

Pole Shed App Ver 01 2022

Job No.: Edwards

Address: 43 Rata Road, Whakatiwai, New Zealand

Date: 02/12/2024

Latitude: -37.087739

Longitude: 175.29588

Elevation: 8 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N1	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	D
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.1 m
Wind Region	NZ1	Terrain Category	2.33	Design Wind Speed	38.32 m/s
Wind Pressure	0.88 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Open

For roof $C_{p,i} = 0.6876$

For roof $C_{p,e}$ from 0 m To 2.75 m $C_{p,e} = -0.9$ $p_e = -0.57$ KPa $p_{net} = -1.09$ KPa

For roof $C_{p,e}$ from 2.75 m To 5.50 m $C_{p,e} = -0.5$ $p_e = -0.32$ KPa $p_{net} = -0.84$ KPa

For wall Windward $C_{p,i} = 0.6876$ side Wall $C_{p,i} = -0.6271$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 7 m $C_{p,e} = 0.7$ $p_e = 0.55$ KPa $p_{net} = 1.14$ KPa

For side wall $C_{p,e}$ from 0 m To 2.75 m $C_{p,e} =$ $p_e = -0.51$ KPa $p_{net} = 0.08$ KPa

Maximum Upward pressure used in roof member Design = 1.09 KPa

Maximum Downward pressure used in roof member Design = 0.67 KPa

Maximum Wall pressure used in Design = 1.14 KPa

Maximum Racking pressure used in Design = 0.96 KPa

Design Summary

Purlin Design

Purlin Spacing = 700 mm

Purlin Span = 3350 mm

Try Purlin 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

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K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.68 S1 Downward = 10.36 S1 Upward = 19.84

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.33 Kn-m	Capacity	0.99 Kn-m	Passing Percentage	300.00 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	1.22 Kn-m	Capacity	1.32 Kn-m	Passing Percentage	108.20 %
M _{0.9D-WnUp}	-0.85 Kn-m	Capacity	-1.12 Kn-m	Passing Percentage	131.76 %
V _{1.35D}	0.40 Kn	Capacity	6.08 Kn	Passing Percentage	1520.00 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	1.14 Kn	Capacity	8.10 Kn	Passing Percentage	710.53 %
V _{0.9D-WnUp}	-1.01 Kn	Capacity	-10.13 Kn	Passing Percentage	1002.97 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 9.41 mm Limit by Woolcock et al, 1999 Span/240 = 13.75 mm

Deflection under Dead and Service Wind = 13.09 mm Limit by Woolcock et al, 1999 Span/100 = 33.00 mm

Reactions

Maximum downward = 1.14 kn Maximum upward = -1.01 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 3500 mm Internal Rafter Span = 6850 mm Try Rafter 2x240x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 4.59 S1 Upward = 4.59

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	6.93 Kn-m	Capacity	27.86 Kn-m	Passing Percentage	402.02 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	19.91 Kn-m	Capacity	37.16 Kn-m	Passing Percentage	186.64 %

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M _{0.9D-WnUp}	-17.76 Kn-m	Capacity	-46.44 Kn-m	Passing Percentage	261.49 %
V _{1.35D}	4.05 Kn	Capacity	51.54 Kn	Passing Percentage	1272.59 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	11.63 Kn	Capacity	68.72 Kn	Passing Percentage	590.89 %
V _{0.9D-WnUp}	-10.37 Kn	Capacity	-85.9 Kn	Passing Percentage	828.35 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 18.505 mm Limit by Woolcock et al, 1999 Span/240 = 29.17 mm

Deflection under Dead and Service Wind = 28.61 mm Limit by Woolcock et al, 1999 Span/100 = 70.00 mm

Reactions

Maximum downward = 11.63 kn Maximum upward = -10.37 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K₁₁ = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -10.37 Kn

Rafter Design External

External Rafter Load Width = 1750 mm External Rafter Span = 3318 mm Try Rafter 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 0.98

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K8 Upward =0.98 S1 Downward =12.23 S1 Upward =12.23

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.81 Kn-m	Capacity	1.79 Kn-m	Passing Percentage	220.99 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.34 Kn-m	Capacity	2.38 Kn-m	Passing Percentage	101.71 %
M _{0.9D-W_nUp}	-2.08 Kn-m	Capacity	-2.98 Kn-m	Passing Percentage	143.27 %
V _{1.35D}	0.98 Kn	Capacity	8.25 Kn	Passing Percentage	841.84 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.82 Kn	Capacity	11.00 Kn	Passing Percentage	390.07 %
V _{0.9D-W_nUp}	-2.51 Kn	Capacity	-13.75 Kn	Passing Percentage	547.81 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 7.39 mm Limit by Woolcock et al, 1999 Span/240= 14.58 mm

Deflection under Dead and Service Wind = 10.28 mm Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

Reactions

Maximum downward =2.82 kn Maximum upward = -2.51 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k₁ x k₄ x k₅ x f_s x b x d_s (Eq 4.12) = -12.28 kn > -2.51 Kn

Single Shear Capacity under short term loads = -9.75 Kn > -2.51 Kn

Girt Design Front and Back

Girt's Spacing = 800 mm

Girt's Span = 3500 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.92 S1 Downward =10.36 S1 Upward =14.45

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.40 Kn-m	Capacity	1.51 Kn-m	Passing Percentage	107.86 %
V _{0.9D-WnUp}	1.60 Kn	Capacity	10.13 Kn	Passing Percentage	633.13 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 25.85 mm Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

Sag during installation = 11.23 mm

Reactions

Maximum = 1.60 kn

Girt Design Sides

Girt's Spacing = 800 mm

Girt's Span = 3500 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.92 S1 Downward =10.36 S1 Upward =14.45

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.40 Kn-m	Capacity	1.51 Kn-m	Passing Percentage	107.86 %
V _{0.9D-WnUp}	1.60 Kn	Capacity	10.13 Kn	Passing Percentage	633.13 %

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Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 25.85 mm Limit by Woolcock et al. 1999 Span/100 = 35.00 mm

Sag during installation = 11.23 mm

Reactions

Maximum = 1.60 kn

Middle Pole Design

Geometry

150 SED H5 (Minimum 175 dia. at Floor Level)	Dry Use	Height	2860 mm
Area	20729 mm ²	As	15546.6796875 mm ²
Ix	34210793 mm ⁴	Zx	421056 mm ³
Iy	34210793 mm ⁴	Zy	421056 mm ³
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 12.25 m²

Dead	3.06 Kn	Live	3.06 Kn
Wind Down	8.21 Kn	Snow	0.00 Kn
Moment wind	6.04 Kn-m		
Phi	0.8	K8	1.00
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	298.50 Kn	PhiMnx Wind	12.23 Kn-m	PhiVnx Wind	36.81 Kn
PhiNcx Dead	179.10 Kn	PhiMnx Dead	7.34 Kn-m	PhiVnx Dead	22.09 Kn

Checks

$$(M_x/\phi M_{nx}) + (N/\phi N_{cx}) = 0.54 < 1 \text{ OK}$$

$$(M_x/\phi M_{nx})^2 + (N/\phi N_{cx}) = 0.29 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 23.76 \text{ mm} < 28.60 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

Geometry For Middle Bay Pole

Ds = 0.6 mm Pile Diameter
L = 1300 mm Pile embedment length
f1 = 2325 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 6.04 Kn-m
Shear Wind = 2.60 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.40 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 7.57 Kn-m Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.80 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

Geometry

150 SED H5 (Minimum 175 dia. at Floor Level) Dry Use Height 2900 mm

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Area	20729 mm ²	As	15546.6796875 mm ²
Ix	34210793 mm ⁴	Zx	421056 mm ³
Iy	34210793 mm ⁴	Zy	421056 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 6.125 m²

Dead	1.53 Kn	Live	1.53 Kn
Wind Down	4.10 Kn	Snow	0.00 Kn
Moment Wind	2.01 Kn-m		
Phi	0.8	K8	0.78
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	231.52 Kn	PhiMnx Wind	9.48 Kn-m	PhiVnx Wind	36.81 Kn
PhiNcx Dead	138.91 Kn	PhiMnx Dead	5.69 Kn-m	PhiVnx Dead	22.09 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.24 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.08 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 8.56 \text{ mm} < 30.92 \text{ mm}$$

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2325 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 6.125 m²

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Moment Wind = 2.01 Kn-m
Shear Wind = 0.87 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.40 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 7.57 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.27 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³
K0 = $(1 - \sin(30)) / (1 + \sin(30))$
Kp = $(1 + \sin(30)) / (1 - \sin(30))$

Geometry For End Bay Pole

Ds = 0.6 mm Pile Diameter
L = 1300 mm Pile embedment length
f1 = 2325 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 2.01 Kn-m
Shear Wind = 0.87 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.40 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 7.57 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.27 < 1 OK

Uplift Check

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Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1300) x Ks(1.5) x $0.5 \times \tan(30) \times \pi \times \text{Dia of Pile}(0.6) \times \text{Height of Pile}(1300)$

Skin Friction = 13.65 Kn

Weight of Pile + Pile Skin Friction = 17.91 Kn

Uplift on one Pile = 10.60 Kn

Uplift is ok