

Pole Shed App Ver 01 2022

**Job No.:** 2207039 - 1

**Address:** 1869 Takaka Coillingwood Highway,  
Parapara 7182, New Zealand

**Date:** 10/30/2023

**Latitude:** -40.717703

**Longitude:** 172.676802

**Elevation:** 16.5 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N2	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	D
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.6 m
Wind Region	NZ2	Terrain Category	1.71	Design Wind Speed	39.15 m/s
Wind Pressure	0.92 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.30 m  $C_{p,e} = -0.9$   $p_e = -0.74$  KPa  $p_{net} = -0.74$  KPa

For roof  $C_{p,e}$  from 3.30 m To 6.60 m  $C_{p,e} = -0.5$   $p_e = -0.41$  KPa  $p_{net} = -0.41$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 18.0 m  $C_{p,e} = 0.7$   $p_e = 0.58$  KPa  $p_{net} = 0.86$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.30 m  $C_{p,e} =$   $p_e = -0.54$  KPa  $p_{net} = -0.54$  KPa

Maximum Upward pressure used in roof member Design = 0.74 KPa

Maximum Downward pressure used in roof member Design = 0.45 KPa

Maximum Wall pressure used in Design = 0.86 KPa

Maximum Racking pressure used in Design = 0.88 KPa

**Design Summary**

**Purlin Design**

Purlin Spacing = 900 mm

Purlin Span = 5850 mm

Try Purlin 250x50 SG8 Dry

### Pole Shed App Ver 01 2022

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 0.97

K8 Upward = 0.28    S1 Downward = 12.68    S1 Upward = 32.18

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>1.35D</sub>	1.3 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	<b>261.54 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	2.89 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	<b>156.75 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-1.98 Kn-m	Capacity	-1.65 Kn-m	Passing Percentage	<b>351.06 %</b>
V <sub>1.35D</sub>	0.89 Kn	Capacity	12.06 Kn	Passing Percentage	<b>1355.06 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	1.97 Kn	Capacity	16.08 Kn	Passing Percentage	<b>816.24 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-1.36 Kn	Capacity	-20.10 Kn	Passing Percentage	<b>1477.94 %</b>

#### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 18.24 mm    Limit by Woolcock et al, 1999 Span/240 = 24.17 mm

Deflection under Dead and Service Wind = 22.04 mm    Limit by Woolcock et al, 1999 Span/100 = 58.00 mm

#### **Reactions**

Maximum downward = 1.97 kn    Maximum upward = -1.36 kn

Number of Blocking = 0    if 0 then no blocking required, if 1 then one midspan blocking required

#### **Rafter Design Internal**

Internal Rafter Load Width = 4500 mm    Internal Rafter Span = 5850 mm    Try Rafter 2x240x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 1.00    S1 Downward = 6.71    S1 Upward = 6.71

Shear Capacity of timber = 5.3 MPa    Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

Pole Shed App Ver 01 2022

M <sub>1.35D</sub>	6.50 Kn-m	Capacity	19.9 Kn-m	Passing Percentage	<b>306.15 %</b>
M <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	14.44 Kn-m	Capacity	26.54 Kn-m	Passing Percentage	<b>183.80 %</b>
M <sub>0.9D-WnUp</sub>	-9.91 Kn-m	Capacity	-33.18 Kn-m	Passing Percentage	<b>334.81 %</b>
V <sub>1.35D</sub>	4.44 Kn	Capacity	36.82 Kn	Passing Percentage	<b>829.28 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	9.87 Kn	Capacity	49.08 Kn	Passing Percentage	<b>497.26 %</b>
V <sub>0.9D-WnUp</sub>	-6.78 Kn	Capacity	-61.36 Kn	Passing Percentage	<b>905.01 %</b>

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 17.98 mm      Limit by Woolcock et al, 1999 Span/240 = 25.00 mm

Deflection under Dead and Service Wind = 24.135 mm      Limit by Woolcock et al, 1999 Span/100 = 60.00 mm

**Reactions**

Maximum downward = 9.87 kn    Maximum upward = -6.78 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -6.78 Kn

**Girt Design Front and Back**

Girt's Spacing = 900 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after

installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.79    S1 Downward =9.63    S1 Upward =17.59

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>Wind+Snow</sub>	0.87 Kn-m	Capacity	1.65 Kn-m	Passing Percentage	<b>189.66 %</b>
V <sub>0.9D-WnUp</sub>	1.16 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	<b>1039.66 %</b>

#### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 8.66 mm    Limit by Woolcock et al, 1999 Span/100 = 30.00 mm  
Sag during installation = 4.91 mm

#### **Reactions**

Maximum = 1.16 kn

#### **Girt Design Sides**

Girt's Spacing = 900 mm                      Girt's Span = 3000 mm                      Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.79    S1 Downward =9.63    S1 Upward =17.59

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>Wind+Snow</sub>	0.87 Kn-m	Capacity	1.65 Kn-m	Passing Percentage	<b>189.66 %</b>
V <sub>0.9D-WnUp</sub>	1.16 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	<b>1039.66 %</b>

#### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 8.66 mm    Limit by Woolcock et al. 1999 Span/100 = 30.00 mm

Pole Shed App Ver 01 2022

Sag during installation = 4.91 mm

**Reactions**

Maximum = 1.16 kn

**Middle Pole Design**

**Geometry**

225 SED H5 (Minimum 250 dia. at Floor Level)	Dry Use	Height	3360 mm
Area	11875 mm <sup>2</sup>	As	8906.25 mm <sup>2</sup>
I <sub>x</sub>	55818685 mm <sup>4</sup>	Z <sub>x</sub>	470052 mm <sup>3</sup>
I <sub>y</sub>	55818685 mm <sup>4</sup>	Z <sub>y</sub>	470052 mm <sup>3</sup>
Lateral Restraint	3400 mm c/c		

**Loads**

Total Area over Pole = 13.5 m<sup>2</sup>

Dead	3.38 Kn	Live	3.38 Kn
Wind Down	6.08 Kn	Snow	0.00 Kn
Moment wind	9.60 Kn-m		
Phi	0.8	K <sub>8</sub>	0.92
K <sub>1</sub> snow	0.8	K <sub>1</sub> Dead	0.6
K <sub>1</sub> wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
f <sub>b</sub> =	36.3 MPa	f <sub>s</sub> =	2.96 MPa
f <sub>c</sub> =	18 MPa	f <sub>p</sub> =	7.2 MPa
f <sub>t</sub> =	22 MPa	E =	9257 MPa

**Capacities**

PhiN <sub>cx</sub> Wind	158.13 Kn	PhiM <sub>nx</sub> Wind	12.62 Kn-m	PhiV <sub>nx</sub> Wind	21.09 Kn
PhiN <sub>cx</sub> Dead	94.88 Kn	PhiM <sub>nx</sub> Dead	7.57 Kn-m	PhiV <sub>nx</sub> Dead	12.65 Kn

**Checks**

$$(M_x/\Phi M_{nx}) + (N/\phi N_{cx}) = 0.84 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\phi N_{cx}) = 0.66 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 31.57 mm < 33.60 mm

## **Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

### **Assumed Soil Properties**

Gamma 18 Kn/m<sup>3</sup> Friction angle 30 deg Cohesion 0 Kn/m<sup>3</sup>  
K<sub>0</sub> =  $(1 - \sin(30)) / (1 + \sin(30))$   
K<sub>p</sub> =  $(1 + \sin(30)) / (1 - \sin(30))$

### **Geometry For Middle Bay Pole**

D<sub>s</sub> = 0.6 mm Pile Diameter  
L = 1450 mm Pile embedment length  
f<sub>1</sub> = 2700 mm Distance at which the shear force is applied  
f<sub>2</sub> = 0 mm Distance of top soil at rest pressure

### **Loads**

Moment Wind = 9.60 Kn-m  
Shear Wind = 3.56 Kn

### **Pile Properties**

Safety Factory 0.55  
H<sub>u</sub> = 6.55 Kn Ultimate Lateral Strength of the Pile, Short pile  
M<sub>u</sub> = 10.61 Kn-m Ultimate Moment Capacity of Pile

### **Checks**

Applied Forces/Capacities = 0.91 < 1 OK

## **Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K<sub>s</sub> (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1450) x K<sub>s</sub>(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1450)

Skin Friction = 16.98 Kn

Weight of Pile + Pile Skin Friction = 20.30 Kn

Uplift on one Pile = 6.95 Kn

Uplift is ok