

Pole Shed App Ver 01 2022

Job No.: 166 Millar - 1

Address: 166 Millar Way, Mahurangi East, New Zealand

Date: 09/12/2024

Latitude: -36.464636

Longitude: 174.746802

Elevation: 37.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	D
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	6 m
Wind Region	NZ1	Terrain Category	1.04	Design Wind Speed	55.1 m/s
Wind Pressure	1.83 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	extra High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 6.0 m $C_{p,e} = -1.1472$ $p_e = -1.89$ KPa $p_{net} = -1.89$ KPa

For roof $C_{p,e}$ from m To m $C_{p,e} =$ $p_e =$ KPa $p_{net} =$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 00 m To 6 m $C_{p,e} = 0.7$ $p_e = 1.15$ KPa $p_{net} = 1.70$ KPa

For side wall $C_{p,e}$ from 0 m To 5.40 m $C_{p,e} =$ $p_e = -1.07$ KPa $p_{net} = -1.07$ KPa

Maximum Upward pressure used in roof member Design = 1.89 KPa

Maximum Downward pressure used in roof member Design = 0.30 KPa

Maximum Wall pressure used in Design = 1.70 KPa

Maximum Racking pressure used in Design = 1.64 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 3600 mm

Try Purlin 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

Second page

Pole Shed App Ver 01 2022

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.87 S1 Downward = 11.27 S1 Upward = 15.83

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.49 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	455.10 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.43 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	207.69 %
M _{0.9D-W_nUp}	-2.43 Kn-m	Capacity	-3.24 Kn-m	Passing Percentage	191.72 %
V _{1.35D}	0.55 Kn	Capacity	9.65 Kn	Passing Percentage	1754.55 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.09 Kn	Capacity	12.86 Kn	Passing Percentage	1179.82 %
V _{0.9D-W_nUp}	-2.70 Kn	Capacity	-16.08 Kn	Passing Percentage	595.56 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 5.00 mm Limit by Woolcock et al, 1999 Span/240 = 14.79 mm

Deflection under Dead and Service Wind = 5.42 mm Limit by Woolcock et al, 1999 Span/100 = 35.50 mm

Reactions

Maximum downward = 1.09 kn Maximum upward = -2.70 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Girt Design Front and Back

Girt's Spacing = 900 mm

Girt's Span = 3750 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.85 S1 Downward = 11.27 S1 Upward = 16.27

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+S_now}	2.69 Kn-m	Capacity	3.17 Kn-m	Passing Percentage	117.84 %
V _{0.9D-W_nUp}	2.87 Kn	Capacity	16.08 Kn	Passing Percentage	560.28 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 17.64 mm Limit by Woolcock et al, 1999 Span/100 = 37.50 mm
Sag during installation = 11.99 mm

Reactions

Maximum = 2.87 kn

Girt Design Sides

Girt's Spacing = 900 mm Girt's Span = 3000 mm Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.98 S1 Downward =9.63 S1 Upward =12.44

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.72 Kn-m	Capacity	2.05 Kn-m	Passing Percentage	119.19 %
V _{0.9D-WnUp}	2.29 Kn	Capacity	12.06 Kn	Passing Percentage	526.64 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 17.13 mm Limit by Woolcock et al. 1999 Span/100 = 30.00 mm
Sag during installation =4.91 mm

Reactions

Maximum = 2.29 kn

Middle Pole Design

Geometry

300 SED H5 (Minimum 325 dia. at Floor Level)	Dry Use	Height	5700 mm
Area	76660 mm ²	As	57495.1171875 mm ²

Pole Shed App Ver 01 2022

Ix	467896461 mm ⁴	Zx	2994537 mm ³
Iy	467896461 mm ⁴	Zx	2994537 mm ³
Lateral Restraint	5700 mm c/c		

Loads

Total Area over Pole = 11.25 m²

Dead	2.81 Kn	Live	2.81 Kn
Wind Down	3.38 Kn	Snow	0.00 Kn
Moment wind	41.41 Kn-m		
Phi	0.8	K8	0.76
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	834.98 Kn	PhiMnx Wind	65.78 Kn-m	PhiVnx Wind	136.15 Kn
PhiNcx Dead	500.99 Kn	PhiMnx Dead	39.47 Kn-m	PhiVnx Dead	81.69 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.64 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.41 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 45.94 \text{ mm} < 57.00 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	2300 mm	Pile embedment length
f1 =	4500 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	41.41 Kn-m
Shear Wind =	9.20 Kn

Pile Properties

Safety Factory	0.55	
Hu =	15.94 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	42.82 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.97 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(2300) x Ks(1.5) x $0.5 \times \tan(30)$ x Pi x Dia of Pile(0.6) x Height of Pile(2300)

Skin Friction = 42.72 Kn

Weight of Pile + Pile Skin Friction = 46.20 Kn

Uplift on one Pile = 18.73 Kn

Uplift is ok