Job No.:	2405053-5	**Address:**	270 Mt Heslington Road, Brightwater, New Zealand	**Date:**	29/07/2024
Latitude:	-41.403304	**Longitude:**	173.101144	**Elevation:**	82 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	6.7 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	40.91 m/s
Wind Pressure	1 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Gable   Enclosed

For roof Cp,i = -0.3

For roof   CP,e   from 0 m   To 3.08 m   Cpe = -0.992   pe = -1.06 KPa   pnet = -1.06 KPa

For roof   CP,e   from 3.08 m   To 6.15 m   Cpe = -0.854   pe = -0.91 KPa   pnet = -0.91 KPa

For wall Windward Cp,i = -0.3   side Wall Cp,i = -0.3

For wall Windward and Leeward   CP,e   from 0 m   To 8 m   Cpe = 0.7   pe = 0.75 KPa   pnet = 1.11 KPa

For side wall   CP,e   from 0 m   To 6.15 m   Cpe =   pe = -0.7 KPa   pnet = -0.7 KPa

Maximum Upward pressure used in roof member Design = 1.06 KPa

Maximum Downward pressure used in roof member Design = 0.29 KPa

Maximum Wall pressure used in Design = 1.11 KPa

Maximum Racking pressure used in Design = 1.08 KPa

Design Summary

Rafter Design Internal

Internal Rafter Load Width = 5000 mm   Internal Rafter Span = 7850 mm   Try Rafter 2x360x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =1.00   S1 Downward =8.40   S1 Upward =8.40

Shear Capacity of timber =5.3 MPa   Bending Capacity of timber =48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	13.00 Kn-m	Capacity	43.44 Kn-m	Passing Percentage	**334.15 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	26.00 Kn-m	Capacity	57.92 Kn-m	Passing Percentage	**222.77 %**
$M_{0.9D-WnUp}$	-32.16 Kn-m	Capacity	-72.42 Kn-m	Passing Percentage	**225.19 %**
$V_{1.35D}$	6.62 Kn	Capacity	55.22 Kn	Passing Percentage	**834.14 %**

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$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	13.25 Kn	Capacity	73.64 Kn	Passing Percentage	**555.77 %**
$V_{0.9D-WnUp}$	-16.39 Kn	Capacity	-92.04 Kn	Passing Percentage	**561.56 %**

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 18.705 mm	Limit by Woolcock et al, 1999 Span/240 = 33.33 mm
Deflection under Dead and Service Wind = 22.345 mm	Limit by Woolcock et al, 1999 Span/100 = 80.00 mm

Reactions

Maximum downward =13.25 kn   Maximum upward = -16.39 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -16.39 Kn

Rafter Design External

External Rafter Load Width = 2500 mm   External Rafter Span = 8097 mm   Try Rafter 300x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =0.88

K8 Upward =0.88   S1 Downward =15.50   S1 Upward =15.50

Shear Capacity of timber =5.3 MPa   Bending Capacity of timber =48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	6.91 Kn-m	Capacity	13.69 Kn-m	Passing Percentage	**198.12 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	13.83 Kn-m	Capacity	18.26 Kn-m	Passing Percentage	**132.03 %**
$M_{0.9D-WnUp}$	-17.11 Kn-m	Capacity	-22.82 Kn-m	Passing Percentage	**133.37 %**
$V_{1.35D}$	3.42 Kn	Capacity	23.01 Kn	Passing Percentage	**672.81 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	6.83 Kn	Capacity	30.68 Kn	Passing Percentage	**449.19 %**
$V_{0.9D-WnUp}$	-8.45 Kn	Capacity	-38.35 Kn	Passing Percentage	**453.85 %**

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 35.91 mm	Limit by Woolcock et al, 1999 Span/240= 33.33 mm
Deflection under Dead and Service Wind = 38.61 mm	Limit by Woolcock et al, 1999 Span/100 = 80.00 mm

Reactions

Maximum downward =6.83 kn   Maximum upward = -8.45 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds .... (Eq 4.12) = -40.07 kn > -8.45 Kn

Single Shear Capacity under short term loads = -14.56 Kn > -8.45 Kn

Intermediate Design Front and Back

Intermediate Spacing = 2500 mm	Intermediate Span = 5450 mm	Try Intermediate 2x250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =0.97

K8 Upward =1.00   S1 Downward =12.68   S1 Upward =0.99

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	10.30 Kn-m	Capacity	11.66 Kn-m	Passing Percentage	**113.20 %**
$V_{0.9D-WnUp}$	7.56 Kn	Capacity	-40.2 Kn	Passing Percentage	**531.75 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 45.33 mm	Limit byWoolcock et al, 1999 Span/100 = 54.50 mm

Reactions

Maximum = 7.56 kn

Girt Design Front and Back

Girt's Spacing = 1200 mm   Girt's Span = 2500 mm   Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =0.86   S1 Downward =9.63   S1 Upward =16.05

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.04 Kn-m	Capacity	1.80 Kn-m	Passing Percentage	**173.08 %**
$V_{0.9D-WnUp}$	1.67 Kn	Capacity	12.06 Kn	Passing Percentage	**722.16 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 7.19 mm   Limit by Woolcock et al, 1999 Span/100 = 25.00 mm
Sag during installation = 2.37 mm

Reactions

Maximum = 1.67 kn

Girt Design Sides

Girt's Spacing = 0 mm   Girt's Span = 4000 mm   Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =0.65   S1 Downward =9.63   S1 Upward =20.31

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.00 Kn-m	Capacity	1.38 Kn-m	Passing Percentage	**Infinity %**
$V_{0.9D-WnUp}$	0.00 Kn	Capacity	12.06 Kn	Passing Percentage	**Infinity %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 0.00 mm   Limit by Woolcock et al. 1999 Span/100 = 40.00 mm
Sag during installation =15.52 mm

Reactions

Maximum = 0.00 kn

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(2400) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(2400)

Skin Friction = 46.52 Kn

Weight of Pile + Pile Skin Friction = 50.15 Kn

Uplift on one Pile = 16.70 Kn

Uplift is ok