| | | |
|---|---|---|
| **Job No.:** Ormond Nurseries Enclosed Lean To | **Address:** 13 Rowley Crescent Grovetown, Grovetown, New Zealand | **Date:** 10/11/2023 |
| **Latitude:** -41.491377 | **Longitude:** 173.958501 | **Elevation:** 6 m |

**General Input**

| | | | | | |
|---|---|---|---|---|---|
| Roof Live Load | 0.25 KPa | Roof Dead Load | 0.25 KPa | Roof Live Point Load | 1.1 Kn |
| Snow Zone | N3 | Ground Snow Load | 0 KPa | Roof Snow Load | 0 KPa |
| Earthquake Zone | 3 | Subsoil Category | D | Exposure Zone | B |
| Importance Level | 1 | Ultimate wind & Earthquake ARI | 100 Years | Max Height | 6 m |
| Wind Region | NZ2 | Terrain Category | 2.0 | Design Wind Speed | 38.64 m/s |
| Wind Pressure | 0.9 KPa | Lee Zone | NO | Ultimate Snow ARI | 50 Years |
| Wind Category | High | Earthquake ARI | 100 | | |

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressues**

Shed Type = Mono Enclosed

For roof Cp,i = -0.3

For roof CP,e from 0 m To 5.55 m Cpe = -0.9 pe = -0.73 KPa pnet = -0.73 KPa

For roof CP,e from 5.55 m To 11.10 m Cpe = -0.5 pe = -0.40 KPa pnet = -0.40 KPa

For wall Windward Cp,i = -0.3 side Wall Cp,i = -0.3

For wall Windward and Leeward CP,e from 0 m To 11.90 m Cpe = 0.7 pe = 0.56 KPa pnet = 0.83 KPa

For side wall CP,e from 0 m To 5.55 m Cpe = pe = -0.52 KPa pnet = -0.52 KPa

Maximum Upward pressure used in roof member Design = 0.73 KPa

Maximum Downward pressure used in roof member Design = 0.43 KPa

Maximum Wall pressure used in Design = 0.83 KPa

Maximum Racking pressure used in Design = 0.96 KPa

**Design Summary**

**Purlin Design**

Purlin Spacing = 900 mm  Purlin Span = 5250 mm  Try Purlin 250x50 SG8 Dry

<u>Pole Shed App Ver 01 2022</u>

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.97

K8 Upward =0.31 S1 Downward =12.68 S1 Upward =30.47

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

| | | | | | |
|---|---|---|---|---|---|
| $M_{1.35D}$ | 1.05 Kn-m | Capacity | 3.40 Kn-m | Passing Percentage | **323.81 %** |
| $M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$ | 2.37 Kn-m | Capacity | 4.53 Kn-m | Passing Percentage | **191.14 %** |
| $M_{0.9D-WnUp}$ | -1.57 Kn-m | Capacity | -1.83 Kn-m | Passing Percentage | **116.56 %** |
| $V_{1.35D}$ | 0.80 Kn | Capacity | 12.06 Kn | Passing Percentage | **1507.50 %** |
| $V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$ | 1.72 Kn | Capacity | 16.08 Kn | Passing Percentage | **934.88 %** |
| $V_{0.9D-WnUp}$ | -1.19 Kn | Capacity | -20.10 Kn | Passing Percentage | **1689.08 %** |

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

| | |
|---|---|
| Deflection under Dead and Live Load = 11.79 mm | Limit by Woolcock et al, 1999 Span/240 = 21.67 mm |
| Deflection under Dead and Service Wind = 14.04 mm | Limit by Woolcock et al, 1999 Span/100 = 52.00 mm |

**Reactions**

Maximum downward =1.72 kn Maximum upward = -1.19 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

## Rafter Design Internal

Internal Rafter Load Width = 5400 mm Internal Rafter Span = 11750 mm Try Rafter 2x450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =6.68 S1 Upward =6.68

Shear Capacity of timber =5.3 MPa Bending Capacity of timber =48 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

Second page

| | | | | | |
|---|---|---|---|---|---|
| $M_{1.35D}$ | 31.45 Kn-m | Capacity | 91.56 Kn-m | Passing Percentage | **291.13 %** |
| $M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$ | 68.03 Kn-m | Capacity | 122.08 Kn-m | Passing Percentage | **179.45 %** |
| $M_{0.9D\text{-}WnUp}$ | -47.06 Kn-m | Capacity | -152.6 Kn-m | Passing Percentage | **324.27 %** |
| $V_{1.35D}$ | 10.71 Kn | Capacity | 96.64 Kn | Passing Percentage | **902.33 %** |
| $V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$ | 23.16 Kn | Capacity | 128.86 Kn | Passing Percentage | **556.39 %** |
| $V_{0.9D\text{-}WnUp}$ | -16.02 Kn | Capacity | -161.08 Kn | Passing Percentage | **1005.49 %** |

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 36.17 mm Limit by Woolcock et al, 1999 Span/240 = 49.58 mm
Deflection under Dead and Service Wind = 47.895 mm Limit by Woolcock et al, 1999 Span/100 = 119.00 mm

**Reactions**

Maximum downward =23.16 kn Maximum upward = -16.02 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 43.67 Kn > -16.02 Kn

## Rafter Design External

External Rafter Load Width = 2700 mm External Rafter Span = 5767 mm Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.94

K8 Upward =0.94 S1 Downward =13.93 S1 Upward =13.93

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

| | | | | | |
|---|---|---|---|---|---|
| $M_{1.35D}$ | 3.79 Kn-m | Capacity | 4.72 Kn-m | Passing Percentage | **124.54 %** |
| $M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$ | 8.19 Kn-m | Capacity | 6.30 Kn-m | Passing Percentage | **76.92 %** |
| $M_{0.9D-WnUp}$ | -5.67 Kn-m | Capacity | -7.87 Kn-m | Passing Percentage | **138.80 %** |
| $V_{1.35D}$ | 2.63 Kn | Capacity | 14.47 Kn | Passing Percentage | **550.19 %** |
| $V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$ | 5.68 Kn | Capacity | 19.30 Kn | Passing Percentage | **339.79 %** |
| $V_{0.9D-WnUp}$ | -3.93 Kn | Capacity | -24.12 Kn | Passing Percentage | **613.74 %** |

**Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

| | |
|---|---|
| Deflection under Dead and Live Load = 21.76 mm | Limit by Woolcock et al, 1999 Span/240= 24.79 mm |
| Deflection under Dead and Service Wind = 25.93 mm | Limit by Woolcock et al, 1999 Span/100 = 59.50 mm |

**Reactions**

Maximum downward =5.68 kn Maximum upward = -3.93 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds ........... (Eq 4.12) = -25.20 kn > -3.93 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -3.93 Kn

## Intermediate Design Sides

Intermediate Spacing = 2975 mm    Intermediate Span = 5626 mm    Try Intermediate 2x250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =0.97

K8 Upward =1.00    S1 Downward =12.68    S1 Upward =1.00

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

| | | | | | |
|---|---|---|---|---|---|
| $M_{Wind+Snow}$ | 4.89 Kn-m | Capacity | 11.66 Kn-m | Passing Percentage | **238.45 %** |
| $V_{0.9D-WnUp}$ | 3.47 Kn-m | Capacity | 40.2 Kn-m | Passing Percentage | **1158.50 %** |

**Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 45.82 mm    Limit by Woolcock et al, 1999 Span/100 = 56.26 mm

**Reactions**

Maximum = 3.47 kn

## Girt Design Front and Back

Girt's Spacing = 900 mm    Girt's Span = 5400 mm    Try Girt 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =0.97

K8 Upward =0.58    S1 Downward =12.68    S1 Upward =21.96

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

| | | | | | |
|---|---|---|---|---|---|
| $M_{Wind+Snow}$ | 2.72 Kn-m | Capacity | 3.37 Kn-m | Passing Percentage | **123.90 %** |

| | | | | | |
|---|---|---|---|---|---|
| $V_{0.9D\text{-}WnUp}$ | 2.02 Kn-m | Capacity | 20.10 Kn-m | Passing Percentage | **995.05 %** |

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 18.96 mm Limit by Woolcock et al, 1999 Span/100 = 54.00 mm
Sag during installation = 51.56 mm

#### Reactions

Maximum = 2.02 kn

### Girt Design Sides

Girt's Spacing = 900 mm Girt's Span = 2975 mm Try Girt 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.97

K8 Upward =0.53 S1 Downward =12.68 S1 Upward =23.05

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

| | | | | | |
|---|---|---|---|---|---|
| $M_{Wind+Snow}$ | 0.83 Kn-m | Capacity | 3.09 Kn-m | Passing Percentage | **372.29 %** |
| $V_{0.9D\text{-}WnUp}$ | 1.11 Kn-m | Capacity | 20.10 Kn-m | Passing Percentage | **1810.81 %** |

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 1.75 mm Limit by Woolcock et al. 1999 Span/100 = 29.75 mm
Sag during installation =4.75 mm

#### Reactions

Maximum = 1.11 kn

### Middle Pole Design

#### Geometry

| | | | |
|---|---|---|---|
| 275 SED H5 (Minimum 300 dia. at Floor Level) | Dry Use | Height | 5600 mm |
| Area | 64885 mm2 | As | 48663.8671875 mm2 |
| Ix | 335197731 mm4 | Zx | 2331810 mm3 |
| Iy | 335197731 mm4 | Zx | 2331810 mm3 |
| Lateral Restraint | 3400 mm c/c | | |

**Loads**

Total Area over Pole = 32.13 m2

| | | | |
|---|---|---|---|
| Dead | 8.03 Kn | Live | 8.03 Kn |
| Wind Down | 13.82 Kn | Snow | 0.00 Kn |
| Moment wind | 34.90 Kn-m | | |
| Phi | 0.8 | K8 | 0.99 |
| K1 snow | 0.8 | K1 Dead | 0.6 |
| K1wind | 1 | | |

**Material**

| | | | |
|---|---|---|---|
| Peeling | Steaming | Normal | Dry Use |
| fb = | 36.3 MPa | fs = | 2.96 MPa |
| fc = | 18 MPa | fp = | 7.2 MPa |
| ft = | 22 MPa | E = | 9257 MPa |

**Capacities**

| | | | | | |
|---|---|---|---|---|---|
| PhiNcx Wind | 923.16 Kn | PhiMnx Wind | 66.90 Kn-m | PhiVnx Wind | 115.24 Kn |
| PhiNcx Dead | 553.89 Kn | PhiMnx Dead | 40.14 Kn-m | PhiVnx Dead | 69.14 Kn |

**Checks**

(Mx/PhiMnx)+(N/phiNcx) = 0.55 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.30 < 1 OK

Deflection at top under service lateral loads = 53.11 mm < 56.00 mm

# Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

**Assumed Soil Properties**

| | | | | | |
|---|---|---|---|---|---|
| Gamma | 18 Kn/m3 | Friction angle | 30 deg | Cohesion | 0 Kn/m3 |
| K0 = | (1-sin(30)) / (1+sin(30)) | | | | |

Kp = (1+sin(30)) / (1-sin(30))

**Geometry For Middle Bay Pole**

| | | |
|---|---|---|
| Ds = | 0.6 mm | Pile Diameter |
| L = | 2150 mm | Pile embedment length |
| f1 = | 4500 mm | Distance at which the shear force is applied |
| f2 = | 0 mm | Distance of top soil at rest pressure |

**Loads**

| | |
|---|---|
| Moment Wind = | 34.90 Kn-m |
| Shear Wind = | 7.76 Kn |

**Pile Properties**

| | | |
|---|---|---|
| Safety Factory | 0.55 | |
| Hu = | 13.32 Kn | Ultimate Lateral Strength of the Pile, Short pile |
| Mu = | 35.53 Kn-m | Ultimate Moment Capacity of Pile |

**Checks**

Applied Forces/Capacities = 0.98 < 1 OK

## End Pole Design

**Geometry For End Bay Pole**

**Geometry**

| | | | |
|---|---|---|---|
| 225 SED H5 (Minimum 250 dia. at Floor Level) | Dry Use | Height | 5700 mm |
| Area | 44279 mm2 | As | 33209.1796875 mm2 |
| Ix | 156100441 mm4 | Zx | 1314530 mm3 |
| Iy | 156100441 mm4 | Zx | 1314530 mm3 |
| Lateral Restraint | mm c/c | | |

**Loads**

Total Area over Pole = 16.065 m2

| | | | |
|---|---|---|---|
| Dead | 4.02 Kn | Live | 4.02 Kn |
| Wind Down | 6.91 Kn | Snow | 0.00 Kn |
| Moment Wind | 11.63 Kn-m | | |
| Phi | 0.8 | K8 | 0.49 |

| | | | |
|---|---|---|---|
| K1 snow | 0.8 | K1 Dead | 0.6 |
| K1wind | 1 | | |

**Material**

| | | | |
|---|---|---|---|
| Peeling | Steaming | Normal | Dry Use |
| fb = | 36.3 MPa | fs = | 2.96 MPa |
| fc = | 18 MPa | fp = | 7.2 MPa |
| ft = | 22 MPa | E = | 9257 MPa |

**Capacities**

| | | | | | |
|---|---|---|---|---|---|
| PhiNcx Wind | 314.62 Kn | PhiMnx Wind | 18.84 Kn-m | PhiVnx Wind | 78.64 Kn |
| PhiNcx Dead | 188.77 Kn | PhiMnx Dead | 11.30 Kn-m | PhiVnx Dead | 47.18 Kn |

**Checks**

(Mx/PhiMnx)+(N/phiNcx) = 0.67 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.43 < 1 OK

Deflection at top under service lateral loads = 40.63 mm < 59.85 mm

| | | |
|---|---|---|
| Ds = | 0.6 mm | Pile Diameter |
| L = | 1500 mm | Pile embedment length |
| f1 = | 4500 mm | Distance at which the shear force is applied |
| f2 = | 0 mm | Distance of top soil at rest pressure |

**Loads**

Total Area over Pole = 16.065 m2

| | |
|---|---|
| Moment Wind = | 11.63 Kn-m |
| Shear Wind = | 2.59 Kn |

**Pile Properties**

| | | |
|---|---|---|
| Safety Factory | 0.55 | |
| Hu = | 5.01 Kn | Ultimate Lateral Strength of the Pile, Short pile |
| Mu = | 13.00 Kn-m | Ultimate Moment Capacity of Pile |

**Checks**

Applied Forces/Capacities = 0.90 < 1 OK

# Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

**Assumed Soil Properties**

| | | | | | |
|---|---|---|---|---|---|
| Gamma | 18 Kn/m3 | Friction angle | 30 deg | Cohesion | 0 Kn/m3 |

| | |
|---|---|
| K0 = | (1-sin(30)) / (1+sin(30)) |
| Kp = | (1+sin(30)) / (1-sin(30)) |

**Geometry For End Bay Pole**

| | | |
|---|---|---|
| Ds = | 0.6 mm | Pile Diameter |
| L = | 1500 mm | Pile embedment length |
| f1 = | 4500 mm | Distance at which the shear force is applied |
| f2 = | 0 mm | Distance of top soil at rest pressure |

**Loads**

| | |
|---|---|
| Moment Wind = | 11.63 Kn-m |
| Shear Wind = | 2.59 Kn |

**Pile Properties**

| | | |
|---|---|---|
| Safety Factory | 0.55 | |
| Hu = | 5.01 Kn | Ultimate Lateral Strength of the Pile, Short pile |
| Mu = | 13.00 Kn-m | Ultimate Moment Capacity of Pile |

**Checks**

Applied Forces/Capacities = 0.90 < 1 OK

## Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(2150) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(2150)

Skin Friction = 37.33 Kn

Weight of Pile + Pile Skin Friction = 41.09 Kn

Uplift on one Pile = 16.23 Kn

Uplift is ok