

Pole Shed App Ver 01 2022

**Job No.:** 2312001 - 1

**Address:** 49 Martin Loop, Mariri, New Zealand

**Date:** 11/30/2023

**Latitude:** -41.168836

**Longitude:** 173.03198

**Elevation:** 2.5 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N3	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	D
Importance Level	2	Ultimate wind & Earthquake ARI	100 Years	Max Height	5.4 m
Wind Region	NZ2	Terrain Category	1.0	Design Wind Speed	40.91 m/s
Wind Pressure	1 KPa	Lee Zone	NO	Ultimate Snow ARI	150 Years
Wind Category	High	Earthquake ARI	500		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.05 m  $C_{p,e} = -0.988$   $p_e = -0.89$  KPa  $p_{net} = -0.89$  KPa

For roof  $C_{p,e}$  from 3.05 m To 6.10 m  $C_{p,e} = -0.856$   $p_e = -0.77$  KPa  $p_{net} = -0.77$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 10 m  $C_{p,e} = 0.7$   $p_e = 0.63$  KPa  $p_{net} = 0.93$  KPa

For side wall  $C_{p,e}$  from 0 m To 6.10 m  $C_{p,e} =$   $p_e = -0.59$  KPa  $p_{net} = -0.59$  KPa

Maximum Upward pressure used in roof member Design = 0.89 KPa

Maximum Downward pressure used in roof member Design = 0.24 KPa

Maximum Wall pressure used in Design = 0.93 KPa

Maximum Racking pressure used in Design = 0.81 KPa

**Design Summary**

**Purlin Design**

Purlin Spacing = 900 mm

Purlin Span = 4850 mm

Try Purlin 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after

installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 0.56    S1 Downward = 9.63    S1 Upward = 22.25

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>1.35D</sub>	0.89 Kn-m	Capacity	1.26 Kn-m	Passing Percentage	<b>141.57 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	2.13 Kn-m	Capacity	1.68 Kn-m	Passing Percentage	<b>78.87 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-1.76 Kn-m	Capacity	-1.18 Kn-m	Passing Percentage	<b>67.05 %</b>
V <sub>1.35D</sub>	0.74 Kn	Capacity	7.24 Kn	Passing Percentage	<b>978.38 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	1.47 Kn	Capacity	9.65 Kn	Passing Percentage	<b>656.46 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-1.45 Kn	Capacity	-12.06 Kn	Passing Percentage	<b>831.72 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 39.62 mm    Limit by Woolcock et al, 1999 Span/360 = 13.33 mm

Deflection under Dead and Service Wind = 40.94 mm    Limit by Woolcock et al, 1999 Span/250 = 32.00 mm

#### Reactions

Maximum downward = 1.47 kn    Maximum upward = -1.45 kn

Number of Blocking = 0    if 0 then no blocking required, if 1 then one midspan blocking required

#### Rafter Design Internal

Internal Rafter Load Width = 5000 mm    Internal Rafter Span = 9850 mm    Try Rafter 2x450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 1.00    S1 Downward = 6.68    S1 Upward = 6.68

Shear Capacity of timber = 5.3 MPa    Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

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M <sub>1.35D</sub>	20.47 Kn-m	Capacity	91.56 Kn-m	Passing Percentage	<b>447.29 %</b>
M <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	40.93 Kn-m	Capacity	122.08 Kn-m	Passing Percentage	<b>298.27 %</b>
M <sub>0.9D-WnUp</sub>	-40.32 Kn-m	Capacity	-152.6 Kn-m	Passing Percentage	<b>378.47 %</b>
V <sub>1.35D</sub>	8.31 Kn	Capacity	96.64 Kn	Passing Percentage	<b>1162.94 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	16.62 Kn	Capacity	128.86 Kn	Passing Percentage	<b>775.33 %</b>
V <sub>0.9D-WnUp</sub>	-16.38 Kn	Capacity	-161.08 Kn	Passing Percentage	<b>983.39 %</b>

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 16.7 mm      Limit by Woolcock et al, 1999 Span/360 = 27.78 mm

Deflection under Dead and Service Wind = 19.175 mm      Limit by Woolcock et al, 1999 Span/250 = 66.67 mm

**Reactions**

Maximum downward = 16.62 kn    Maximum upward = -16.38 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 f<sub>pj</sub> = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -16.38 Kn

**Rafter Design External**

External Rafter Load Width = 2500 mm      External Rafter Span = 3062 mm      Try Rafter 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

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K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 1.00    S1 Downward = 11.27    S1 Upward = 11.27

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>1.35D</sub>	0.99 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	<b>225.25 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	1.98 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	<b>150.00 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-1.95 Kn-m	Capacity	-3.72 Kn-m	Passing Percentage	<b>190.77 %</b>
V <sub>1.35D</sub>	1.29 Kn	Capacity	9.65 Kn	Passing Percentage	<b>748.06 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	2.58 Kn	Capacity	12.86 Kn	Passing Percentage	<b>498.45 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-2.55 Kn	Capacity	-16.08 Kn	Passing Percentage	<b>630.59 %</b>

#### **Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 6.11 mm      Limit by Woolcock et al, 1999 Span/360 = 9.05 mm  
Deflection under Dead and Service Wind = 6.31 mm      Limit by Woolcock et al, 1999 Span/250 = 21.72 mm

#### **Reactions**

Maximum downward = 2.58 kn    Maximum upward = -2.55 kn

#### **Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 14.9 f<sub>pj</sub> = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V =  $\phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s$  ..... (Eq 4.12) = -14.70 kn > -2.55 Kn

Single Shear Capacity under short term loads =  $-10.84 \text{ Kn} > -2.55 \text{ Kn}$

### **Girt Design Front and Back**

Girt's Spacing = 600 mm

Girt's Span = 5000 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 0.73    S1 Downward = 11.27    S1 Upward = 18.79

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>Wind+Snow</sub>	1.74 Kn-m	Capacity	2.72 Kn-m	Passing Percentage	<b>156.32 %</b>
V <sub>0.9D-WnUp</sub>	1.40 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	<b>1148.57 %</b>

#### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 20.33 mm    Limit by Woolcock et al, 1999 Span/250 = 20.00 mm

Sag during installation = 37.90 mm

#### **Reactions**

Maximum = 1.40 kn

### **Girt Design Sides**

Girt's Spacing = 600 mm

Girt's Span = 3257 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 0.89    S1 Downward = 11.27    S1 Upward = 15.16

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>Wind+Snow</sub>	0.74 Kn-m	Capacity	3.34 Kn-m	Passing Percentage	<b>451.35 %</b>
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V<sub>0.9D-WnUp</sub>      0.91 Kn-m      Capacity      16.08 Kn-m      Passing Percentage      **1767.03 %**

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 3.66 mm      Limit by Woolcock et al. 1999 Span/100 = 13.03 mm  
Sag during installation = 6.83 mm

**Reactions**

Maximum = 0.91 kn

**Middle Pole Design**

**Geometry**

300 SED H5 (Minimum 325 dia. at Floor Level)	Dry Use	Height	5100 mm
Area	76660 mm <sup>2</sup>	As	57495.1171875 mm <sup>2</sup>
I <sub>x</sub>	467896461 mm <sup>4</sup>	Z <sub>x</sub>	2994537 mm <sup>3</sup>
I <sub>y</sub>	467896461 mm <sup>4</sup>	Z <sub>y</sub>	2994537 mm <sup>3</sup>
Lateral Restraint	5100 mm c/c		

**Loads**

Total Area over Pole = 25 m<sup>2</sup>

Dead	6.25 Kn	Live	6.25 Kn
Wind Down	6.00 Kn	Snow	0.00 Kn
Moment wind	22.09 Kn-m		
Phi	0.8	K <sub>8</sub>	0.85
K <sub>1</sub> snow	0.8	K <sub>1</sub> Dead	0.6
K <sub>1</sub> wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
f <sub>b</sub> =	36.3 MPa	f <sub>s</sub> =	2.96 MPa
f <sub>c</sub> =	18 MPa	f <sub>p</sub> =	7.2 MPa
f <sub>t</sub> =	22 MPa	E =	9257 MPa

**Capacities**

PhiN <sub>Cx</sub> Wind	933.65 Kn	PhiM <sub>Nx</sub> Wind	73.55 Kn-m	PhiV <sub>Nx</sub> Wind	136.15 Kn
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PhiNcx Dead      560.19 Kn      PhiMnx Dead      44.13 Kn-m      PhiVnx Dead      81.69 Kn

#### Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.32 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.11 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 19.73 mm < 34.00 mm

### Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

#### Assumed Soil Properties

Gamma    18 Kn/m<sup>3</sup>      Friction angle    30 deg    Cohesion    0 Kn/m<sup>3</sup>

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

#### Geometry For Middle Bay Pole

Ds =      0.6 mm      Pile Diameter  
L =      1900 mm      Pile embedment length  
f1 =      4050 mm      Distance at which the shear force is applied  
f2 =      0 mm      Distance of top soil at rest pressure

#### Loads

Moment Wind =      22.09 Kn-m  
Shear Wind =      5.45 Kn

#### Pile Properties

Safety Factory      0.55  
Hu =      10.27 Kn      Ultimate Lateral Strength of the Pile, Short pile  
Mu =      24.62 Kn-m      Ultimate Moment Capacity of Pile

#### Checks

$$\text{Applied Forces/Capacities} = 0.90 < 1 \text{ OK}$$

### End Pole Design

#### Geometry For End Bay Pole

#### Geometry

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200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	5200 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
Ix	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Iy	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Lateral Restraint	mm c/c		

**Loads**

Total Area over Pole = 8.143322475570033 m<sup>2</sup>

Dead	2.04 Kn	Live	2.04 Kn
Wind Down	1.95 Kn	Snow	0.00 Kn
Moment Wind	5.43 Kn-m		
Phi	0.8	K8	0.48
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

**Capacities**

PhiNcx Wind	243.49 Kn	PhiMnx Wind	13.04 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	146.10 Kn	PhiMnx Dead	7.83 Kn-m	PhiVnx Dead	37.77 Kn

**Checks**

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.44 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.20 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 23.95 mm < 35.91 mm

Ds =	0.6 mm	Pile Diameter
L =	1400 mm	Pile embedment length
f1 =	4050 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**



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Total Area over Pole = 8.143322475570033 m<sup>2</sup>

Moment Wind = 5.43 Kn-m

Shear Wind = 1.34 Kn

**Pile Properties**

Safety Factory 0.55

Hu = 4.49 Kn Ultimate Lateral Strength of the Pile, Short pile

Mu = 10.49 Kn-m Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.52 < 1 OK

**Drained Lateral Strength of End pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma 18 Kn/m<sup>3</sup> Friction angle 30 deg Cohesion 0 Kn/m<sup>3</sup>

K0 =  $(1 - \sin(30)) / (1 + \sin(30))$

Kp =  $(1 + \sin(30)) / (1 - \sin(30))$

**Geometry For End Bay Pole**

Ds = 0.6 mm Pile Diameter

L = 1400 mm Pile embedment length

f1 = 4050 mm Distance at which the shear force is applied

f2 = 0 mm Distance of top soil at rest pressure

**Loads**

Moment Wind = 5.43 Kn-m

Shear Wind = 1.34 Kn

**Pile Properties**

Safety Factory 0.55

Hu = 4.49 Kn Ultimate Lateral Strength of the Pile, Short pile

Mu = 10.49 Kn-m Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.52 < 1 OK

## **Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1900) x Ks(1.5) x  $0.5 \times \tan(30)$  x  $\pi$  x Dia of Pile(0.6) x Height of Pile(1900)

Skin Friction = 29.16 Kn

Weight of Pile + Pile Skin Friction = 32.03 Kn

Uplift on one Pile = 16.63 Kn

Uplift is ok