

Part 1: Principle of Minimum Total Potential Energy (PMTPE)

FEEG3001/SESM6047 FEA in Solid Mechanics Prof A S Dickinson

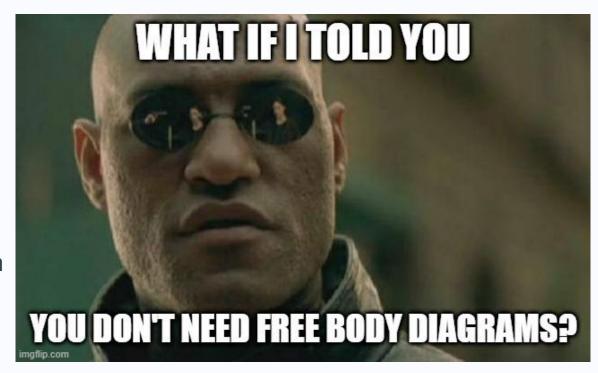
From 3rd October 2024

Principle of Minimum Total Potential Energy

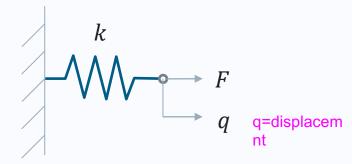
- PMTPE works for Statics:
 - Very simple problems on springs and masses
 - Elastic rods in tension and compression
 - Beam bending formulations (combined)
 - Trusses and Frames
- Hamilton's Principle for structural dynamics
 - Requires Lagrange's equations
 - special cases in Dynamics (free vibration, and rigid body motion)
- Then onto 2D elements (can still be 3D, it just means that the thickness of things like plates is really thing -> sheet type behaviour)

Structural mechanics - how?

- Free body diagrams, reaction forces
- Isolate the object of our attention (free body), and replace rest of universe with two things: Forces, and Moments.
- Classical or Newtonian Mechanics = Newton's Law + Euler's Law
 - Sum of external force = mass x acceleration
 - and Euler equivalent for rotation
- We will not draw FBDs but we will still do Newtonian Mechanics
- FEA goes back to 19th century, precomputers!



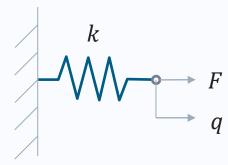
PMTPE for a Spring:



- Elastic structure has deformed under the influence of static forces.
 - U represents the stored elastic strain energy in the system
 - V represents the potential energy, the negative of work done by external forces
- You can imagine where the material points have moved to (e.g. a wing under pressure, a bridge under gravity, a drive shaft under torque) i.e. deformation.
- We imagine an infinite number of possible responses, and work out the PE for them; the response that corresponds to the minimum of Π is the true answer.

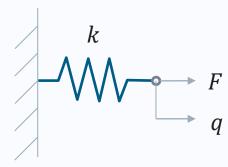
$$\Pi = U + V$$

PMTPE for a Spring:



- This means we don't solve the problem, we search for an answer to it.
- The true answer will have lower value of Π than all the values for Π for the wrong answers
- i.e. minimum with respect to *comparisons* with other, wrong answers, not with respect to increments of a function (i.e. d).
- Instead we'll use delta δ . $\delta\Pi=0$, using principle 'variational calculus'. $\delta\Pi$ is known as the first variation of Π .
- Valid for all mechanical systems under equilibrium (linear, nonlinear, differential, etc).
- We can't prove it, but we can verify it (then inductive inference instead of deduction).

Questions:



• What is the strain energy in the spring if the end moves by distance q?

$$U = \frac{1}{2} kq^2$$

What potential energy is there?

Negative of the work done by the external force:

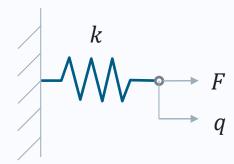
$$V = -W = -Fq$$

Reaction does no work, because displacement is zero. Therefore

$$\Pi = \frac{1}{2}kq^2 - Fq$$

• q is described as a 'generalised coordinate' or 'Degree of Freedom' (DoF) of the problem, because it's not just a coordinate – may not have units of distance or length, etc. But if we know q, we know everything about the mechanical system.

Minimum energy scenario:

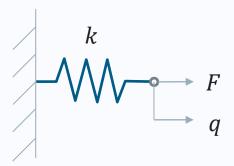


- So $\Pi = \Pi(q)$ and we need to find out what q is
- Without proof of variational calculus, by minimisation with respect to comparison with the false answers, the statement:

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\delta\Pi=0 < This means \Rightarrow \text{(implies)} \qquad \text{means} \frac{\partial\Pi(q)}{\partial q}=0 \qquad \text{(because reasons we skip over)}
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- (that is, differentiating Π with respect to q and setting it to zero)
- I'm not proving the 'implies' arrow. But if you trust me, the next step is easy:

Minimum energy scenario:



$$\frac{\partial \Pi(q)}{\partial q} = 0$$

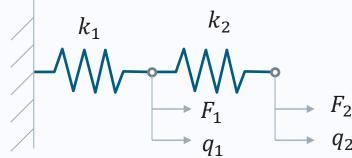
$$\Pi = \frac{1}{2}kq^2 - Fq \text{ so}$$

$$\frac{\partial \Pi(q)}{\partial q} = \frac{1}{2}k \times 2q - F = 0 \text{ or simply}$$

$$F = kq$$

- Look familiar?
- Imagine the alternative scenarios, if q was larger or smaller (Both result in non possible cases)

A more interesting problem:



- We can extend this to another problem: how about two springs in series on the same wall:
- Π becomes a function of two generalised coordinates (system has 2 DoF):

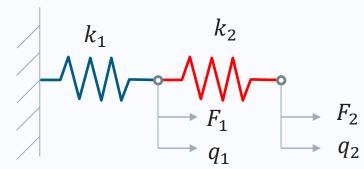
$$\Pi = \Pi(q_1, q_2)$$

• and the location of BOTH qs must obey the variational, comparison principle of minimum potential energy:

$$\frac{\partial \Pi}{\partial a} = 0$$
 2 degrees of freedom -> 2 unknowns -> 2 equations

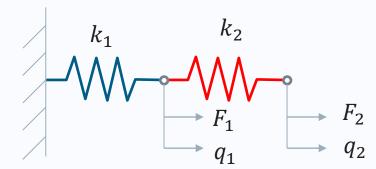
$$\frac{\partial \Pi}{\partial q_2} = 0$$

Again PMTPE:



- $\Pi = U + V$ so what is U? $U = \frac{1}{2}k_1q_1^2 + \frac{1}{2}k_2(q_2 q_1)^2 \text{ (strain energy in first spring + second spring)}$
- Question: is it $q_1 q_2$ or $q_2 q_1$?
- The squaring is forgiving, it doesn't matter!

Again PMTPE:



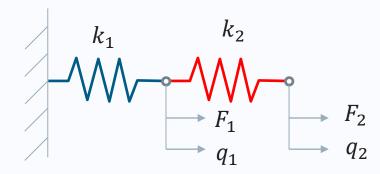
V = -W negative of work done by external forces

- Fixed end doesn't move, so doesn't do work.
- The other points move, so do work

$$W = F_1 q_1 + F_2 q_2$$

- Question: why F_2q_2 , and not $F_2(q_2-q_1)$?
- q_1 and q_2 are displacements in your reference frame; absolute displacement. Work done is just force x how far it moves.

Thank You



•
$$U = \frac{1}{2}k_1q_1^2 + \frac{1}{2}k_2(q_2 - q_1)^2$$

•
$$V = -(F_1q_1 + F_2q_2)$$

• Next session: we will combine, differentiate, and present in a matrix form...

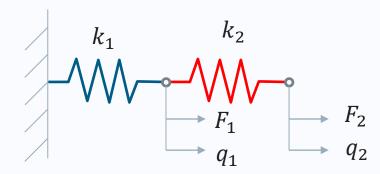


Part 1b: PMTPE: a 'Stiffness Matrix' for more complex problems

FEEG3001/SESM6047 FEA in Solid Mechanics Prof A S Dickinson

From 3rd October 2024

Reminder:

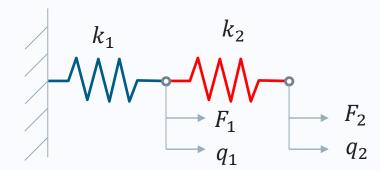


- Springs as Finite Elements
- What are the values of q_1 and q_2 when equilibrium is achieved?

$$\Pi=U+V$$
 (Total potential energy: strain energy plus external loading energy) $U={}^1\!/_2 k_1 q_1^2 + {}^1\!/_2 k_2 (q_2-q_1)^2$ (strain energy in first spring + second spring) $V=-W$ (negative of work done by external forces) $V=-(F_1q_1+F_2q_2)$

Combine, differentiate, and present in a matrix form...

Apply PMTPE:



$$\Pi = \frac{1}{2}k_1q_1^2 + \frac{1}{2}k_2(q_2 - q_1)^2 - (F_1q_1 + F_2q_2)$$
 so now we have $\Pi = \Pi(q_1, q_2)$ Two q s, so two degrees of freedom (DOF).

Equilibrium says

$$\delta\Pi(q_1,q_2)=0\Rightarrow \frac{\partial\Pi}{\partial q_1}=0 \text{ and } \frac{\partial\Pi}{\partial q_2}=0$$

(partial derivatives – not proven, but trusting variational calculus)

$$\frac{\partial\Pi}{\partial q_1}=k_1q_1+k_2(q_1-q_2)-F_1=0$$
 (expanding brackets above: chain rule), Or
$$F_1=(k_1+k_2)q_1-k_2q_2$$

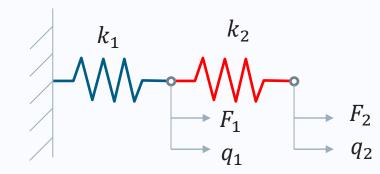
and

$$\frac{\partial \Pi}{\partial q_2} = 0 + k_2(q_2 - q_1) - F_2 = 0 \text{ or}$$

$$F_2 = -k_2q_1 + k_2q_2$$

Apply PMTPE:

$$F_1 = (k_1 + k_2)q_1 - k_2q_2$$
 and $F_2 = -k_2q_1 + k_2q_2$



- What kind of equations are these? qs are unknowns, Fs and ks are knowns...
- Linear, simultaneous, algebraic equations. They can be expressed in matrix form where:

$$\begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \begin{Bmatrix} F_1 \\ F_2 \end{bmatrix}$$
 Experience tells us the size: where each row comes from one equation:

$$\begin{bmatrix} (k_1+k_2) & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ F_2 \end{Bmatrix} \text{ or in general terms,}$$

$$[K]\{q\} = \{F\}$$

- {*F*} is the *force vector*
- $\{q\}$ is the *displacement vector*, and
- [K] is the **stiffness matrix**
- We use a computers to solve matrix equations fast when there are many more DOFs.

How to solve PMTPE...?

 $[K]{q} = {F}$ (with unknowns). This might be expressed as

$${q} = [K]^{-1}{F}$$

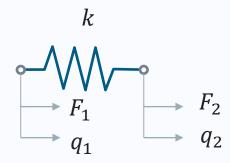
- Matrix inversion is very inefficient computationally, so often algorithms will use methods related to Gaussian elimination: upper triangulate or lower triangulate the matrix then back substitute.
- Mostly in this module we will be breaking down mechanical problems into this form. This one is really simple, but we do this for bones, whole ships, etc.

- (remember, no free body diagrams but you would get to the same result)
- Can we do this without needing to repeat the integration? Can we make a generic Finite Element, a tidy, generic package we can use more quickly?
- The spring element: simplest mechanical element that stores strain energy
- We formulate the element, then we collect information about it. A spring floating alone would have:

$$U = \frac{1}{2}k(q_2 - q_1)^2$$

And we can use a special form:

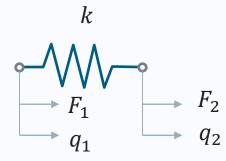
$$U = \frac{1}{2} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}_{1 \times 2}^T \begin{bmatrix} \\ \\ \\ \end{bmatrix}_{2 \times 2} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}_{2 \times 1}$$



Quadratic form is:

$$U = \frac{1}{2} (Aq_1^2 + Bq_1q_2 + Cq_2q_1 + Dq_2^2)$$

$$U = \frac{1}{2} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}^T \begin{bmatrix} A & B \\ C & D \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}$$



The square terms come from the diagonal, and the mixed terms from the off-diagonal

$$U = \frac{1}{2}k_{1}(q_{2} - q_{1})^{2}$$

$$U = \frac{1}{2}(k_{1}q_{1}^{2} - 2k_{1}q_{1}q_{2} + k_{1}q_{2}^{2})$$

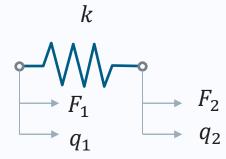
$$U = \frac{1}{2}(k_{1}q_{1}^{2} - k_{1}q_{1}q_{2} - k_{1}q_{1}q_{2} + k_{1}q_{2}^{2})$$

$$U = \frac{1}{2}\begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}^{T} \begin{bmatrix} ? & ? \\ ? & ? \end{bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}$$

Quadratic form is:

$$U = \frac{1}{2} (Aq_1^2 + Bq_1q_2 + Cq_2q_1 + Dq_2^2)$$

$$U = \frac{1}{2} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}^T \begin{bmatrix} A & B \\ C & D \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}$$



The square terms come from the diagonal, and the mixed terms from the off-diagonal

$$U = \frac{1}{2} k_1 (q_2 - q_1)^2$$

$$U = \frac{1}{2} (k_1 q_1^2 - 2k_1 q_1 q_2 + k_1 q_2^2)$$

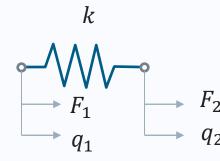
$$U = \frac{1}{2} (k_1 q_1^2 - k_1 q_1 q_2 - k_1 q_1 q_2 + k_1 q_2^2)$$

$$U = \frac{1}{2} {q_1 \choose q_2}^T \begin{bmatrix} k_1 & -k_1 \\ -k_1 & k_1 \end{bmatrix} {q_1 \choose q_2}$$

$$U = \frac{1}{2}k(q_2 - q_1)^2$$

can be written as:

$$U = \frac{1}{2} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}^T \begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}$$



Much easier to prove by back multiplication than forwards!

Recognise the structure from maths? *Quadratic form* in matrix algebra (i.e. will *only* give terms in q_1^2 , q_2^2 and $(q_1q_2)^2$; no terms in just q_1 or q_2).

$$\begin{bmatrix} k & -k \\ -k & k \end{bmatrix}$$
 is known as the **element stiffness matrix**, $[K]$

In short-hand we can write this as something which will reappear frequently:

$$U = \frac{1}{2} \{q\}^T [K] \{q\}$$

What was the point of all this?

- Crucially:
 - The Stiffness Matrix in the generalised equilibrium equation $\{q\} = [K]^{-1}\{F\}$
 - Is exactly the same Stiffness Matrix in the energy equation $U = \frac{1}{2} \{q\}^T [K] \{q\}$
 - and using this variational calculus observation, of energy expressions in the quadratic form, means we can skip the derivation!
- Next week we will join these stiffness matrices together...



Part 1c: Assembling Stiffness Matrices for multiple element problems

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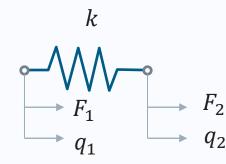
From 8th October 2024

For one spring element we have:

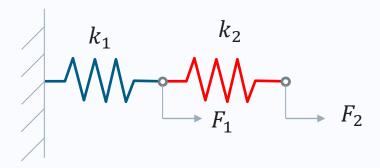
$$U = \frac{1}{2} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}^T \begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}$$

which is the same as:

$$U = \frac{1}{2} \{q\}^T [K] \{q\}$$



But what about when we have many? Let's chain two springs as before: and standardise them (like software would)



Redefine our *q* generalised coordinates:

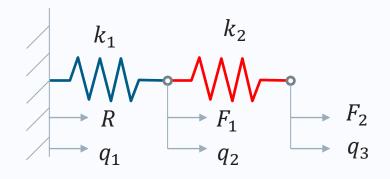
(Ignoring for now that we know $q_1 = 0...$)

$$U_1 = ?$$

$$U_1 = \frac{1}{2} k_1 (q_2 - q_1)^2$$

$$U_2 = ?$$

$$U_2 = \frac{1}{2} k_2 (q_3 - q_2)^2$$

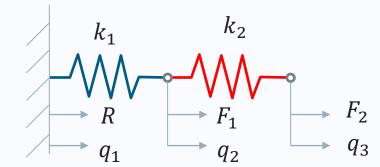


so that we can apply our general, off-the-shelf spring elements.

If we put these into quadratic form:

$$U = \frac{1}{2} \{q\}^T [K] \{q\}$$

we should get:



$$U_1 = \frac{1}{2} k_1 (q_2 - q_1)^2$$

can be expressed as:

$$U_1 = \frac{1}{2} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}^T \begin{bmatrix} k_1 & -k_1 \\ -k_1 & k_1 \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}$$
 (very similar, but now in k_1 instead of k)

and

$$U_2 = \frac{1}{2}k_2(q_3 - q_2)^2$$

can be expressed as:

$$U_2 = \frac{1}{2} \begin{Bmatrix} q_2 \\ q_3 \end{Bmatrix}^T \begin{bmatrix} k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{Bmatrix} q_2 \\ q_3 \end{Bmatrix}$$

A fundamental principle of finite element analysis is that the strain energy of the system will be the sum of its parts, because it is a simple, additive quantity (unlike e.g. temperature):

$$U = U_1 + U_2$$

So, for the whole system this time:

$$U = \frac{1}{2} \left\{ \begin{array}{c} \\ \\ \\ \\ \end{array} \right\}_{1 \times 3} \left[\begin{array}{c} \\ \\ \\ \end{array} \right]_{3 \times 3} \left\{ \begin{array}{c} \\ \\ \\ \\ \end{array} \right\}_{3 \times 1}$$

Because we have 3 degrees of freedom, it becomes a 3 sized. The energy is scalar and there are 3 of em

What is the product of a 1x3, a 3x3 and a 3x1 matrix?

A 1x1. Makes sense!

$$U_{1} = \frac{1}{2} k_{1} (q_{2} - q_{1})^{2}$$

$$U_{1} = \frac{1}{2} k_{1} (q_{1}^{2} - 2q_{1}q_{2} + q_{2}^{2})$$

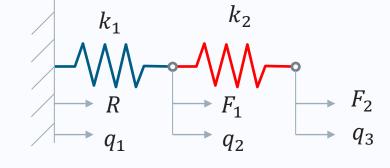
$$U_{1} = \frac{1}{2} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}^{T} \begin{bmatrix} k_{1} & -k_{1} \\ -k_{1} & k_{1} \end{Bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}$$

$$U_{2} = \frac{1}{2} k_{2} (q_{3} - q_{2})^{2}$$

$$U_{2} = \frac{1}{2} k_{2} (q_{2}^{2} - 2q_{2}q_{3} + q_{3}^{2})$$

$$U_{2} = \frac{1}{2} \begin{Bmatrix} q_{2} \end{Bmatrix}^{T} \begin{bmatrix} k_{2} & -k_{2} \\ -k_{2} & k_{3} \end{Bmatrix} \begin{Bmatrix} q_{2} \\ q_{3} \end{Bmatrix}$$

$$U = \frac{1}{2} \left\{ \begin{array}{c} \\ \\ \\ \\ \end{array} \right\}_{1 \times 3}^{T} \left[\begin{array}{c} \\ \\ \\ \\ \end{array} \right]_{3 \times 3} \left\{ \begin{array}{c} \\ \\ \\ \\ \end{array} \right\}_{3 \times 1}$$

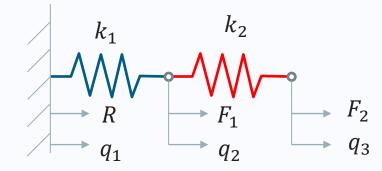


Expressing this requires a bit of imagination and matrix algebra...

Remember: the form is:

$$U_{1} = \frac{1}{2} (Aq_{1}^{2} + Bq_{1}q_{2} + Cq_{2}q_{1} + Dq_{2}^{2})$$

$$U_{1} = \frac{1}{2} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}^{T} \begin{bmatrix} A & B \\ C & D \end{bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}$$



The square terms come from the diagonal, and the mixed terms from the off-diagonal

$$U_{1} = \frac{1}{2} k_{1} (q_{2} - q_{1})^{2}$$

$$U_{1} = \frac{1}{2} (k_{1} q_{1}^{2} - k_{1} q_{1} q_{2} - k_{1} q_{1} q_{2} + k_{1} q_{2}^{2})$$

$$U_{1} = \frac{1}{2} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}^{T} \begin{bmatrix} k_{1} & -k_{1} \\ -k_{1} & k_{1} \end{Bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}$$

Assembly of Elements

$$U_{1} = \frac{1}{2} k_{1} (q_{2} - q_{1})^{2}$$

$$U_{1} = \frac{1}{2} k_{1} (q_{1}^{2} - 2q_{1}q_{2} + q_{2}^{2})$$

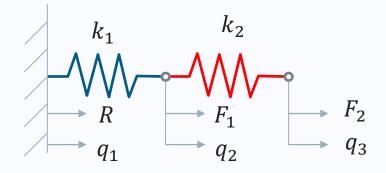
$$U_{1} = \frac{1}{2} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}^{T} \begin{bmatrix} k_{1} & -k_{1} \\ -k_{1} & k_{1} \end{Bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}$$

$$U_{2} = \frac{1}{2} k_{2} (q_{3} - q_{2})^{2}$$

$$U_{2} = \frac{1}{2} k_{2} (q_{2}^{2} - 2q_{2}q_{3} + q_{3}^{2})$$

$$U_{2} = \frac{1}{2} \begin{Bmatrix} q_{2} \\ q_{3} \end{Bmatrix}^{T} \begin{bmatrix} k_{2} & -k_{2} \\ -k_{2} & k_{2} \end{Bmatrix} \begin{Bmatrix} q_{2} \\ q_{3} \end{Bmatrix}$$

 $U = \frac{1}{2} \begin{cases} q_1 \\ q_2 \\ q_3 \end{cases}^T \qquad \begin{bmatrix} ? & ? & ? \\ ? & ? & ? \\ ? & ? & ? \end{bmatrix}_{2\times 3} \begin{cases} q_1 \\ q_2 \\ q_3 \end{pmatrix}_{3}.$



Basically we need to put the constituent parts of the 2x2 unit spring into the correct parts of the 3x3

 $U = U_1 + U_2$

Assembly of Elements

$$U_{1} = \frac{1}{2} k_{1} (q_{2} - q_{1})^{2}$$

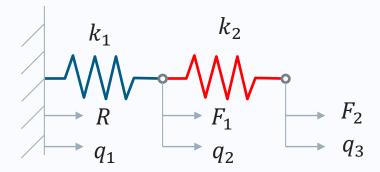
$$U_{1} = \frac{1}{2} k_{1} (q_{1}^{2} - 2q_{1}q_{2} + q_{2}^{2})$$

$$U_{1} = \frac{1}{2} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}^{T} \begin{bmatrix} k_{1} & -k_{1} \\ -k_{1} & k_{1} \end{Bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \end{Bmatrix}$$

$$U_{2} = \frac{1}{2} k_{2} (q_{3} - q_{2})^{2}$$

$$U_{2} = \frac{1}{2} k_{2} (q_{2}^{2} - 2q_{2}q_{3} + q_{3}^{2})$$

$$U_{2} = \frac{1}{2} \begin{Bmatrix} q_{2} \\ q_{3} \end{Bmatrix}^{T} \begin{bmatrix} k_{2} & -k_{2} \\ -k_{2} & k_{2} \end{Bmatrix} \begin{Bmatrix} q_{2} \\ q_{3} \end{Bmatrix}$$



In the 3x3 matrix, we kind of have a "q1" and "q2" row and also column

$$U = U_1 + U_2$$

$$U = \frac{1}{2} \begin{cases} q_1 \\ q_2 \\ q_3 \end{cases}^T \begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix}_{3 \times 3} \begin{cases} q_1 \\ q_2 \\ q_3 \end{pmatrix}_{3 \times 1}$$
 which could be checked by multiplying out.

Why are there zeroes on the two corners?

Assembly of Elements

$$U = \frac{1}{2} \begin{cases} q_1 \\ q_2 \\ q_3 \end{cases}^T \begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix}_{3 \times 3} \begin{cases} q_1 \\ q_2 \\ q_3 \end{pmatrix}_{3 \times 1}$$
 which contains $[K]$, the assembled stiffness matrix K and K are K and K are K are K are K are K and K are K are K and K are K are K are K and K are K are K and K are K are K and K are K are K are K and K are K are K and K are K and K are K are K and K are K are K and K are K and K are K are K and K are K are K and K are K and K are K are K are K and K are K are K and K are K are K and K are K and K are K are K and K are K are K and K are K and K are K are K are K are K are K and K are K are K are K and K are K and K are K and K are K are K are K are K are K are K and K are K are K are K and K are K are K and K are K and K are K are K are K are K are K are

- In performing assembly of the elements, we added the energies, but we did not simply add the matrices. Why?
- The dimensions and/or $\{q\}$ vectors don't match.
- Practically, how?

$$U_{1} = \frac{1}{2} k_{1} (q_{2} - q_{1})^{2}$$

$$U_{1} = \frac{1}{2} \begin{Bmatrix} q_{1} \\ q_{2} \\ q_{3} \end{Bmatrix}^{T} \begin{bmatrix} k_{1} & -k_{1} & 0 \\ -k_{1} & k_{1} & 0 \\ 0 & 0 & 0 \end{Bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \\ q_{3} \end{Bmatrix}$$

$$U_{2} = \frac{1}{2} k_{2} (q_{3} - q_{2})^{2}$$

$$U_{2} = \frac{1}{2} \begin{Bmatrix} q_{1} \\ q_{2} \\ q_{3} \end{Bmatrix}^{T} \begin{bmatrix} 0 & 0 & 0 \\ 0 & k_{2} & -k_{2} \\ 0 & -k_{2} & k_{2} \end{Bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \\ q_{3} \end{Bmatrix}$$

$$U = U_{1} + U_{2}$$

$$U = \frac{1}{2} \begin{Bmatrix} q_{1} \\ q_{2} \\ q_{3} \end{Bmatrix}^{T} \begin{bmatrix} k_{1} & -k_{1} & 0 \\ -k_{1} & k_{1} + k_{2} & -k_{2} \\ 0 & -k_{2} & k_{2} \end{Bmatrix} \begin{Bmatrix} q_{1} \\ q_{2} \\ q_{3} \end{Bmatrix}_{3 \times 1}$$

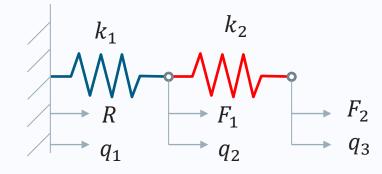
A practical, computational step, 'padding'.

Now, we will introduce the forces

What about the external loading terms?

Work done by external forces:

$$V=-W=$$
 ?
$$V=\left(Rq_{1}+F_{1}q_{2}+F_{2}q_{3}\right) \text{ (missing minus sign)}$$



Again we aim for a more general matrix form:

$$V = -\{? ? ?\}_{1\times 3} \{q\}_{3\times 1}$$

$$V = -\{R F_1 F_2\} \begin{Bmatrix} q_1 \\ q_2 \\ q_3 \end{Bmatrix}$$

So together with:

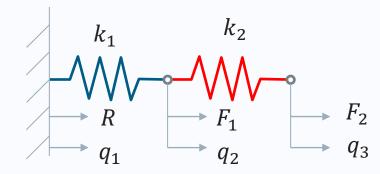
$$U = 1/2\{q\}_{1\times 3}^{T}[K]_{3\times 3}\{q\}_{3\times 1}$$
$$V = -\{F\}_{1\times 3}^{T}\{q\}_{3\times 1}$$

again both will multiply out to scalar energy values, which makes sense!

So applying PMTPE, in *i* notation:

$$\Pi = U + V$$

Equilibrium says
$$\delta\Pi(q_i) = 0 \Rightarrow \frac{\partial\Pi}{\partial q_i} = 0, i = 1, 2, ...$$



So using these quite general energy terms:

$$U = \frac{1}{2} \{q\}_{1 \times 3}^{T} [K]_{3 \times 3} \{q\}_{3 \times 1}$$

$$V = -\{F\}_{1\times 3}^T \{q\}_{3\times 1}$$

with potential energy terms taking quadratic form, our system is linear.

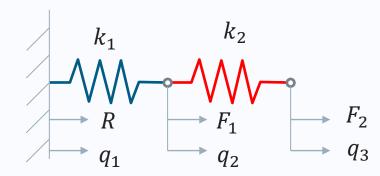
Do the partial differentiation if you want to... the *governing equation of equilibrium* takes the form:

$$[K]{q} = {F}$$

where [K] is our assembled stiffness matrix, and we can recycle the elemental stiffness matrices for each spring in our model, to *assemble* them (instead of simply adding them)

Note that now we have a reaction force!

$$\begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix} \begin{pmatrix} q_1 \\ q_2 \\ q_3 \end{pmatrix} = \begin{pmatrix} R \\ F_1 \\ F_2 \end{pmatrix}$$

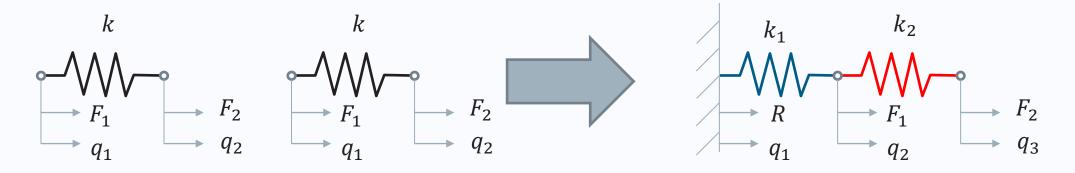


What is our motivation?

We use this like putting these ingredients together in a recipe,

'assembling' them,

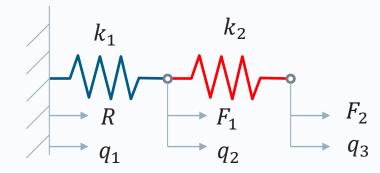
so our generalised coordinates of the generic element q_1 and q_2 are 'mapped' onto the generalised coordinates of the new problem.



- Can we solve it though? Can we invert the stiffness matrix?
- Remember the properties for a matrix to be invertible?
- The stiffness matrix is singular (its determinant is zero), so it cannot be inverted.
- The same is true for the elemental stiffness matrices.

(we can't solve the following, it's singular! So we need to add another constraint)

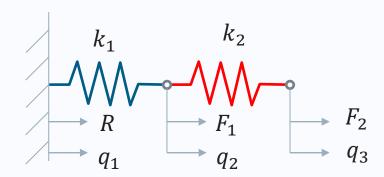
$$\begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix} \begin{pmatrix} q_1 \\ q_2 \\ q_3 \end{pmatrix} = \begin{pmatrix} R \\ F_1 \\ F_2 \end{pmatrix}$$



Is [K] invertible? What to do...?

$$[A] = \begin{bmatrix} a & b & c \\ d & e & f \\ g & h & i \end{bmatrix}$$

$$|A| = a \begin{vmatrix} e & f \\ h & i \end{vmatrix} - b \begin{vmatrix} d & f \\ g & i \end{vmatrix} + c \begin{vmatrix} d & e \\ g & h \end{vmatrix}$$



$$[K] = \begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ 0 & -k_2 & k_2 \end{bmatrix}$$

$$|K| = k_1 \begin{vmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{vmatrix} - -k_1 \begin{vmatrix} -k_1 & -k_2 \\ 0 & k_2 \end{vmatrix}$$

$$|K| = k_1 ((k_1 + k_2)k_2 - k_2^2) + k_1(-k_1k_2 - 0)$$

$$|K| = k_1(k_1k_2) + k_1(-k_1k_2)$$

$$|K| = 0$$

We need to assert a **boundary condition**:

$$q_1 = 0$$

This zero-BC case allows us to use an (unproven) trick where we strike out corresponding rows and columns of our governing equation:

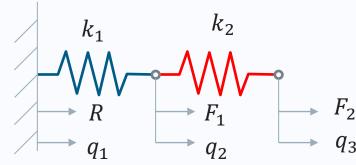
$$\begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{pmatrix} q_1 \\ q_2 \\ q_3 \end{pmatrix} = \begin{cases} R \\ F_1 \\ F_2 \end{pmatrix}$$

and rewrite what is left:

$$\begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{Bmatrix} q_2 \\ q_3 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ F_2 \end{Bmatrix}$$

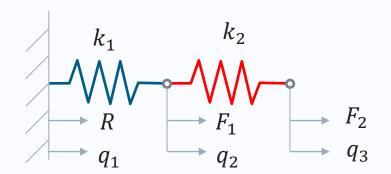
Looks familiar?

We can still calculate R! Using q_1(=0) and q 2, hence just q 2



Is [K] invertible? What to do...?

Can we invert this reduced stiffness matrix?



$$[K] = \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix}$$

$$|K| = ((k_1 + k_2)k_2 - k_2^2)$$

$$|K| = (k_1k_2 + k_2^2 - k_2^2)$$

$$|K| = k_1 k_2 \neq 0$$

So we CAN solve the reduced stiffness matrix!

Conclusion:

- Now you have a first element type on the shelf to pick up and assemble into a model. We don't have to make a new element for each new spring.
- But is this really Finite Element Analysis? Not really...
 - The FEM solves boundary value differential equations approximately
 - This is an ideal spring element, so there is no approximation: it is exact
- Next we will formulate a new type of element, an elastic rod in tension or compression