

Pole Shed App Ver 01 2022

Job No.: 92 Babylon Coast Road Dargaville **Address:** 92 Babylon Coast Road, Dargaville, New Zealand **Date:** 11/22/2023
Latitude: -35.906634 **Longitude:** 173.8002 **Elevation:** 17.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.2 m
Wind Region	NZ1	Terrain Category	2.0	Design Wind Speed	41.31 m/s
Wind Pressure	1.02 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = 0.6555$

For roof $C_{p,e}$ from 0 m To 3.90 m $C_{p,e} = -0.9$ $p_e = -0.80$ KPa $p_{net} = -0.19$ KPa

For roof $C_{p,e}$ from 3.90 m To 7.80 m $C_{p,e} = -0.5$ $p_e = -0.44$ KPa $p_{net} = 0.17$ KPa

For wall Windward $C_{p,i} = 0.6555$ side Wall $C_{p,i} = 0.5674$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 21.0 m $C_{p,e} = 0.7$ $p_e = 0.62$ KPa $p_{net} = 1.23$ KPa

For side wall $C_{p,e}$ from 0 m To 3.90 m $C_{p,e} =$ $p_e = -0.58$ KPa $p_{net} = 0.03$ KPa

Maximum Upward pressure used in roof member Design = 1.37 KPa

Maximum Downward pressure used in roof member Design = 0.79 KPa

Maximum Wall pressure used in Design = 1.23 KPa

Maximum Racking pressure used in Design = 1.11 KPa

Design Summary

Purlin Design

Purlin Spacing = 750 mm Purlin Span = 4050 mm Try Purlin 200x50 SG8 Dry

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.50 S1 Downward = 11.27 S1 Upward = 23.76

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.52 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	428.85 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.68 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	176.79 %
M _{0.9D-W_nUp}	-1.76 Kn-m	Capacity	-1.87 Kn-m	Passing Percentage	106.25 %
V _{1.35D}	0.51 Kn	Capacity	9.65 Kn	Passing Percentage	1892.16 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.66 Kn	Capacity	12.86 Kn	Passing Percentage	774.70 %
V _{0.9D-W_nUp}	-1.74 Kn	Capacity	-16.08 Kn	Passing Percentage	924.14 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 6.72 mm Limit by Woolcock et al, 1999 Span/240 = 16.67 mm

Deflection under Dead and Service Wind = 10.02 mm Limit by Woolcock et al, 1999 Span/100 = 40.00 mm

Reactions

Maximum downward = 1.66 kn Maximum upward = -1.74 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4200 mm Internal Rafter Span = 4350 mm Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.81 S1 Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{1.35D}	3.35 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	300.90 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	10.83 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	124.10 %
M _{0.9D-WnUp}	-11.37 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	147.76 %
V _{1.35D}	3.08 Kn	Capacity	28.94 Kn	Passing Percentage	939.61 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	9.96 Kn	Capacity	38.6 Kn	Passing Percentage	387.55 %
V _{0.9D-WnUp}	-10.46 Kn	Capacity	-48.24 Kn	Passing Percentage	461.19 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 4.985 mm Limit by Woolcock et al, 1999 Span/240 = 18.75 mm

Deflection under Dead and Service Wind = 8.26 mm Limit by Woolcock et al, 1999 Span/100 = 45.00 mm

Reactions

Maximum downward = 9.96 kn Maximum upward = -10.46 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K₁₁ = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -10.46 Kn

Rafter Design External

External Rafter Load Width = 2100 mm External Rafter Span = 4310 mm Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.94

K8 Upward = 0.94 S1 Downward = 13.93 S1 Upward = 13.93

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.65 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	286.06 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	5.32 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	118.42 %
M _{0.9D-W_nUp}	-5.58 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	141.04 %
V _{1.35D}	1.53 Kn	Capacity	14.47 Kn	Passing Percentage	945.75 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	4.93 Kn	Capacity	19.30 Kn	Passing Percentage	391.48 %
V _{0.9D-W_nUp}	-5.18 Kn	Capacity	-24.12 Kn	Passing Percentage	465.64 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 5.54 mm Limit by Woolcock et al, 1999 Span/240 = 18.75 mm

Deflection under Dead and Service Wind = 8.26 mm Limit by Woolcock et al, 1999 Span/100 = 45.00 mm

Reactions

Maximum downward = 4.93 kn Maximum upward = -5.18 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

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$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -25.20 kn > -5.18 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -5.18 Kn

Girt Design Front and Back

Girt's Spacing = 600 mm

Girt's Span = 4200 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.91 S1 Downward =9.63 S1 Upward =14.71

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.63 Kn-m	Capacity	1.91 Kn-m	Passing Percentage	117.18 %
$V_{0.9D-WnUp}$	1.55 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	778.06 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 31.74 mm Limit by Woolcock et al, 1999 Span/100 = 42.00 mm

Sag during installation = 18.87 mm

Reactions

Maximum = 1.55 kn

Girt Design Sides

Girt's Spacing = 600 mm

Girt's Span = 4500 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.89 S1 Downward =9.63 S1 Upward =15.23

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{Wind+Snow}	1.87 Kn-m	Capacity	1.87 Kn-m	Passing Percentage	100.00 %
V _{0.9D-WnUp}	1.66 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	726.51 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 41.82 mm Limit by Woolcock et al. 1999 Span/100 = 45.00 mm
Sag during installation = 24.86 mm

Reactions

Maximum = 1.66 kn

Middle Pole Design

Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3900 mm
Area	35448 mm ²	As	26585.7421875 mm ²
I _x	100042702 mm ⁴	Z _x	941578 mm ³
I _y	100042702 mm ⁴	Z _y	941578 mm ³
Lateral Restraint	3400 mm c/c		

Loads

Total Area over Pole = 18.9 m²

Dead	4.72 Kn	Live	4.72 Kn
Wind Down	14.93 Kn	Snow	0.00 Kn
Moment wind	10.25 Kn-m		
Phi	0.8	K ₈	0.86
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

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PhiNcx Wind	438.78 Kn	PhiMnx Wind	23.50 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	263.27 Kn	PhiMnx Dead	14.10 Kn-m	PhiVnx Dead	37.77 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.49 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.25 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 25.48 \text{ mm} < 39.00 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K ₀ =	$(1 - \sin(30)) / (1 + \sin(30))$				
K _p =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

D _s =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f ₁ =	3150 mm	Distance at which the shear force is applied
f ₂ =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	10.25 Kn-m
Shear Wind =	3.26 Kn

Pile Properties

Safety Factory	0.55	
H _u =	6.47 Kn	Ultimate Lateral Strength of the Pile, Short pile
M _u =	12.07 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.85 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

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Geometry

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	3900 mm
Area	27598 mm ²	As	20698.2421875 mm ²
Ix	60639381 mm ⁴	Zx	646820 mm ³
Iy	60639381 mm ⁴	Zy	646820 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 9.45 m²

Dead	2.36 Kn	Live	2.36 Kn
Wind Down	7.47 Kn	Snow	0.00 Kn
Moment Wind	5.13 Kn-m		
Phi	0.8	K8	0.63
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	250.93 Kn	PhiMnx Wind	11.86 Kn-m	PhiVnx Wind	49.01 Kn
PhiNcx Dead	150.56 Kn	PhiMnx Dead	7.12 Kn-m	PhiVnx Dead	29.41 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.48 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.24 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 22.58 \text{ mm} < 41.90 \text{ mm}$$

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	3150 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 9.45 m²

Moment Wind = 5.13 Kn-m

Shear Wind = 1.63 Kn

Pile Properties

Safety Factory 0.55

Hu = 6.47 Kn Ultimate Lateral Strength of the Pile, Short pile

Mu = 12.07 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.42 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³

K0 = $(1 - \sin(30)) / (1 + \sin(30))$

Kp = $(1 + \sin(30)) / (1 - \sin(30))$

Geometry For End Bay Pole

Ds = 0.6 mm Pile Diameter

L = 1500 mm Pile embedment length

f1 = 3150 mm Distance at which the shear force is applied

f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 5.13 Kn-m

Shear Wind = 1.63 Kn

Pile Properties

Safety Factory 0.55

Hu = 6.47 Kn Ultimate Lateral Strength of the Pile, Short pile

Mu = 12.07 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.42 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m^3

Density of Timber Pole = 5 Kn/m^3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1500) x K_s (1.5) x $0.5 \times \tan(30) \times \pi \times \text{Dia of Pile}(0.6) \times \text{Height of Pile}(1500)$

Skin Friction = 18.17 Kn

Weight of Pile + Pile Skin Friction = 22.07 Kn

Uplift on one Pile = 21.64 Kn

Uplift is ok