Date: **13/06/2024**

Council: **Waipa Council**

BWhite Consulting Ltd

Subject: B2 compliance in respect of Proposed shed at 590 Collinson Road, Pirongia, New Zealand

Waipa Council typically requests a Producer Statement/Other means of compliance for Design for Clause B2 of the Building Code-Durability

We are not able to provide a Producer Statement for durability because compliance needs to be shown on material-by-material basis using a variety of compliance methods, and not all materials used have a clear compliance path.

We can confirm that for the structural elements shown in our documentation under Clause B1:

Timber

Timber treatment has been selected to meet or exceed the requirements of table 1A of B2/AS1 and NZS3602

Steel fixing

Steel fixings are protected against weather as per table 4.1 and 4.2 of NZS3604-2011. Exposure Zone B

Yours Faithfully

BWhite CONSULTING LTD

Bevan Whiite

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Note: This letter shall only be relied on by the Building Consent Authority named in Engineering New Zealand/ACE New Zealand Producer Statement PS1(B1) - Design in relation to the Building Work. Liability under this letter accrues to the Design Review Firm only. The total maximum amount of damages payable arising from this letter and all other statements provided to the Building Consent Authority in relation to this Building Work whether in contract, tort or otherwise (including negligence), is limited to the sum of $200,000

Job No.:	Collinson Street	**Address:**	590 Collinson Road, Pirongia, New Zealand	**Date:**	13/06/2024
Latitude:	-37.998023	**Longitude:**	175.207812	**Elevation:**	33.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	5 m
Wind Region	NZ1	Terrain Category	2.01	Design Wind Speed	38.41 m/s
Wind Pressure	0.89 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Mono Enclosed

For roof Cp,i = -0.3

For roof CP,e from 0 m To 2.25 m Cpe = -1.0 pe = -0.80 KPa pnet = -0.80 KPa

For roof CP,e from 2.25 m To 4.50 m Cpe = -0.5 pe = -0.40 KPa pnet = -0.40 KPa

For wall Windward Cp,i = -0.3 side Wall Cp,i = -0.3

For wall Windward and Leeward CP,e from 0 m To 4.5 m Cpe = 0.7 pe = 0.56 KPa pnet = 0.83 KPa

For side wall CP,e from 0 m To 4.50 m Cpe = pe = -0.52 KPa pnet = -0.52 KPa

Maximum Upward pressure used in roof member Design = 0.80 KPa

Maximum Downward pressure used in roof member Design = 0.31 KPa

Maximum Wall pressure used in Design = 0.83 KPa

Maximum Racking pressure used in Design = 0.80 KPa

Design Summary

Rafter Design Internal

Internal Rafter Load Width = 4000 mm Internal Rafter Span = 4350 mm Try Rafter 2x250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =6.13 S1 Upward =6.13

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	3.19 Kn-m	Capacity	7 Kn-m	Passing Percentage	**219.44 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	6.39 Kn-m	Capacity	9.34 Kn-m	Passing Percentage	**146.17 %**
$M_{0.9D\text{-}WnUp}$	-5.44 Kn-m	Capacity	-11.66 Kn-m	Passing Percentage	**214.34 %**

$V_{1.35D}$	2.94 Kn	Capacity	24.12 Kn	Passing Percentage	**820.41 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	5.87 Kn	Capacity	32.16 Kn	Passing Percentage	**547.87 %**
$V_{0.9D-WnUp}$	-5.00 Kn	Capacity	-40.2 Kn	Passing Percentage	**804.00 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 8.2 mm — Limit by Woolcock et al, 1999 Span/240 = 18.75 mm

Deflection under Dead and Service Wind = 9.95 mm — Limit by Woolcock et al, 1999 Span/100 = 45.00 mm

Reactions

Maximum downward =5.87 kn   Maximum upward = -5.00 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -5.00 Kn

Rafter Design External

External Rafter Load Width = 2000 mm   External Rafter Span = 4317 mm   Try Rafter 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =0.97

K8 Upward =0.97   S1 Downward =12.68   S1 Upward =12.68

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	1.57 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	**216.56 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	3.14 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	**144.27 %**
$M_{0.9D-WnUp}$	-2.68 Kn-m	Capacity	-5.67 Kn-m	Passing Percentage	**211.57 %**
$V_{1.35D}$	1.46 Kn	Capacity	12.06 Kn	Passing Percentage	**826.03 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	2.91 Kn	Capacity	16.08 Kn	Passing Percentage	**552.58 %**
$V_{0.9D-WnUp}$	-2.48 Kn	Capacity	-20.10 Kn	Passing Percentage	**810.48 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 9.11 mm	Limit by Woolcock et al, 1999 Span/240= 18.75 mm
Deflection under Dead and Service Wind = 9.95 mm	Limit by Woolcock et al, 1999 Span/100 = 45.00 mm

Reactions

Maximum downward =2.91 kn   Maximum upward = -2.48 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds .... (Eq 4.12) = -19.95 kn > -2.48 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.48 Kn

Girt Design Front and Back

Girt's Spacing = 800 mm	Girt's Span = 4000 mm	Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =0.65   S1 Downward =9.63   S1 Upward =20.31

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.33 Kn-m	Capacity	1.38 Kn-m	Passing Percentage	**103.76 %**
$V_{0.9D-WnUp}$	1.33 Kn	Capacity	12.06 Kn	Passing Percentage	**906.77 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 23.49 mm	Limit by Woolcock et al, 1999 Span/100 = 40.00 mm
Sag during installation = 15.52 mm	

Reactions

Maximum = 1.33 kn

Girt Design Sides

Girt's Spacing = 800 mm	Girt's Span = 4500 mm	Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.89 S1 Downward =9.63 S1 Upward =15.23

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.68 Kn-m	Capacity	1.87 Kn-m	Passing Percentage	**111.31 %**
$V_{0.9D-WnUp}$	1.49 Kn	Capacity	12.06 Kn	Passing Percentage	**809.40 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 37.63 mm	Limit by Woolcock et al. 1999 Span/100 = 45.00 mm
Sag during installation =24.86 mm	

Reactions

Maximum = 1.49 kn

Middle Pole Design

Geometry

225 UNI H5	Dry Use	Height	4700 mm
Area	39741 mm2	As	29805.46875 mm2
Ix	125741821 mm4	Zx	1117705 mm3
Iy	125741821 mm4	Zx	1117705 mm3
Lateral Restraint	4700 mm c/c		

Loads

Total Area over Pole = 9 m2

Dead	2.25 Kn	Live	2.25 Kn
Wind Down	2.79 Kn	Snow	0.00 Kn
Moment wind	14.96 Kn-m		
Phi	0.8	K8	0.63
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Shaving	Steaming	Normal	Dry Use
fb =	34.325 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	20.75 MPa	E =	8793 MPa

Capacities

PhiNcx Wind	358.75 Kn	PhiMnx Wind	19.24 Kn-m	PhiVnx Wind	70.58 Kn
PhiNcx Dead	215.25 Kn	PhiMnx Dead	11.54 Kn-m	PhiVnx Dead	42.35 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.80 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.63 < 1 OK

Deflection at top under service lateral loads = 44.68 mm < 47.00 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1650 mm	Pile embedment length
f1 =	3750 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	14.96 Kn-m
Shear Wind =	3.99 Kn

Pile Properties

Safety Factory	0.55	
Hu =	7.41 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	16.35 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.91 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

200 UNI H5	Dry Use	Height	4750 mm
Area	31400 mm2	As	23550 mm2
Ix	78500000 mm4	Zx	785000 mm3
Iy	78500000 mm4	Zx	785000 mm3
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 9 m2

Dead	2.25 Kn	Live	2.25 Kn
Wind Down	2.79 Kn	Snow	0.00 Kn
Moment Wind	7.48 Kn-m		
Phi	0.8	K8	0.50
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Shaving	Steaming	Normal	Dry Use
fb =	34.325 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	20.75 MPa	E =	8793 MPa

Capacities

PhiNcx Wind	227.28 Kn	PhiMnx Wind	10.84 Kn-m	PhiVnx Wind	55.77 Kn
PhiNcx Dead	136.37 Kn	PhiMnx Dead	6.50 Kn-m	PhiVnx Dead	33.46 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.72 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.51 < 1 OK

Deflection at top under service lateral loads = 37.98 mm < 49.88 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3750 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 9 m2

Moment Wind =	7.48 Kn-m
Shear Wind =	2.00 Kn

Pile Properties

Safety Factory	0.55	
Hu =	3.88 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.40 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.89 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3750 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	7.48 Kn-m
Shear Wind =	2.00 Kn

Pile Properties

Safety Factory	0.55	
Hu =	3.88 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.40 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.89 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(1650) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1650)

Skin Friction = 21.99 Kn

Weight of Pile + Pile Skin Friction = 26.02 Kn

Uplift on one Pile = 5.18 Kn

Uplift is ok