



Pole Shed App Ver 01 2022

**Job No.:** 2501033 - 1

**Address:** 214 Eves Valley Road,, Brightwater, New Zealand

**Date:** 3/13/2025

**Latitude:** -41.332654

**Longitude:** 173.077902

**Elevation:** 96 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N3	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.65 m
Wind Region	NZ2	Terrain Category	2.73	Design Wind Speed	40.56 m/s
Wind Pressure	0.99 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Open

For roof  $C_{p,i} = 0.5647$

For roof  $C_{p,e}$  from 0 m To 3.0 m  $C_{pe} = -0.8492$   $p_e = -0.65$  KPa  $p_{net} = -1.13$  KPa

For roof  $C_{p,e}$  from 3 m To 6.0 m  $C_{pe} = -0.5299$   $p_e = -0.40$  KPa  $p_{net} = -0.88$  KPa

For wall Windward  $C_{p,i} = 0.5647$  side Wall  $C_{p,i} = -0.5838$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 10 m  $C_{pe} = 0.7$   $p_e = 0.62$  KPa  $p_{net} = 1.20$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.65 m  $C_{pe} =$   $p_e = -0.58$  KPa  $p_{net} = 0.00$  KPa

Maximum Upward pressure used in roof member Design = 1.13 KPa

Maximum Downward pressure used in roof member Design = 0.67 KPa

Maximum Wall pressure used in Design = 1.20 KPa

Maximum Racking pressure used in Design = 0.89 KPa

**Design Summary**

**Girt Design Front and Back**

Girt's Spacing = 750 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

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### Pole Shed App Ver 01 2022

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.98    S1 Downward =9.63    S1 Upward =12.44

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>Wind+Snow</sub>	1.01 Kn-m	Capacity	2.05 Kn-m	Passing Percentage	<b>202.97 %</b>
V <sub>0.9D-WnUp</sub>	1.35 Kn	Capacity	12.06 Kn	Passing Percentage	<b>893.33 %</b>

#### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 10.07 mm    Limit by Woolcock et al, 1999 Span/100 = 30.00 mm

Sag during installation = 4.91 mm

#### **Reactions**

Maximum = 1.35 kn

#### **Girt Design Sides**

Girt's Spacing = 750 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

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## Middle Pole Design

### Geometry

150 SED H5 (Minimum 175 dia. at Floor Level)	Dry Use	Height	3360 mm
Area	20729 mm <sup>2</sup>	As	15546.6796875 mm <sup>2</sup>
Ix	34210793 mm <sup>4</sup>	Zx	421056 mm <sup>3</sup>
Iy	34210793 mm <sup>4</sup>	Zy	421056 mm <sup>3</sup>
Lateral Restraint	1300 mm c/c		

### Loads

Total Area over Pole = 9 m<sup>2</sup>

Dead	2.25 Kn	Live	2.25 Kn
Wind Down	6.03 Kn	Snow	0.00 Kn
Moment wind	4.44 Kn-m		
Phi	0.8	K8	1.00
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

### Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

### Capacities

PhiNcx Wind	298.50 Kn	PhiMnx Wind	12.23 Kn-m	PhiVnx Wind	36.81 Kn
PhiNcx Dead	179.10 Kn	PhiMnx Dead	7.34 Kn-m	PhiVnx Dead	22.09 Kn

### Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.40 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.17 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 24.13 mm < 33.60 mm

## **Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

### **Assumed Soil Properties**

Gamma 18 Kn/m<sup>3</sup>                      Friction angle 30 deg    Cohesion 0 Kn/m<sup>3</sup>  
K<sub>0</sub> =  $(1 - \sin(30)) / (1 + \sin(30))$   
K<sub>p</sub> =  $(1 + \sin(30)) / (1 - \sin(30))$

### **Geometry For Middle Bay Pole**

D<sub>s</sub> = 0.6 mm                      Pile Diameter  
L = 1300 mm                      Pile embedment length  
f<sub>1</sub> = 2738 mm                      Distance at which the shear force is applied  
f<sub>2</sub> = 0 mm                      Distance of top soil at rest pressure

### **Loads**

Moment Wind = 4.44 Kn-m  
Shear Wind = 1.62 Kn

### **Pile Properties**

Safety Factory 0.55  
H<sub>u</sub> = 4.85 Kn                      Ultimate Lateral Strength of the Pile, Short pile  
M<sub>u</sub> = 7.86 Kn-m                      Ultimate Moment Capacity of Pile

### **Checks**

Applied Forces/Capacities = 0.56 < 1 OK

## **Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K<sub>s</sub> (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1300) x K<sub>s</sub>(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1300)

Skin Friction = 13.65 Kn

Weight of Pile + Pile Skin Friction = 17.91 Kn

Uplift on one Pile = 8.14 Kn

Uplift is ok