

RECTANGULAR SPREAD FOOTING - PRELIMINARY

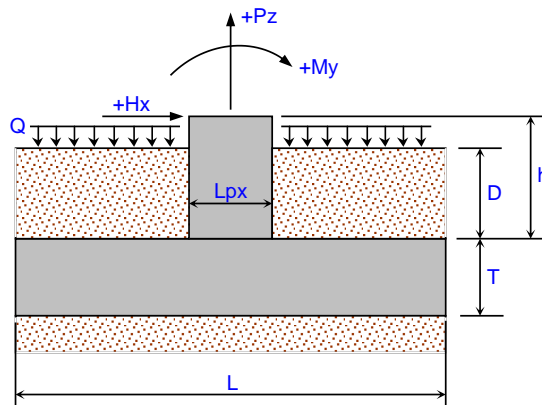
**For Assumed Rigid Footing with from 1 To 8 Piers
Subjected to Uniaxial or Biaxial Eccentricity**

| | | | |
|-------------|--|-------------|----------|
| Job Name: | | Subject: | |
| Job Number: | | Originator: | Checker: |

Input Data:

Footing Data:

Footing Length, L = 9,000 ft.
 Footing Width, B = 7,250 ft.
 Footing Thickness, T = 1,300 ft.
 Concrete Unit Wt., γ_c = 0,150 kcf
 Soil Depth, D = 2,500 ft.
 Soil Unit Wt., γ_s = 0,110 kcf
 Pass. Press. Coef., Kp = 2,050
 Coef. of Base Friction, μ = 0,280
 Uniform Surcharge, Q = 0,000 ksf

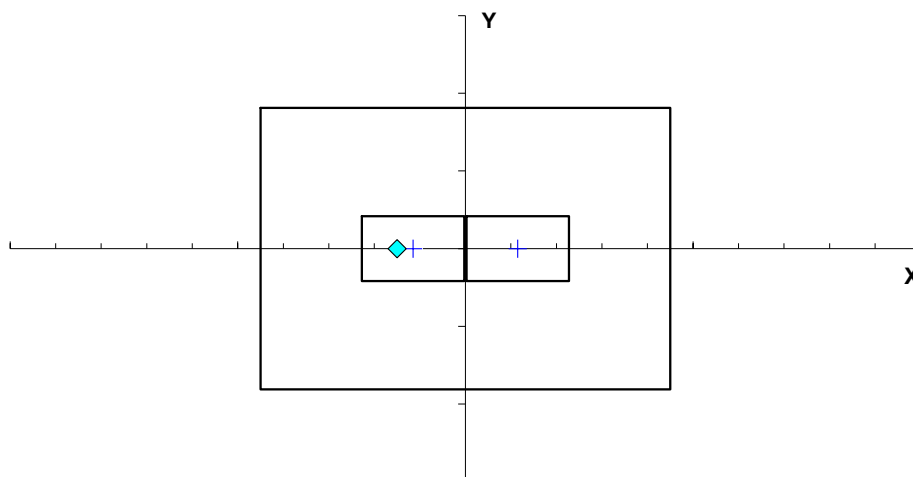


Pier/Loading Data:

Number of Piers = 2

Nomenclature

| | Pier #1 | Pier #2 | | | | | | |
|-------------|---------|---------|--|--|--|--|--|--|
| Xp (ft.) = | -1,150 | 1,150 | | | | | | |
| Yp (ft.) = | 0,000 | 0,000 | | | | | | |
| Lpx (ft.) = | 2,250 | 2,250 | | | | | | |
| Lpy (ft.) = | 1,670 | 1,670 | | | | | | |
| h (ft.) = | 2,500 | 2,500 | | | | | | |
| Pz (k) = | -88,50 | 0,00 | | | | | | |
| Hx (k) = | -20,70 | 0,00 | | | | | | |
| Hy (k) = | 0,00 | 0,00 | | | | | | |
| Mx (ft-k) = | 0,00 | 0,00 | | | | | | |
| My (ft-k) = | 0,00 | 0,00 | | | | | | |



FOOTING PLAN

(continued)

Results:

Total Resultant Load and Eccentricities:

| | | |
|----------------|---------|--------------------|
| $\Sigma P_z =$ | -119,92 | kips |
| $e_x =$ | -1,50 | ft. ($\leq L/6$) |
| $e_y =$ | 0,00 | |

Overturning Check:

| | | |
|-------------------|--------|----------------|
| $\Sigma M_{rx} =$ | N.A. | ft-kips |
| $\Sigma M_{oy} =$ | N.A. | ft-kips |
| $FS(ot)x =$ | N.A. | |
| $\Sigma M_{ry} =$ | 437,86 | ft-kips |
| $\Sigma M_{ox} =$ | -78,66 | ft-kips |
| $FS(ot)y =$ | 5,566 | (≥ 1.5) |

Sliding Check:

| | | |
|---------------|-------|----------------|
| Pass(x) = | 6,69 | kips |
| Frict(x) = | 33,58 | kips |
| $FS(slid)x =$ | 1,946 | (≥ 1.5) |
| Passive(y) = | 8,31 | kips |
| Frict(y) = | 33,58 | kips |
| $FS(slid)y =$ | N.A. | |

Uplift Check:

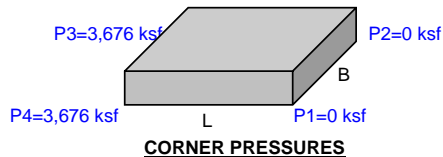
| | | |
|-------------------------------|---------|------|
| $\Sigma P_z(\text{down}) =$ | -119,92 | kips |
| $\Sigma P_z(\text{uplift}) =$ | 0,00 | kips |
| $FS(\text{uplift}) =$ | N.A. | |

Bearing Length and % Bearing Area:

| | | |
|----------------|--------|-----|
| Dist. x = | N.A. | ft. |
| Dist. y = | N.A. | ft. |
| Brg. Lx = | 9,000 | ft. |
| Brg. Ly = | 7,250 | ft. |
| %Brg. Area = | 100,00 | % |
| Biaxial Case = | N.A. | |

Gross Soil Bearing Corner Pressures:

| | | |
|------|-------|-----|
| P1 = | 0,000 | ksf |
| P2 = | 0,000 | ksf |
| P3 = | 3,676 | ksf |
| P4 = | 3,676 | ksf |



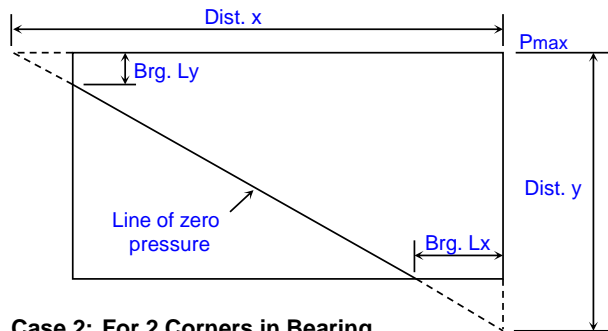
Maximum Net Soil Pressure:

$$P_{\max}(\text{net}) = P_{\max}(\text{gross}) - (D+T) \cdot \gamma_s$$

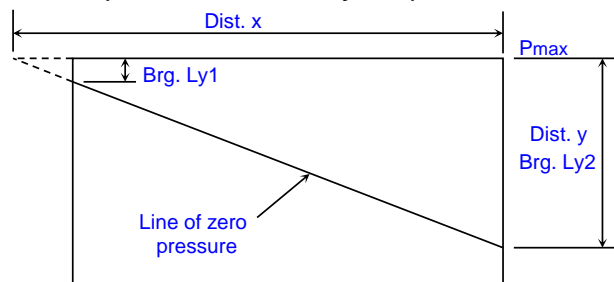
$$P_{\max}(\text{net}) = 3,258 \text{ ksf}$$

Nomenclature for Biaxial Eccentricity:

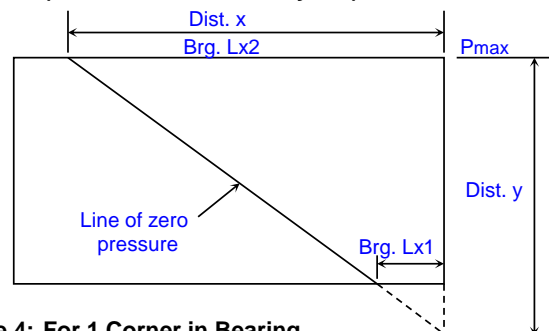
Case 1: For 3 Corners in Bearing (Dist. x > L and Dist. y > B)



Case 2: For 2 Corners in Bearing (Dist. x > L and Dist. y ≤ B)



Case 3: For 2 Corners in Bearing (Dist. x ≤ L and Dist. y > B)



Case 4: For 1 Corner in Bearing (Dist. x ≤ L and Dist. y ≤ B)

