

Job No.: Q2256 - 2	Address: 1130 Thongcaster Rd, Burnt Hill, New Zealand	Date: 04/03/2024
Latitude: -43.385953	Longitude: 172.12497	Elevation: 248.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N4	Ground Snow Load	1.25 KPa	Roof Snow Load	0.88 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.6 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	41.45 m/s
Wind Pressure	1.03 KPa	Lee Zone	YES	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Open

For roof $C_{p,i} = 0.6731$

For roof $C_{p,e}$ from 0 m To 1.65 m $C_{p,e} = -0.94$ $p_e = -0.60$ KPa $p_{net} = -1.12$ KPa

For roof $C_{p,e}$ from 1.65 m To 3.30 m $C_{p,e} = -0.88$ $p_e = -0.56$ KPa $p_{net} = -1.08$ KPa

For wall Windward $C_{p,i} = 0.6731$ side Wall $C_{p,i} = -0.6$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 11.70 m $C_{p,e} = 0.7$ $p_e = 0.65$ KPa $p_{net} = 1.32$ KPa

For side wall $C_{p,e}$ from 0 m To 3.30 m $C_{p,e} =$ $p_e = -0.60$ KPa $p_{net} = 0.07$ KPa

Maximum Upward pressure used in roof member Design = 1.12 KPa

Maximum Downward pressure used in roof member Design = 0.63 KPa

Maximum Wall pressure used in Design = 1.32 KPa

Maximum Racking pressure used in Design = 1.11 KPa

Design Summary

Purlin Design

Purlin Spacing = 800 mm	Purlin Span = 3450 mm	Try Purlin 150x50 SG8 Dry
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Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.73 S1 Downward =9.63 S1 Upward =18.72

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.4 Kn-m	Capacity	1.26 Kn-m	Passing Percentage	315.00 %
$M_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nDn}$	1.4 Kn-m	Capacity	1.68 Kn-m	Passing Percentage	120.00 %
$M_{0.9D-W_nUp}$	-1.07 Kn-m	Capacity	-1.54 Kn-m	Passing Percentage	143.93 %

Pole Shed App Ver 01 2022

V _{1.35D}	0.47 Kn	Capacity	7.24 Kn	Passing Percentage	1540.43 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.63 Kn	Capacity	9.65 Kn	Passing Percentage	592.02 %
V _{0.9D-W_nUp}	-1.24 Kn	Capacity	-12.06 Kn	Passing Percentage	972.58 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 8.86 mm

Limit by Woolcock et al, 1999 Span/240 = 14.17 mm

Deflection under Dead and Service Wind = 12.04 mm

Limit by Woolcock et al, 1999 Span/100 = 34.00 mm

Reactions

Maximum downward = 1.63 kn Maximum upward = -1.24 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 3600 mm

Internal Rafter Span = 5850 mm

Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 6.81 S₁ Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	5.20 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	193.85 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	18.17 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	73.97 %
M _{0.9D-W_nUp}	-13.78 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	121.92 %
V _{1.35D}	3.55 Kn	Capacity	28.94 Kn	Passing Percentage	815.21 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	12.43 Kn	Capacity	38.6 Kn	Passing Percentage	310.54 %
V _{0.9D-W_nUp}	-9.42 Kn	Capacity	-48.24 Kn	Passing Percentage	512.10 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 13.5 mm

Limit by Woolcock et al, 1999 Span/240 = 25.00 mm

Deflection under Dead and Service Wind = 20.375 mm

Limit by Woolcock et al, 1999 Span/100 = 60.00 mm

Reactions

Maximum downward = 12.43 kn Maximum upward = -9.42 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

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Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$ $f_{pj} = 12.9$ Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

$K_{11} = 2.0$ $f_{cj} = 36.1$ Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -9.42 Kn

Intermediate Design Sides

Intermediate Spacing = 3000 mm

Intermediate Span = 3150 mm

Try Intermediate 2x200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 1.00 S_1 Downward = 11.27 S_1 Upward = 0.67

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	2.46 Kn-m	Capacity	7.46 Kn-m	Passing Percentage	303.25 %
$V_{0.9D-WnUp}$	3.12 Kn-m	Capacity	32.16 Kn-m	Passing Percentage	1030.77 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 27.44 mm

Limit by Woolcock et al, 1999 Span/100 = 31.50 mm

Reactions

Maximum = 3.12 kn

Girt Design Front and Back

Girt's Spacing = 700 mm

Girt's Span = 3600 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.95 S_1 Downward = 9.63 S_1 Upward = 13.62

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.50 Kn-m	Capacity	1.99 Kn-m	Passing Percentage	132.67 %
$V_{0.9D-WnUp}$	1.66 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	726.51 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 35.75 mm

Limit by Woolcock et al, 1999 Span/100 = 36.00 mm

Sag during installation = 10.18 mm

Reactions

Maximum = 1.66 kn

Girt Design Sides

Girt's Spacing = 700 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.79 S1 Downward =9.63 S1 Upward =17.59

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.04 Kn-m	Capacity	1.65 Kn-m	Passing Percentage	158.65 %
$V_{0.9D-WnUp}$	1.39 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	867.63 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 17.24 mm

Limit by Woolcock et al. 1999 Span/100 = 30.00 mm

Sag during installation =4.91 mm

Reactions

Maximum = 1.39 kn

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1550) x Ks(1.5) x 0.5 x tan(30) x π x Dia of Pile(0.6) x Height of Pile(1550)

Skin Friction = 19.40 Kn

Weight of Pile + Pile Skin Friction = 23.43 Kn

Uplift on one Pile = 9.67 Kn

Uplift is ok