



Pole Shed App Ver 01 2022

**Job No.:** GSH501 - 1

**Address:** 32 Steel Road, West Plains, Invercargill,  
9874, New Zealand

**Date:** 17/11/2024

**Latitude:** -46.366541

**Longitude:** 168.330122

**Elevation:** 8 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4 m
Wind Region	NZ4	Terrain Category	2.62	Design Wind Speed	40.45 m/s
Wind Pressure	0.98 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Gable Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 4.52 m  $C_{p,e} = -0.9$   $p_e = -0.78$  KPa  $p_{net} = -0.78$  KPa

For roof  $C_{p,e}$  from 4.52 m To 9.04 m  $C_{p,e} = -0.5$   $p_e = -0.43$  KPa  $p_{net} = -0.43$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 11.8 m  $C_{p,e} = 0.7$   $p_e = 0.62$  KPa  $p_{net} = 0.91$  KPa

For side wall  $C_{p,e}$  from 0 m To 4.52 m  $C_{p,e} =$   $p_e = -0.57$  KPa  $p_{net} = -0.57$  KPa

Maximum Upward pressure used in roof member Design = 0.78 KPa

Maximum Downward pressure used in roof member Design = 0.46 KPa

Maximum Wall pressure used in Design = 0.91 KPa

Maximum Racking pressure used in Design = 0.89 KPa

**Design Summary**

**Rafter Design Internal**

Internal Rafter Load Width = 4800 mm Internal Rafter Span = 11650 mm Try Rafter 2x450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

Second page

### Pole Shed App Ver 01 2022

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 1.00    S1 Downward = 6.68    S1 Upward = 6.68

Shear Capacity of timber = 5.3 MPa    Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>1.35D</sub>	27.48 Kn-m	Capacity	91.56 Kn-m	Passing Percentage	<b>333.19 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	75.73 Kn-m	Capacity	122.08 Kn-m	Passing Percentage	<b>161.20 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-45.20 Kn-m	Capacity	-152.6 Kn-m	Passing Percentage	<b>337.61 %</b>
V <sub>1.35D</sub>	9.44 Kn	Capacity	96.64 Kn	Passing Percentage	<b>1023.73 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	26.00 Kn	Capacity	128.86 Kn	Passing Percentage	<b>495.62 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-15.52 Kn	Capacity	-161.08 Kn	Passing Percentage	<b>1037.89 %</b>

#### **Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 31.085 mm    Limit by Woolcock et al, 1999 Span/240 = 49.17 mm

Deflection under Dead and Service Wind = 42.02 mm    Limit by Woolcock et al, 1999 Span/100 = 118.00 mm

#### **Reactions**

Maximum downward = 26.00 kn    Maximum upward = -15.52 kn

#### **Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 4

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 f<sub>pj</sub> = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 58.22 Kn > -15.52 Kn

## Girt Design Front and Back

Girt's Spacing = 900 mm

Girt's Span = 4800 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.43    S1 Downward =11.27    S1 Upward =26.03

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

M <sub>Wind+Snow</sub>	2.36 Kn-m	Capacity	1.59 Kn-m	Passing Percentage	<b>67.37 %</b>
V <sub>0.9D-WnUp</sub>	1.97 Kn	Capacity	16.08 Kn	Passing Percentage	<b>816.24 %</b>

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 42.90 mm    Limit by Woolcock et al, 1999 Span/100 = 48.00 mm

Sag during installation = 32.19 mm

### Reactions

Maximum = 1.97 kn

## Girt Design Sides

Girt's Spacing = 1300 mm

Girt's Span = 2950 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.65    S1 Downward =11.27    S1 Upward =20.41

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

M <sub>Wind+Snow</sub>	1.29 Kn-m	Capacity	2.43 Kn-m	Passing Percentage	<b>188.37 %</b>
V <sub>0.9D-WnUp</sub>	1.74 Kn	Capacity	16.08 Kn	Passing Percentage	<b>924.14 %</b>

## Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 8.84 mm      Limit by Woolcock et al. 1999 Span/100 = 29.50 mm  
Sag during installation = 4.59 mm

## Reactions

Maximum = 1.74 kn

## Middle Pole Design

### Geometry

225 SED H5 (Minimum 250 dia. at Floor Level)	Dry Use	Height	4740 mm
Area	44279 mm <sup>2</sup>	As	33209.1796875 mm <sup>2</sup>
I <sub>x</sub>	156100441 mm <sup>4</sup>	Z <sub>x</sub>	1314530 mm <sup>3</sup>
I <sub>y</sub>	156100441 mm <sup>4</sup>	Z <sub>y</sub>	1314530 mm <sup>3</sup>
Lateral Restraint	4740 mm c/c		

### Loads

Total Area over Pole = 28.32 m<sup>2</sup>

Dead	7.08 Kn	Live	7.08 Kn
Wind Down	13.03 Kn	Snow	17.84 Kn
Moment wind	12.78 Kn-m	Moment snow	4.31 Kn-m
Phi	0.8	K <sub>8</sub>	0.67
K <sub>1</sub> snow	0.8	K <sub>1</sub> Dead	0.6
K <sub>1</sub> wind	1		

### Material

Peeling	Steaming	Normal	Dry Use
f <sub>b</sub> =	36.3 MPa	f <sub>s</sub> =	2.96 MPa
f <sub>c</sub> =	18 MPa	f <sub>p</sub> =	7.2 MPa
f <sub>t</sub> =	22 MPa	E =	9257 MPa

### Capacities

PhiN <sub>cx</sub> Wind	428.52 Kn	PhiM <sub>nx</sub> Wind	25.66 Kn-m	PhiV <sub>nx</sub> Wind	78.64 Kn
PhiN <sub>cx</sub> Dead	257.11 Kn	PhiM <sub>nx</sub> Dead	15.39 Kn-m	PhiV <sub>nx</sub> Dead	47.18 Kn
PhiN <sub>cx</sub> Snow	342.82 Kn	PhiM <sub>nx</sub> Snow	20.52 Kn-m	PhiV <sub>nx</sub> Snow	62.91 Kn

#### Checks

$$(M_x/\phi M_{nx}) + (N/\phi N_c) = 0.58 < 1 \text{ OK}$$

$$(M_x/\phi M_{nx})^2 + (N/\phi N_c) = 0.33 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 23.57 \text{ mm} < 47.40 \text{ mm}$$

### **Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

#### Assumed Soil Properties

Gamma 18 Kn/m<sup>3</sup> Friction angle 30 deg Cohesion 0 Kn/m<sup>3</sup>

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

#### Geometry For Middle Bay Pole

Ds = 0.6 mm Pile Diameter

L = 1600 mm Pile embedment length

f1 = 3000 mm Distance at which the shear force is applied

f2 = 0 mm Distance of top soil at rest pressure

#### Loads

Moment Wind = 12.78 Kn-m Moment Snow = Kn-m

Shear Wind = 4.26 Kn Shear Snow = 4.31 Kn

#### Pile Properties

Safety Factory 0.55

Hu = 7.93 Kn Ultimate Lateral Strength of the Pile, Short pile

Mu = 14.27 Kn-m Ultimate Moment Capacity of Pile

#### Checks

$$\text{Applied Forces/Capacities} = 0.90 < 1 \text{ OK}$$

### **Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Pole Shed App Ver 01 2022

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1600) x Ks(1.5) x  $0.5 \times \tan(30)$  x  $\pi$  x Dia of Pile(0.6) x Height of Pile(1600)

Skin Friction = 20.68 Kn

Weight of Pile + Pile Skin Friction = 24.34 Kn

Uplift on one Pile = 15.72 Kn

Uplift is ok