we call this configuration the  $\star$ -problem
- then we compute  $\underline{u}^*(X_1, X_2)$ 
  - because of uniformity in  $s$  this is a 2D elasticity problem
- in the context of a finite element discretization, one such 2D problem is solved for every quadrature point within an element to approximate the average warping  $\underline{u}^*(X_1, X_2)$  of the cross sections within that element ;  $(\underline{v}^*, \underline{k}^*)$  are the strains at the quadrature points

## Uniformly strained rod

- we have a rod with the same  $(\underline{v}^*, \underline{k}^*)$  for every  $s$
- all its cross sections warp / relax in exactly the same way
  - we must consider just one cross section, i.e. we have a 2D problem
- this cross section is completely relaxed
- the deformation map of this relaxed configuration is
$$\underline{f}^*(X_1, X_2, s) = \underline{r}^*(s) + \underline{R}^*(s) \cdot (X_\alpha \underline{e}_\alpha + \underline{u}^*(X_1, X_2))$$

## accuracy of the approximation

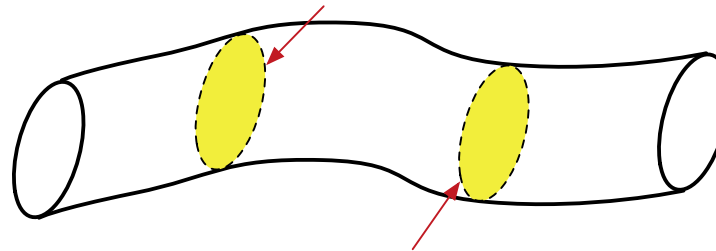
- consider an arbitrary rod in a deformed configuration and one particular cross section  $s^*$
- set  $(\underline{v}^*, \underline{k}^*) := (\underline{v}(s^*), \underline{k}(s^*))$
- consider the same rod but in the configuration with  $(\underline{v}(s), \underline{k}(s)) = (\underline{v}^*, \underline{k}^*) \forall s$ , i.e. the particular  $\star$ -problem at  $s = s^*$
- if in the arbitrarily deformed configuration  $(\underline{v}(s), \underline{k}(s))$  change slowly with  $s$  in some neighborhood of  $s^*$ , then  $\underline{u}^*(X_1, X_2)$ , obtained from the  $\star$ -problem, is a good *local* approximation for  $\underline{u}(X_1, X_2, s = s^*)$ , obtained from 3D theory

## Strain energy of a cross section

the warping of the cross section will be determined by a minimization of strain energy

$$\psi(\underline{v}, \underline{k}; \underline{u}(\underline{v}, \underline{k})) = \lim_{s_2 \rightarrow s_1} \frac{1}{s_2 - s_1} \int_{s_1}^{s_2} \left( \iint_{\Omega_0(s)} W(\underline{F}) \, d\Omega_0 \right) ds = \iint_{\Omega_0(s)} W(\underline{F}) \, d\Omega_0$$

$$\psi_1 = \psi(\underline{v}_1, \underline{k}_1) = \psi(\underline{v}(s_1), \underline{k}(s_1))$$



$$\psi_2 = \psi(\underline{v}_2, \underline{k}_2) = \psi(\underline{v}(s_2), \underline{k}(s_2))$$

in order to compute the strain energy  $\psi$ , considering relaxation of the cross section, we require a material model  $W(\cdot)$  from 3D elasticity, such as the Neo-Hookean solid or the Mooney-Rivlin solid



## Deformation gradient of the star-problem

starting with the deformation map of the  $\star$ -problem

$$\underline{f}^*(X_1, X_2, s) = \underline{r}^*(s) + \underline{R}^*(s) \cdot \underbrace{\left( X_\alpha \underline{e}_\alpha + \underline{u}^*(X_1, X_2) \right)}_{\underline{x}_0^*(X_1, X_2)}$$

we compute its deformation gradient

$$\begin{aligned} \underline{F}^*(X_1, X_2, s) &= \\ &= \underline{r}^{*\prime}(s) \otimes \underline{e}_3 + \underline{R}^*(s) \cdot \left( \underline{e}_\alpha \otimes \underline{e}_\alpha + \frac{\partial \underline{u}^*}{\partial X_\alpha} \otimes \underline{e}_\alpha \right) + \underline{R}^{*\prime}(s) \cdot \left( X_\alpha \underline{e}_\alpha + \underline{u}^*(X_1, X_2) \right) \otimes \underline{e}_3 = \\ &= \underline{R}^*(s) \cdot \left( \underline{v}^*(s) \otimes \underline{e}_3 + \underline{e}_\alpha \otimes \underline{e}_\alpha + \frac{\partial \underline{u}^*}{\partial X_\alpha} \otimes \underline{e}_\alpha + \underline{K}^*(s) \cdot \left( X_\alpha \underline{e}_\alpha + \underline{u}^*(X_1, X_2) \right) \otimes \underline{e}_3 \right) = \\ &= \underline{R}^*(s) \cdot \left( \underline{v}^* \otimes \underline{e}_3 + \underline{e}_\alpha \otimes \underline{e}_\alpha + \frac{\partial \underline{u}^*}{\partial X_\alpha} \otimes \underline{e}_\alpha + \underline{k}^* \times \left( X_\alpha \underline{e}_\alpha + \underline{u}^*(X_1, X_2) \right) \otimes \underline{e}_3 \right) = \\ &=: \underline{R}^*(s) \cdot \underline{\tilde{F}}^*(X_1, X_2, s) \end{aligned}$$

## Warping of the cross section in the star-problem

without loss of generality ...

- we consider the cross section at  $s = 0$
- we prescribe  $\underline{\underline{R}}^*(s = 0) = \underline{\underline{R}}_0^* = \underline{\underline{I}}$  (rigid body rotation)
- we prescribe  $\underline{\underline{r}}^*(s = 0) = \underline{\underline{r}}_0^* = \underline{\underline{0}}$  (rigid body translation)

### minimization of energy

$$\underline{\underline{u}}^*(X_1, X_2) = \arg \min_{\underline{\underline{u}}(X_1, X_2)} \psi(\underline{\underline{v}}^*, \underline{\underline{k}}^*; \underline{\underline{u}}) = \arg \min_{\underline{\underline{u}}(X_1, X_2)} \iint_{\Omega_0} W(\underline{\underline{\tilde{F}}}^*(X_1, X_2, s = 0)) \, d\Omega_0$$

- here  $\underline{\underline{\tilde{F}}}^*$  is the deformation gradient we just computed *but* we consider  $\underline{\underline{u}}(X_1, X_2)$  as a free variable, for which we optimize, i.e.  $\underline{\underline{\tilde{F}}}^* = \underline{\underline{\tilde{F}}}^*(X_1, X_2, s = 0; \underline{\underline{u}})$
- the minimization is carried out subject to constraints:  
(mass) center and orientation of the cross section must be preserved

## Displacements in the star-problem (part 1)

### rotation / orientation of cross section

$$\begin{aligned}\underline{\underline{R}}^{\star T}(s) \cdot \underline{\underline{R}}^{\star'}(s) &= \underline{\underline{K}}^{\star} \text{ (const.)} \Rightarrow \underline{\underline{R}}^{\star'}(s) = \underline{\underline{R}}^{\star}(s) \cdot \underline{\underline{K}}^{\star} \\ \Rightarrow \underline{\underline{R}}^{\star}(s) &= \underline{\underline{R}}_0^{\star} \cdot \exp(\underline{\underline{K}}^{\star} \cdot s) = \exp(\underline{\underline{K}}^{\star} \cdot s)\end{aligned}$$

### translation / displacement of cross section

$$\begin{aligned}\underline{\underline{R}}^{\star T}(s) \cdot \underline{\underline{r}}^{\star'}(s) &= \underline{\underline{v}}^{\star} \text{ (const.)} \Rightarrow \underline{\underline{r}}^{\star'}(s) = \underline{\underline{R}}^{\star}(s) \cdot \underline{\underline{v}}^{\star} = \exp(\underline{\underline{K}}^{\star} \cdot s) \cdot \underline{\underline{v}}^{\star} \\ \underline{\underline{r}}^{\star}(s) &= \underline{\underline{r}}_0^{\star} + \left( \int_0^s \exp(\underline{\underline{K}}^{\star} \cdot l) \, dl \right) \cdot \underline{\underline{v}}^{\star}\end{aligned}$$

## Displacements in the star-problem (part 2)

### decomposition of $\underline{v}^*$

we split  $\underline{v}^*$  in a part that is parallel to  $\underline{k}^*$  and a part that is perpendicular to  $\underline{k}^*$

$$\underline{v}^* = \left( \cos(\phi) \cdot \underline{\hat{k}} + \sin(\phi) \cdot \underline{\hat{k}}^\perp \right) \cdot \|\underline{v}^*\|$$

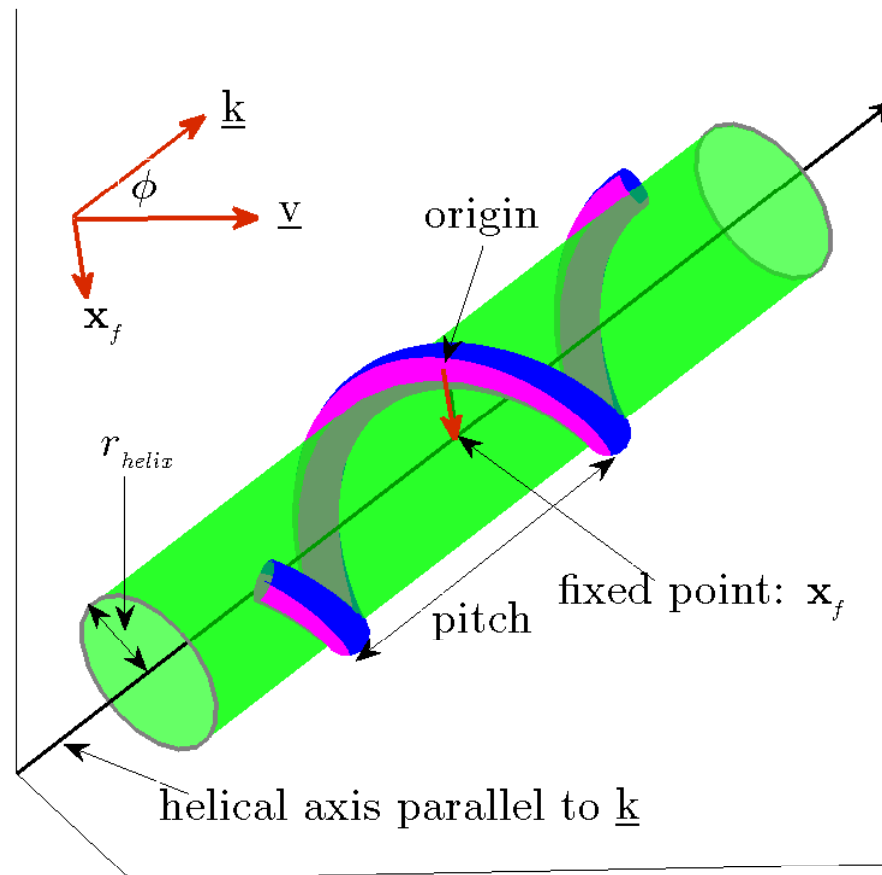
- $\phi$  is the angle between  $\underline{v}^*$  and  $\underline{k}^*$ , i.e.  $\phi = \arcsin\left(\frac{\underline{v}^* \cdot \underline{k}^*}{\|\underline{v}^*\| \cdot \|\underline{k}^*\|}\right)$
- $\underline{\hat{k}}$  is the unit vector along  $\underline{k}^*$ , i.e.  $\underline{\hat{k}} = \frac{\underline{k}^*}{\|\underline{k}^*\|}$
- $\underline{\hat{k}}^\perp$  is a unit vector perpendicular to  $\underline{\hat{k}}$

### translation / displacement of cross section (revisited)

$$\underline{r}^*(s) = \underbrace{\left( \cos(\phi) \cdot \int_0^s \overbrace{\exp(\underline{K}^* \cdot l) \cdot \underline{\hat{k}}}^{\underline{\hat{k}}} dl \right)}_{\text{straight line along } \underline{\hat{k}}} + \underbrace{\left( \sin(\phi) \cdot \int_0^s \exp(\underline{K}^* \cdot l) \cdot \underline{\hat{k}}^\perp dl \right)}_{\text{circle with normal along } \underline{\hat{k}}} \cdot \|\underline{v}^*\|$$

## The centerline turns into a helix

$$\underline{r}^*(s) = \left( \cos(\phi) \cdot \hat{\underline{k}} \cdot s + \sin(\phi) \cdot \int_0^s \exp(\underline{\underline{K}}^* \cdot l) \cdot \hat{\underline{k}}^\perp dl \right) \cdot \|\underline{v}^*\|$$



## Helix equation

rewriting the equation we get ...

$$\underline{r}^*(s) = \|\underline{v}^*\| \cdot \left( \cos(\phi) \cdot \hat{\underline{k}} \cdot s + \sin(\phi) \cdot \frac{1}{\|\underline{k}^*\|} \cdot \left( \underline{I} - \exp(\underline{K}^* \cdot s) \right) \cdot \hat{\underline{k}} \times \hat{\underline{k}}^\perp \right)$$

we introduce the abbreviations

- $\tau = \|\underline{v}^*\| \cdot \cos(\phi)$
- $\underline{x}_f = \frac{\sin(\phi) \cdot \hat{\underline{k}} \times \hat{\underline{k}}^\perp}{\|\underline{k}^*\|} = \frac{\underline{v}^* \times \underline{k}^*}{\|\underline{k}^*\|^2}$  (fixed point of the helix)

and get ...

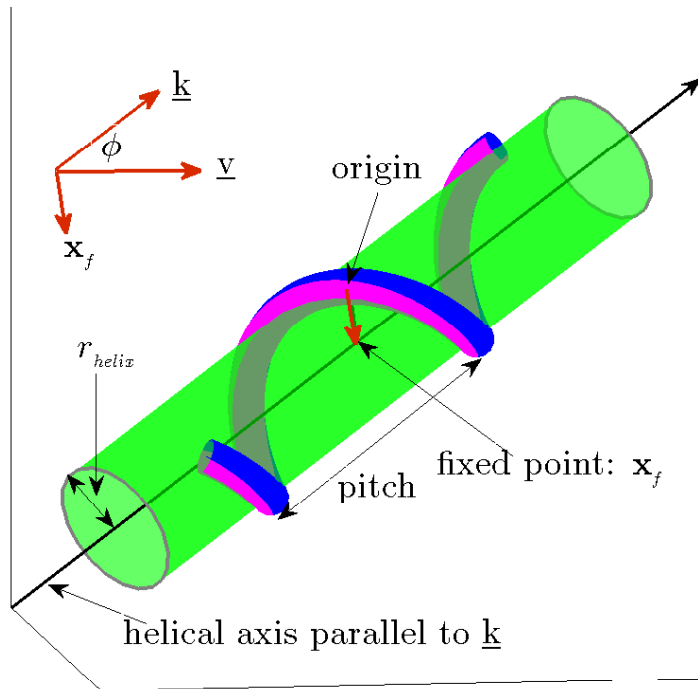
$$\underline{r}^*(s) = s \cdot \tau \cdot \hat{\underline{k}} + \left( \underline{I} - \exp(\underline{K}^* \cdot s) \right) \cdot \underline{x}_f = s \cdot \tau \cdot \hat{\underline{k}} + \underbrace{\underline{x}_f + \underline{R}^*(s) \cdot (\underline{r}_0^* - \underline{x}_f)}_{\text{rotation of the origin about } \underline{x}_f}$$

## examples

- $\underline{v}^* \parallel \underline{k}^*$  : centerline degenerates into a straight line (e.g. combined extension & torsion)
- $\underline{v}^* \perp \underline{k}^*$  : centerline degenerates into a circle (e.g. pure bending)

## Helix

$$\underline{r}^*(s) = s \cdot \tau \cdot \hat{\underline{k}} + \underline{x}_f + \exp(\underline{K}^* \cdot s) \cdot (\underline{r}_0^* - \underline{x}_f)$$



### pitch

for one full turn of the helix we have

$$\|\underline{k}^*\| \cdot \dot{s} \stackrel{!}{=} 2 \cdot \pi$$

plugging  $\dot{s}$  into the part of the helix equation that generates the axial motion we obtain the pitch of the helix as

$$\dot{s} \cdot \tau = \frac{2\pi}{\|\underline{k}^*\|} \cdot \|\underline{v}^*\| \cdot \cos(\phi)$$

### radius

$$\|\underline{r}_0^* - \underline{x}_f\| = \|\underline{x}_f\|$$

## Deformation map and deformation gradient of the star-problem

we rewrite the **deformation map** of the  $\star$ -problem, using the helix equation ...

$$\begin{aligned} \underline{f}^*(X_1, X_2, s) &= \underline{r}^*(s) + \underline{\underline{R}}^*(s) \cdot \left( X_\alpha \underline{e}_\alpha + \underline{u}^*(X_1, X_2) \right) = \underline{r}^*(s) + \underline{\underline{R}}^*(s) \cdot \underline{x}_0^*(X_1, X_2) = \\ &= s \cdot \tau \cdot \hat{\underline{k}} + \underline{x}_f + \underline{\underline{R}}^*(s) \cdot \left( \underline{x}_0^*(X_1, X_2) - \underline{x}_f \right) \end{aligned}$$

→ rotating  $\underline{r}^* = \underline{0}$  about  $\underline{x}_f$  creates centerline ; rotating  $\underline{x}_0^*$  about  $\underline{x}_f$  creates entire rod

and then we compute its **deformation gradient** ...

$$\begin{aligned} \underline{\underline{F}}^*(X_1, X_2, s) &= \underline{r}^{*\prime}(s) \otimes \underline{e}_3 + \underline{\underline{R}}^{*\prime}(s) \cdot \underline{x}_0^*(X_1, X_2) \otimes \underline{e}_3 + \underline{\underline{R}}^*(s) \cdot \frac{\partial \underline{x}_0^*}{\partial X_\alpha} \otimes \underline{e}_\alpha = \\ &= \underline{\underline{R}}^*(s) \cdot \left( \underline{\underline{R}}^{*\text{T}}(s) \cdot \underline{r}^{*\prime}(s) \otimes \underline{e}_3 + \underline{\underline{R}}^{*\text{T}}(s) \cdot \underline{\underline{R}}^{*\prime}(s) \cdot \underline{x}_0^*(X_1, X_2) \otimes \underline{e}_3 + \frac{\partial \underline{x}_0^*}{\partial X_\alpha} \otimes \underline{e}_\alpha \right) = \\ &= \underline{\underline{R}}^*(s) \cdot \left( \underline{v}^* \otimes \underline{e}_3 + \left( \underline{k}^* \times \underline{x}_0^*(X_1, X_2) \right) \otimes \underline{e}_3 + \frac{\partial \underline{x}_0^*}{\partial X_\alpha} \otimes \underline{e}_\alpha \right) \end{aligned}$$



## 2D minimization problem (part 1)

$$\min_{\underline{x}_0^*} \iint_{\Omega_0} W(\underline{\tilde{F}}^*(X_1, X_2, s=0)) \, d\Omega_0$$

such that ...

- the center(line) remains at the origin or more precisely that the mass center of the cross section remains in the origin, i.e.

$$\iint_{\Omega_0} \rho_0 \cdot \underline{x}_0^* \, d\Omega_0 = \underline{0}$$

- and such that orientation of the cross section remains in the  $\underline{e}_1$ - $\underline{e}_2$ -plane or more precisely that the principal axis of the inertia tensor remains aligned with the  $\underline{e}_3$ -axis ; this is the same as saying that the mixed moments of inertia must vanish

$$\underbrace{\iint_{\Omega_0} \rho_0 \cdot \begin{bmatrix} x_2 \cdot x_3 \\ x_1 \cdot x_3 \\ x_1 \cdot x_2 \end{bmatrix} \, d\Omega_0}_{=:\underline{M}} = \underline{0}$$

with  $x_i$  the components of  $\underline{x}_0^*$

## 2D minimization problem (part 2)

minimization problem with augmented Lagrangian

$$\min_{\underline{x}_0^*, \underline{\lambda}, \underline{\mu}} \iint_{\Omega_0} \left( W(\underline{\tilde{F}}^*(X_1, X_2, s=0)) + \underline{\lambda} \cdot \rho_0 \cdot \underline{x}_0^* + \underline{\mu} \cdot \underline{M} \right) d\Omega_0$$

perturbed version of  $\underline{x}_0^*$

$$\underline{x}_0^\epsilon(X_1, X_2) = \underline{x}_0^*(X_1, X_2) + \epsilon \cdot \delta \underline{x}_0^*(X_1, X_2)$$

perturbed version of the cross section energy

$$\psi_\epsilon = \iint_{\Omega_0} \left( W(\underline{\tilde{F}}^\epsilon(X_1, X_2, s=0)) + \underline{\lambda} \cdot \rho_0 \cdot \underline{x}_0^\epsilon(X_1, X_2) + \underline{\mu} \cdot \underline{M}^\epsilon(X_1, X_2) \right) d\Omega_0$$

first variation of the cross section energy

$$\delta\psi = \left. \frac{d\psi_\epsilon}{d\epsilon} \right|_{\epsilon=0} = \iint_{\Omega_0} \left( \underbrace{\frac{\partial W}{\partial \underline{\tilde{F}}}}_{\underline{P}} : \left. \frac{d\underline{\tilde{F}}^\epsilon}{d\epsilon} \right|_{\epsilon=0} + \underline{\lambda} \cdot \rho_0 \cdot \left. \frac{d\underline{x}_0^\epsilon}{d\epsilon} \right|_{\epsilon=0} + \underline{\mu} \cdot \left. \frac{d\underline{M}^\epsilon}{d\epsilon} \right|_{\epsilon=0} \right) d\Omega_0$$

## 2D minimization problem (part 3)

from

$$\underline{\underline{F}}^\epsilon(X_1, X_2, s) = \underline{\underline{R}}^\star(s) \cdot \left( \underline{v}^\star \otimes \underline{e}_3 + \left( \underline{k}^\star \times \underline{x}_0^\epsilon(X_1, X_2) \right) \otimes \underline{e}_3 + \frac{\partial \underline{x}_0^\epsilon}{\partial X_\alpha} \otimes \underline{e}_\alpha \right)$$

it follows that

$$\left. \frac{d\tilde{\underline{\underline{F}}}^\epsilon}{d\epsilon} \right|_{\epsilon=0} = \left( \underline{k}^\star \times \delta \underline{x}_0^\star(X_1, X_2) \right) \otimes \underline{e}_3 + \frac{\partial \delta \underline{x}_0^\star}{\partial X_\alpha} \otimes \underline{e}_\alpha$$

and from

$$\underline{\underline{M}}^\epsilon = \rho_0 \cdot \begin{bmatrix} x_2^\epsilon \cdot x_3^\epsilon \\ x_1^\epsilon \cdot x_3^\epsilon \\ x_1^\epsilon \cdot x_2^\epsilon \end{bmatrix} = \rho_0 \cdot \begin{bmatrix} (x_2 + \epsilon \cdot \delta x_2) \cdot (x_3 + \epsilon \cdot \delta x_3) \\ (x_1 + \epsilon \cdot \delta x_1) \cdot (x_3 + \epsilon \cdot \delta x_3) \\ (x_1 + \epsilon \cdot \delta x_1) \cdot (x_2 + \epsilon \cdot \delta x_2) \end{bmatrix}$$

it follows that

$$\left. \frac{d\underline{\underline{M}}^\epsilon}{d\epsilon} \right|_{\epsilon=0} = \rho_0 \cdot \begin{bmatrix} x_2 \cdot \delta x_3 + x_3 \cdot \delta x_2 \\ x_1 \cdot \delta x_3 + x_3 \cdot \delta x_1 \\ x_1 \cdot \delta x_2 + x_2 \cdot \delta x_1 \end{bmatrix} = \rho_0 \cdot \underbrace{\begin{bmatrix} 0 & x_3 & x_2 \\ x_3 & 0 & x_1 \\ x_2 & x_1 & 0 \end{bmatrix}}_{=:\underline{\underline{M}}(X_1, X_2)} \cdot \delta \underline{x}_0^\star(X_1, X_2)$$

## 2D minimization problem (part 4)

note that

$$\underline{\underline{P}} : (\underline{a} \otimes \underline{b}) \hat{=} P_{ij} a_i b_j = P_{ij} b_j a_i \hat{=} (\underline{\underline{P}} \cdot \underline{b}) \cdot \underline{a}$$

and that

$$\underline{a} \cdot (\underline{b} \times \underline{c}) = \underline{b} \cdot (\underline{c} \times \underline{a}) = \underline{c} \cdot (\underline{a} \times \underline{b})$$

### first variation of the cross section energy

$$\begin{aligned} \delta\psi &= \iint_{\Omega_0} \left( \underline{\underline{P}} : \left( (\underline{k}^* \times \delta \underline{x}_0^*) \otimes \underline{e}_3 + \frac{\partial \delta \underline{x}_0^*}{\partial X_\alpha} \otimes \underline{e}_\alpha \right) + \underline{\lambda} \cdot \left( \rho_0 \cdot \delta \underline{x}_0^* \right) + \underline{\mu} \cdot \left( \underline{\underline{M}} \cdot \delta \underline{x}_0^* \right) \right) d\Omega_0 = \\ &= \iint_{\Omega_0} \left( (\underline{\underline{P}} \cdot \underline{e}_3) \cdot (\underline{k}^* \times \delta \underline{x}_0^*) + \overbrace{(\underline{\underline{P}} \cdot \underline{e}_\alpha) \cdot \frac{\partial \delta \underline{x}_0^*}{\partial X_\alpha}}^{\rightarrow \text{IBP}} + (\rho_0 \cdot \underline{\lambda} + \underline{\underline{M}} \cdot \underline{\mu}) \cdot \delta \underline{x}_0^* \right) dX_1 dX_2 = \\ &= \iint_{\Omega_0} \left( \left( (\underline{\underline{P}} \cdot \underline{e}_3) \times \underline{k}^* \right) \cdot \delta \underline{x}_0^* + \frac{\partial}{\partial X_\alpha} \left( (\underline{\underline{P}} \cdot \underline{e}_\alpha) \cdot \delta \underline{x}_0^* \right) - \frac{\partial}{\partial X_\alpha} \left( \underline{\underline{P}} \cdot \underline{e}_\alpha \right) \cdot \delta \underline{x}_0^* + \right. \\ &\quad \left. + (\rho_0 \cdot \underline{\lambda} + \underline{\underline{M}} \cdot \underline{\mu}) \cdot \delta \underline{x}_0^* \right) d\Omega_0 = \dots \end{aligned}$$

## 2D minimization problem (part 5)

$$\begin{aligned}\delta\psi &= \iint_{\Omega_0} \left( \left( (\underline{\underline{P}} \cdot \underline{e}_3) \times \underline{k}^* \right) \cdot \delta \underline{x}_0^* + \frac{\partial}{\partial X_\alpha} \left( (\underline{\underline{P}} \cdot \underline{e}_\alpha) \cdot \delta \underline{x}_0^* \right) - \frac{\partial}{\partial X_\alpha} \left( \underline{\underline{P}} \cdot \underline{e}_\alpha \right) \cdot \delta \underline{x}_0^* + \right. \\ &\quad \left. + (\rho_0 \cdot \underline{\lambda} + \underline{\underline{M}} \cdot \underline{\mu}) \cdot \delta \underline{x}_0^* \right) d\Omega_0 = \\ &= \iint_{\Omega_0} - \left( \frac{\partial}{\partial X_\alpha} \left( \underline{\underline{P}} \cdot \underline{e}_\alpha \right) + \underline{k}^* \times (\underline{\underline{P}} \cdot \underline{e}_3) - (\rho_0 \cdot \underline{\lambda} + \underline{\underline{M}} \cdot \underline{\mu}) \right) \cdot \delta \underline{x}_0^* d\Omega_0 + \\ &\quad + \iint_{\Omega_0} \frac{\partial}{\partial X_\alpha} \left( (\underline{\underline{P}} \cdot \underline{e}_\alpha) \cdot \delta \underline{x}_0^* \right) d\Omega_0\end{aligned}$$

## 2D minimization problem (part 6)

for the second integral we can use the divergence theorem ...

$$\begin{aligned} \iint_{\Omega_0} \frac{\partial}{\partial X_\alpha} \left( (\underline{P} \cdot \underline{e}_\alpha) \cdot \delta \underline{x}_0^* \right) d\Omega_0 &= \iint_{\Omega_0} \frac{\partial}{\partial X_\alpha} \left( (\underline{P}^T \cdot \delta \underline{x}_0^*) \cdot \underline{e}_\alpha \right) d\Omega_0 = \\ &= \iint_{\Omega_0} \underline{\nabla} \cdot (\underline{P}^T \cdot \delta \underline{x}_0^*) d\Omega_0 = \int_{\partial\Omega_0} (\underline{P}^T \cdot \delta \underline{x}_0^*) \cdot \underline{n}_0 dl = \int_{\partial\Omega_0} (\underline{P} \cdot \underline{n}_0) \cdot \delta \underline{x}_0^* dl \end{aligned}$$

with  $\underline{n}_0$  the unit normal of the cross section boundary

setting  $\delta\psi = 0$  gives us the **Euler-Lagrange equations** of the  $\star$ -problem

$$\begin{aligned} \underline{\nabla}_\alpha \cdot \underline{P} + \underline{k}^* \times (\underline{P} \cdot \underline{e}_3) &= \rho_0 \cdot \underline{\lambda} + \underline{M} \cdot \underline{\mu} \quad \text{in } \Omega_0 \\ \underline{P} \cdot \underline{n}_0 &= \underline{0} \quad \text{on } \partial\Omega_0 \quad (\text{traction free boundary condition}) \end{aligned}$$

together with the constraints

$$\iint_{\Omega_0} \rho_0 \cdot \underline{x}_0^* d\Omega_0 = \underline{0} \quad \text{and} \quad \iint_{\Omega_0} \underline{M} d\Omega_0 = \underline{0}$$

the Euler-Lagrange equations are the (necessary) conditions for a minimum of the cross section energy ;  $\underline{x}_0^*(X_1, X_2)$ ,  $\underline{\lambda}$  and  $\underline{\mu}$  are the unknowns of the problem

## Recap

**strain energy density function** (per unit of undeformed length)

$$\psi(\underline{v}, \underline{k}; \underline{u}(\underline{v}, \underline{k})) = \iint_{\Omega_0} W(\underline{\underline{F}}) \, d\Omega_0$$

**deformation gradient** of the  $\star$ -problem

$$\underline{\underline{F}}^\star(X_1, X_2, s) = \underline{\underline{R}}^\star(s) \cdot \left( \underline{v}^\star \otimes \underline{e}_3 + \left( \underline{k}^\star \times \underbrace{\underline{x}_0^\star(X_1, X_2)}_{\text{warping function}} \right) \otimes \underline{e}_3 + \frac{\partial \underline{x}_0^\star}{\partial X_\alpha} \otimes \underline{e}_\alpha \right)$$

in order to find  $\underline{x}_0^\star(X_1, X_2)$  we must find a solution to the **Euler-Lagrange equations** of the  $\star$ -problem

$$\begin{aligned} \nabla_\alpha \cdot \underline{\underline{P}} + \underline{k}^\star \times (\underline{\underline{P}} \cdot \underline{e}_3) &= \rho_0 \cdot \underline{\lambda} + \underline{\underline{M}} \cdot \underline{\mu} \quad \text{in } \Omega_0 \\ \underline{\underline{P}} \cdot \underline{n}_0 &= \underline{0} \quad \text{on } \partial\Omega_0 \quad (\text{traction free boundary condition}) \end{aligned}$$

that also respects the **kinematic constraints**

$$\iint_{\Omega_0} \rho_0 \cdot \underline{x}_0^\star \, d\Omega_0 = \underline{0} \quad \text{and} \quad \iint_{\Omega_0} \underline{\underline{M}} \, d\Omega_0 = \underline{0}$$

## Modeling of continuum and nanorods using molecular approaches

- what is the form of  $W(\underline{\underline{F}})$  ?
  - either obtain it from experiments ...
  - or use theory  $\rightarrow$  molecular approach called *Cauchy Born rule*
- if the radius of the rod is at the nanoscale surface effects become also important, i.e.

$$\psi(\underline{v}, \underline{k}) = \underbrace{\iint_{\Omega_0} W(\underline{\underline{F}}) \, d\Omega_0}_{\text{bulk energy}} + \underbrace{\int_{\partial\Omega_0} \psi^S(\underline{\underline{E}}^S) \, dl}_{\text{surface energy}}$$

$\rightarrow$  extension of the approach called *Surface Cauchy Born rule*

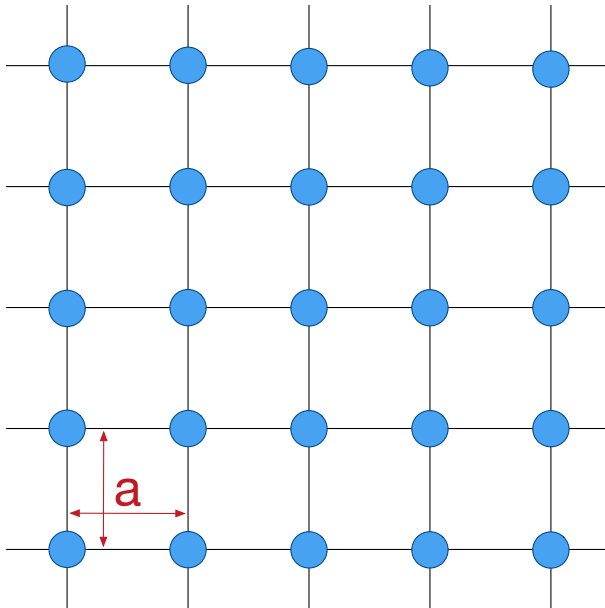
- hollow tube at nanoscale (SWCNT = single wall carbon nanotube)
  - can not be thought of as a 3D continuum anymore!
  - $\psi(\underline{v}, \underline{k}) \rightarrow$  direct approach called *Helical Cauchy Born rule*



## Arrangement of atoms in crystalline materials (part 1)

crystals have a regular arrangement of atoms: translational periodicity

consider for example a simple 2D crystal:



to generate any crystal we need

- lattice vectors:  $(\underline{A}_1, \underline{A}_2, \underline{A}_3)$
- basis atoms:  $\underline{X}_{0,j}$  with  $j = 1, \dots, M$   
 $M$  the number of atoms per unit cell ;  
the zero index indicates the unit cell

in this 2D example we have

- $(\underline{A}_1, \underline{A}_2) = (a \cdot \underline{e}_1, a \cdot \underline{e}_2)$
- $\underline{X}_{0,1} = (0, 0)$  ;  $M = 1$

## Arrangement of atoms in crystalline materials (part 2)

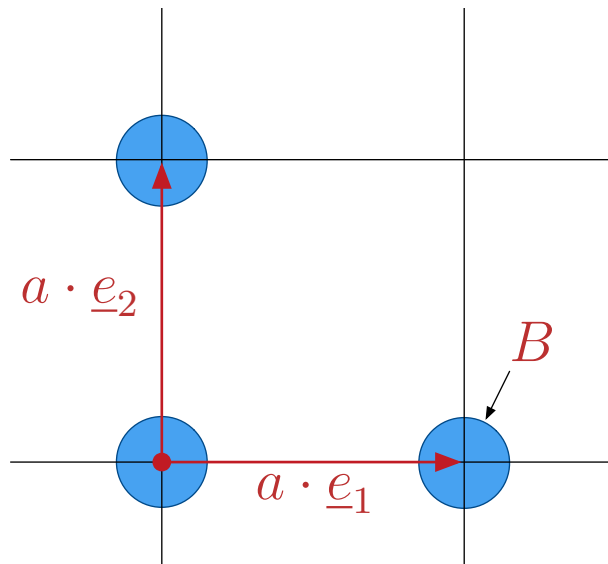
### generators of a crystal

$$\underline{X}_{(n_1, n_2, n_3, j)} = n_i \cdot \underline{A}_i + \left( \underline{X}_j \right)_{j=1, \dots, M} = \underbrace{n_1 \cdot \underline{A}_1 + n_2 \cdot \underline{A}_2 + n_3 \cdot \underline{A}_3}_{\text{generates lattice}} + \underbrace{\left( \underline{X}_j \right)_{j=1, \dots, M}}_{\text{puts atoms on lattice}}$$

$n_1, n_2, n_3 \in \mathbb{Z}$  gives the translation of the unit cell and

$j = 1, \dots, M$  represents the different atoms that are present within the unit cell

### 2D example from last slide



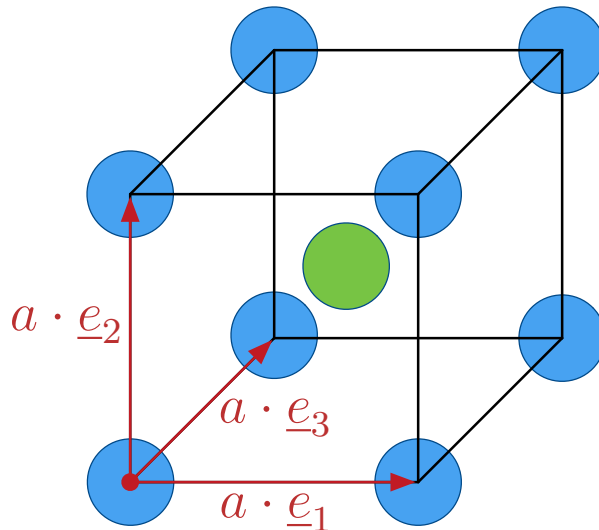
atom  $B$  is generated by

$$\begin{aligned} \underline{X}_B &= \underline{X}_{(1,0,1)} = \\ &= \underbrace{1}_{n_1} \cdot \underbrace{(a \cdot \underline{e}_1)}_{\underline{A}_1} + \underbrace{0}_{n_2} \cdot \underbrace{(a \cdot \underline{e}_2)}_{\underline{A}_2} + 0 \cdot \underline{e}_1 + 0 \cdot \underline{e}_2 \end{aligned}$$

## Arrangement of atoms in crystalline materials (part 3)

another example: **BCC crystal** (body centered cubic)

unit cell with two atoms

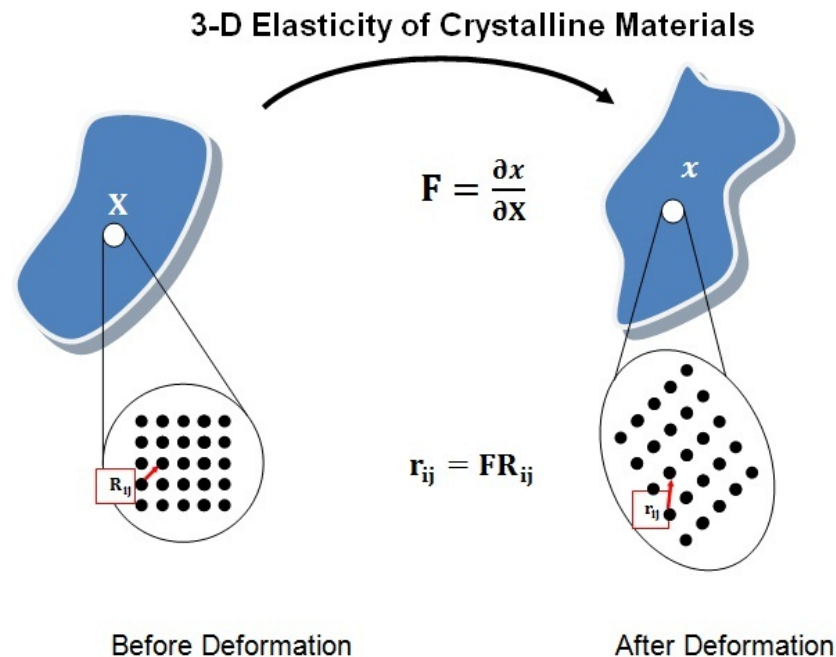


the 7 blue atoms, that are not at the origin, actually belong to the neighboring unit cells!

- lattice vectors:  
 $(\underline{A}_1, \underline{A}_2, \underline{A}_3) = (a \cdot \underline{e}_1, a \cdot \underline{e}_2, a \cdot \underline{e}_3)$
- basis atoms:  
 $\underline{X}_{0,1} = (0, 0, 0)$   
 $\underline{X}_{0,2} = \frac{a}{2} \cdot (\underline{e}_1 + \underline{e}_2 + \underline{e}_3) \quad ; \quad M = 2$

## Cauchy Born rule (part 1)

- at the continuum level : straining a rod
- at the molecular level : the atoms are moving
- the movement of atoms is determined by the bond energy between the atoms
- assumption: periodicity is maintained by slowly varying deformations



**Cauchy Born-Rule:  $\mathbf{a}_i = \mathbf{F} \mathbf{A}_i$**

## Cauchy Born rule (part 2)

as an example consider the homogeneous deformation map

$$\begin{aligned}x_1 &= X_1 \\x_2 &= X_2 \\x_3 &= \lambda \cdot X_3 \quad \Rightarrow \quad \underline{a}_3 = \lambda \cdot \underline{A}_3\end{aligned}$$

if the continuum deformation map is also valid at the atomic level, we have  $\underline{a}_3 = \lambda \cdot \underline{A}_3$   
or more generally the Cauchy Born rule

$$\underline{a}_i = \underline{\underline{F}} \cdot \underline{A}_i$$

- bridges the continuum and the atomistic viewpoint
- the crystal lattice deforms like the continuum
- the positions of the atoms are still unknown  
(they are not dictated by the macroscopic deformation gradient)

the position of *any* atom in the deformed configuration is given by

$$\underline{x}_{(n_1, n_2, n_3, j)} = n_i \cdot \underline{\underline{F}} \cdot \underline{A}_i + \underline{x}_{0, j}$$

where  $\underline{x}_{0, j}$  are the positions of the basis atoms within the unit cell

→ unknowns : can be determined by minimizing the energy of the unit cell

## Minimizing the energy of the unit cell (part 1)

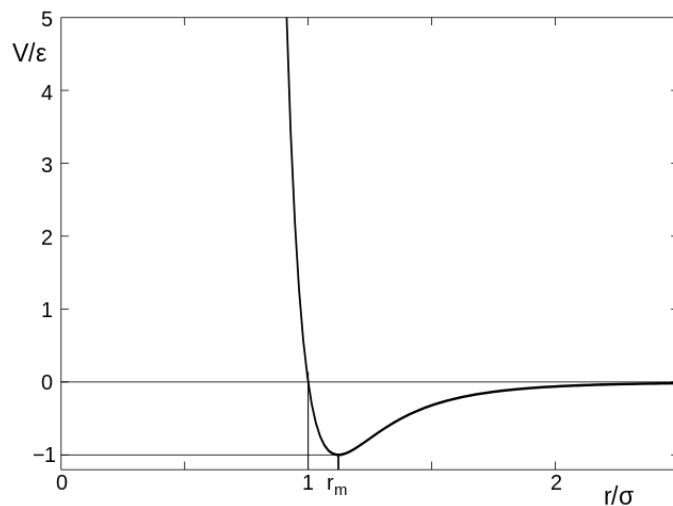
average strain energy density of the unit cell

$$W(\underline{\underline{F}}) = \min_{(\underline{x}_{0,1}, \dots, \underline{x}_{0,M})} \frac{E(\underline{x}_{0,1}, \underline{x}_{0,2}, \dots, \underline{x}_{0,M}; \underline{\underline{F}}) - E(\underline{X}_{0,1}, \underline{X}_{0,2}, \dots, \underline{X}_{0,M}; \underline{\underline{I}})}{(\underline{A}_1 \times \underline{A}_2) \cdot \underline{A}_3}$$

where  $E$  is a measure of the interatomic energy of the unit cell

a **interatomic potential** determines the energy due to atomic bonds

in the simplest case this is a pair potential, i.e. the energy that is present within an individual bond between two atoms or molecules



the **Lennard-Jones potential** models the interaction between two atoms or molecules that are neutral and have no chemical bonds between them

← potential energy as a function of the distance between the two atoms or molecules

$$V(r) = 4 \cdot \epsilon \cdot \left( \left( \frac{\sigma}{r} \right)^{12} - \left( \frac{\sigma}{r} \right)^6 \right)$$

## Minimizing the energy of the unit cell (part 2)

### unit cell energy

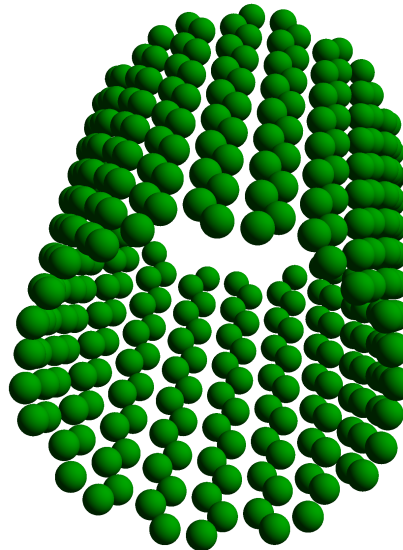
$$E = \frac{1}{2} \sum_{\alpha} \sum_{\beta} V(r_{\alpha,\beta})$$

- $\alpha$  iterates over the basis atoms of the unit cell
- $\beta$  iterates over all neighbors of  $\alpha$  (also within the neighboring unit cells)
- a cutoff-radius defines the size of this neighborhood
- the factor  $1/2$  comes from the fact that every bond appears twice:  $(\alpha, \beta) \leftrightarrow (\beta, \alpha)$
- bonds that cross the boundary of the unit cell have a part inside and a part outside of the unit cell ; this symmetry implies that for every such bond only the part inside the unit cell is taken into account because the inside part of  $(\alpha, \beta)$  equals exactly the outside part of  $(\beta, \alpha)$  ; this works out regardless of the energy potential model  $V$ , i.e. also for models that are not pair-potentials

## Direct molecular approach (part 1)

- for larger deformations of a rod / nanotube the translational periodicity of the crystalline material is not preserved  
→ standard Cauchy Born rule cannot be used to compute the energy of a unit cell
- the idea is to *directly* obtain  $\psi(\underline{v}, \underline{k})$ , i.e. without first computing  $W(\underline{\underline{F}})$
- the direct molecular approach takes care of surface effects
- and it can still be used whenever the rod cannot be thought of as a 3D continuum, because e.g. the rod consists only of a wall with a thickness of one atom (SWCNT)

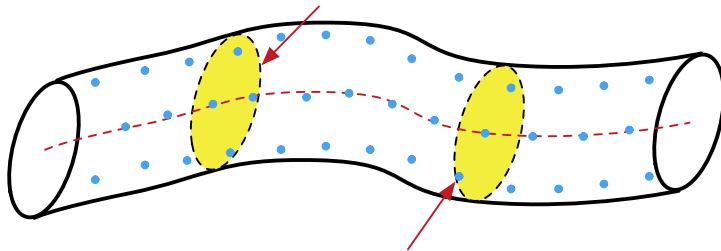
schematic of a SWCNT:





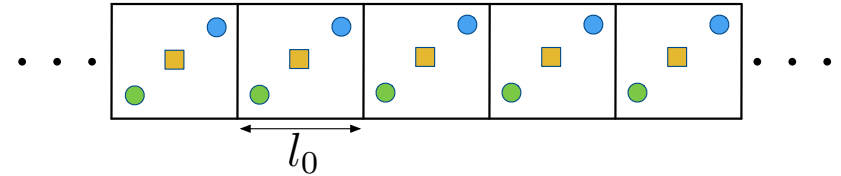
## Direct molecular approach (part 2)

$$\psi_1 = \psi(\underline{v}_1, \underline{k}_1) = \psi(\underline{v}(s_1), \underline{k}(s_1))$$



$$\psi_2 = \psi(\underline{v}_2, \underline{k}_2) = \psi(\underline{v}(s_2), \underline{k}(s_2))$$

→ we will again subject the rod to constant  $(\underline{v}, \underline{k}) \forall s$  to obtain  $\psi(\underline{v}, \underline{k})$



→ the nanorod can be thought of as a 1D crystal, i.e. it has a unit cell that repeats only along  $\underline{e}_3$ -axis ; this repeating unit is sometimes also called fundamental domain

→ all unit cells deform in the same way for constant  $(\underline{v}, \underline{k}) \forall s$

## Direct molecular approach (part 3)

**deformation map** for the repeating unit cells

the reference state is given by  $\underline{X}_{i,j}$

- $i$  is an integer that indicates a particular unit cell (analog to the continuous scalar  $s$ )
- $j$  is an integer that indicates a particular basis atom within the unit cell (analog to the continuous scalars  $X_1, X_2$ )

we want to determine  $\underline{x}_{i,j}$  for a nanorod subjected to constant  $(\underline{v}, \underline{k}) \forall s$

the idea is to find a discrete analog of the 3D deformation map

$$\underline{x}(X_1, X_2, s) = \underbrace{\underline{x}_f + \exp(s \cdot \underline{K}) \cdot (\underline{x}_0(X_1, X_2) - \underline{x}_f)}_{\text{rotate cross section } s=0} + \underbrace{s \cdot \tau \cdot \hat{k}}_{\text{translate cross section}}$$

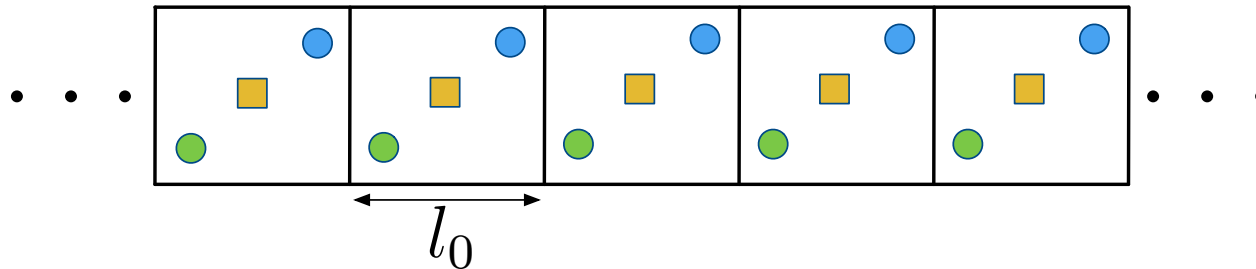
with  $s \rightarrow i \cdot l_0$  we get

$$\underline{x}_{i,j} = \underbrace{\underline{x}_f + \exp(i \cdot l_0 \cdot \underline{K}) \cdot (\underline{x}_{0,j} - \underline{x}_f)}_{\text{rotate unit cell } i=0} + \underbrace{i \cdot l_0 \cdot \tau \cdot \hat{k}}_{\text{translate unit cell}}$$

→ the atomic positions  $\underline{x}_{0,j}$  in the unit cell  $i = 0$  are the unknowns we have to solve for

## Direct molecular approach (part 4)

### short recap



- unit cells are the analogs of cross sections:  $s \rightarrow i \cdot l_0$  with  $i = 0, \dots$ ,
- the atomic positions in the unit cell  $i = 0$  are given by  $(\underline{x}_{0,j})_{j=0,\dots,M}$  with the number of basis atoms  $M = 3$  in the example picture above
- for constant  $(\underline{v}, \underline{k}) \forall s$  we get the discrete deformation map

$$\underline{x}_{i,j} = \underline{x}_f + \exp(i \cdot l_0 \cdot \underline{\underline{K}}) \cdot (\underline{x}_{0,j} - \underline{x}_f) + i \cdot l_0 \cdot \tau \cdot \hat{\underline{k}}$$

- $(\underline{x}_{0,j})_{j=1,\dots,M}$  are not functions as in the continuous case (no dependence on  $X_1, X_2$ )

## Direct molecular approach (part 5)

- reference state of the unit cell at  $i = 0$  given by  $(\underline{X}_{0,j})_{j=1,\dots,M}$
- reference state of entire crystal given by  $\underline{X}_{0,j} = i \cdot l_0 \cdot \underline{e}_3 + \underline{X}_{0,j}$
- we apply a deformation such that the strains  $\underline{v}$ ,  $\underline{k}$  are constant for all  $s$
- we obtain the unknown  $\underline{x}_{0,j}$  by minimizing the unit cell energy:

$$\underline{x}_{0,j} = \arg \min_{(\underline{x}_{0,j})} E(\underline{x}_{0,1}, \dots, \underline{x}_{0,M}; \underline{v}, \underline{k})$$

- the minimization is carried out subject to constraints:  
(mass) center and orientation of the unit cell must be preserved, i.e.

$$\sum_{j=1}^M m_j \cdot \underline{x}_{0,j} = \underline{0}$$

and

$$\sum_{j=1}^M m_j \cdot \underbrace{\begin{bmatrix} x_2 \cdot x_3 \\ x_1 \cdot x_3 \\ x_1 \cdot x_2 \end{bmatrix}}_{=: \underline{M}_j} = \underline{0}$$

with  $x_1, x_2, x_3$  the components of  $\underline{x}_{0,j}$

## Direct molecular approach (part 6)

### minimization of energy under constraints

$$\min_{(\underline{x}_{0,j}; \underline{\lambda}, \underline{\mu})} E(\underline{x}_{0,1}, \dots, \underline{x}_{0,M}; \underline{v}, \underline{k}) + \underline{\lambda} \cdot \sum_{j=1}^M m_j \cdot \underline{x}_{0,j} + \underline{\mu} \cdot \sum_{j=1}^M \underline{M}_j$$

setting the **first derivative** to zero we obtain

$$\frac{\partial E}{\partial \underline{x}_{0,j}} + m_j \cdot \underline{\lambda} + m_j \cdot \underbrace{\begin{bmatrix} 0 & x_3 & x_2 \\ x_3 & 0 & x_1 \\ x_2 & x_1 & 0 \end{bmatrix}}_{=:\underline{\underline{M}}_j} \cdot \underline{\mu} = \underline{0} \quad \forall j = 1, \dots, M$$

together with the 2 constraint equations this gives a system of  $3M + 6$  algebraic equations

## Direct molecular approach (part 7)

$$-\frac{\partial E}{\partial \underline{x}_{0,j}} = m_j \cdot \underline{\lambda} + \underline{\underline{M}}_j \cdot \underline{\mu} \quad \forall j = 1, \dots, M$$

physical meaning:

- $-\frac{\partial E}{\partial \underline{x}_{0,j}}$  is the force on basis atom  $j$  (due to its bond interactions with all other atoms)
- $(m_j \cdot \underline{\lambda} + \underline{\underline{M}}_j \cdot \underline{\mu})$  is the external force on basis atom  $j$  that is required to keep the nanorod in the uniform configuration  $(\underline{v}, \underline{k})$

in the case of pure bending or combined extension & torsion the extra constraint forces  $(m_j \cdot \underline{\lambda} + \underline{\underline{M}}_j \cdot \underline{\mu})$  are zero, i.e. no such forces are required to hold the beam in that configuration

## Direct molecular approach (part 8)

the **strain energy density of the nanorod** is then given by

$$\psi(\underline{v}, \underline{k}) = \frac{1}{l_0} \cdot E(\hat{\underline{x}}_{0,1}(\underline{v}, \underline{k}), \dots, \hat{\underline{x}}_{0,M}(\underline{v}, \underline{k}); \underline{v}, \underline{k})$$

with

$$(\hat{\underline{x}}_{0,j}(\underline{v}, \underline{k}))_{j=1,\dots,M} = \arg \min_{(\underline{x}_{0,j})_{j=1,\dots,M}} E(\underline{x}_{0,1}, \dots, \underline{x}_{0,M}; \underline{v}, \underline{k})$$

such that the kinematic constraints are satisfied

for further information on this topic please consider reading the paper “A Helical Cauchy-Born Rule for Special Cosserat Rod Modeling of Nano and Continuum Rods” by Kumar et al. in Journal of Elasticity 124, June 2016

## Weak form of the equations (part 1)

by multiplying the **strong form** of the equations

$$\underline{n}'(s) + \hat{\underline{n}}(s) = \rho_0 \cdot A \cdot \ddot{\underline{r}}(s)$$

$$\underline{m}'(s) + \underline{r}'(s) \times \underline{n}(s) + \hat{\underline{m}}(s) = \rho_0 \cdot \frac{d}{dt} \left( \underline{I}_{\equiv 0} \cdot \underline{\omega} \right)$$

with test functions  $\delta \underline{r}(s)$  and  $\delta \underline{\Theta}(s)$  respectively and integrating over the length of the rod we get a weak form of the equations ...

$$\int_0^L \left( \underline{n}'(s) + \hat{\underline{n}}(s) \right) \cdot \delta \underline{r}(s) \, ds = \int_0^L \rho_0 \cdot A \cdot \ddot{\underline{r}}(s) \cdot \delta \underline{r}(s) \, ds$$

$$\int_0^L \left( \underline{m}'(s) + \underline{r}'(s) \times \underline{n}(s) + \hat{\underline{m}}(s) \right) \cdot \delta \underline{\Theta}(s) \, ds = \int_0^L \rho_0 \cdot \frac{d}{dt} \left( \underline{I}_{\equiv 0} \cdot \underline{\omega} \right) \cdot \delta \underline{\Theta}(s) \, ds$$

where  $\underline{I}_{\equiv 0}$  is the moment of area tensor



## Weak form of the equations (part 2)

by adding both equations together we obtain the **weak form**

$$G = \underbrace{\int_0^L \left( \rho_0 \cdot A \cdot \ddot{\underline{r}}(s) \cdot \delta \underline{r}(s) + \rho_0 \cdot \frac{d}{dt} (\underline{I}_{=0} \cdot \underline{\omega}) \cdot \delta \underline{\Theta}(s) \right) ds}_{=: G_{\text{dyn}}} - \underbrace{\int_0^L \left( \underline{n}'(s) + \hat{\underline{n}}(s) \right) \cdot \delta \underline{r}(s) ds - \int_0^L \left( \underline{m}'(s) + \underline{r}'(s) \times \underline{n}(s) + \hat{\underline{m}}(s) \right) \cdot \delta \underline{\Theta}(s) ds}_{=: G_{\text{stat}}}$$

since the internal contact force  $\underline{n}(s)$  and the internal moment  $\underline{m}(s)$  depend on the strains  $\underline{v}(s)$  and  $\underline{k}(s)$  they are of first order in the kinematic variables  $\underline{r}(s)$  and  $\underline{\Theta}(s)$   
with  $\underline{\underline{R}}(s) = \exp(\underline{\underline{\Theta}}(s)) = \exp([\underline{\Theta}]_{\times})$

→  $\underline{n}'(s)$  and  $\underline{m}'(s)$  are therefore of second order in the kinematic variables

## Weak form of the equations (part 3)

we can relax the regularity requirements for a solution by transferring one order of the derivatives onto the test functions via integration by parts ...

$$\begin{aligned}
 G_{\text{stat}} &= - \int_0^L \left( \left( \underline{n}(s) \cdot \delta \underline{r}(s) \right)' - \underline{n}(s) \cdot \delta \underline{r}'(s) + \left( \underline{m}(s) \cdot \delta \underline{\Theta}(s) \right)' - \underline{m}(s) \cdot \delta \underline{\Theta}'(s) + \right. \\
 &\quad \left. + \hat{\underline{n}}(s) \cdot \delta \underline{r}(s) + \hat{\underline{m}}(s) \cdot \delta \underline{\Theta}(s) + \left( \underline{r}'(s) \times \underline{n}(s) \right) \cdot \delta \underline{\Theta}(s) \right) ds = \\
 &\quad \underbrace{\int_0^L \left( \underline{n}(s) \cdot \delta \underline{r}'(s) + \underline{m}(s) \cdot \delta \underline{\Theta}'(s) + \left( \underline{n}(s) \times \underline{r}'(s) \right) \cdot \delta \underline{\Theta}(s) \right) ds}_{\text{internal response}} + \\
 &\quad \underbrace{- \int_0^L \left( \hat{\underline{n}}(s) \cdot \delta \underline{r}(s) + \hat{\underline{m}}(s) \cdot \delta \underline{\Theta}(s) \right) ds}_{\text{external distributed loads}} - \underbrace{\left( \underline{n}_p(s) \cdot \delta \underline{r}(s) + \underline{m}_p(s) \cdot \delta \underline{\Theta}(s) \right) \Big|_0^L}_{\text{external loads at boundaries}}
 \end{aligned}$$

with  $\underline{n}_p$  and  $\underline{m}_p$  as prescribed quantities at the boundary  
 whereas  $\underline{n}(s)$  and  $\underline{m}(s)$  are obtained from constitutive law

## Weak form of the equations (part 4)

changing the order of the scalar triple product gives ...

$$G_{\text{stat}} = \overbrace{\int_0^L \left( \underline{n}(s) \cdot \delta \underline{r}'(s) + \underline{m}(s) \cdot \delta \underline{\Theta}'(s) + \underline{n}(s) \cdot \left( \underline{r}'(s) \times \delta \underline{\Theta}(s) \right) \right) ds}^{\text{internal response}} +$$

$$\underbrace{- \int_0^L \left( \hat{\underline{n}}(s) \cdot \delta \underline{r}(s) + \hat{\underline{m}}(s) \cdot \delta \underline{\Theta}(s) \right) ds}_{\text{external distributed loads}} - \underbrace{\left( \underline{n}_p(s) \cdot \delta \underline{r}(s) + \underline{m}_p(s) \cdot \delta \underline{\Theta}(s) \right) \bigg|_0^L}_{\text{external loads at boundaries}}$$

we can write the internal part in a more compact form by introducing the operator

$$\underline{\underline{E}}^T(s) = \begin{bmatrix} \underline{I} \frac{d}{ds} & [\underline{r}'(s)]_{\times} \\ \underline{0} & \underline{I} \frac{d}{ds} \end{bmatrix}$$

$$G_{\text{stat}} = \overbrace{\int_0^L \begin{bmatrix} \underline{n} \\ \underline{m} \end{bmatrix} \cdot \underline{\underline{E}}^T(s) \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds}_{\text{internal response}} - \underbrace{\int_0^L \begin{bmatrix} \hat{\underline{n}} \\ \hat{\underline{m}} \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds}_{\text{external distributed loads}} - \underbrace{\begin{bmatrix} \underline{n}_p \\ \underline{m}_p \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} \bigg|_0^L}_{\text{external loads at boundaries}}$$

## Weak form of the equations (part 5)

the weak form of the problem is

$$G = G_{\text{dyn}} + G_{\text{stat}} = 0 \quad \forall (\delta \underline{r}, \delta \underline{\Theta}) \text{ admissible}$$

admissible means that the test functions must satisfy the kinematic boundary conditions

in the sequel we will work with the **static problem**, i.e. we assume  $G_{\text{dyn}} \equiv 0$

the weak form  $G_{\text{stat}}(\underline{r}, \underline{\Theta}; \delta \underline{r}, \delta \underline{\Theta})$  is

- nonlinear in the unknowns  $(\underline{r}, \underline{\Theta})$
- and linear in the test functions  $(\delta \underline{r}, \delta \underline{\Theta})$

in order to find a solution we must linearize the weak form and solve iteratively ...

## Linearization of the weak form (part 1)

we introduce **perturbed versions of the unknowns**

$$\underline{r}_\epsilon(s) = \underline{r}(s) + \epsilon \cdot \Delta \underline{r}(s) \quad \text{and} \quad \underline{\underline{R}}_\epsilon(s) = \exp(\epsilon \cdot \Delta \underline{\underline{\Theta}}(s)) \cdot \exp(\underline{\underline{\Theta}}(s))$$

with  $\Delta \underline{r}$  the increment in  $\underline{r}$  and  $\Delta \underline{\underline{\Theta}} = \text{axial}(\Delta \underline{\underline{\Theta}})$  the increment in  $\underline{\underline{\Theta}}$

we then obtain the **linearized weak form** by truncating the Taylor expansion of  $G_{\text{stat}}(\underline{r}_\epsilon, \underline{\underline{\Theta}}_\epsilon; \delta \underline{r}, \delta \underline{\underline{\Theta}})$  in  $\epsilon$  after the linear term

$$\underbrace{G_{\text{stat}}(\underline{r}_\epsilon, \underline{\underline{\Theta}}_\epsilon; \delta \underline{r}, \delta \underline{\underline{\Theta}})}_{\text{should equal 0}} \approx \underbrace{G_{\text{stat}}(\underline{r}, \underline{\underline{\Theta}}; \delta \underline{r}, \delta \underline{\underline{\Theta}}) + \left. \frac{d}{d\epsilon} G_{\text{stat}}(\underline{r}_\epsilon, \underline{\underline{\Theta}}_\epsilon; \delta \underline{r}, \delta \underline{\underline{\Theta}}) \right|_{\epsilon=0}}_{\text{linearized weak form}} \cdot \epsilon + \text{HOT}$$

## Linearization of the weak form (part 2)

for simplicity we now assume  $\hat{n}(s) \equiv 0$ ,  $\hat{m}(s) \equiv 0$  and we also ignore the boundary terms so we only have

$$G_{\text{stat}} = \int_0^L \begin{bmatrix} \underline{n}(s) \\ \underline{m}(s) \end{bmatrix} \cdot \underline{\underline{E}}^T(s) \cdot \begin{bmatrix} \delta \underline{r}(s) \\ \delta \underline{\Theta}(s) \end{bmatrix} ds$$

we introduce the perturbed version of  $G_{\text{stat}}$

$$G_{\text{stat}}^\epsilon = \int_0^L \begin{bmatrix} \underline{n}_\epsilon(s) \\ \underline{m}_\epsilon(s) \end{bmatrix} \cdot \underline{\underline{E}}_\epsilon^T(s) \cdot \begin{bmatrix} \delta \underline{r}(s) \\ \delta \underline{\Theta}(s) \end{bmatrix} ds$$

and compute the first derivative at  $\epsilon = 0$

$$\left. \frac{d}{d\epsilon} G_{\text{stat}}^\epsilon \right|_{\epsilon=0} = \int_0^L \left. \frac{d}{d\epsilon} \begin{bmatrix} \underline{n}_\epsilon \\ \underline{m}_\epsilon \end{bmatrix} \right|_{\epsilon=0} \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds + \int_0^L \begin{bmatrix} \underline{n} \\ \underline{m} \end{bmatrix} \cdot \left. \frac{d}{d\epsilon} \underline{\underline{E}}_\epsilon^T \right|_{\epsilon=0} \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds$$

in the sequel we will compute the individual terms of the first derivative step by step and then recompile the results

## Linearization of the weak form (part 3)

the linearization of the internal contact force is

$$\left. \frac{d}{d\epsilon} \underline{n}_\epsilon \right|_{\epsilon=0} = \left. \frac{d}{d\epsilon} \left( \underline{R}_\epsilon \cdot \frac{\partial \psi}{\partial \underline{v}}(\underline{v}_\epsilon, \underline{k}_\epsilon) \right) \right|_{\epsilon=0} = \left. \frac{d}{d\epsilon} \underline{R}_\epsilon \right|_{\epsilon=0} \cdot \frac{\partial \psi}{\partial \underline{v}}(\underline{v}, \underline{k}) + \underline{R} \cdot \left. \frac{d}{d\epsilon} \left( \frac{\partial \psi}{\partial \underline{v}}(\underline{v}_\epsilon, \underline{k}_\epsilon) \right) \right|_{\epsilon=0}$$

with

$$\left. \frac{d}{d\epsilon} \underline{R}_\epsilon \right|_{\epsilon=0} = \left. \frac{d}{d\epsilon} \left( \exp(\epsilon \cdot \Delta \underline{\Theta}) \cdot \underline{R} \right) \right|_{\epsilon=0} = \left. \frac{d}{d\epsilon} \exp(\epsilon \cdot \Delta \underline{\Theta}) \right|_{\epsilon=0} \cdot \underline{R} = \Delta \underline{\Theta} \cdot \underline{R}$$

and

$$\left. \frac{d}{d\epsilon} \left( \frac{\partial \psi}{\partial \underline{v}}(\underline{v}_\epsilon, \underline{k}_\epsilon) \right) \right|_{\epsilon=0} = \left. \frac{\partial^2 \psi}{\partial \underline{v}^2} \cdot \frac{d\underline{v}_\epsilon}{d\epsilon} \right|_{\epsilon=0} + \left. \frac{\partial^2 \psi}{\partial \underline{v} \partial \underline{k}} \cdot \frac{d\underline{k}_\epsilon}{d\epsilon} \right|_{\epsilon=0}$$

and the linearized strains (cf. pages 50 and 51)

$$\left. \frac{d\underline{v}_\epsilon}{d\epsilon} \right|_{\epsilon=0} = \left. \frac{d}{d\epsilon} \left( \underline{R}_\epsilon^T \cdot \underline{r}'_\epsilon \right) \right|_{\epsilon=0} = \underline{R}^T \cdot \left( \Delta \underline{r}' + \underline{r}' \times \Delta \underline{\Theta} \right) \quad \text{and} \quad \left. \frac{d\underline{k}_\epsilon}{d\epsilon} \right|_{\epsilon=0} = \underline{R}^T \cdot \Delta \underline{\Theta}'$$

## Linearization of the weak form (part 4)

plugging back in gives ...

$$\begin{aligned} \left. \frac{d}{d\epsilon} \underline{n}_\epsilon \right|_{\epsilon=0} &= \Delta \underline{\Theta} \cdot \underline{R} \cdot \frac{\partial \psi}{\partial \underline{v}} + \underline{R} \cdot \left[ \frac{\partial^2 \psi}{\partial \underline{v}^2} \quad \frac{\partial^2 \psi}{\partial \underline{v} \partial \underline{k}} \right] \cdot \left[ \begin{array}{l} \underline{R}^T \cdot (\Delta \underline{r}' + \underline{r}' \times \Delta \underline{\Theta}) \\ \underline{R}^T \cdot \Delta \underline{\Theta}' = \end{array} \right] \\ &= -\underline{n} \times \Delta \underline{\Theta} + \underline{R} \cdot \left[ \frac{\partial^2 \psi}{\partial \underline{v}^2} \quad \frac{\partial^2 \psi}{\partial \underline{v} \partial \underline{k}} \right] \cdot \left[ \begin{array}{l} \underline{R}^T \cdot (\Delta \underline{r}' + \underline{r}' \times \Delta \underline{\Theta}) \\ \underline{R}^T \cdot \Delta \underline{\Theta}' = \end{array} \right] \end{aligned}$$

likewise the linearization of the internal moment is

$$\left. \frac{d}{d\epsilon} \underline{m}_\epsilon \right|_{\epsilon=0} = \left. \frac{d}{d\epsilon} \left( \underline{R}_\epsilon \cdot \frac{\partial \psi}{\partial \underline{k}}(\underline{v}_\epsilon, \underline{k}_\epsilon) \right) \right|_{\epsilon=0} = \left. \frac{d}{d\epsilon} \underline{R}_\epsilon \right|_{\epsilon=0} \cdot \frac{\partial \psi}{\partial \underline{k}}(\underline{v}, \underline{k}) + \underline{R} \cdot \left. \frac{d}{d\epsilon} \left( \frac{\partial \psi}{\partial \underline{k}}(\underline{v}_\epsilon, \underline{k}_\epsilon) \right) \right|_{\epsilon=0}$$

with the first term already computed above and with

$$\left. \frac{d}{d\epsilon} \left( \frac{\partial \psi}{\partial \underline{k}}(\underline{v}_\epsilon, \underline{k}_\epsilon) \right) \right|_{\epsilon=0} = \frac{\partial^2 \psi}{\partial \underline{k} \partial \underline{v}} \cdot \left. \frac{d \underline{v}_\epsilon}{d\epsilon} \right|_{\epsilon=0} + \frac{\partial^2 \psi}{\partial \underline{k}^2} \cdot \left. \frac{d \underline{k}_\epsilon}{d\epsilon} \right|_{\epsilon=0}$$



## Linearization of the weak form (part 5)

plugging back in gives ...

$$\begin{aligned} \left. \frac{d}{d\epsilon} \underline{m}_\epsilon \right|_{\epsilon=0} &= \Delta \underline{\Theta} \cdot \underline{R} \cdot \frac{\partial \psi}{\partial \underline{k}} + \underline{R} \cdot \left[ \frac{\partial^2 \psi}{\partial \underline{k} \partial \underline{v}} \quad \frac{\partial^2 \psi}{\partial \underline{k}^2} \right] \cdot \left[ \begin{array}{c} \underline{R}^T \cdot (\Delta \underline{r}' + \underline{r}' \times \Delta \underline{\Theta}) \\ \underline{R}^T \cdot \Delta \underline{\Theta}' = \end{array} \right] = \\ &= -\underline{m} \times \Delta \underline{\Theta} + \underline{R} \cdot \left[ \frac{\partial^2 \psi}{\partial \underline{k} \partial \underline{v}} \quad \frac{\partial^2 \psi}{\partial \underline{k}^2} \right] \cdot \left[ \begin{array}{c} \underline{R}^T \cdot (\Delta \underline{r}' + \underline{r}' \times \Delta \underline{\Theta}) \\ \underline{R}^T \cdot \Delta \underline{\Theta}' = \end{array} \right] \end{aligned}$$

we also linearize the introduced operator matrix ...

$$\underline{E}_\epsilon^T(s) = \begin{bmatrix} \underline{I} \frac{d}{ds} & [\underline{r}'_\epsilon(s)]_\times \\ \underline{0} & \underline{I} \frac{d}{ds} \end{bmatrix} \Rightarrow \left. \frac{d}{d\epsilon} \underline{E}_\epsilon^T \right|_{\epsilon=0} = \begin{bmatrix} \underline{0} & [\Delta \underline{r}'(s)]_\times \\ \underline{0} & \underline{0} \end{bmatrix}$$

## Linearization of the weak form (part 6)

finally we obtain the linearized weak form ...

$$\begin{aligned}
 \left. \frac{d}{d\epsilon} G_{\text{stat}}^\epsilon \right|_{\epsilon=0} &= \int_0^L \left( \begin{bmatrix} \underline{\underline{0}} & -[\underline{n}]_\times \\ \underline{\underline{0}} & -[\underline{m}]_\times \end{bmatrix} \cdot \begin{bmatrix} \underline{\Delta r} \\ \underline{\Delta \Theta} \end{bmatrix} + \right. \\
 &+ \underbrace{\begin{bmatrix} \underline{R} & \underline{0} \\ \underline{0} & \underline{R} \end{bmatrix}}_{=:\underline{\underline{\pi}}} \cdot \underbrace{\begin{bmatrix} \frac{\partial^2 \psi}{\partial v^2} & \frac{\partial^2 \psi}{\partial v \partial k} \\ \frac{\partial^2 \psi}{\partial k \partial v} & \frac{\partial^2 \psi}{\partial k^2} \end{bmatrix}}_{=:\underline{\underline{H}}} \cdot \begin{bmatrix} \underline{R}^T & \underline{0} \\ \underline{0} & \underline{R}^T \end{bmatrix} \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \underline{\Delta r} \\ \underline{\Delta \Theta} \end{bmatrix} \Bigg) \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \underline{\delta r} \\ \underline{\delta \Theta} \end{bmatrix} ds + \\
 &+ \int_0^L \underbrace{\begin{bmatrix} \underline{n} \\ \underline{m} \end{bmatrix} \cdot \begin{bmatrix} \underline{0} & [\underline{\Delta r'}]_\times \\ \underline{0} & \underline{0} \end{bmatrix} \cdot \begin{bmatrix} \underline{\delta r} \\ \underline{\delta \Theta} \end{bmatrix}}_{\underline{n} \cdot (\underline{\Delta r'} \times \underline{\delta \Theta})} ds = \\
 &= \int_0^L \begin{bmatrix} \underline{0} & -[\underline{n}]_\times \\ \underline{0} & -[\underline{m}]_\times \end{bmatrix} \cdot \begin{bmatrix} \underline{\Delta r} \\ \underline{\Delta \Theta} \end{bmatrix} \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \underline{\delta r} \\ \underline{\delta \Theta} \end{bmatrix} ds + \int_0^L \underline{\underline{\pi}} \cdot \underline{\underline{H}} \cdot \underline{\underline{\pi}}^T \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \underline{\Delta r} \\ \underline{\Delta \Theta} \end{bmatrix} \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \underline{\delta r} \\ \underline{\delta \Theta} \end{bmatrix} ds + \\
 &+ \int_0^L [\underline{n}]_\times \cdot \underline{\Delta r'} \cdot \underline{\delta \Theta} ds
 \end{aligned}$$

## Linearization of the weak form (recap)

$$\overbrace{G_{\text{stat}}(\underline{r}_\epsilon, \underline{\Theta}_\epsilon; \delta \underline{r}, \delta \underline{\Theta})}^{\text{nonlinear weak form}} \equiv G_{\text{stat}}(\Delta \underline{r}, \Delta \underline{\Theta}; \underline{r}, \underline{\Theta}; \delta \underline{r}, \delta \underline{\Theta}) \approx$$

$$\approx \underbrace{G_{\text{stat}}(\underline{r}, \underline{\Theta}; \delta \underline{r}, \delta \underline{\Theta}) + DG_{\text{stat}}(\Delta \underline{r}, \Delta \underline{\Theta}; \underline{r}, \underline{\Theta}; \delta \underline{r}, \delta \underline{\Theta})}_{\text{linearized weak form}}$$

with the nonlinear weak form

$$G_{\text{stat}} = \int_0^L \begin{bmatrix} \underline{n} \\ \underline{m} \end{bmatrix} \cdot \underline{\underline{E}}^T(s) \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds - \int_0^L \begin{bmatrix} \hat{\underline{n}} \\ \hat{\underline{m}} \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds - \begin{bmatrix} \underline{n}_p \\ \underline{m}_p \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} \bigg|_0^L$$

and the first order term from its Taylor expansion

$$DG_{\text{stat}} = \int_0^L \begin{bmatrix} \underline{0} & -[\underline{n}]_\times \\ \underline{0} & -[\underline{m}]_\times \end{bmatrix} \cdot \begin{bmatrix} \Delta \underline{r} \\ \Delta \underline{\Theta} \end{bmatrix} \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds + \int_0^L \underline{\underline{\pi}} \cdot \underline{\underline{H}} \cdot \underline{\underline{\pi}}^T \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \Delta \underline{r} \\ \Delta \underline{\Theta} \end{bmatrix} \cdot \underline{\underline{E}}^T \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds +$$

$$+ \int_0^L [\underline{n}]_\times \cdot \Delta \underline{r}' \cdot \delta \underline{\Theta} ds + \dots \text{(first order terms of distributed load and boundary terms)}$$

## Iteratively solving the nonlinear problem

note that  $DG_{\text{stat}}(\Delta \underline{r}, \Delta \underline{\Theta}; \underline{r}, \underline{\Theta}; \delta \underline{r}, \delta \underline{\Theta})$  is

- linear in  $(\Delta \underline{r}, \Delta \underline{\Theta})$
- nonlinear in  $(\underline{r}, \underline{\Theta})$
- and linear in  $(\delta \underline{r}, \delta \underline{\Theta})$

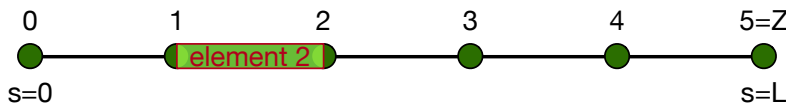
## Newton-Rhapson method for solving the nonlinear problem

$$G_{\text{stat}}(\underline{r}_{\epsilon}, \underline{\Theta}_{\epsilon}; \delta \underline{r}, \delta \underline{\Theta}) = 0$$

- in first iteration guess initial  $\underline{r}(s)$  and  $\underline{\Theta}(s)$
- linearize the weak form about  $\underline{r}(s)$  and  $\underline{\Theta}(s)$
- obtain  $\Delta \underline{r}(s)$  and  $\Delta \underline{\Theta}(s)$  by solving the linearized problem
- update:  $\underline{r}(s) \leftarrow \underline{r}(s) + \Delta \underline{r}(s)$  and  $\underline{\Theta}(s) \leftarrow \dots$  (a bit more tricky)
- check if updated  $\underline{r}(s)$  and  $\underline{\Theta}(s)$  solve the nonlinear problem with a small enough error (an error bound needs to be defined)
- if the error is too large then reiterate by linearizing about the updated  $\underline{r}(s)$  and  $\underline{\Theta}(s)$  ...

## Discretization of the weak form (part 1)

we discretize a rod of length  $L$



with  $Z$  elements and  $Z + 1$  nodes

we discretize the displacements by

$$\Delta \underline{r}(s) \approx \Delta \underline{r}_i \cdot N_i(s)$$

$$\Delta \underline{\Theta}(s) \approx \Delta \underline{\Theta}_i \cdot N_i(s)$$

and the test functions by

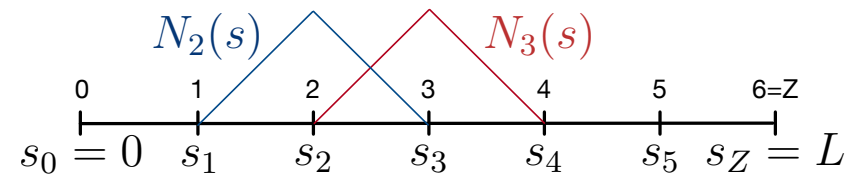
$$\delta \underline{r}(s) \approx \delta \underline{r}_i \cdot N_i(s)$$

$$\delta \underline{\Theta}(s) \approx \delta \underline{\Theta}_i \cdot N_i(s)$$

with shape functions  $N_i(s)$  such that

$$N_i(s) = \begin{cases} 1 & \text{at node } i \\ 0 & \text{at every other node} \end{cases}$$

example with linear shape functions:



meaning of  $\Delta \underline{r}_i$ ,  $\Delta \underline{\Theta}_i$ :

$$\Delta \underline{r}(s_j) \approx \Delta \underline{r}_i \cdot N_i(s_j) = \Delta \underline{r}_i \cdot \delta_{ij} = \underline{r}_j$$

→ approximated displacements at the nodes

→ interpolation of displacements via shape functions between the nodes

## Discretization of the weak form (part 2)

recall the nonlinear weak form

$$G_{\text{stat}} = \int_0^L \begin{bmatrix} \underline{n} \\ \underline{m} \end{bmatrix} \cdot \underline{\underline{E}}^T(s) \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds - \int_0^L \begin{bmatrix} \hat{\underline{n}} \\ \hat{\underline{m}} \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds - \begin{bmatrix} \underline{n}_p \\ \underline{m}_p \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} \Big|_0^L$$

we plug in the discrete approximations of the test functions

$$\delta \underline{r}(s) \approx \delta \underline{r}_i \cdot N_i(s) \quad \text{and} \quad \delta \underline{\Theta}(s) \approx \delta \underline{\Theta}_i \cdot N_i(s)$$

- we take notice that  $\underline{n}(s)$ ,  $\underline{m}(s)$  depend on the current state  $(\underline{r}, \underline{\Theta})$
- then we work out the next part ...

$$\begin{aligned} \underline{\underline{E}}^T(s) \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} &= \begin{bmatrix} \underline{\underline{I}} \frac{d}{ds} [\underline{r}'(s)]_{\times} \\ \underline{0} \quad \underline{\underline{I}} \frac{d}{ds} \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} = \begin{bmatrix} \delta \underline{r}' + \underline{r}' \times \delta \underline{\Theta} \\ \delta \underline{\Theta}' \end{bmatrix} \approx \\ &\approx \begin{bmatrix} \delta \underline{r}_i \cdot N_i'(s) + \underline{r}' \times (N_i(s) \cdot \delta \underline{\Theta}_i) \\ \delta \underline{\Theta}_i \cdot N_i'(s) \end{bmatrix} = \underbrace{\begin{bmatrix} N_i'(s) \cdot \underline{\underline{I}} & N_i(s) \cdot [\underline{r}'(s)]_{\times} \\ \underline{0} & N_i' \cdot \underline{\underline{I}} \end{bmatrix}}_{=: \underline{\underline{E}}_i^T(s)} \cdot \begin{bmatrix} \delta \underline{r}_i \\ \delta \underline{\Theta}_i \end{bmatrix} \end{aligned}$$

note that  $\underline{\underline{E}}_i^T$  is a regular matrix instead of being an operator written in matrix form

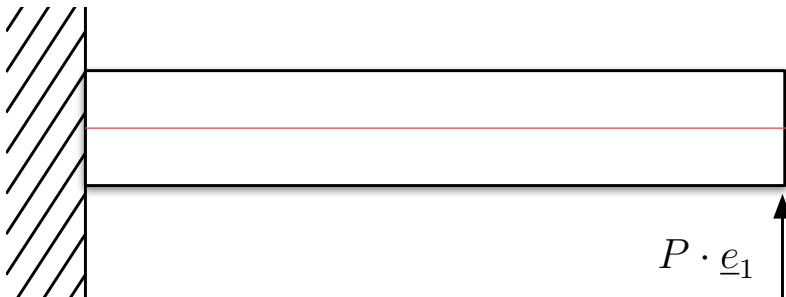
## Discretization of the weak form (part 3)

- the term for the distributed forces is straightforward:  
we just dot the forces with the approximated test functions
- the boundary term can be simplified: e.g. at  $s = L$  we have  
 $\delta \underline{r}(L) \approx N_i(L) \cdot \delta \underline{r}_i = N_Z(L) \cdot \delta \underline{r}_Z = \delta \underline{r}_Z$

we obtain  $G_{\text{stat}}^h(\underline{r}, \underline{\Theta}; \delta \underline{r}, \delta \underline{\Theta}) =$

$$\int_0^L \begin{bmatrix} \underline{n} \\ \underline{m} \end{bmatrix} \cdot \underline{\underline{E}}_i^T \cdot \begin{bmatrix} \delta \underline{r}_i \\ \delta \underline{\Theta}_i \end{bmatrix} ds - \int_0^L \begin{bmatrix} \hat{\underline{n}} \\ \hat{\underline{m}} \end{bmatrix} \cdot N_i \cdot \begin{bmatrix} \delta \underline{r}_i \\ \delta \underline{\Theta}_i \end{bmatrix} ds - \begin{bmatrix} \underline{n}_p(L) \\ \underline{m}_p(L) \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r}_Z \\ \delta \underline{\Theta}_Z \end{bmatrix} + \begin{bmatrix} \underline{n}_p(0) \\ \underline{m}_p(0) \end{bmatrix} \cdot \begin{bmatrix} \delta \underline{r}_0 \\ \delta \underline{\Theta}_0 \end{bmatrix}$$

for a cantilever problem



we have

- $\underline{n}_p(L) = P \cdot \underline{e}_1$  and  $\underline{m}_p(L) = 0$
- and on the clamped end the boundary term vanishes since  $\delta \underline{r}_0 = \delta \underline{\Theta}_0 = 0$  (admissibility)

## Discretization of the weak form (part 4)

we can rewrite the discretized weak form using a global **residual vector**  $\underline{R}$

$$G_{\text{stat}}^h(\underline{r}, \underline{\Theta}; \delta \underline{r}, \delta \underline{\Theta}) = \underbrace{\begin{bmatrix} \underline{R}_0 \\ \underline{R}_1 \\ \vdots \\ \underline{R}_Z \end{bmatrix}}_{=:\underline{R}} \cdot \underbrace{\begin{bmatrix} \begin{bmatrix} \delta \underline{r}_0 \\ \delta \underline{\Theta}_0 \end{bmatrix} \\ \begin{bmatrix} \delta \underline{r}_1 \\ \delta \underline{\Theta}_1 \end{bmatrix} \\ \vdots \\ \begin{bmatrix} \delta \underline{r}_Z \\ \delta \underline{\Theta}_Z \end{bmatrix} \end{bmatrix}}_{=:\underline{\delta}}$$

with  $Z + 1$  blocks / 6 dimensional subvectors of  $\underline{R}$  defined as

$$\underline{R}_i = \int_0^L \underline{E}_i(s) \cdot \begin{bmatrix} \underline{n}(s) \\ \underline{m}(s) \end{bmatrix} ds - \int_0^L N_i(s) \begin{bmatrix} \hat{\underline{n}}(s) \\ \hat{\underline{m}}(s) \end{bmatrix} ds$$

the boundary terms are added to  $\underline{R}_0$  and  $\underline{R}_Z$

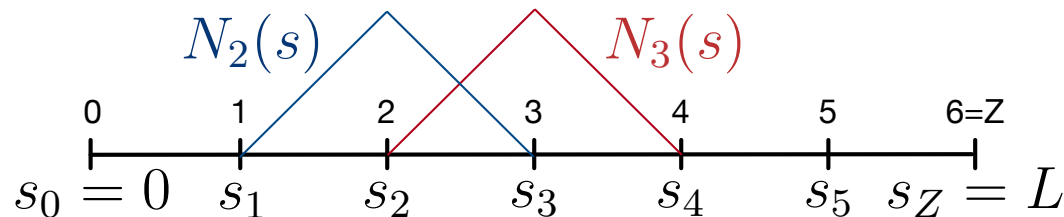


## Assembly of the residual vector (part 1)

for each of the  $Z + 1$  subvectors we integrate from  $s = 0$  to  $s = L$   
the integration domain can be divided into the  $Z$  elements

$$\underline{R}_i = \sum_{e=1}^Z \left( \int_{s_{e-1}}^{s_e} \underline{E}_i(s) \cdot \begin{bmatrix} \underline{n}(s) \\ \underline{m}(s) \end{bmatrix} ds - \int_{s_{e-1}}^{s_e} N_i(s) \begin{bmatrix} \hat{n}(s) \\ \hat{m}(s) \end{bmatrix} ds \right)$$

now we benefit from the compact support of the shape functions,  
i.e. a shape function can only be non-zero within a bounded region



in the 1D case every element has two adjacent nodes

→ there are two shape functions that can contribute to each integral

example: for element  $e = 3$  only  $N_{e-1} = N_2$  and  $N_e = N_3$  need to be considered

## Assembly of the residual vector (part 2)

we can assemble  $\underline{R}$  with a for-loop over the elements

for  $e = 1 : Z$

$$\begin{aligned}\underline{R}_{e-1} & += \int_{s_{e-2}}^{s_{e-1}} \underline{E}_{e-1}(s) \cdot \begin{bmatrix} \underline{n}(s) \\ \underline{m}(s) \end{bmatrix} ds - \int_{s_{e-2}}^{s_{e-1}} N_{e-1}(s) \begin{bmatrix} \hat{n}(s) \\ \hat{m}(s) \end{bmatrix} ds \\ \underline{R}_e & += \int_{s_{e-1}}^{s_e} \underline{E}_e(s) \cdot \begin{bmatrix} \underline{n}(s) \\ \underline{m}(s) \end{bmatrix} ds - \int_{s_{e-1}}^{s_e} N_e(s) \begin{bmatrix} \hat{n}(s) \\ \hat{m}(s) \end{bmatrix} ds\end{aligned}$$

end

remarks:

- after the loop, contributions of the boundary terms at the first and last node are added
- the integration within the elements is done with a numerical integration scheme

## Discretization of the linearized weak form (part 1)

recall the increment  $DG_{\text{stat}}$  of  $G_{\text{stat}}$  that is linear in  $\Delta \underline{r}$ ,  $\Delta \underline{\Theta}$

$$DG_{\text{stat}} = \int_0^L \begin{bmatrix} \underline{0} & -[\underline{n}]_{\times} \\ \underline{0} & -[\underline{m}]_{\times} \end{bmatrix} \cdot \begin{bmatrix} \Delta \underline{r} \\ \Delta \underline{\Theta} \end{bmatrix} \cdot \underline{E}^T \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds + \int_0^L \underline{\pi} \cdot \underline{H} \cdot \underline{\pi}^T \cdot \underline{E}^T \cdot \begin{bmatrix} \Delta \underline{r} \\ \Delta \underline{\Theta} \end{bmatrix} \cdot \underline{E}^T \cdot \begin{bmatrix} \delta \underline{r} \\ \delta \underline{\Theta} \end{bmatrix} ds +$$

$$+ \int_0^L [\underline{n}]_{\times} \cdot \Delta \underline{r}' \cdot \delta \underline{\Theta} ds + \dots \text{(first order terms of distributed load and boundary terms)}$$

remarks regarding the missing terms:

- in the case that distributed loads act independently of the configuration we have  $\hat{\underline{n}}_{\epsilon} = \hat{\underline{n}}$  and therefore  $\frac{d\hat{\underline{n}}_{\epsilon}}{d\epsilon} = 0$ ,  $\rightarrow$  no contribution to  $DG_{\text{stat}}$
- likewise for the cantilever with  $\underline{n}_p(L) = P \cdot \underline{e}_1$  there is no contribution to  $DG_{\text{stat}}$
- on the other hand, for a follower load with  $\underline{n}_p(L) = P \cdot \underline{d}_1 = P \cdot \underline{R} \cdot \underline{e}_1$  there will be a contribution to  $DG_{\text{stat}}$
- for now we will forget about these extra contributions

## Discretization of the linearized weak form (part 2)

we plug the discrete approximations of the test functions into  $DG_{\text{stat}}$

$$\delta \underline{r}(s) \approx \delta \underline{r}_i \cdot N_i(s) \quad \text{and} \quad \delta \underline{\Theta}(s) \approx \delta \underline{\Theta}_i \cdot N_i(s)$$

and also the discrete approximations of the increments

$$\Delta \underline{r}(s) \approx \Delta \underline{r}_j \cdot N_j(s) \quad \text{and} \quad \Delta \underline{\Theta}(s) \approx \Delta \underline{\Theta}_j \cdot N_j(s)$$

we obtain  $DG_{\text{stat}}^h(\Delta \underline{r}_j, \Delta \underline{\Theta}_j; \underline{r}, \underline{\Theta}; \delta \underline{r}_i, \delta \underline{\Theta}_i) =$

$$\begin{aligned} & \int_0^L \begin{bmatrix} \underline{0} & -[\underline{n}]_{\times} \\ \underline{0} & -[\underline{m}]_{\times} \end{bmatrix} \cdot N_j \cdot \begin{bmatrix} \Delta \underline{r}_j \\ \Delta \underline{\Theta}_j \end{bmatrix} \cdot \underline{E}_i^T \cdot \begin{bmatrix} \delta \underline{r}_i \\ \delta \underline{\Theta}_i \end{bmatrix} ds + \int_0^L \underline{\pi} \cdot \underline{H} \cdot \underline{\pi}^T \cdot \underline{E}_j^T \cdot \begin{bmatrix} \Delta \underline{r}_j \\ \Delta \underline{\Theta}_j \end{bmatrix} \cdot \underline{E}_i^T \cdot \begin{bmatrix} \delta \underline{r}_i \\ \delta \underline{\Theta}_i \end{bmatrix} ds + \\ & + \int_0^L N_i \cdot N_j' \cdot (\underline{n} \times \Delta \underline{r}_j) \cdot \delta \underline{\Theta}_i ds + \dots \text{ (first order terms of distributed load and boundary terms) } \end{aligned}$$

## Discretization of the linearized weak form (part 3)

we want to rewrite  $DG_{\text{stat}}^h$  in the form

$$DG_{\text{stat}}^h(\underline{\Delta}; \underline{r}, \underline{\Theta}; \underline{\delta}) = (\underline{K} \cdot \underline{\Delta}) \cdot \underline{\delta} = (\underline{K}_{ij} \cdot \underline{\Delta}_j) \cdot \underline{\delta}_i = \left( \underline{K}_{ij} \cdot \begin{bmatrix} \underline{\Delta}_{r_j} \\ \underline{\Delta}_{\Theta_j} \end{bmatrix} \right) \cdot \begin{bmatrix} \underline{\delta}_{r_i} \\ \underline{\delta}_{\Theta_i} \end{bmatrix}$$

- where  $\underline{K}$  is the stiffness matrix and  $\underline{K}_{ij}$  refers to the  $6 \times 6$  submatrices of  $\underline{K}$
- $\underline{\Delta}$  and  $\underline{\delta}$  are the vectors that contain the discrete approximations of the increments and test functions, respectively
- note that  $\underline{\delta}$  was already defined (cf. page 128) ;  $\underline{\Delta}$  is of the same structure

we get

$$\underline{K}_{ij} = \int_0^L \underbrace{\underline{E}_i \cdot \underline{\pi} \cdot \underline{H} \cdot \underline{\pi}^T \cdot \underline{E}_j^T}_{\text{material stiffness matrix}} ds + \int_0^L \underbrace{N_j \cdot \underline{E}_i \cdot \begin{bmatrix} \underline{0} & -[\underline{n}]_{\times} \\ \underline{0} & -[\underline{m}]_{\times} \end{bmatrix} + N_i \cdot N_j' \cdot \begin{bmatrix} \underline{0} & \underline{0} \\ [\underline{n}]_{\times} & \underline{0} \end{bmatrix}}_{\text{geometric stiffness matrix}} ds$$

- the material stiffness matrix is symmetric
- the geometric stiffness matrix is not symmetric ; but it becomes symmetric for the configuration that corresponds to static equilibrium (cf. Simo & Vu-Quoc 1986)

## Iteratively solving the nonlinear problem (revisited, part 1)

recall the truncated Taylor expansion of the weak form about the configuration  $(\underline{r}, \underline{\Theta})$

$$\begin{aligned}
 & \underbrace{G_{\text{stat}}\left(\underline{r} + \Delta\underline{r}, \exp(\Delta\underline{\Theta}) \cdot \exp(\underline{\Theta}); \delta\underline{r}, \delta\underline{\Theta}\right)}_{\text{nonlinear weak form} \stackrel{!}{=} 0 \text{ but intractable problem}} \approx \\
 & \approx \underbrace{G_{\text{stat}}\left(\underline{r}, \exp(\underline{\Theta}); \delta\underline{r}, \delta\underline{\Theta}\right) + DG_{\text{stat}}\left(\Delta\underline{r}, \Delta\underline{\Theta}; \underline{r}, \exp(\underline{\Theta}); \delta\underline{r}, \delta\underline{\Theta}\right)}_{\text{linearized weak form}} \approx \\
 & \approx \underbrace{\underline{R} \cdot \underline{\delta} + (\underline{K} \cdot \underline{\Delta}) \cdot \underline{\delta}}_{\text{discretized linearized weak form}} = (\underline{R} + \underline{K} \cdot \underline{\Delta}) \cdot \underline{\delta} \stackrel{!}{=} 0 \quad \forall \underline{\delta} \text{ (admissible)} \\
 & \Rightarrow \underline{R} + \underline{K} \cdot \underline{\Delta} = \underline{0} \quad \Rightarrow \underline{\Delta} = -\underline{K}^{-1} \cdot \underline{R}
 \end{aligned}$$

## Iteratively solving the nonlinear problem (revisited, part 2)

- by solving the linearized (about some configuration) and discrete version of the underlying nonlinear problem we obtain an updated configuration (via  $\underline{\Delta}$ ), that is a better solution of the nonlinear problem than the configuration we started with
- the updated configuration is given by

$$\underline{r} \leftarrow \underline{r} + \underline{\Delta} r$$
$$\underline{\Theta} \leftarrow \Theta\left(\exp(\underline{\Delta}\underline{\Theta}) \cdot \exp(\underline{\Theta})\right)$$

where  $\Theta(\cdot)$  means extraction of  $\underline{\Theta}$  from  $\underline{R}$

- a solution/configuration needs to be guessed to start the Newton-Rhapson iteration
- this guess converges to the true solution with each iteration
- we stop to iterate if the solution is close enough to 0  
(theoretical limit: machine precision)

## Iteratively solving the nonlinear problem (revisited, part 3)

- to obtain  $\underline{\underline{K}}_{ij}$  in each iteration we use numerical integration
- therefore we need the strains at the quadrature points  
but displacements are given at the nodes
- within element  $e$  we have

$$\underline{r}(s) = \underline{r}_i \cdot N_i(s) = \underline{r}_{e-1} \cdot N_{e-1}(s) + \underline{r}_e \cdot N_e(s)$$

$$\underline{\Theta}(s) = \underline{\Theta}_i \cdot N_i(s) = \underline{\Theta}_{e-1} \cdot N_{e-1}(s) + \underline{\Theta}_e \cdot N_e(s)$$

- from this we compute the updated strains

$$\underline{v}_{\text{new}} = \underline{\underline{R}}_{\text{new}}^T \cdot \underline{r}'_{\text{new}} \quad \text{and} \quad \underline{k}_{\text{new}} = \text{axial}(\underline{\underline{R}}_{\text{new}}^T \cdot \underline{R}'_{\text{new}})$$

but it is not trivial to obtain  $\underline{R}'_{\text{new}}$  at the quadrature points

because an interpolation like  $\underline{\underline{R}} = \underline{\underline{R}}_i \cdot N_i$  is nonsense ; luckily, Simo has worked out an update formula:

$$\underline{k}_{\text{new}} = \underline{k}_{\text{old}} + \underline{\underline{R}}'_{\text{new}} \cdot \underline{\beta}$$

with  $\underline{\beta} = \dots$  (next slide)



## Iteratively solving the nonlinear problem (revisited, part 4)

$$\underline{\beta} = \frac{\sin\|\underline{\Delta\theta}\|}{\|\underline{\Delta\theta}\|} \cdot \underline{\Delta\theta}' + \left(1 - \frac{\sin\|\underline{\Delta\theta}\|}{\|\underline{\Delta\theta}\|}\right) \cdot \left(\frac{\underline{\Delta\theta} \cdot \underline{\Delta\theta}'}{\|\underline{\Delta\theta}\|^2}\right) \cdot \underline{\Delta\theta} + \frac{1}{2} \cdot \left(\frac{\sin\left(\frac{1}{2} \cdot \|\underline{\Delta\theta}\|\right)}{\frac{1}{2} \cdot \|\underline{\Delta\theta}\|}\right)^2 \cdot \underline{\Delta\theta} \times \underline{\Delta\theta}'$$

where we evaluate  $\underline{\Delta\theta}$  at the quadrature points

for  $\underline{\Delta\theta} \approx \underline{0}$  we have  $\underline{\beta} \approx \underline{\Delta\theta}'$