

Pole Shed App Ver 01 2022

Job No.: 511-5024695--Full Closed

Address: 241 Grahams Rd, Ashburton, New Zealand

Date: 09/08/2024

Latitude: -43.93688

Longitude: 171.739282

Elevation: 81.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N4	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.8 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	38.22 m/s
Wind Pressure	0.88 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.30 m $C_{p,e} = -0.9$ $p_e = -0.71$ KPa $p_{net} = -0.71$ KPa

For roof $C_{p,e}$ from 3.30 m To 6.60 m $C_{p,e} = -0.5$ $p_e = -0.39$ KPa $p_{net} = -0.39$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 9 m $C_{p,e} = 0.7$ $p_e = 0.55$ KPa $p_{net} = 0.81$ KPa

For side wall $C_{p,e}$ from 0 m To 3.30 m $C_{p,e} =$ $p_e = -0.51$ KPa $p_{net} = -0.51$ KPa

Maximum Upward pressure used in roof member Design = 0.71 KPa

Maximum Downward pressure used in roof member Design = 0.42 KPa

Maximum Wall pressure used in Design = 0.81 KPa

Maximum Racking pressure used in Design = 0.94 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 5850 mm

Try Purlin 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.97

K8 Upward = 0.54 S1 Downward = 12.68 S1 Upward = 22.76

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.3 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	261.54 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	3.58 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	126.54 %
M _{0.9D-W_nUp}	-1.87 Kn-m	Capacity	-3.16 Kn-m	Passing Percentage	196.27 %
V _{1.35D}	0.89 Kn	Capacity	12.06 Kn	Passing Percentage	1355.06 %

Pole Shed App Ver 01 2022

V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.45 Kn	Capacity	16.08 Kn	Passing Percentage	656.33 %
V _{0.9D-W_nUp}	-1.28 Kn	Capacity	-20.10 Kn	Passing Percentage	1570.31 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 18.24 mm Limit by Woolcock et al, 1999 Span/240 = 24.17 mm

Deflection under Dead and Service Wind = 21.59 mm Limit by Woolcock et al, 1999 Span/100 = 58.00 mm

Reactions

Maximum downward = 2.45 kn Maximum upward = -1.28 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 6000 mm Internal Rafter Span = 8850 mm Try Rafter 2x360x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 5.90 S₁ Upward = 5.90

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	19.83 Kn-m	Capacity	60.82 Kn-m	Passing Percentage	306.71 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	54.63 Kn-m	Capacity	81.1 Kn-m	Passing Percentage	148.45 %
M _{0.9D-W_nUp}	-28.49 Kn-m	Capacity	-101.38 Kn-m	Passing Percentage	355.84 %
V _{1.35D}	8.96 Kn	Capacity	77.32 Kn	Passing Percentage	862.95 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	24.69 Kn	Capacity	103.08 Kn	Passing Percentage	417.50 %
V _{0.9D-W_nUp}	-12.88 Kn	Capacity	-128.86 Kn	Passing Percentage	1000.47 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 25.68 mm Limit by Woolcock et al, 1999 Span/240 = 37.50 mm

Deflection under Dead and Service Wind = 33.77 mm Limit by Woolcock et al, 1999 Span/100 = 90.00 mm

Reactions

Maximum downward = 24.69 kn Maximum upward = -12.88 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 4

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 12.6$ fpj = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

$K_{11} = 2.0$ fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 58.22 Kn > -12.88 Kn

Rafter Design External

External Rafter Load Width = 3000 mm

External Rafter Span = 4310 mm

Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 K_1 Medium term = 0.8 K_1 Long term = 0.6 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.94

K_8 Upward = 0.94 S_1 Downward = 13.93 S_1 Upward = 13.93

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	2.35 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	200.85 %
$M_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	6.48 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	97.22 %
$M_{0.9D-W_nUp}$	-3.38 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	232.84 %
$V_{1.35D}$	2.18 Kn	Capacity	14.47 Kn	Passing Percentage	663.76 %
$V_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	6.01 Kn	Capacity	19.30 Kn	Passing Percentage	321.13 %
$V_{0.9D-W_nUp}$	-3.14 Kn	Capacity	-24.12 Kn	Passing Percentage	768.15 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k_2 for Long Term Loads = 2

Deflection under Dead and Live Load = 7.91 mm

Limit by Woolcock et al, 1999 Span/240 = 18.75 mm

Deflection under Dead and Service Wind = 9.36 mm

Limit by Woolcock et al, 1999 Span/100 = 45.00 mm

Reactions

Maximum downward = 6.01 kn Maximum upward = -3.14 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$ fpj = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

$K_{11} = 2.0$ $f_{c,j} = 36.1$ Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi_i \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -25.20 kn > -3.14 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -3.14 Kn

Intermediate Design Front and Back

Intermediate Spacing = 3000 mm

Intermediate Span = 3049 mm

Try Intermediate 2x200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 1.00 S_1 Downward = 11.27 S_1 Upward = 0.66

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	3.14 Kn-m	Capacity	7.46 Kn-m	Passing Percentage	237.58 %
$V_{0.9D-WnUp}$	4.12 Kn	Capacity	-32.16 Kn	Passing Percentage	780.58 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 16.035 mm

Limit by Woolcock et al, 1999 Span/100 = 30.49 mm

Reactions

Maximum = 4.12 kn

Intermediate Design Sides

Intermediate Spacing = 2250 mm

Intermediate Span = 3500 mm

Try Intermediate 2x200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 1.00 S_1 Downward = 11.27 S_1 Upward = 0.70

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.55 Kn-m	Capacity	7.46 Kn-m	Passing Percentage	481.29 %
$V_{0.9D-WnUp}$	1.77 Kn	Capacity	32.16 Kn	Passing Percentage	1816.95 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 20.875 mm

Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

Reactions

Maximum = 1.77 kn

Girt Design Front and Back

Girt's Spacing = 1300 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.79 S1 Downward =9.63 S1 Upward =17.59

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.18 Kn-m	Capacity	1.65 Kn-m	Passing Percentage	139.83 %
$V_{0.9D-WnUp}$	1.58 Kn	Capacity	12.06 Kn	Passing Percentage	763.29 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 20.96 mm

Limit by Woolcock et al, 1999 Span/100 = 30.00 mm

Sag during installation = 4.91 mm

Reactions

Maximum = 1.58 kn

Girt Design Sides

Girt's Spacing = 1300 mm

Girt's Span = 2250 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.89 S1 Downward =9.63 S1 Upward =15.23

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.67 Kn-m	Capacity	1.87 Kn-m	Passing Percentage	279.10 %
$V_{0.9D-WnUp}$	1.18 Kn	Capacity	12.06 Kn	Passing Percentage	1022.03 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 6.63 mm

Limit by Woolcock et al. 1999 Span/100 = 22.50 mm

Sag during installation =1.55 mm

Reactions

Maximum = 1.18 kn

Middle Pole Design

Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3440 mm
Area	35448 mm ²	As	26585.7421875 mm ²
I _x	100042702 mm ⁴	Z _x	941578 mm ³
I _y	100042702 mm ⁴	Z _y	941578 mm ³
Lateral Restraint	3440 mm c/c		

Loads

Total Area over Pole = 27 m²

Dead	6.75 Kn	Live	6.75 Kn
Wind Down	11.34 Kn	Snow	17.01 Kn
Moment wind	15.23 Kn-m	Moment snow	5.12 Kn-m
Phi	0.8	K ₈	0.85
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

PhiN _c Wind	434.65 Kn	PhiM _n Wind	23.28 Kn-m	PhiV _n Wind	62.96 Kn
PhiN _c Dead	260.79 Kn	PhiM _n Dead	13.97 Kn-m	PhiV _n Dead	37.77 Kn
PhiN _c Snow	347.72 Kn	PhiM _n Snow	18.63 Kn-m	PhiV _n Snow	50.36 Kn

Checks

$$(M_x/\Phi M_n) + (N/\Phi N_c) = 0.73 < 1 \text{ OK}$$

$$(M_x/\Phi M_n)^2 + (N/\Phi N_c) = 0.51 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 30.21 \text{ mm} < 34.40 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K ₀ =	(1-sin(30)) / (1+sin(30))				
K _p =	(1+sin(30)) / (1-sin(30))				

Geometry For Middle Bay Pole

D _s =	0.6 mm	Pile Diameter
L =	1700 mm	Pile embedment length
f _l =	2850 mm	Distance at which the shear force is applied

Pole Shed App Ver 01 2022

$f_2 =$ 0 mm Distance of top soil at rest pressure

Loads

Moment Wind =	15.23 Kn-m	Moment Snow =	Kn-m
Shear Wind =	5.34 Kn	Shear Snow =	5.12 Kn

Pile Properties

Safety Factory	0.55		
$H_u =$	9.62 Kn	Ultimate Lateral Strength of the Pile, Short pile	
$M_u =$	16.66 Kn-m	Ultimate Moment Capacity of Pile	

Checks

Applied Forces/Capacities = 0.91 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	3500 mm
Area	27598 mm ²	A_s	20698.2421875 mm ²
I_x	60639381 mm ⁴	Z_x	646820 mm ³
I_y	60639381 mm ⁴	Z_y	646820 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 13.5 m²

Dead	3.38 Kn	Live	3.38 Kn
Wind Down	5.67 Kn	Snow	8.51 Kn
Moment Wind	5.08 Kn-m	Moment snow	1.71 Kn-m
Φ	0.8	K ₈	0.74
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
$f_b =$	36.3 MPa	$f_s =$	2.96 MPa
$f_c =$	18 MPa	$f_p =$	7.2 MPa
$f_t =$	22 MPa	E =	9257 MPa

Capacities

ΦH_{Ncx} Wind	292.42 Kn	ΦH_{Mnx} Wind	13.82 Kn-m	ΦH_{Vnx} Wind	49.01 Kn
ΦH_{Ncx} Dead	175.45 Kn	ΦH_{Mnx} Dead	8.29 Kn-m	ΦH_{Vnx} Dead	29.41 Kn
ΦH_{Ncx} Snow	233.94 Kn	ΦH_{Mnx} Snow	11.06 Kn-m	ΦH_{Vnx} Snow	39.21 Kn

Checks

$$(M_x/\phi M_{nx}) + (N/\phi N_{cx}) = 0.43 < 1 \text{ OK}$$

$$(M_x/\phi M_{nx})^2 + (N/\phi N_{cx}) = 0.19 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 18.31 mm < 37.90 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2850 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 13.5 m²

Moment Wind =	5.08 Kn-m	Moment Snow =	1.71 Kn-m
Shear Wind =	1.78 Kn	Shear Snow =	1.71 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.72 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.94 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.64 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2850 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	5.08 Kn-m	Moment Snow =	1.71 Kn-m
Shear Wind =	1.78 Kn	Shear Snow =	1.71 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.72 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.94 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.64 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1700) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1700)

Skin Friction = 23.34 Kn

Weight of Pile + Pile Skin Friction = 27.76 Kn

Uplift on one Pile = 13.09 Kn

Uplift is ok