

**Job No.:** Jeremy Croft

**Address:** 477 Crane Rd, Kauri, New Zealand

**Date:** 12/01/2024

**Latitude:** -35.645642

**Longitude:** 174.277507

**Elevation:** 150.5 m

### General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.35 m
Wind Region	NZ1	Terrain Category	2.59	Design Wind Speed	39.46 m/s
Wind Pressure	0.93 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

### Pressure Coefficients and Pressures

Shed Type = Gable Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 4.35 m  $C_{p,e} = -0.9$   $p_e = -0.76$  KPa  $p_{net} = -0.76$  KPa

For roof  $C_{p,e}$  from 4.35 m To 8.70 m  $C_{p,e} = -0.5$   $p_e = -0.42$  KPa  $p_{net} = -0.42$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 11 m  $C_{p,e} = 0.7$   $p_e = 0.59$  KPa  $p_{net} = 0.87$  KPa

For side wall  $C_{p,e}$  from 0 m To 4.35 m  $C_{p,e} =$   $p_e = -0.55$  KPa  $p_{net} = -0.55$  KPa

Maximum Upward pressure used in roof member Design = 0.76 KPa

Maximum Downward pressure used in roof member Design = 0.43 KPa

Maximum Wall pressure used in Design = 0.87 KPa

Maximum Racking pressure used in Design = 1.01 KPa

### Design Summary

#### Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 5850 mm

Try Purlin 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.97

K8 Upward = 0.54 S1 Downward = 12.68 S1 Upward = 22.76

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{1.35D}$	1.3 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	<b>261.54 %</b>
$M_{1.2D+1.5L\ 1.2D+S_n\ 1.2D+W_nDn}$	2.81 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	<b>161.21 %</b>
$M_{0.9D-W_nUp}$	-2.06 Kn-m	Capacity	-3.16 Kn-m	Passing Percentage	<b>153.40 %</b>

Pole Shed App Ver 01 2022

V <sub>1.35D</sub>	0.89 Kn	Capacity	12.06 Kn	Passing Percentage	<b>1355.06 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	1.92 Kn	Capacity	16.08 Kn	Passing Percentage	<b>837.50 %</b>
V <sub>0.9D-WnUp</sub>	-1.41 Kn	Capacity	-20.10 Kn	Passing Percentage	<b>1425.53 %</b>

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 18.24 mm

Limit by Woolcock et al, 1999 Span/240 = 24.17 mm

Deflection under Dead and Service Wind = 21.74 mm

Limit by Woolcock et al, 1999 Span/100 = 58.00 mm

**Reactions**

Maximum downward = 1.92 kn Maximum upward = -1.41 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

**Rafter Design Internal**

Internal Rafter Load Width = 6000 mm

Internal Rafter Span = 4850 mm

Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1 K<sub>1</sub> Medium term = 0.8 K<sub>1</sub> Long term = 0.6 K<sub>4</sub> = 1 K<sub>5</sub> = 1 K<sub>8</sub> Downward = 1.00

K<sub>8</sub> Upward = 1.00 S<sub>1</sub> Downward = 6.81 S<sub>1</sub> Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>1.35D</sub>	5.95 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	<b>169.41 %</b>
M <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	12.88 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	<b>104.35 %</b>
M <sub>0.9D-WnUp</sub>	-9.44 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	<b>177.97 %</b>
V <sub>1.35D</sub>	4.91 Kn	Capacity	28.94 Kn	Passing Percentage	<b>589.41 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	10.62 Kn	Capacity	38.6 Kn	Passing Percentage	<b>363.47 %</b>
V <sub>0.9D-WnUp</sub>	-7.78 Kn	Capacity	-48.24 Kn	Passing Percentage	<b>620.05 %</b>

**Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 10.85 mm

Limit by Woolcock et al, 1999 Span/240 = 20.83 mm

Deflection under Dead and Service Wind = 14.365 mm

Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

**Reactions**

Maximum downward = 10.62 kn Maximum upward = -7.78 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Second page

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$   $f_{pj} = 12.9$  Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

$K_{11} = 2.0$   $f_{cj} = 36.1$  Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -7.78 Kn

### **Girt Design Front and Back**

Girt's Spacing = 900 mm

Girt's Span = 6000 mm

Try Girt 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1  $K_4 = 1$   $K_5 = 1$   $K_8$  Downward = 0.97

$K_8$  Upward = 0.72  $S_1$  Downward = 12.68  $S_1$  Upward = 18.90

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### **Capacity Checks**

$M_{Wind+Snow}$	3.52 Kn-m	Capacity	4.22 Kn-m	Passing Percentage	<b>119.89 %</b>
$V_{0.9D-WnUp}$	2.35 Kn-m	Capacity	20.10 Kn-m	Passing Percentage	<b>855.32 %</b>

### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 30.29 mm

Limit by Woolcock et al, 1999 Span/100 = 60.00 mm

Sag during installation = 78.58 mm

### **Reactions**

Maximum = 2.35 kn

### **Girt Design Sides**

Girt's Spacing = 1300 mm

Girt's Span = 5000 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1  $K_4 = 1$   $K_5 = 1$   $K_8$  Downward = 1.00

$K_8$  Upward = 0.96  $S_1$  Downward = 11.27  $S_1$  Upward = 13.28

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### **Capacity Checks**

$M_{Wind+Snow}$	3.53 Kn-m	Capacity	3.57 Kn-m	Passing Percentage	<b>101.13 %</b>
$V_{0.9D-WnUp}$	2.83 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	<b>568.20 %</b>

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 41.21 mm

Limit by Woolcock et al. 1999 Span/100 = 50.00 mm

Sag during installation = 37.90 mm

### Reactions

Maximum = 2.83 kn

### Middle Pole Design

#### Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	4400 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
I <sub>x</sub>	100042702 mm <sup>4</sup>	Z <sub>x</sub>	941578 mm <sup>3</sup>
I <sub>y</sub>	100042702 mm <sup>4</sup>	Z <sub>y</sub>	941578 mm <sup>3</sup>
Lateral Restraint	4400 mm c/c		

#### Loads

Total Area over Pole = 30 m<sup>2</sup>

Dead	7.50 Kn	Live	7.50 Kn
Wind Down	12.90 Kn	Snow	0.00 Kn
Moment wind	14.30 Kn-m		
Phi	0.8	K <sub>8</sub>	0.64
K <sub>1</sub> snow	0.8	K <sub>1</sub> Dead	0.6
K <sub>1</sub> wind	1		

#### Material

Peeling	Steaming	Normal	Dry Use
f <sub>b</sub> =	36.3 MPa	f <sub>s</sub> =	2.96 MPa
f <sub>c</sub> =	18 MPa	f <sub>p</sub> =	7.2 MPa
f <sub>t</sub> =	22 MPa	E =	9257 MPa

#### Capacities

PhiN <sub>c</sub> Wind	324.48 Kn	PhiM <sub>n</sub> Wind	17.38 Kn-m	PhiV <sub>n</sub> Wind	62.96 Kn
PhiN <sub>c</sub> Dead	194.69 Kn	PhiM <sub>n</sub> Dead	10.43 Kn-m	PhiV <sub>n</sub> Dead	37.77 Kn

#### Checks

$$(M_x/\Phi M_n) + (N/\Phi N_c) = 0.91 < 1 \text{ OK}$$

$$(M_x/\Phi M_n)^2 + (N/\Phi N_c) = 0.76 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 41.52 mm < 44.00 mm

### Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

#### Assumed Soil Properties

Pole Shed App Ver 01 2022

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For Middle Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	4000 mm	Pile embedment length
f1 =	3262 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	14.30 Kn-m
Shear Wind =	4.38 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	78.52 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	177.67 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.08 < 1 OK

**End Pole Design**

**Geometry For End Bay Pole**

**Geometry**

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	4050 mm
Area	35448 mm2	As	26585.7421875 mm2
Ix	100042702 mm4	Zx	941578 mm3
Iy	100042702 mm4	Zx	941578 mm3
Lateral Restraint	mm c/c		

**Loads**

Total Area over Pole = 15 m2

Dead	3.75 Kn	Live	3.75 Kn
Wind Down	6.45 Kn	Snow	0.00 Kn
Moment Wind	7.15 Kn-m		
Phi	0.8	K8	0.72
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa

ft = 22 MPa E = 9257 MPa

**Capacities**

PhiNcx Wind	365.76 Kn	PhiMnx Wind	19.59 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	219.46 Kn	PhiMnx Dead	11.76 Kn-m	PhiVnx Dead	37.77 Kn

**Checks**

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.40 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.17 < 1$  OK

Deflection at top under service lateral loads = 20.47 mm < 43.39 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3262 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Total Area over Pole = 15 m<sup>2</sup>

Moment Wind =	7.15 Kn-m
Shear Wind =	2.19 Kn

**Pile Properties**

Safety Factor	0.55	
Hu =	4.29 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.17 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.88 < 1 OK

**Drained Lateral Strength of End pile in cohesionless soils Free Head short pile****Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For End Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3262 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	7.15 Kn-m
Shear Wind =	2.19 Kn

**Pile Properties**

Safety Factor	0.55	
$H_u =$	4.29 Kn	Ultimate Lateral Strength of the Pile, Short pile
$M_u =$	8.17 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.88 < 1 OK

**Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

$K_s$  (Lateral Earth Pressure Coefficient) for cast in place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil (18) x Height of Pile (4000) x  $K_s$  (1.5) x 0.5 x  $\tan(30)$  x  $\pi$  x Dia of Pile (0.6) x Height of Pile (4000)

Skin Friction = 129.22 Kn

Weight of Pile + Pile Skin Friction = 139.61 Kn

Uplift on one Pile = 16.05 Kn

Uplift is ok