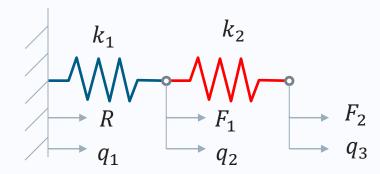


# Part 2a: Elastic Rods in Tension and Compression

FEEG3001/SESM6047 FEA in Solid Mechanics Prof A S Dickinson

From 11th October 2024

## Reminder from last time:



- We search for solutions for  $q_i$  in problems like this:
  - We express the system's Elastic Strain Energy  $\it U$  and its Potential Energy  $\it V$
  - by finding  $U_i$  for each element and V based on all applied loads.
  - We could apply PMTPE and calculate  $\frac{\partial \Pi(q_i)}{\partial q_i} = 0$  to obtain  $\{F\} = [K]\{q\}$
  - but because we can express  $U_i$  in quadratic form, we can take a shortcut where  $U_i = 1/2\{q\}^T[K_i]\{q\}$
  - and assemble our elemental  $[K_i]$  into a global [K]
  - and by expressing V in matrix form too, we find  $\{F\}$  and can therefore solve  $\{F\} = [K]\{q\}$
  - by inverting the stiffness matrix, allowing  $\{q\} = [K]^{-1}\{F\}$

#### This week:

#### Similarities:

- PMTPE gives us our governing equation of equilibrium (a set of linear quadratic equations)
- How we fetch and assemble our element stiffness matrices into an assembled stiffness matrix

#### Differences:

- Unlike for springs, we have some approximation, for 'elastic rods'

#### Next week:

Solving Rod FE Systems

Combining them with point Masses, and Springs

Example Questions

# Reminder for Springs - why don't we lose R?

We need to assert a **boundary condition**:

$$q_1 = 0$$

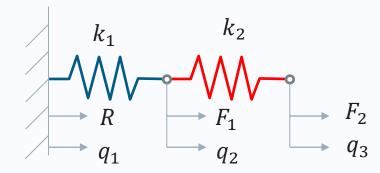
This zero-BC case allows us to use an (unproven) trick where we strike out corresponding rows and columns of our governing equation:

$$\begin{bmatrix} k_1 & -k_1 & 0 \\ -k_1 & k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{pmatrix} q_1 \\ q_2 \\ q_3 \end{pmatrix} = \begin{cases} R \\ F_1 \\ F_2 \end{pmatrix}$$

and rewrite what is left:

$$\begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix} \begin{Bmatrix} q_2 \\ q_3 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ F_2 \end{Bmatrix}$$

After solving for  $\{q\}$  we can come back and find R.



#### The FEA Procedure

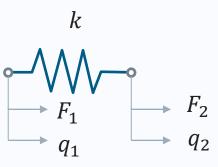
#### The Complete Procedure:

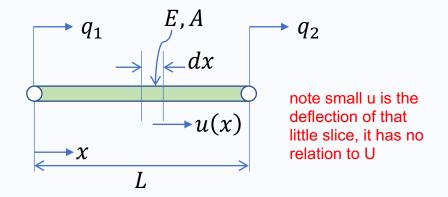
- 1. Describe the problem
- 2. Select a displacement estimation/approximation function
- 3. Relate displacement field to nodal displacements
- 4. Estimate strain from displacement
- 5. Estimate stress from strain
- 6. Apply PMTPE
- 7. Apply Boundary Conditions
- 8. Solve for nodal displacements
- 9. Calculate displacement field (optional)
- 10. Calculate strain field (optional)
- 11. Calculate stress field (optional)
- 12. Calculate reaction forces (optional)

#### Applied Practically:

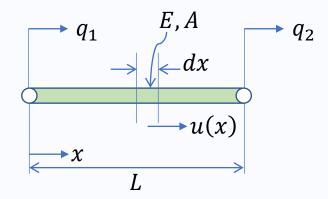
- 1. Describe the problem
- 2. Select an appropriate element type
- 3. Apply the elements to the problem, assembling them

- 4. Apply Boundary Conditions
- 5. Solve for nodal displacements
  - . Calculate displacement field (optional)
- 7. Calculate strain field (optional)
- 8. Calculate stress field (optional)
- 9. Calculate reaction forces (optional)





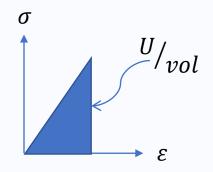
- Can deform only in tension or compression
- Unlike our spring, properties are described by E, A (cross section) and L (length), and the axial displacement of the endpoints  $q_1$  and  $q_2$  in coordinate system x
- We look at a small portion dx, and its deformation u(x) depends on where we take the slice
- Every point on the cross-section slice displaces the same amount
- So x is the 'label' (which cross section) and u(x) is the displacement.



• PMTPE requires an expression for the strain energy U in terms of the displacement field in the rod u.

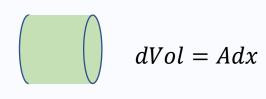
$$U = \frac{1}{2} \int_0^L EA \left(\frac{du}{dx}\right)^2 dx$$

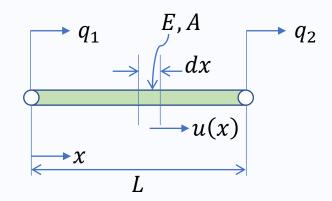
- Where does this come from? You should remember...
- Elastic strain energy per unit volume is the area under the stress-strain curve, for simplified 1D (uniaxial) elasticity:



$$U = \frac{1}{2} \int \sigma_x \varepsilon_x dVol$$

$$U = \frac{1}{2} \int \sigma_x \varepsilon_x Adx$$





• and because in 1D elasticity (highly simplified case) we can say that  $\sigma_x = E \varepsilon_x$ ,

$$U = \frac{1}{2} \int E \varepsilon_x \times \varepsilon_x \times A dx$$

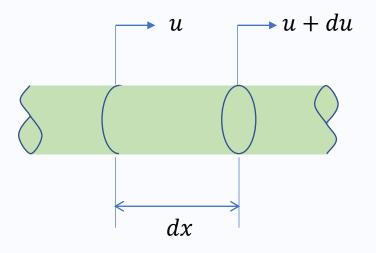
$$U = \frac{1}{2} \int E A \varepsilon_x^2 dx$$

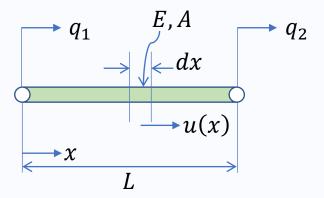
• What is  $\varepsilon_x$  given the displacement field?

$$\varepsilon_{x} = \frac{(u + du) - u}{dx} = \frac{du}{dx}$$

Hence

$$U = \frac{1}{2} \int EAu'^2 dx$$





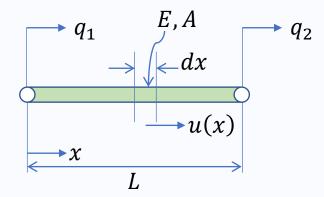
- **Aim:** we want to express the displacement throughout the element u(x) from the nodal displacements,  $q_1$  and  $q_2$ , (currently unknown).
- Now our first approximation: let's say u(x) is a linear function of x. i.e.:

$$u(x) = a + bx$$

- (We can make this linear function assumption for 1D elasticity, but we couldn't make that assumption for more complex cases).
- This is inconvenient though, as a and b have no meaning. A more convenient version, more general:

$$u(x) = N_1(x)q_1 + N_2(x)q_2$$

•  $q_1$  and  $q_2$  are unknowns,  $N_1$  and  $N_2$  are prescribed **shape functions** or **interpolation functions** for approximation: a common approach taken in all finite elements.



$$u(x) = N_1(x)q_1 + N_2(x)q_2$$

• The trick is we choose for these shape functions  $N_1$  and  $N_2$  some linear functions,  $g_1$  and  $g_2$ . (We could choose other functions...)

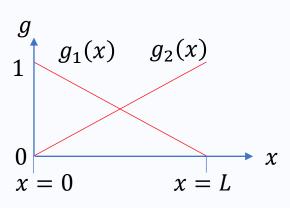
$$u(x) = g_1(x)q_1 + g_2(x)q_2$$

• We can define these by drawing our domain 0 to *L*:

$$g_1(x) = 1 - \frac{x}{L}$$

$$g_2(x) = \frac{x}{L}$$

$$u(x) = \left(1 - \frac{x}{L}\right)q_1 + \left(\frac{x}{L}\right)q_2$$



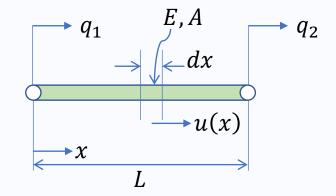
$$u(x) = \left(1 - \frac{x}{L}\right)q_1 + \left(\frac{x}{L}\right)q_2$$

- But what are these qs?
- Why didn't we just use u(x) = a + bx?
- Work out u(x) at each end:

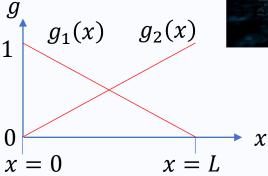
$$u(0) = \left(1 - \frac{0}{L}\right)q_1 + \left(\frac{0}{L}\right)q_2 = q_1$$

$$u(L) = \left(1 - \frac{L}{L}\right)q_1 + \left(\frac{L}{L}\right)q_2 = q_2$$

• The qs, which aren't functions of x, describe the end displacements! This is why we choose shape functions in that form.







 and assuming we have solved to find the q unknowns, we can use

$$u(x) = \left(1 - \frac{x}{L}\right)q_1 + \left(\frac{x}{L}\right)q_2$$

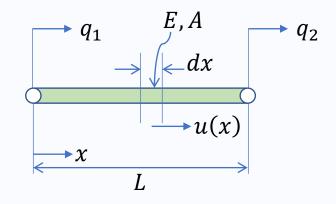
to give us values for strain:

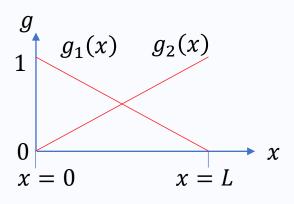
$$\varepsilon(x) = \frac{du}{dx} = \left(-\frac{1}{L}\right)q_1 + \left(\frac{1}{L}\right)q_2$$

• and in this simple 1D case, for stress:

$$\sigma(x) = E\varepsilon(x) = \left(-\frac{E}{L}\right)q_1 + \left(\frac{E}{L}\right)q_2$$

(check dimensional analysis if you like!)





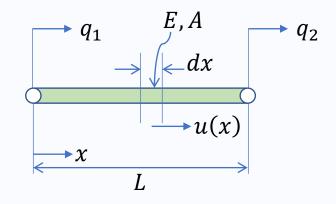
 or in matrix form (as more complex element types will require):

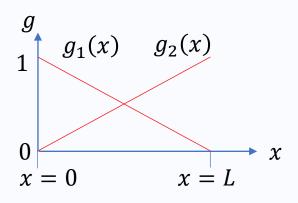
$$\varepsilon(x) = \frac{du}{dx} = \left(-\frac{1}{L}\right)q_1 + \left(\frac{1}{L}\right)q_2$$

$$\varepsilon(x) = \left[-\frac{1}{L} \quad \frac{1}{L}\right] \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}$$



$$\sigma(x) = E\varepsilon(x) = \left(-\frac{E}{L}\right)q_1 + \left(\frac{E}{L}\right)q_2$$
$$\sigma(x) = [E] \left[-\frac{1}{L} \quad \frac{1}{L}\right] \begin{Bmatrix} q_1 \\ q_2 \end{Bmatrix}$$





## Summary:

- We have written the internal field of displacement u(x) in terms of two chosen functions that happen here to be linear (Shape Functions)  $g_1(x)$  and  $g_2(x)$ .
- $g_1(x)$  has a property where its value is 1 at left and 0 at right, and
- $g_2(x)$  has a value 0 at left and 1 at right.
- Because they are linear functions, what about their combination? It must also be a linear function, if q values do not depend on x (they are single, nodal values).
- Next class will continue towards how we find values for q!