

Pole Shed App Ver 01 2022

**Job No.:** Morris Workshop - 1    **Address:** 1195D Pohangina Road, Pohangina, New Zealand    **Date:** 10/5/2023  
**Latitude:** -40.176613    **Longitude:** 175.786834    **Elevation:** 187 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N1	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.8 m
Wind Region	NZ2	Terrain Category	2.4	Design Wind Speed	46.23 m/s
Wind Pressure	1.28 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Very High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Gable Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 4.50 m  $C_{p,e} = -0.962$   $p_e = -1.11$  KPa  $p_{net} = -1.11$  KPa

For roof  $C_{p,e}$  from 4.50 m To 9.0 m  $C_{p,e} = -0.531$   $p_e = -0.61$  KPa  $p_{net} = -0.61$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 9 m  $C_{p,e} = 0.7$   $p_e = 0.81$  KPa  $p_{net} = 1.19$  KPa

For side wall  $C_{p,e}$  from 0 m To 5.20 m  $C_{p,e} =$   $p_e = -0.75$  KPa  $p_{net} = -0.75$  KPa

Maximum Upward pressure used in roof member Design = 1.11 KPa

Maximum Downward pressure used in roof member Design = 0.61 KPa

Maximum Wall pressure used in Design = 1.19 KPa

Maximum Racking pressure used in Design = 1.16 KPa

**Design Summary**

**Rafter Design External**

External Rafter Load Width = 2300 mm    External Rafter Span = 8939 mm    Try Rafter 360x45 LVL13

### Pole Shed App Ver 01 2022

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 0.81

K8 Upward = 0.81    S1 Downward = 17.01    S1 Upward = 17.01

Shear Capacity of timber = 5.3 MPa    Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>1.35D</sub>	7.75 Kn-m	Capacity	17.70 Kn-m	Passing Percentage	<b>228.39 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	20.91 Kn-m	Capacity	23.60 Kn-m	Passing Percentage	<b>112.86 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-20.33 Kn-m	Capacity	-29.50 Kn-m	Passing Percentage	<b>145.11 %</b>
V <sub>1.35D</sub>	3.47 Kn	Capacity	27.61 Kn	Passing Percentage	<b>795.68 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	9.35 Kn	Capacity	36.82 Kn	Passing Percentage	<b>393.80 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-9.10 Kn	Capacity	-46.02 Kn	Passing Percentage	<b>505.71 %</b>

#### **Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 30.63 mm    Limit by Woolcock et al, 1999 Span/240 = 37.50 mm

Deflection under Dead and Service Wind = 41.09 mm    Limit by Woolcock et al, 1999 Span/100 = 90.00 mm

#### **Reactions**

Maximum downward = 9.35 kn    Maximum upward = -9.10 kn

#### **Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$  (Eq 4.12) = -50.09 kn > -9.10 Kn

Single Shear Capacity under short term loads = -14.56 Kn > -9.10 Kn

### **Girt Design Front and Back**

Girt's Spacing = 0 mm

Girt's Span = 2300 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =NaN

K8 Upward =NaN    S1 Downward =NaN    S1 Upward =NaN

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

### **Capacity Checks**

$M_{Wind+Snow}$	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	NaN %
$V_{0.9D-WnUp}$	0.00 Kn-m	Capacity	0.00 Kn-m	Passing Percentage	NaN %

### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm    Limit by Woolcock et al, 1999 Span/100 = 23.00 mm

Sag during installation = NaN mm

### **Reactions**

Maximum = 0.00 kn

### **Girt Design Sides**

Girt's Spacing = 0 mm

Girt's Span = 4500 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =NaN

K8 Upward =NaN    S1 Downward =NaN    S1 Upward =NaN

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

$M_{\text{Wind+Snow}}$	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	NaN %
$V_{0.9D-WnUp}$	0.00 Kn-m	Capacity	0.00 Kn-m	Passing Percentage	NaN %

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm      Limit by Woolcock et al. 1999 Span/100 = 45.00 mm  
Sag during installation = NaN mm

### Reactions

Maximum = 0.00 kn

### Uplift Check

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

$K_s$  (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(2100) x  $K_s$ (1.5) x  $0.5 \times \tan(30) \times \pi \times \text{Dia of Pile}(0.6) \times \text{Height of Pile}(2100)$

Skin Friction = 35.62 Kn

Weight of Pile + Pile Skin Friction = 39.84 Kn

Uplift on one Pile = 23.10 Kn

Uplift is ok