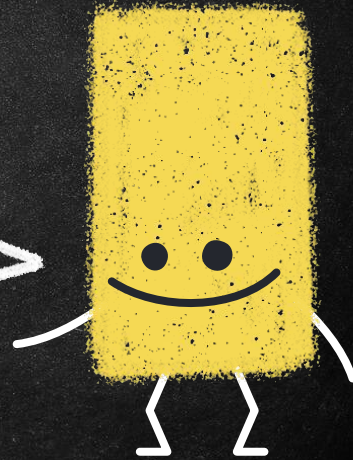



Car Jack

MAE 157 Final Project

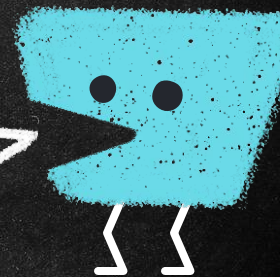
Allison Eiler, Denesse Gomez,
Matthieu Lu, Curtis Abe



Motivation For The Project

A yellow, round, textured cartoon character with a simple smile and two thin legs, positioned to the left of the first speech bubble.

The motivation behind this project is that most heavy duty car jacks that are able to lift trucks are large and unwieldy.

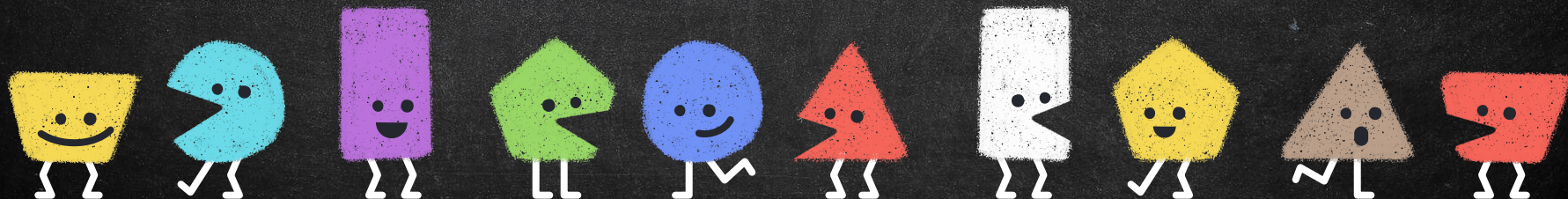
A blue, angular, textured cartoon character with two dots for eyes and two thin legs, positioned to the right of the second speech bubble.

They are required to be wheeled around on rollers and not normally able to be casually stored in smaller vehicles.

Project Objectives

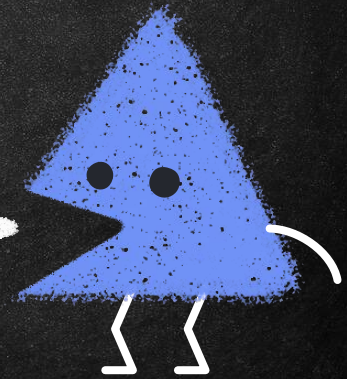
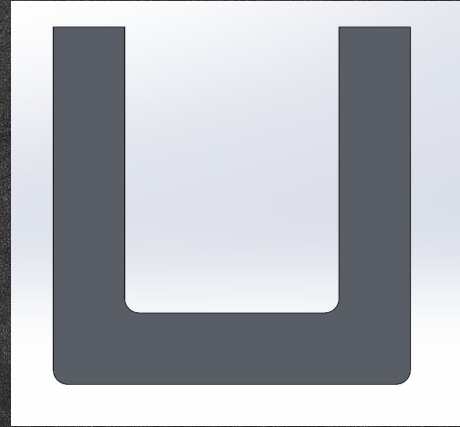
The goal for this project is to design a scissor car jack with:

- Minimum mass
- Ability to lift $\frac{1}{4}$ the weight of a car (up to a midsize pickup truck)



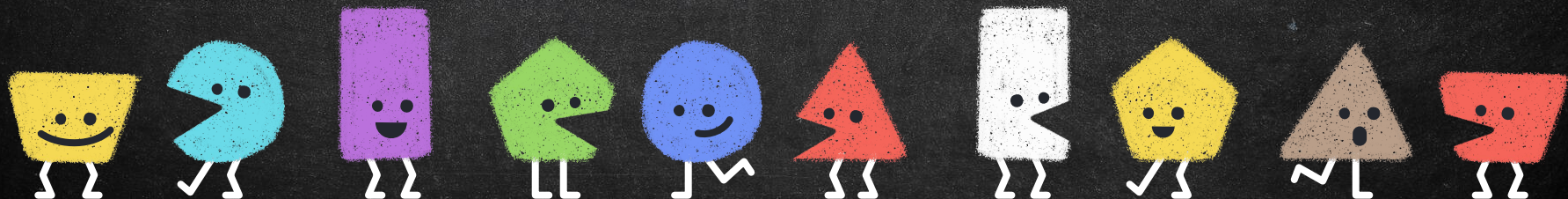
Description of the car jack

- The jack is made up of 4 equal length beams with a U-shaped cross section arranged in a diamond shape.
- The weight of the car will be applied to the top of the diamond





Approach



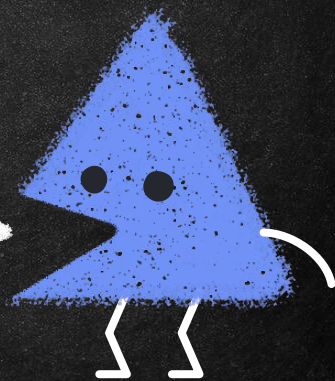
For Structural Analysis

The values we need are the stress in the beams, and the displacements of the nodes.

- Stress failure indicates material failure (the jack would break and collapse)
- Displacement failure means that the jack would not be able to lift the car high enough.

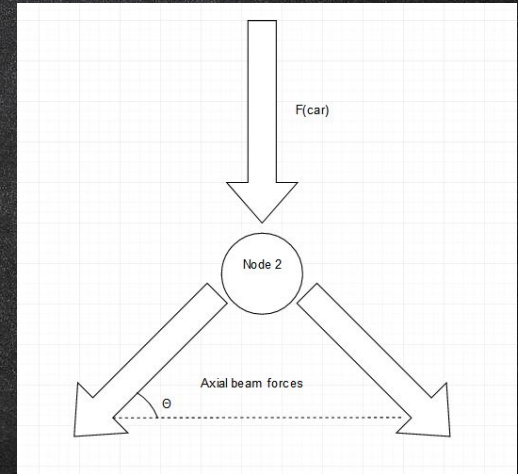
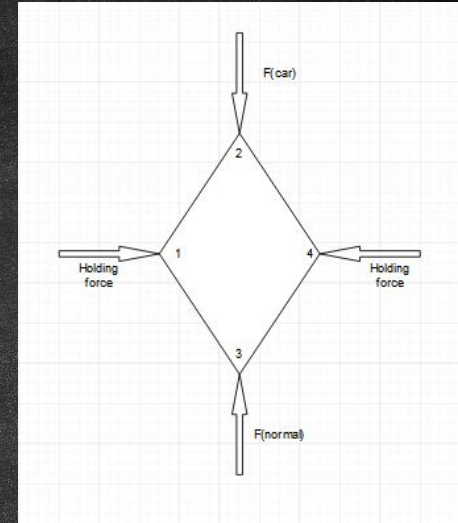
Structural analysis: Initial assumptions

- Weight of the car: 18,894.4 N, based on the weight of a Toyota Tacoma
 - Divided by 4
- Car ground clearance: 15 cm, based on the average ground clearance of a sedan
- Desired lift height: 30 cm, based on the average ground clearance of midsize trucks
- Beam length: 18cm



STRUCTURAL ANALYSIS: STRESS

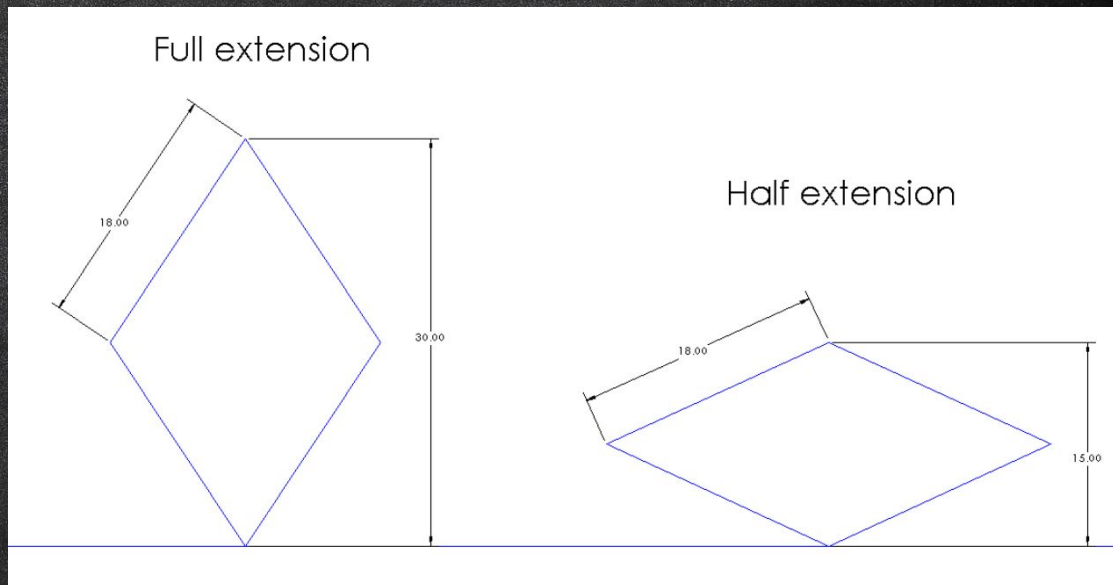
1. Define our boundary conditions for the entire truss
2. Find reaction forces using Newtons Second Law: $\Sigma F_x = 0$ and $\Sigma F_y = 0$
3. Use Newton's second law again on each node to find the axial forces in each beam
4. Find the axial stress in each beam using $\sigma = F / A$



- We found the axial stress in each beam to be:
- “A” denotes the cross sectional area of the beam
- The stresses are negative denoting compressive stress
- We can disregard the stress at full extension. Since it is lower than our half extension stress, it will not dictate our minimum mass

$[\sigma = -5668 / A] \text{ Pa}$ at half extension

$[\sigma = -2833 / A] \text{ Pa}$ at full extension



STRUCTURAL ANALYSIS: DISPLACEMENT

1. Define our boundary conditions
2. Using these boundary conditions define our vectors for node forces (F) and node displacements (U)
3. Find the stiffness matrix of each beam
4. Find the stiffness matrix of the entire truss K
5. Create the system of equations $F = K * U$
6. Solve the system of equations to find the displacement of each node.

```
%set the matrix equal to boundary conditions and solve
for i = 1:8
    e(i, 1) = K(i) == forces(i);
end
vpa(e, 6)

sol = solve(e, [fx1 fx3 fy3 fx4 uy1 ux2 uy2 uy4]);
```

```
%stiffness matrices of each beam
K12 = [c12^2      c12*s12      -(c12^2)      -(c12 * s12)  0 0 0 0;
       c12*s12    s12^2        -(c12 * s12)    -(s12^2)  0 0 0 0;
       -(c12^2)   -(c12 * s12)  c12^2         c12*s12    0 0 0 0;
       -(c12*s12) -(s12^2)      c12*s12        s12^2    0 0 0 0;
       0 0 0 0 0 0 0;
       0 0 0 0 0 0 0;
       0 0 0 0 0 0 0;
       0 0 0 0 0 0 0;];

K13 = [c13^2      c13 * s13    0 0 -(c13^2)      -(c13 * s13)  0 0;
       c13*s13    s13^2        0 0 -(c13 * s13)    -(s13^2)  0 0;
       0 0 0 0 0 0 0;
       0 0 0 0 0 0 0;
       -(c13^2)   -(c13 * s13)  0 0 c13^2         c13*s13  0 0;
       -(c13*s13) -(s13^2)      0 0 c13*s13        s13^2  0 0;
       0 0 0 0 0 0 0;
       0 0 0 0 0 0 0;
       1];

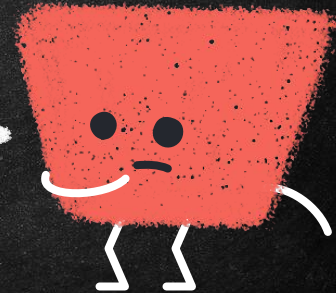
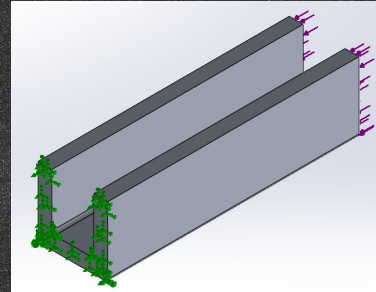
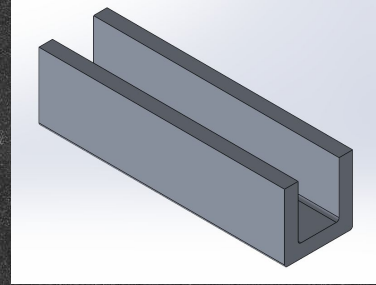
K24 = [0 0 0 0 0 0 0 0;
       0 0 0 0 0 0 0 0;
       0 0 c24^2      (c24 * s24)  0 0 -(c24^2)      -(c24 * s24);
       0 0 c24*s24    s24^2        0 0 -(c24 * s24)    -(s24^2);
       0 0 0 0 0 0 0 0;
       0 0 0 0 0 0 0 0;
       0 0 -(c24^2)   -(c24 * s24)  0 0 c24^2         c24*s24;
       0 0 -(c24*s24) -(s24^2)      0 0 c24*s24        s24^2];

K34 = [0 0 0 0 0 0 0 0;
       0 0 0 0 0 0 0 0;
       0 0 0 0 0 0 0 0;
       0 0 0 0 0 0 0 0;
       0 0 0 c34^2      c34 * s34    -(c34^2)      -(c34 * s34);
       0 0 0 c34*s34    s34^2        -(c34 * s34)    -(s34^2);
       0 0 0 -(c34^2)   -(c34 * s34)  c34^2         c34*s34;
       0 0 0 -(c24*s34) -(s34^2)      c34*s34        s34^2];

%combine the stiffness matrices
K = (K12 + K13 + K24 + K34);
vpa(K, 6);
K = (E * A / L) * K * u;
```


Simulation Setup (Bar Member)

1. Model:
 - a. Simple Bar with consistent cross section
2. FEA Setup:
 - a. Pink Arrows are a Force Applied
 - i. 5668N applied evenly over face
 - b. Green Arrows are Fixed Geometry at the Bottom.



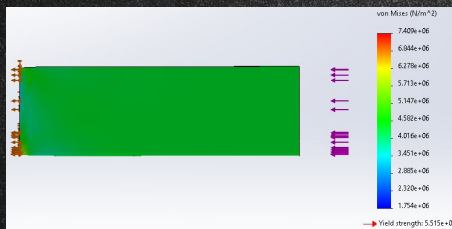
Simulation Validation

Material: 6061 Alloy

$t = 0.01M$

B and $H = 0.05M$

Stress

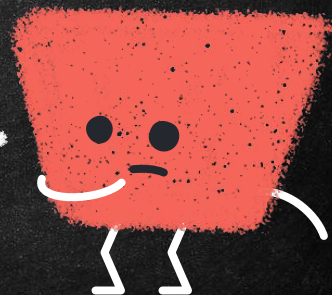


\approx

$$\sigma = 4.3604e+06$$

```
%Yield Criterion
syms sigma sigmatar t B F A SF
sigmatar = 5.515e7
SF = 1.5
B = 0.05
F = 5668.53
t = 0.01
A = 3*B*t - 2*t^2
sigma = F/A
```

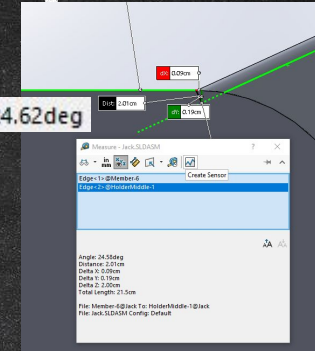
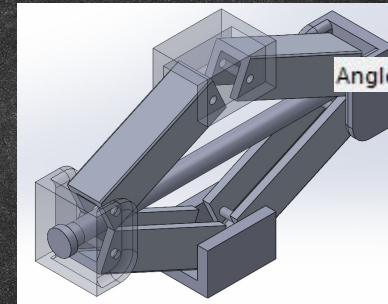
In the right ballpark!



Simulation Setup (Jack Assembly)

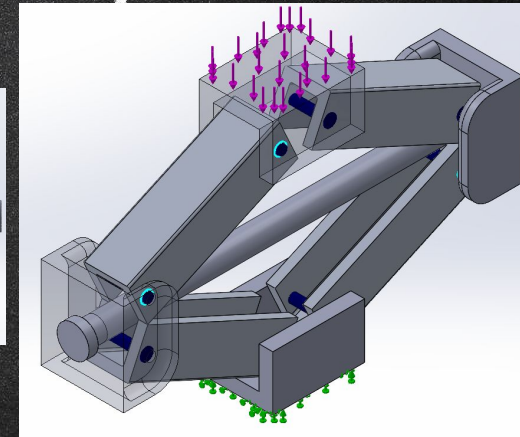
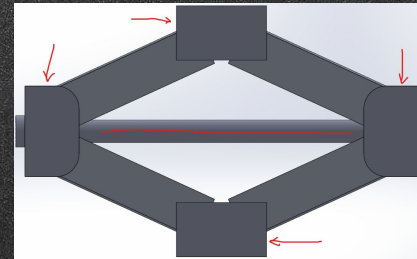
1. Model:

- Cross sections of bars
- Length of bars
- Angles match Truss Analysis



2. FEA Setup:

- Red marked components were made rigid
- Blue Cylinders are Pin Connections
- Pink Arrows are a Force Applied (4733N)
- Green Arrows are Fixed Geometry at the Bottom.



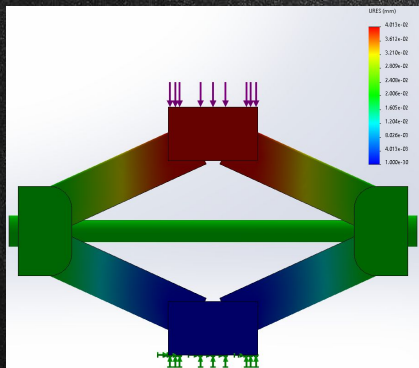
Simulation Validation

Material: 6061 Alloy

$t = 0.01M$

B and H = $0.05M$

Displacement



URES (mm)

4.013e-02

\approx

dtopymeters =

-0.000054609

Within 1/100 mm!

```
K13 = [c13^2 c13 * s13 0 0 -(c13^2) -(c13 * s13) 0 0;
c13*s13 s13^2 0 0 -(c13 * s13) -(s13^2) 0 0;
0 0 0 0 0 0 0 0;
0 0 0 0 0 0 0 0;
-(c13^2) -(c13 * s13) 0 0 c13^2 c13*s13 0 0;
-(c13*s13) -(s13^2) 0 0 c13*s13 s13^2 0 0;
0 0 0 0 0 0 0 0;
0 0 0 0 0 0 0 0;
1];

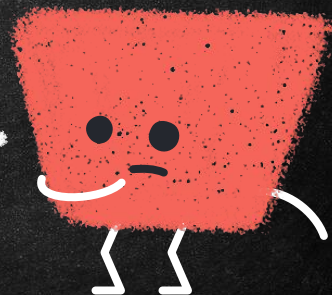
K24 = [0 0 0 0 0 0 0 0;
0 0 0 0 0 0 0 0;
0 0 c24^2 c24 * s24 0 0 -(c24^2) -(c24 * s24);
0 0 c24*s24 s24^2 0 0 -(c24 * s24) -(s24^2);
0 0 0 0 0 0 0 0;
0 0 0 0 0 0 0 0;
0 0 -(c24^2) -(c24 * s24) 0 0 c24^2 c24*s24;
0 0 -(c24*s24) -(s24^2) 0 0 c24*s24 s24^2];

K34 = [0 0 0 0 0 0 0 0;
0 0 0 0 0 0 0 0;
0 0 0 0 0 0 0 0;
0 0 0 0 0 0 0 0;
0 0 0 c34^2 c34 * s34 -(c34^2) -(c34 * s34);
0 0 0 c34*s34 s34^2 -(c34 * s34) -(s34^2);
0 0 0 -(c34^2) -(c34 * s34) c34^2 c34*s34;
0 0 0 -(c34*s34) -(s34^2) c34*s34 s34^2];

%combine the stiffness matrices
K = (K12 + K13 + K24 + K34);
vpa(K, 6);
K = (E * A / L) * K * u;
vpa(K, 6);

%set the matrix equal to boundary conditions and solve
for i = 1:n
    s(i, 1) = K(i) == forces(i);
end
vpa(s, 6);

sol = solve(s, [fx1 fx3 fy3 fx4 uy1 ux2 uy2 uy3]);
figure % vpa(sol, fx1, 3)
```



FAILURE CRITERIA

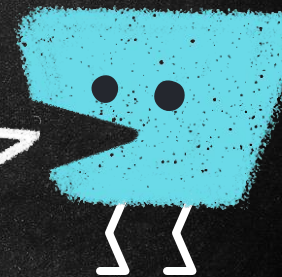
Yield Failure Criterion

- Stress values from structural analysis
- Found material failure stress though yield stress
- Used 25 as Safety factor

→ Plugged into and solved for t

$$\Phi_{\sigma} \sigma \leq \sigma^*$$

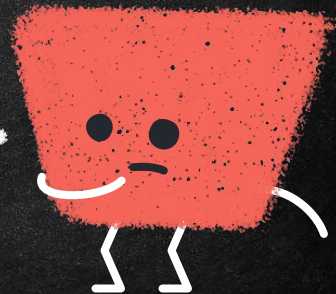
**We created Matlab code to repeat calculations



MATLAB CODE

```
clear
clc
%Yield Criterion
syms sigma sigmastar t B F A SF
sigmastar = 5.515e7
SF = 1.5
B = 0.05
F = 5668.53
t = 0.01
A = 3*B*t - 2*t^2
sigma = F/A

eqn = SF*sigma == sigmastar
S = vpa(solve(eqn,t))
```



FAILURE CRITERIA

Buckling Failure Criterion

- Stress values from structural analysis
- Used 25 as Safety factor
- Found Critical Stress through:

$$\sigma_{cr} = \pi^2 EI / (KL)^2$$

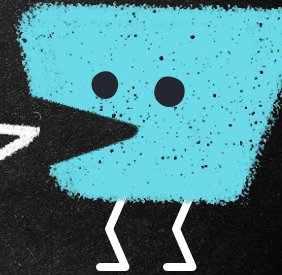
Inertia

$$I = -2t^4 + 35t^3 - 150t^2 + 250t$$

- Plugged into and solved for t

$$\Phi_b \sigma \leq \sigma_{cr}$$

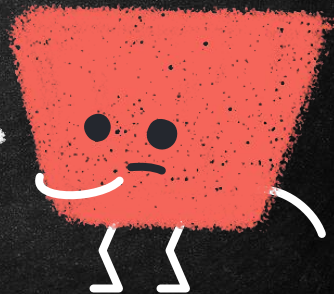
**We created Matlab code to repeat calculations



MATLAB CODE

```
clear
clc
%Buckling Criterion
syms L t SF B F E I K A Sigma Sigmacr
SF = 25;
B = 0.05;
F = 5668.53;
E = 69e9;
K = 1;
L = .18;
I = (B*(t^3)+2*t*((B-t)^3))/3;
A = 3*B*t - 2*t^2;
Pcrit = ((pi^2)*E*I/((K*L)^2));
Sigma = F/A;
Sigmacr = Pcrit/A

eqn = SF*Sigma == Sigmacr
solutions = vpa(solve(eqn,t))
```

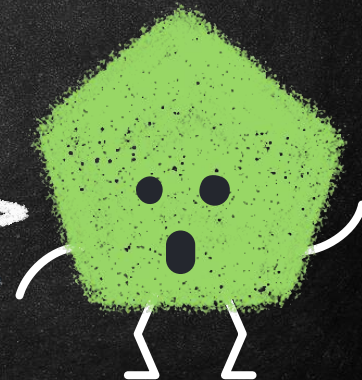


MINIMUM MASS DESIGN

- Equation used : $m = \rho \times L \times A$
- Used minimum thickness found through the failure criteria
- Used three different materials , so found three different densities
- Created a Matlab script to do the calculations for the minimum mass.
- Repeated process for the rest of the minimum thickness

```
clear
clc
% Minimum Mass
syms Density L A B t m_i;
Density = 2810;
L = 0.18;
t = 0.00051988894197323006737573663303929;
B = 0.05;
A = 3*B*t - 2*t^2;
m_i = Density*L*A;

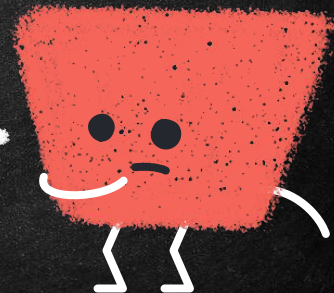
TotalMinimumMass= 4*m_i
```



RESULTS

- Disregarded the full-extension minimum mass because it will be higher
- The minimum masses shown resulted by using the minimum thickness from the yielding criterion

Half-Extension:			From yielding criterion:		
Material	Modulus of Elasticity(Pa)	Density (kg/m ³)	Force(N)	Minimum t (m)	Minimum mass (kg)
6061	69×10^9	2700	1558.15	0.0009530218442230 08596720720973442 93	0.274369891304348
304	190×10^9	8000	1558.15	0.0005076923503991 38061047910370346 44	0.435676893203884
7075	71.7×10^9	2810	1558.15	0.0005198894197323 00673757366330392 9	0.156682359840954

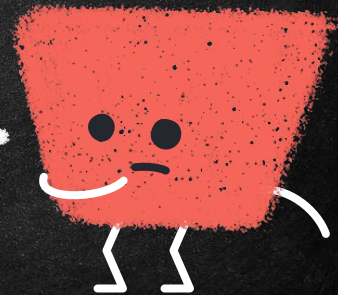


RESULTS

Half-Extension: From Buckling Failure Criterion

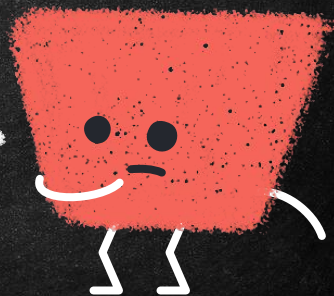
Material	Modulus of Elasticity(Pa)	Density (kg/m ³)	Force(N)	Minimum t (m)	Minimum mass (kg)
6061	69×10^9	2700	1558.15	0.0000222693256266 05371937165562377 606	0.006491807204623
304	190×10^9	8000	1558.15	0.0000080803979590 71838126363314788 0078	0.006980711663223
7075	71.7×10^9	2810	1558.15	0.0000214296517978 55527177339795901 136	0.006501612499438

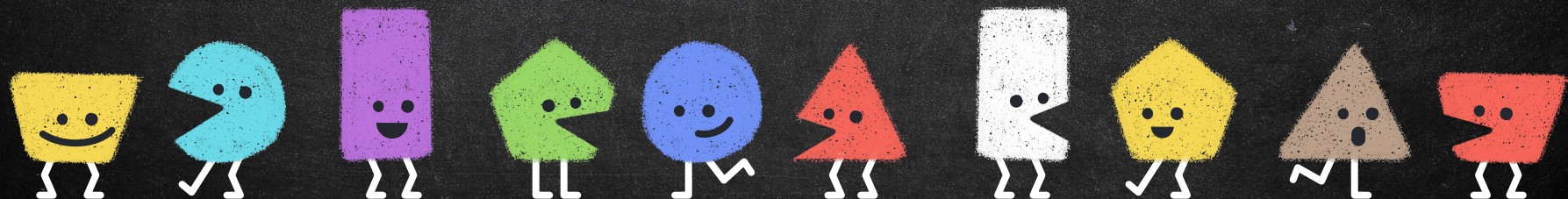
- The above minimum mass resulted from using the minimum thickness from the buckling failure criterion
- We found that the material 6061 Aluminum gives us our overall minimum mass of 6.491×10^{-3} kg.



FEA

- Waiting on minimum thicknesses from Failure Criteria, but models and simulations have been validated.

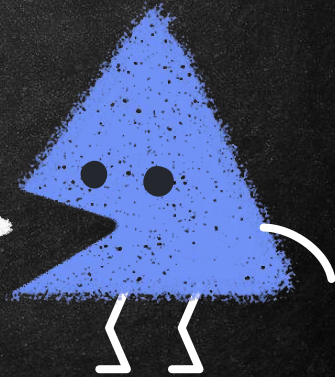




Issues Solved

1. Everyone redid the truss analysis and stress calculations independently
 - a. Still getting nonsense answers for t
2. Stress calculations were validated by hand and FEA.
3. Eventually realized that all our work was correct
 - a. Issues arose after applying Failure Criteria
 - i. Thus our Safety Factor was too Low

Increased Safety Factor from 1.5 to 25
(to keep the humans safe)



Issues Encountered

1. Initially, Safety Factor was set to 1.5

```
%Yield Criterion
syms sigma sigmastar t B F A SF
sigmastar = 5.515e7
SF = 1.5
B = 0.05
F = 5668.53
A = 3*B*t - 2*t^2
sigma = F/A

eqn = SF*sigma == sigmastar
S = vpa(solve(eqn,t))
```

```
S =
0.00104232449326359254485990031994
0.07395767550673640745514009968006
```

2. Caused a lot of issues: Numbers produced were not analyzable on FEA or simply impossible.
3. The truss analysis was confirmed with the simulation

So why aren't we getting realistic values for thickness???

