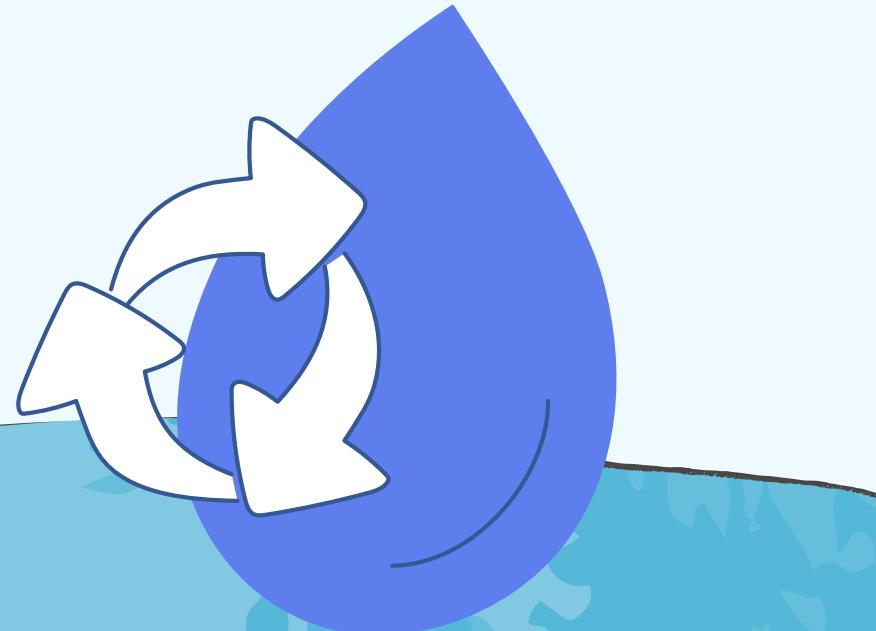
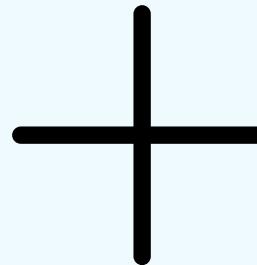
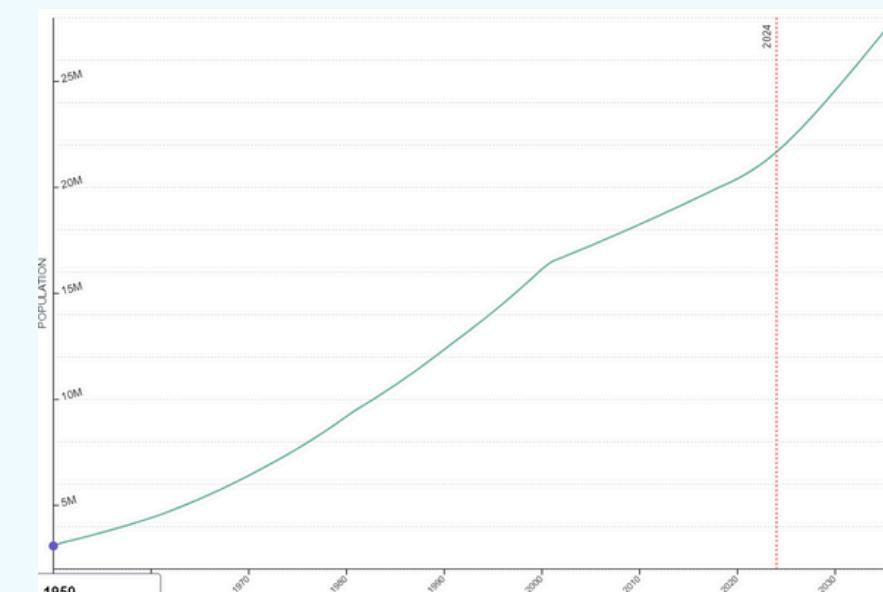


ILP MODELS FOR WATER DISTRIBUTION SYSTEM OPTIMISATION

A PRESENTATION BY
SPARSH AMARNANI



Motivation



Thane Water Crisis: City Faces Water Shortage as Main Pipeline Undergoes Emergency Repairs

By Nirmeeti Patole | Published: July 10, 2024 02:35 PM



The Thane Municipal Corporation (TMC) area is experiencing a perfect storm of water supply issues, as a newly detected ...

Water cut announced for Mumbai

Richa Pinto / TNN / May 25, 2024, 16:40 IST



Mumbai faces water shortage with BMC announcing 5% and 10% water cuts. Citizens urged to conserve water until sufficient rainfall improves reservoir levels. Indian Meteorological Department forecasts timely monsoon arrival. Water cuts also apply to Thane, Bhiwandi-Nizampur Municipal Corporation. BMC provides list of water-

**Tanker economy revealed:
Rajasthan's groundwater level takes
a dip as water mafias emerge**
Water shortages in Rajasthan need no introduction; but the extent of the crisis covers a lot more than what meets the eye

Rapid Urbanisation

Variable Water Supply

Water problems



WHAT IS WDS OPTIMISATION ?

DECISION VARIABLE

Hydralic Components that we can choose like ,pipes, pumps and valves

OBJECTIVE FUNTION

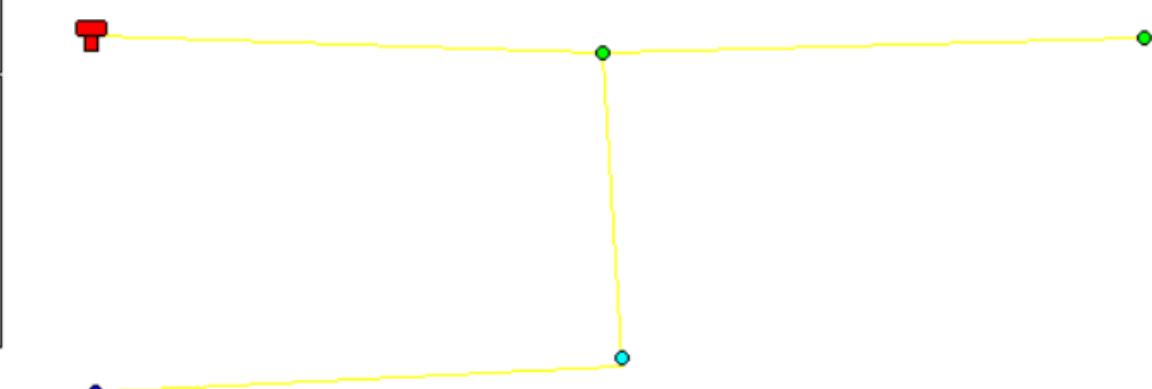
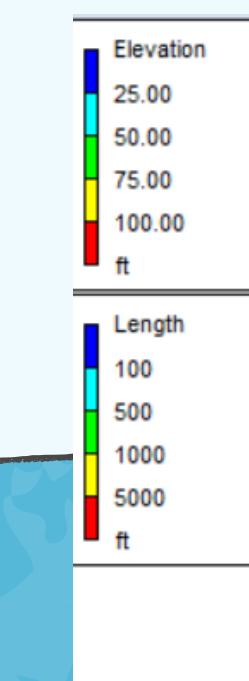
Variables that are dependent on our choices that we want to minimize or maximize, like cost, water quality or greenhouse gas emmisions.

CONSTRAINTS

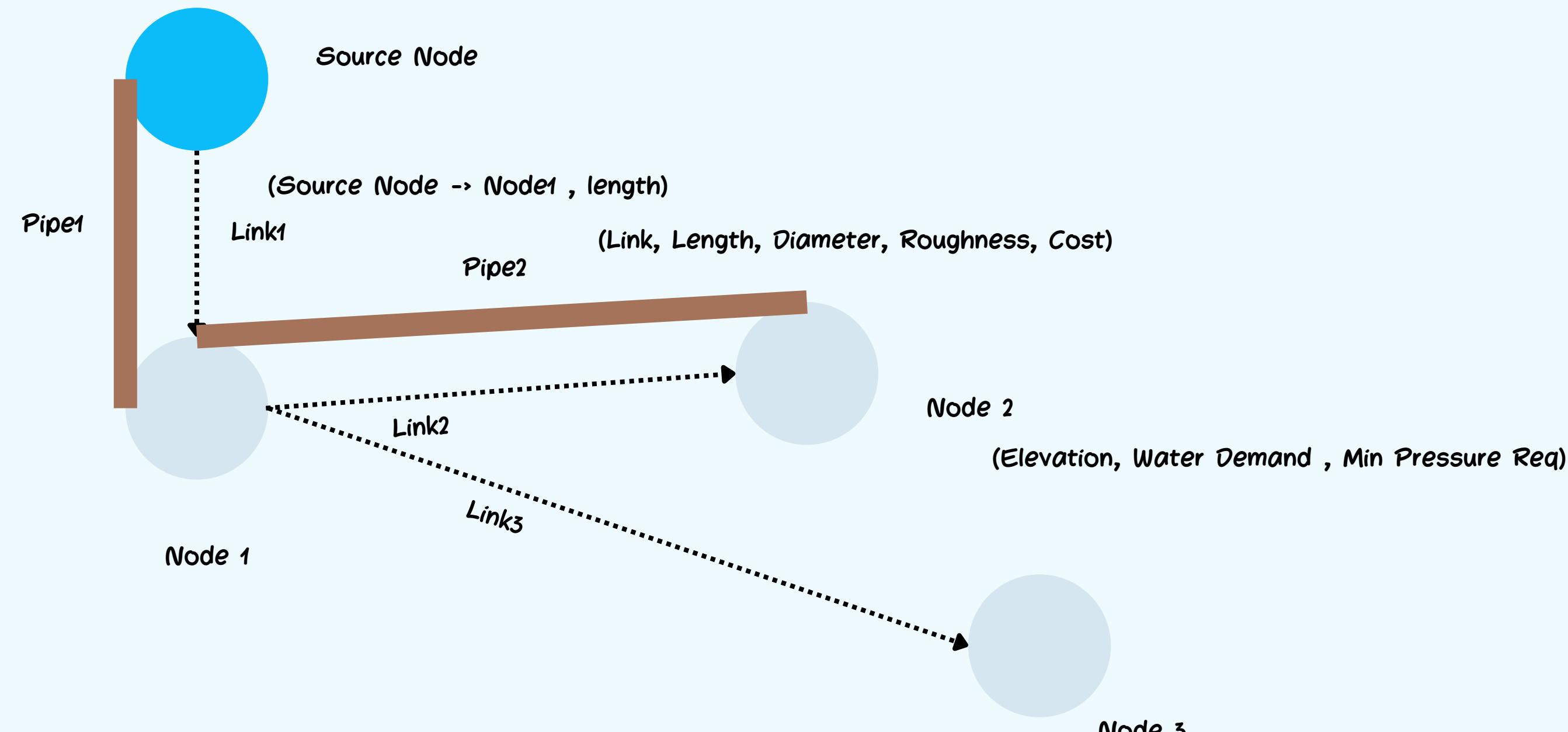
Hydraulic constraints are given by physical laws governing the fluid flow in a pipe network.

System Constraints like water demands and water quality requirement

THE GOAL IN DESIGNING A SOFTWARE WHICH CAN OPTIMISE THIS IS THAT WE CAN UPDATE OUR WATER NETWORKS WITH INCREASING WATER DEMANDS OR CHANGE IN WATER SUPPLY



Inputs of the Optimisation Model



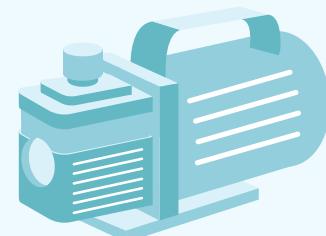
Base Cost, Unit Cost, Upper Limit Capacity, Lower Limit Capacity

Tank 1

Tank 2

Tank 3

Pump Cost, Pump Life Cycle, Minimum Pump Size, Efficiency



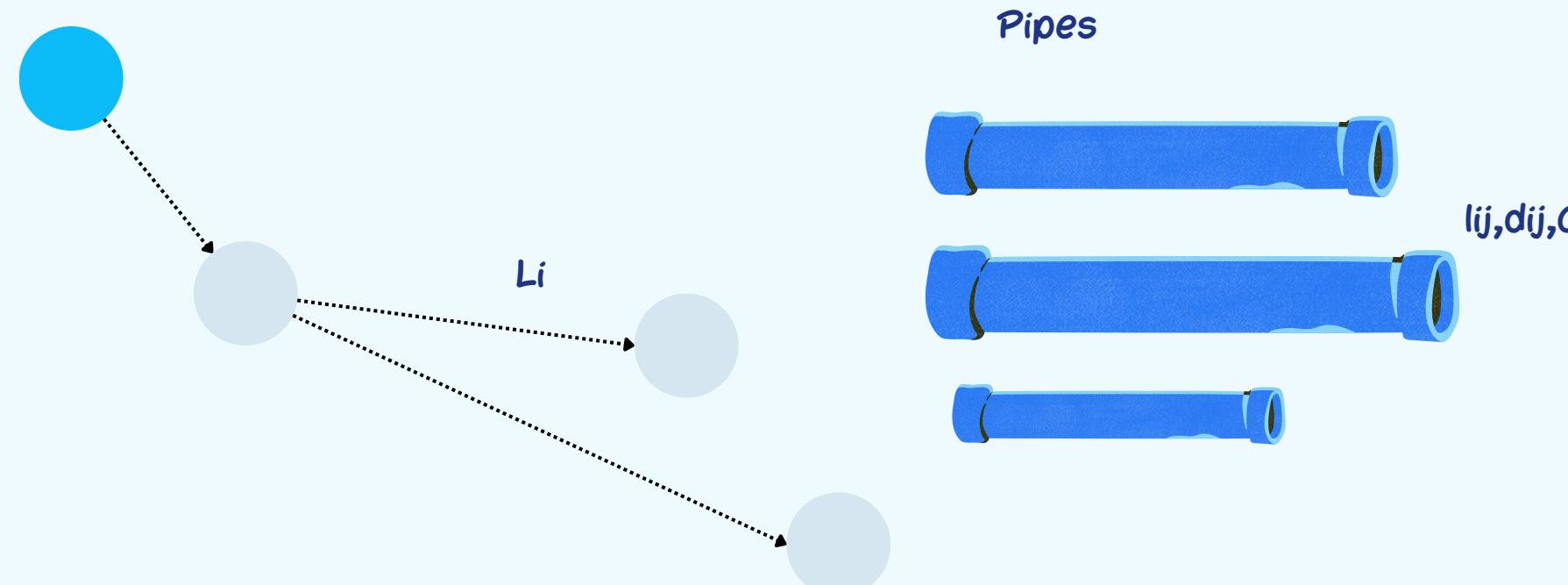
Pump 1



Pump 1

Other tank inputs : Maximum, Min Tank Height allowed,

PIPE ONLY MODEL OPTIMISATION



OBJECTIVE FUNCTION

minimize $L : C(D_{i,j}) * L_{ij}$ sum
over i and j

Length of Pipes

$$\begin{aligned} \text{lij sum over } j &= L_i \\ \text{lij} &\geq 0 \end{aligned}$$

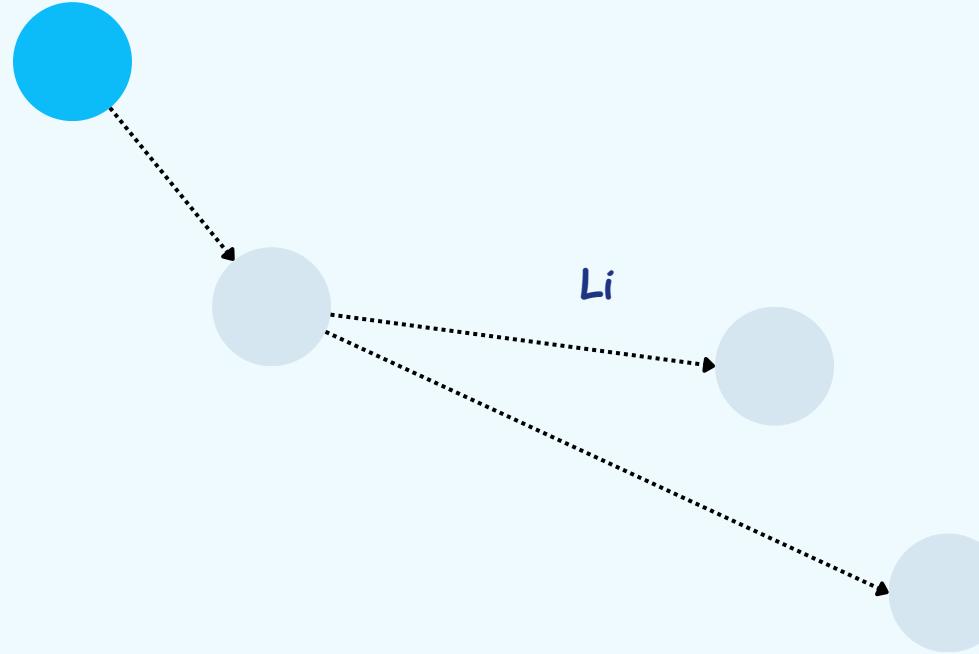
CONSTRAINTS

Pressure requirement
at each node

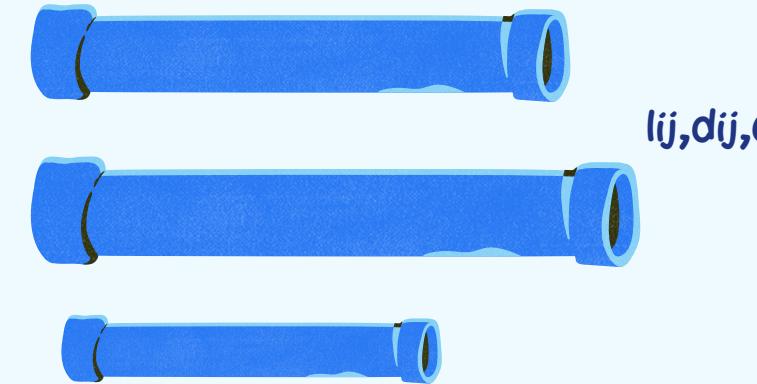
$$P_n \leq H_r - E_n - (H L_{ij} * L_i) \text{ sum over } j \text{ and } i \text{ which are parent links}$$

OUR GOAL IS TO
PUT PIPES IN EACH
LINK WHICH WILL
MINIMIZE THE COST
OF THE NETWORK

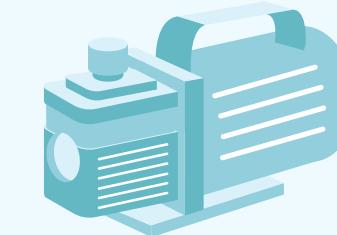
PIPES AND PUMP MODEL OPTIMISATION



Pipes



Pump



What power pump should be applied at which link i

Efficiency of a pump, Max and minimum power allowed by a pump, Capital and Operational Cost of a Pump

OBJECTIVE FUNCTION

minimize L, p : Pipe Cost +
 $(C_p * p_i) + (EF * DF * SH * p_i)$ for all i

Pressure on Link

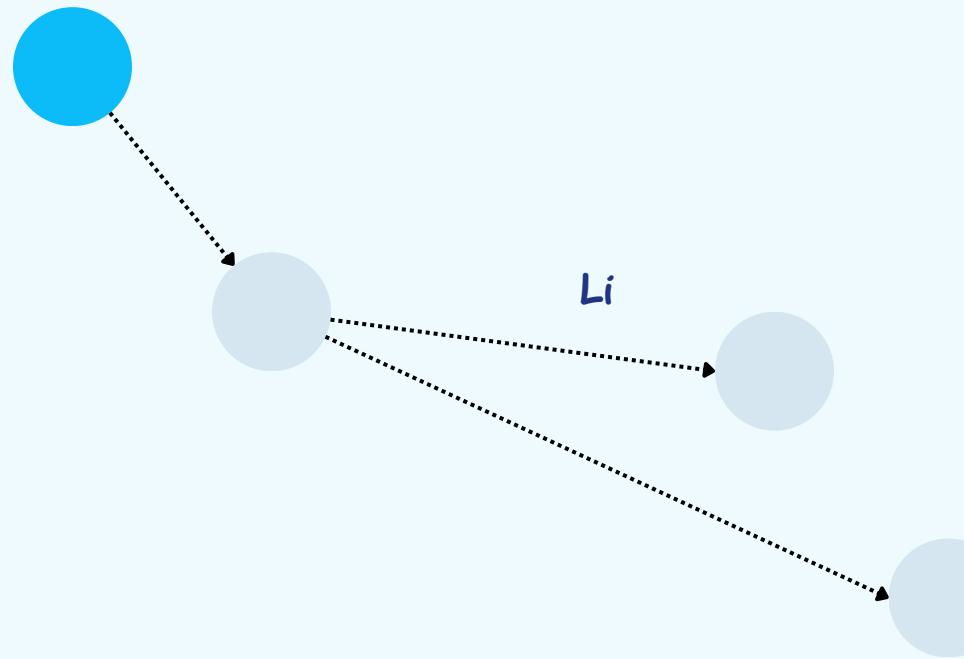
$$\phi_i = \frac{(eff * p_i)}{wtden} * g * F_{Li}$$

CONSTRAINTS

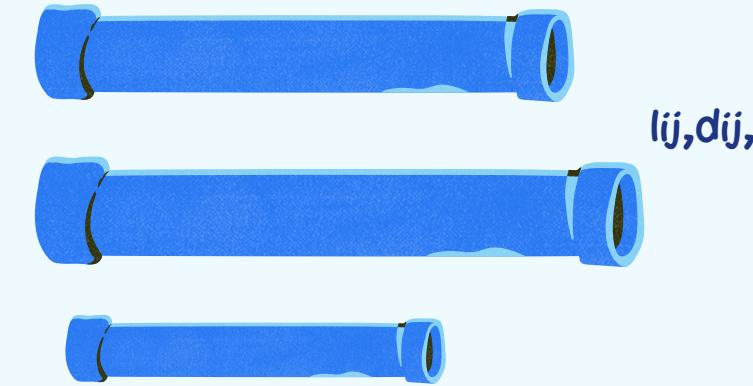
Pressure requirement at each node

$$P_n \leq H_r - E_n - (H L_{ij} * L_i) + \phi_i \text{ sum over } j \text{ and } i \text{ which are parent links}$$

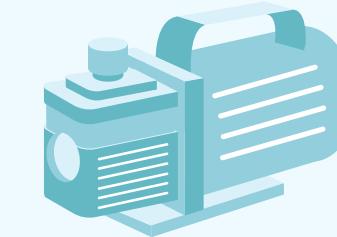
PIPES AND PUMP MODEL OPTIMISATION



Pipes



Pump



What power pump should be applied at which link i

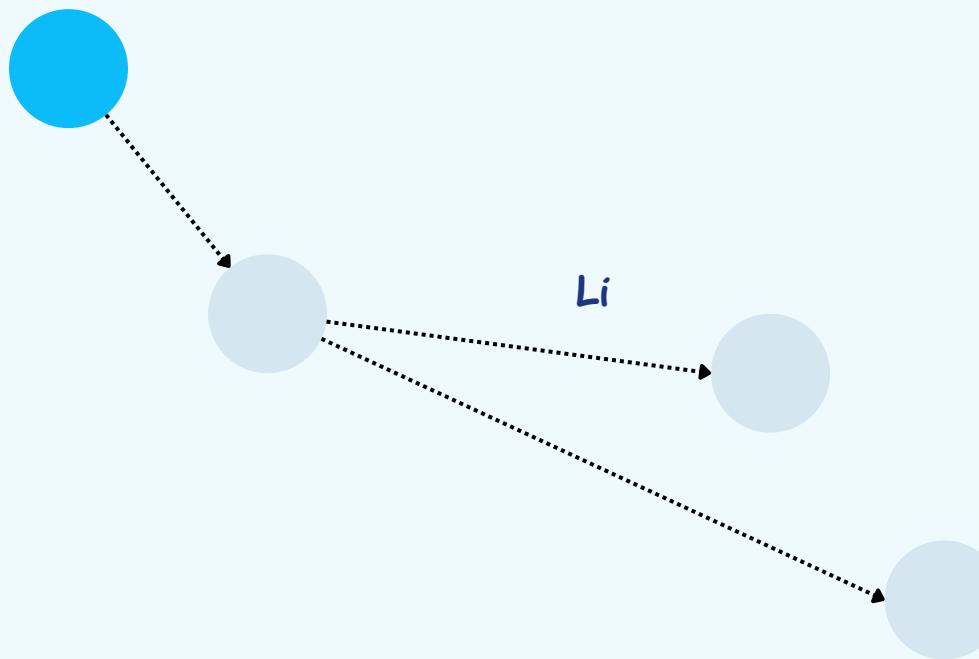
Efficiency of a pump, Max and minimum power allowed by a pump, Capital and Operational Cost of a Pump

CONSTRAINTS CONTINUED

Power Constraints

$$PP_{min} \leq p_i \leq PP_{max}$$

FINAL MODEL INPUTS

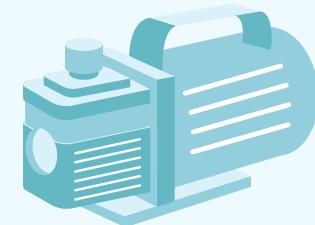


Pipes



l_{ij}, d_{ij}, C

Pump



Efficiency of a pump, Max and minimum power allowed by a pump, Capital and Operational Cost of a Pump

Tank Cost Table

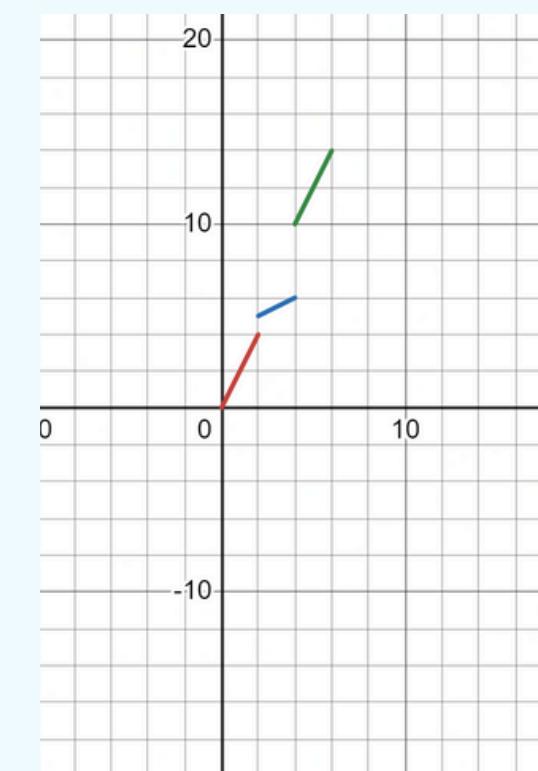
Base Cost, Unit Cost, Upper Limit Capacity, Lower Limit Capacity

Tank 1

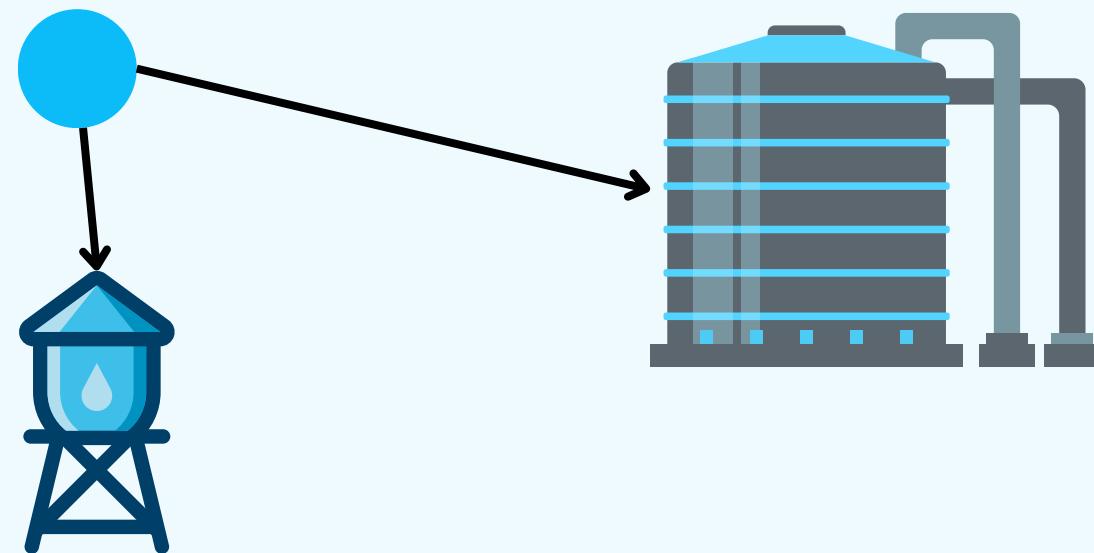
Tank 2

Tank 3

Other tank inputs : Maximum, Min Tank Height allowed,

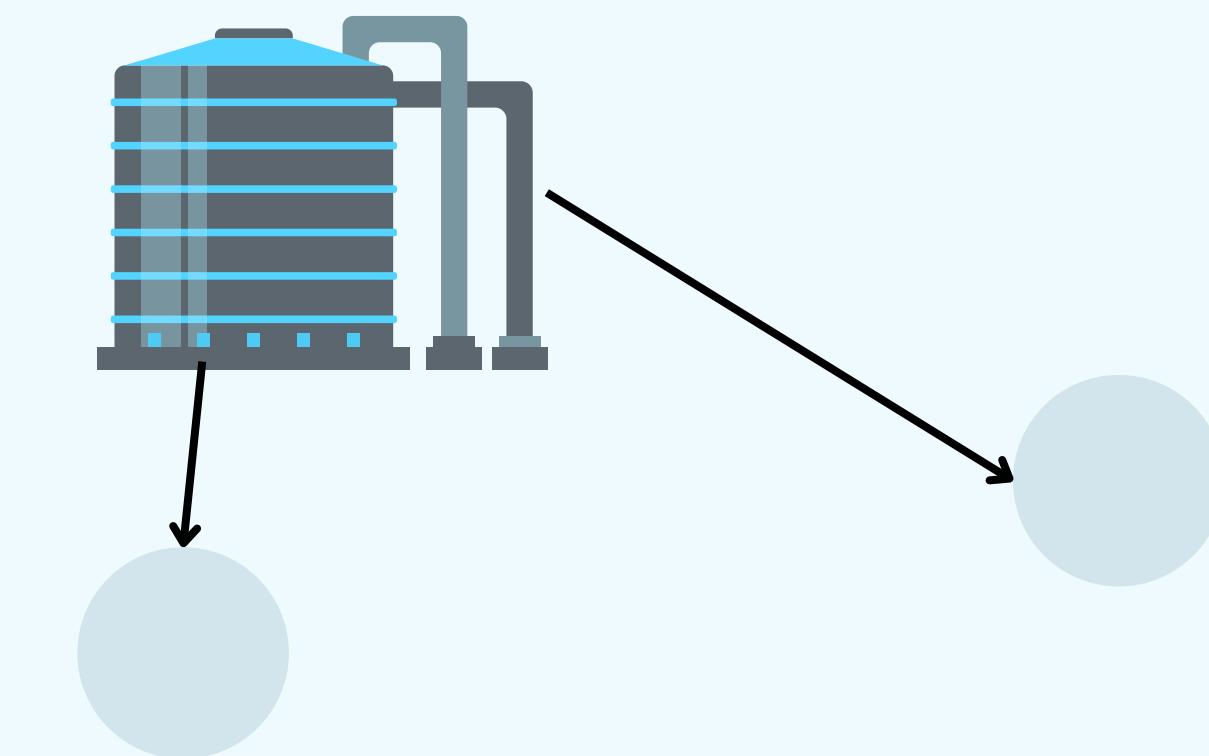


PRIMARY



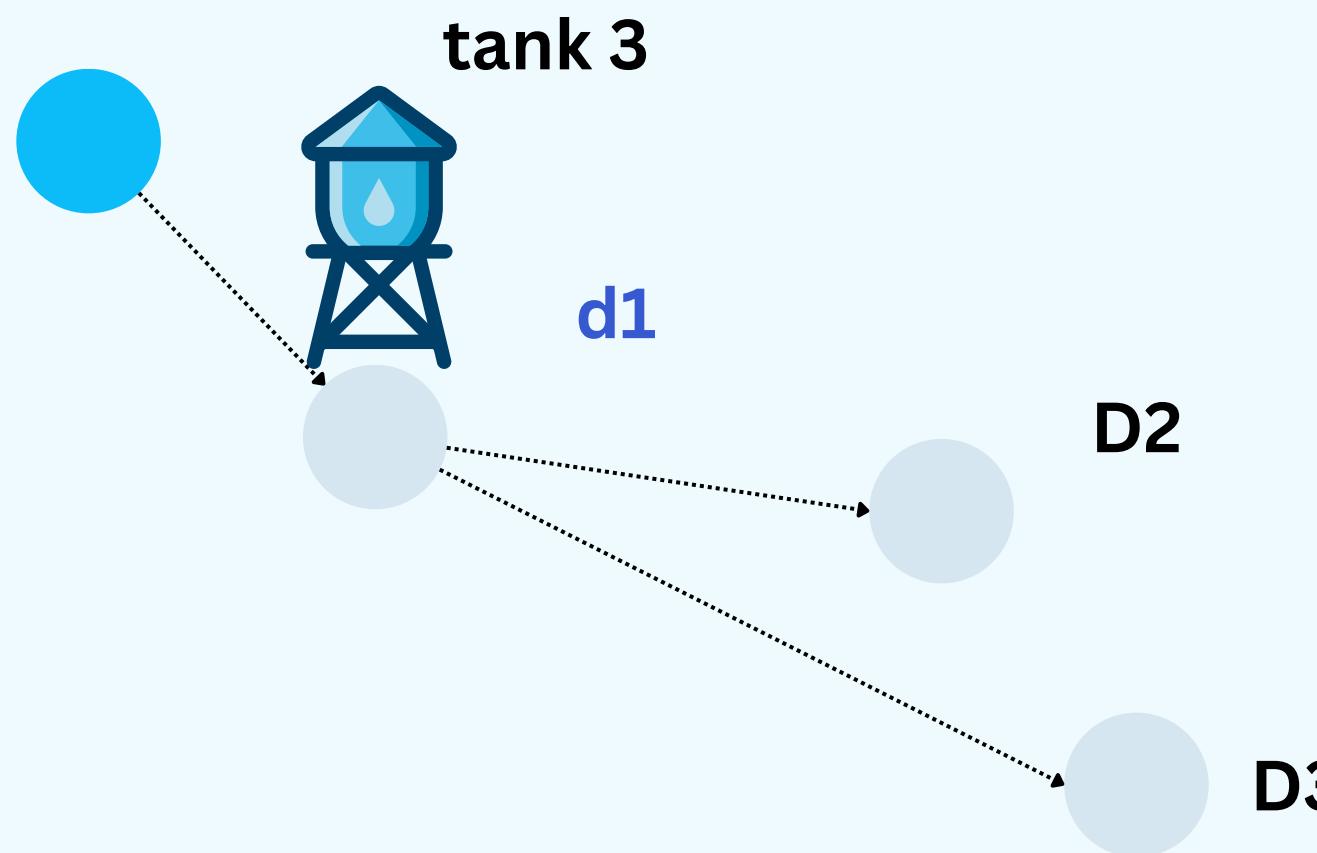
The primary network is the main system of pipes and infrastructure that transports water from the source (such as a water treatment plant or reservoir) to various storage facilities

The secondary network is the subsystem that distributes water from the primary network (or storage tanks) to individual demand nodes, such as households, businesses, and public facilities.



SECONDARY

OBJECTIVE FUNCTION WITH TANK



Head on link if tank is its source

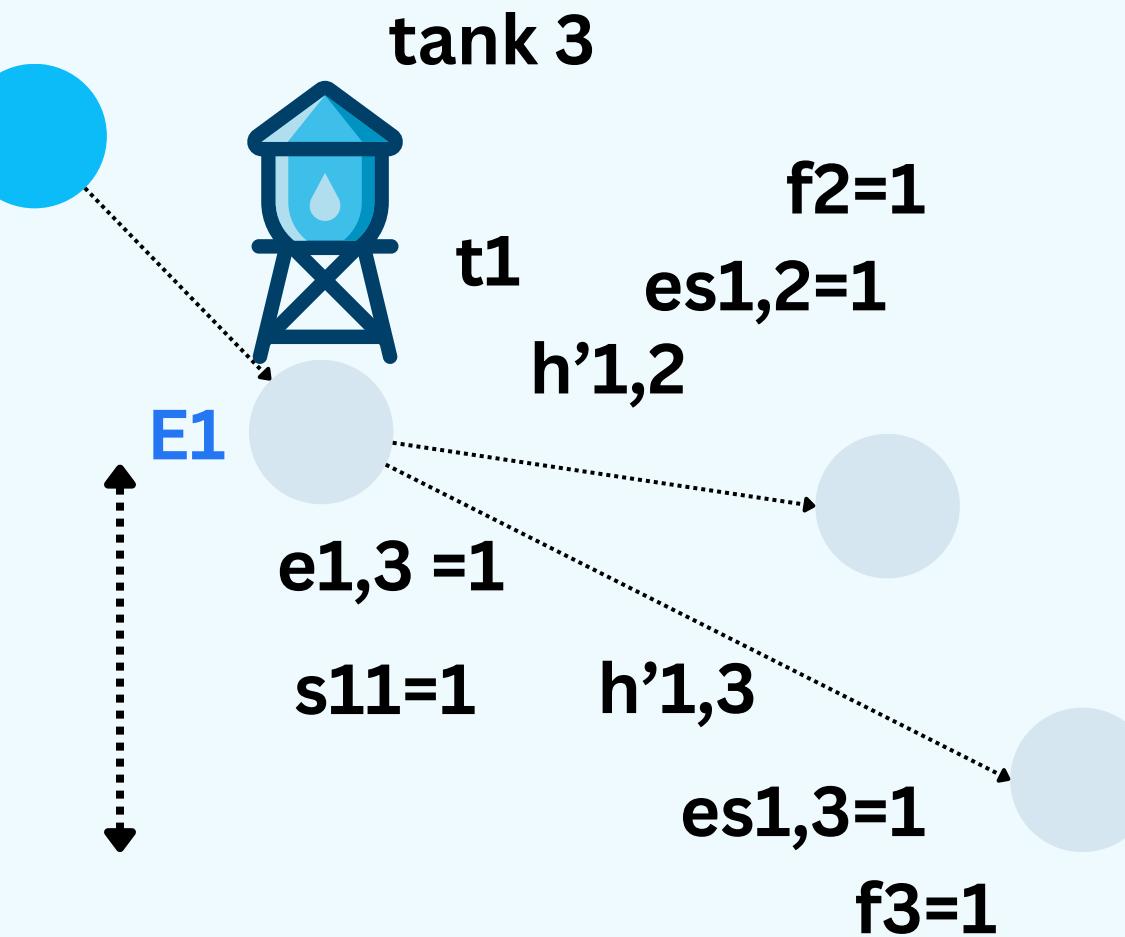
$$d_1 = D_2 + D_3$$

$z_{nk} = d_n * e_{nk}$ (linearisation of
the)

$\min l, p$ Cost of pipes + Cost of Pumps +
 $e_{nk} * (\beta_k + \text{UnitCk} * (d_n - L_{Ok}))$ over all
nodes (n) of the network and rows of
the tank table (k)

PRESSURE CONSTRAINT WITH TANK

Head on link if tank is its source



$$h'^{ni} = (t_n + E_n) * es_n + h_n * (1 - es_n)$$

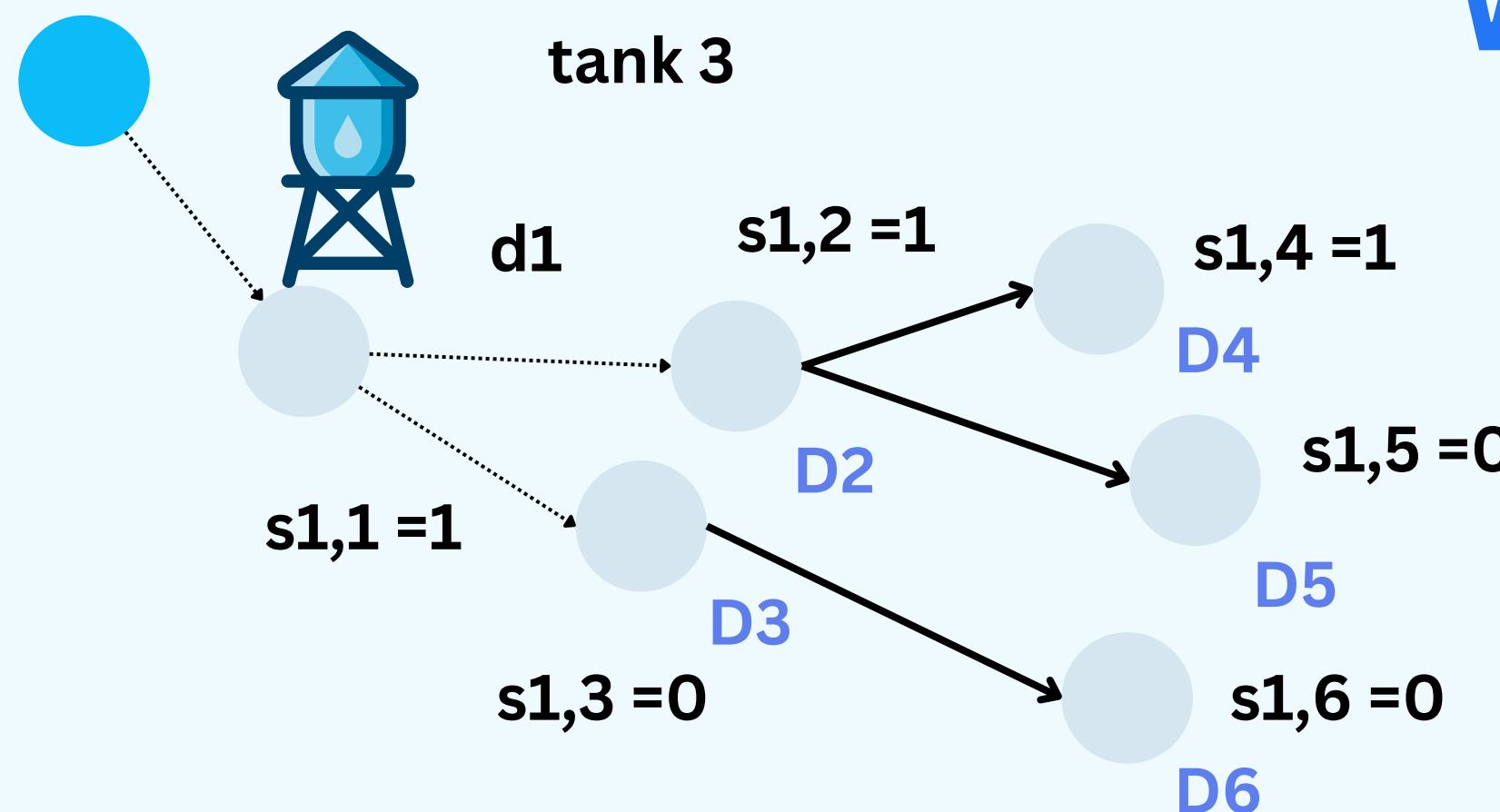
$$h'^{1,2} = t_1 + E_1$$

$$es_{ni} = s_{nn} * (1 - f_i)$$

$$p_n \leq h'^{mi} - h^{li}$$

If the starting node is a source node
then the head loss equation changes

CONSTRAINTS ON TANK AND THEIR DESCENDANTS



Where m is a descendant of n

$$smm \leq snn$$

$$snm \leq snn$$

sum over all Ancestors of m $snm = 1$

$$dn = snm * DEM$$

$snm \leq 1 - soo$ for all o in path of n,m

Nodes can serve their descendants if they serve their own

OTHER CONSTRAINTS

Only 1 tank per row:

sum over k enk =1

Capacity-demand Constraint:

$$dn \geq LOk * enk$$

$$UPk + DE(1-enk) \geq dn$$

EFFECT ON OTHER CONSTRAINTS

Splitting headloss calculation:

$hli = \text{sum over all pipe segments}$

$$(HLPij * lpij + HLSij * [lij - lpij])$$

Primary and Secondary Flows:

$$FLPi / FLSi = SH / PH$$

Primary and Secondary Pumps:

$$pi = ppi + psi$$

HOW IS OUR MODEL IMPROVED

$\min x \mathbf{C} \cdot \mathbf{x}$
 $A\mathbf{x} \leq \mathbf{B}$
 $\mathbf{x} \geq 0$
X is Integer

$\min x \mathbf{C} \cdot \mathbf{x}$
 $A\mathbf{x} \leq \mathbf{B}$
 $\mathbf{x} \geq 0$
X is Rational

SOLVING A LINEAR
INTEGER PROGRAM
INVOLVES LINEAR
RELATION

ILP' ← ILP

New Relaxation'

Old Relaxation

IF NEW RELAXATION IS A STRICT
SUBSET OF THE OLD RELAXATION
THEN IT'S A SMALLER SOLUTION
SPACE FOR THE SAME PROBLEM

HEADLOSS IMPROVEMENT

Old model

$$lijp = lij * fi$$

↓
Linearize

$$lijp \geq 0$$

$$lijp \leq Li * fi$$

$$lij - Li * (1 - fi) \leq lij p_j$$

$$lijp \leq lij$$

New model

We introduce $lijs$ which represents length of link for the secondary network

$$lijp + lijs = lij$$

sum over a link $lijp = Li * fi$

sum over a link $lijs = Li * (1 - fi)$

HEADLOSS IMPROVEMENT STRICT SUBSET PROVE

Consider
the point :

Link Length = L



Primary pipe segment
 $=[L/2, L/2]$, $f = 0.5$



Secondary pipe
segment $=[0, 0]$

Link/Pipe Length = $[L/2, L/2]$

Old model
Satisfied

$$lijp = lij * fi$$



$$L/2 = L * 0.5$$

$$lijp + lijs = lij$$

$$L/2 = L/2$$

$$\text{sum over a link } ljp = L * fi$$

$$L/2 = L/2$$

$$\text{sum over a link } lijs = Li * (1 - fi)$$

$$0 = L/2$$

New model Unsatisfied

TANK COST IMPROVEMENT

Old model

$$znk = enk * dn \quad \text{eq0}$$

↓
Linearize

$$znk \geq 0$$

$$znk \leq DE * enk$$

$$znk - DE * (1 - enk) \leq dn$$

$$dn \leq znk$$

znk is the water demand of
tank k at node n

DE is total wated demand of the
network

New model

$$LOk * enk \leq znk \quad \text{eq1}$$

$$znk \leq UPk * enk \quad \text{eq2}$$

$$\text{eq3} \quad \text{sum for all row for } n \text{ enk} = 1$$

$$\text{eq4} \quad \text{sum for all row for } n \text{ znk} = dn$$

lets say for node n we have 4 tanks

applying eq1&2 to enk

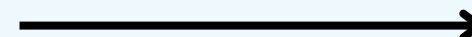
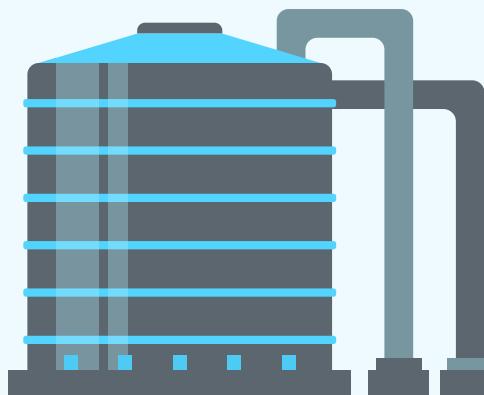
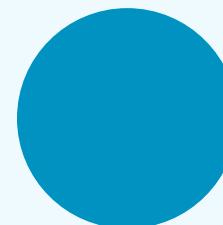
applying eq4 to znk

enk : 0 1 0 0

znk : 0 zn2 0 0

zn2=dn

TANK COST IMPROVEMENT : STRICT SUBSET PROOF



$$\begin{aligned} z_{1k} &: [d, d] \\ e_{1k} &: [1/2, 1/2] \\ d_1 &= d \end{aligned}$$

$$\begin{aligned} L_{0k} &: [0, d] \\ U_{P_k} &: [d, 2d] \\ DE &= 2d \end{aligned}$$



Old model is satisfied

$$z_{nk} \geq 0$$

$$z_{nk} \leq DE * e_{nk}$$

$$z_{nk} - DE * (1 - e_{nk}) \leq d_n$$

$$d_n \leq z_{nk}$$

New model is violated

eq1 $L_{0k} * e_{nk} \leq z_{nk}$

eq2 $z_{nk} \leq U_{P_k} * e_{nk}$

eq3 sum for all row for n $e_{nk} = 1$

eq4 sum for all row for n $z_{nk} = d_n$

TANK COST IMPROVEMENT : BEST RELAXATION PROOF

$z_{1k} : [1,1]$
 $e_{1k} : [1/2, 1/2]$
 $d_1 = 1$

P1

$t = 1/2$
 $n' = 1$
 $k' = 1$

$z_{1k} : [0,2]$
 $e_{1k} : [0,1]$
 $d_1 = 0$

P2

$LO_k : [2,0]$
 $UP_k : [1,0]$
 $DE = 2$

P3

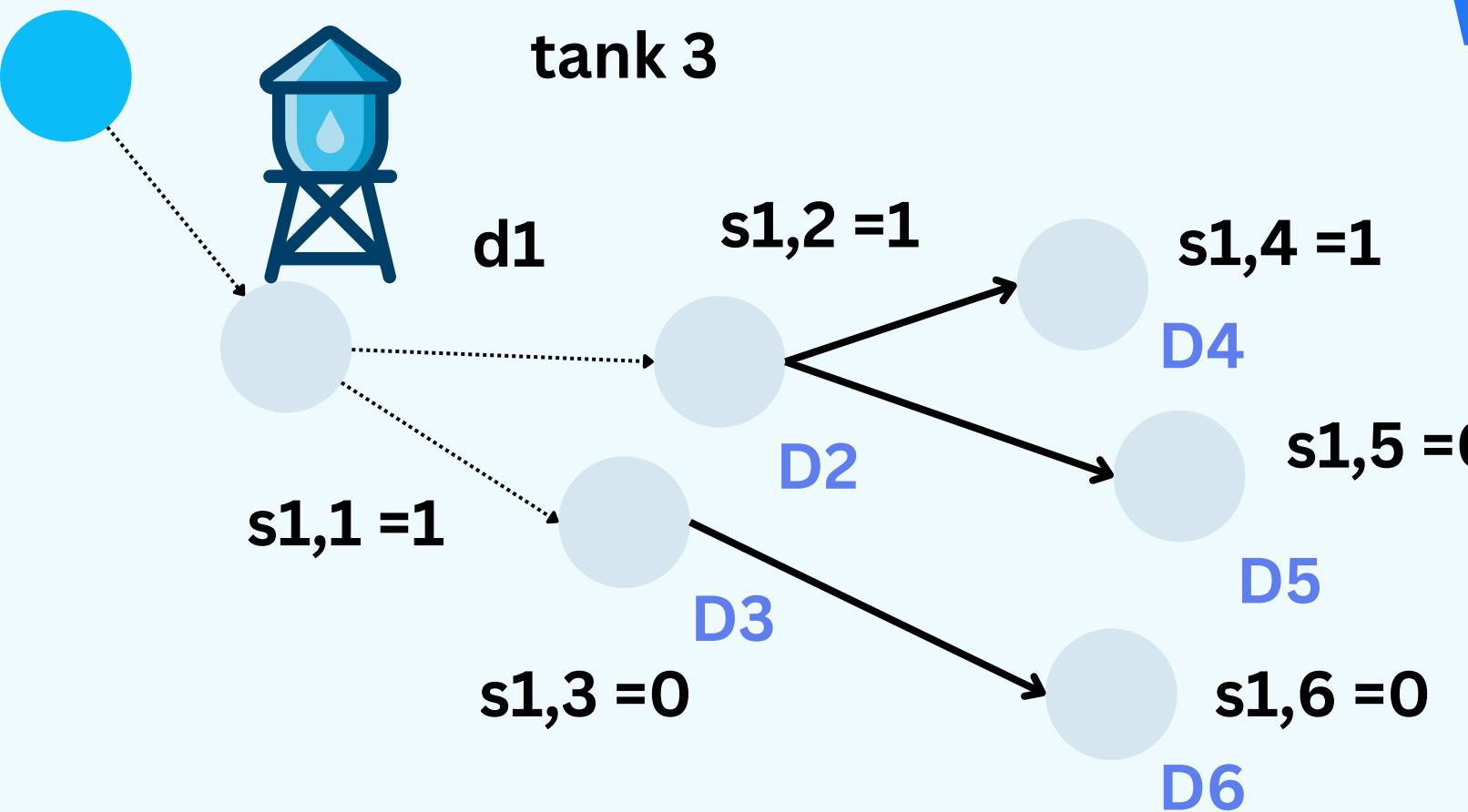
New model is Satisfied

$$P_1 = P_2/2 + P_3/2$$

- eq1 $LO_k * e_{nk} \leq z_{nk}$
eq2 $z_{nk} \leq UP_k * e_{nk}$
eq3 sum for all row for n $e_{nk} = 1$
eq4 sum for all row for n $z_{nk} = d_n$

Since P can be represented as a linear combination of two other points belonging to R_2 , P cannot be a corner point of R_2 .

TANK CONFIGURATION IMPROVEMENT : INITIAL MODEL



Where m is a descendant of n

$$smm \leq snn$$

$$snm \leq snn$$

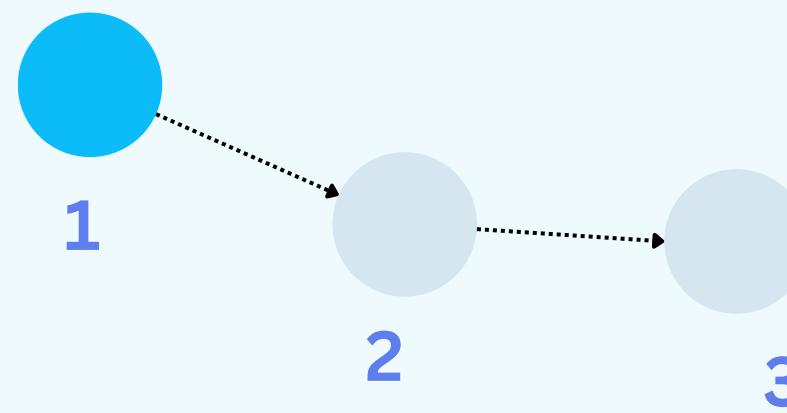
sum over all Ancestors of m $snm = 1$

$$dn = snm * DEM$$

$snm \leq 1 - soo$ for all o in path of n,m

Nodes can serve their descendants if they serve their own

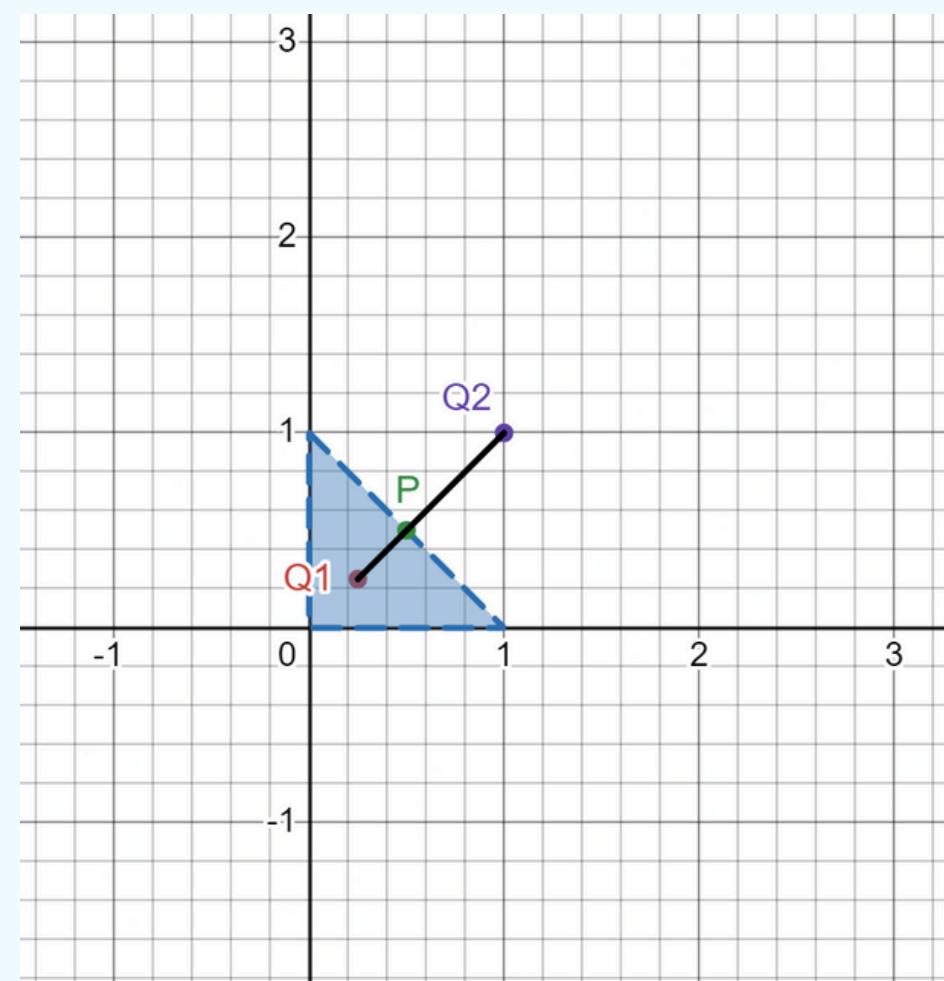
TANK CONFIGURATION IMPROVEMENT : TIGHTNESS PROOF



$$P : \{1, 0.5, 0, 0.5, 0.5, 0.5\}$$

$$Q_1 : \{s_{11}, s_{12}, s_{13}, s_{22}, s_{23}, s_{33}\}$$

$$Q_2 : \{s_{11}, s_{12}, s_{13}, s_{22}, s_{23}, s_{33}\}$$



$$P = Q_1 + t(Q_2 - Q_1)$$

P, Q_1, Q_2 belong in R^2

To prove

P

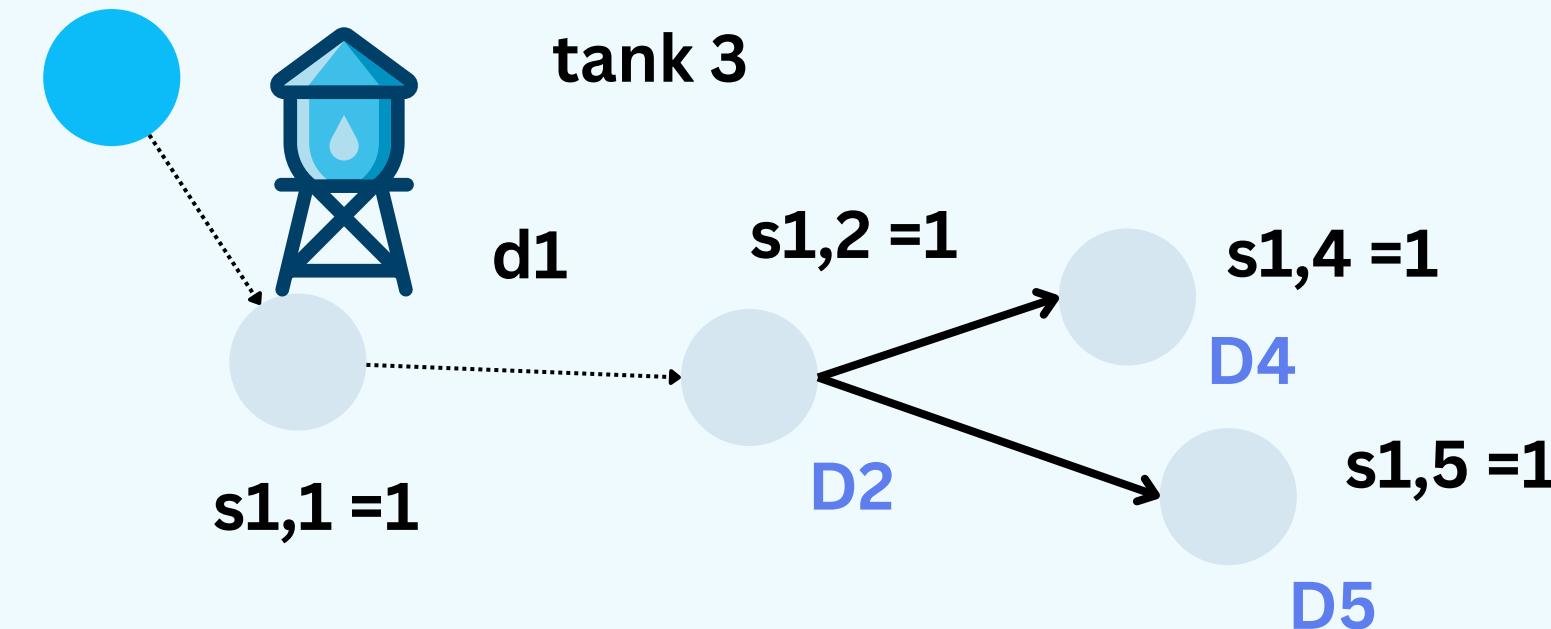
cannot be described as a linear combination of two distinct points that belong to R .



The paper's proof shows that this condition is only possible when $Q_1 = Q_2 = P$

Thus P is a corner point with fractional value

TANK CONFIGURATION IMPROVEMENT : NEW MODEL



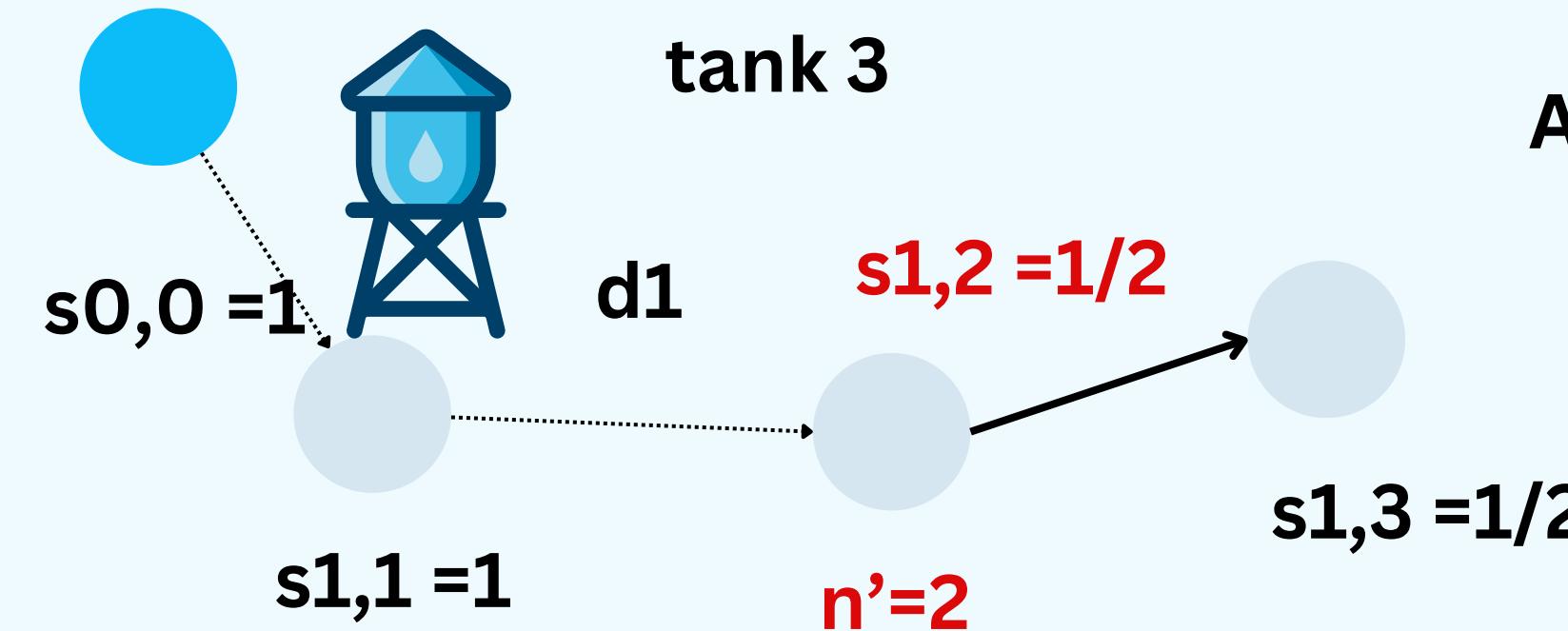
New Constraints

sum over n $s_{nm} = 1$ for all ancestors of m

$s_{nm} = s_{nk}$ where m is the child node and k is the descendant node

$$0 \leq s_{nm} \leq 1$$

TANK CONFIGURATION IMPROVEMENT : NEW MODEL TIGHTNESS



Assume this allocation of tanks is P

Claim 1

$s_{0,0} = 1$, root node can never be fractional or zero

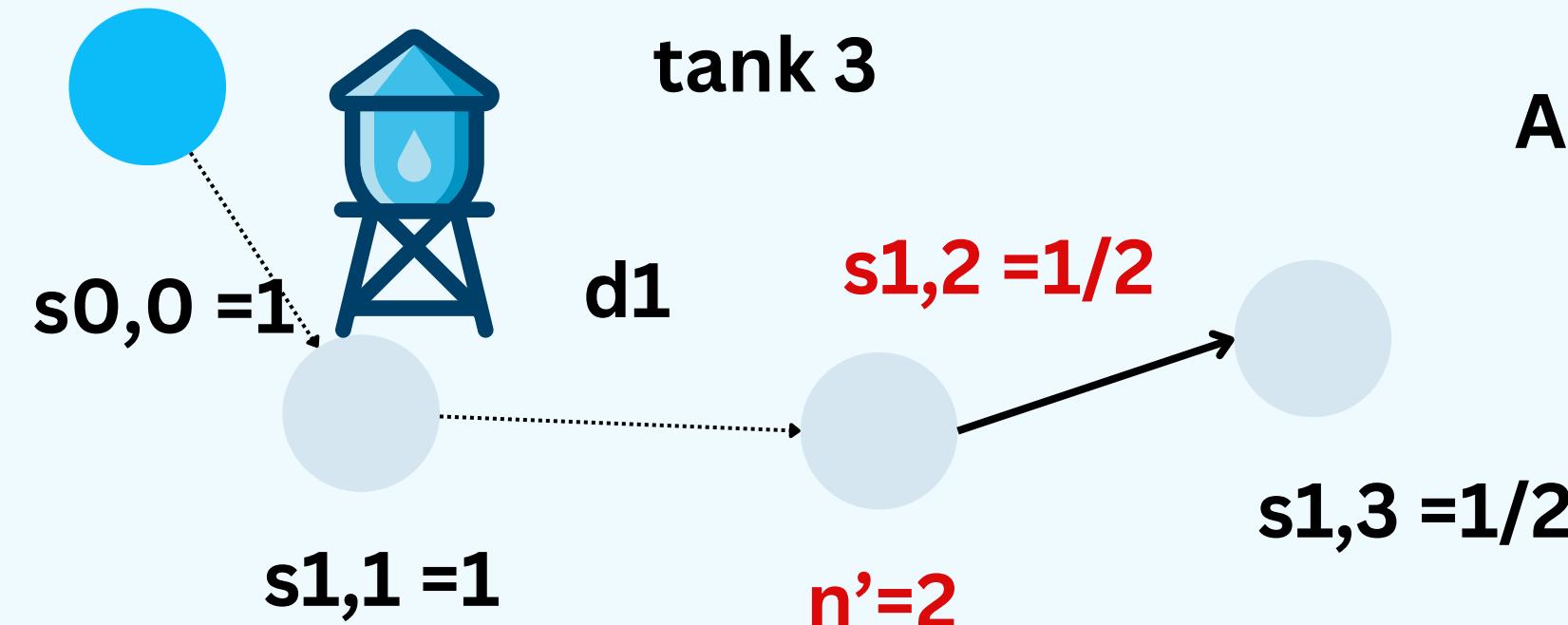
Claim 2

$s_{0,2} = 0$, descendants of secondary networks do not depend on nodes above their source node

Claim 3

- Q_1 is a valid allocation of our tank
- $s_{nm} = s_{nm}$ of P for all m not in 2 's path
- $s_{nm} = s_{nm} / 1 - s_{1,2}$ for all n which are ancestors of 2
- $s_{2m} = 0$ for all m which are descendants of 2

TANK CONFIGURATION IMPROVEMENT : NEW MODEL TIGHTNESS



Assume this allocation of tanks is P

Claim 4

- Q_2 is a valid allocation of our tank
- $s_{nm} = s_{nm}$ of P for all m not in 2 's path
- $s_{nm} = s_{nm} / s_{1,2}$ for all n which are ancestors of 2
- $s_{2m} = 0$ for all m which are descendants of 2

Claim 5

$$P = s_{1,2} * Q_1 + (1 - s_{1,2}) * Q_2$$

CONCLUSION

The first ILP model took 45 mins to solve a 150 nodes network while this one took 5sec

The model is being used by BMC and Gujrat government to model their water networks

Paper models real-life networks from Maharashtra :

Future research opportunities with creating a pump scheduling algorithm and designing a model for looped networks