



GEOTECHNICAL, ENVIRONMENTAL AND CONSTRUCTION MATERIALS CONSULTANTS

N8 W22350 Johnson Drive, Suite A1 / Waukesha, WI 53186 / (262) 544-0118 / FAX (262) 549-5868

CLIENT	FYF, LLC P.O. Box 244 Fort Atkinson, WI 53538	PROJECT	Brent Rouse Soil Testing 640 Main Street Lake Geneva, Wisconsin
DATE	June 14, 2017	PROJECT NO.	1M-1706023
WEATHER	80°F Sunny	FIELD REP.	Alex Calaway / Bill Brauer

FOUNDATION SOIL TESTING

As requested by our client, our representative reported to the above referenced site to test the strength of the soils at foundation grade on a spot-check basis. However, at the time of arrival the foundations were not excavated to bearing grade.

A review of the project documents indicated that a subsurface exploration and foundation analysis has not been performed. Suitable bearing soil strength for foundation support was not provided. It is recommended that a subsurface exploration be performed to test the suitability of the underlying subsurface soils for the proposed construction.

Observations indicated that the footing excavations for east side building addition have not been excavated to planned footing grade. Hand auger probes and dynamic cone penetrometer tests were performed randomly along the existing soil surface to a depth of -2.5 feet below existing surface (approximately -10.5 feet below the top of sidewalk located directly north of planned footing) to visually classify and verify the strength of the existing soils in the planned footing area.

Samples obtained from the auger probes indicated that the existing soils consisted of sand with cobble and trace silt. Dynamic cone penetrometer tests indicated a loose relative density with a correlated standard penetration resistance (N value) of 5 to 6 (report of soil strength test results is attached).

As indicated above, we have only hand-tested the shallow soils. We make no statement regarding the bearing capacity or settlement characteristics of deeper soils. As noted above the area was not excavated to bottom of footing; therefore, it cannot be determined if the footing grades soils are suitable for footings. It should be noted that a wetland was located to the southeast of the footing area; it is possible that at footing elevations that water may be encountered; however, it was not encountered while hand auger excavating.

The contractor was reminded that the exposed footing grade soils must be protected from water, frost, and construction activity since these soils have the potential of softening, thus allowing a decrease in strength and increased settlement characteristics to occur. If the footing grade soils become wet, frozen or soft prior to construction, they must be removed and replaced or other remedial construction may be required.

Locations and elevations of tests were assessed in accordance with the accuracy and proximity of provided survey control. Unless otherwise noted, the test locations and elevations were assessed by pacing and hand-level methods.

We make no warranty, express or implied, except that our services were performed in accordance with engineering principles generally accepted at this time and in this location.

REVIEWING ENGINEER: Steven P. Homar, P.E.



GILES

ENGINEERING ASSOCIATES, INC.

REPORT OF SOIL STRENGTH TESTS

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TEST DATA

TEST NO	FOOTING DIMENSIONS	*ELEV/ DEPTH ft	VISUAL SOIL DESCRIPTION	IN-PLACE SOIL STRENGTH		SPECIFIED SOIL STRENGTH psf/N value	A	B	REMARKS
				qc	DCP				
1A		-8	Organic Soils			N/A			
1B		-9	Organic Soils			N/A			
1C		-9½	Sand with Cobbles, trace Silt		N=6 (1750 psf)	N/A			
1D		-10	Sand with Cobbles, trace Silt		N=5 (1500 psf)	N/A			
1E		-10½	Sand with Cobbles, trace Silt		N=5 (1500 psf)	N/A			

*ELEV/DEPTH REF.: 0 = top of sidewalk directly north of footing

A-test result greater than specification
B-test result less than specification

TEST NO.

LOCATION

1	about 7 feet east of existing building - approximately midpoint of footing
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OBSERVATIONS:

Footings Formed: No
Loose Soil Removed: No
Rebar Placed: No
Concrete Placed: No

SOILS REPORT:

qc-Cone Penetrometer: correlates to unconfined compressive strength, psf.
DCP-Dynamic Cone Penetrometer: correlates to SPT resistance or N value, blows/ft.

REVIEWED BY: Angela A. Jacobi

REVIEWING ENGINEER: Steven P. Homar, P.E.