<u>Pole Shed App Ver 01 2022</u>

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Job No.: 2409031-1 **Address:** 21 Hill View Road, Motupipi, New Zealand **Date:** 16/12/2024

Latitude: -40.884582 **Longitude:** 172.839328 **Elevation:** 113.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	5 m
Wind Region	NZ2	Terrain Category	2.26	Design Wind Speed	44.12 m/s
Wind Pressure	1.17 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Very High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Mono Enclosed

For roof Cp,i = -0.3

For roof CP,e from 0 m To 2.15 m Cpe = -0.93 pe = -0.98 KPa pnet = -0.98 KPa

For roof CP,e from 2.15 m To 4.30 m Cpe = -0.88 pe = -0.93 KPa pnet = -0.93 KPa

For wall Windward Cp,i = -0.3 side Wall Cp,i = -0.3

For wall Windward and Leeward CP,e from 0 m To 8 m Cpe = 0.7 pe = 0.74 KPa pnet = 1.09 KPa

For side wall CP,e from 0 m To 4.30 m Cpe = pe = -0.68 KPa pnet = -0.68 KPa

Maximum Upward pressure used in roof member Design = 0.98 KPa

Maximum Downward pressure used in roof member Design = 0.46 KPa

Maximum Wall pressure used in Design = 1.09 KPa

Maximum Racking pressure used in Design = 1.27 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm Purlin Span = 9850 mm Try Purlin 360x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

Second page

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.81

K8 Upward =0.28 S1 Downward =17.01 S1 Upward =32.41

Shear Capacity of timber =5.3 MPa Bending Capacity of timber =48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	3.68 Kn-m	Capacity	17.70 Kn-m	Passing Percentage	**480.98 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	8.3 Kn-m	Capacity	23.60 Kn-m	Passing Percentage	**284.34 %**
$M_{0.9D-WnUp}$	-8.24 Kn-m	Capacity	-10.11 Kn-m	Passing Percentage	**122.69 %**
$V_{1.35D}$	1.50 Kn	Capacity	27.61 Kn	Passing Percentage	**1840.67 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	3.37 Kn	Capacity	36.82 Kn	Passing Percentage	**1092.58 %**
$V_{0.9D-WnUp}$	-3.35 Kn	Capacity	-46.02 Kn	Passing Percentage	**1373.73 %**

Deflections

Modulus of Elasticity = 12100 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 30.63 mm Limit by Woolcock et al, 1999 Span/240 = 40.83 mm
Deflection under Dead and Service Wind = 37.27 mm Limit by Woolcock et al, 1999 Span/100 = 98.00 mm

Reactions

Maximum downward =3.37 kn Maximum upward = -3.35 kn

Number of Blocking = 2 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design External

External Rafter Load Width = 5000 mm External Rafter Span = 7922 mm Try Rafter 450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.95

K8 Upward =0.95 S1 Downward =13.57 S1 Upward =13.57

Shear Capacity of timber =5.3 MPa Bending Capacity of timber =48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	13.24 Kn-m	Capacity	43.42 Kn-m	Passing Percentage	**327.95 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	29.81 Kn-m	Capacity	57.89 Kn-m	Passing Percentage	**194.20 %**

$M_{0.9D\text{-}WnUp}$	-29.61 Kn-m	Capacity	-72.37 Kn-m	Passing Percentage	**244.41 %**
$V_{1.35D}$	6.68 Kn	Capacity	48.32 Kn	Passing Percentage	**723.35 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	15.05 Kn	Capacity	64.43 Kn	Passing Percentage	**428.11 %**
$V_{0.9D\text{-}WnUp}$	-14.95 Kn	Capacity	-80.54 Kn	Passing Percentage	**538.73 %**

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 15.20 mm	Limit by Woolcock et al, 1999 Span/240= 33.33 mm
Deflection under Dead and Service Wind = 18.50 mm	Limit by Woolcock et al, 1999 Span/100 = 80.00 mm

Reactions

Maximum downward =15.05 kn   Maximum upward = -14.95 kn

Rafter to Pole Connection check

Bolt Size = M16 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 63 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds .... (Eq 4.12) = -86.48 kn > -14.95 Kn

Single Shear Capacity under short term loads = -38.81 Kn > -14.95 Kn

Intermediate Design Front and Back

Intermediate Spacing = 5000 mm   Intermediate Span = 4850 mm   Try Intermediate 2x290x45 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.89

K8 Upward =1.00 S1 Downward =15.23 S1 Upward =1.12

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	16.02 Kn-m	Capacity	14.12 Kn-m	Passing Percentage	**88.14 %**
$V_{0.9D-WnUp}$	13.22 Kn	Capacity	-41.96 Kn	Passing Percentage	**317.40 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 39.75 mm Limit byWoolcock et al, 1999 Span/100 = 48.50 mm

Reactions

Maximum = 13.22 kn

Intermediate Design Sides

Intermediate Spacing = 4000 mm Intermediate Span = 4150 mm Try Intermediate 2x290x45 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.89

K8 Upward =1.00 S1 Downward =15.23 S1 Upward =1.03

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	4.69 Kn-m	Capacity	14.12 Kn-m	Passing Percentage	**301.07 %**
$V_{0.9D-WnUp}$	4.52 Kn	Capacity	41.96 Kn	Passing Percentage	**928.32 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 17.045 mm Limit by Woolcock et al, 1999 Span/100 = 41.50 mm

Reactions

Maximum = 4.52 kn

Girt Design Front and Back

Girt's Spacing = 0 mm Girt's Span = 5000 mm Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =NaN

K8 Upward =NaN S1 Downward =NaN S1 Upward =NaN

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	**NaN %**
$V_{0.9D-WnUp}$	0.00 Kn	Capacity	0.00 Kn	Passing Percentage	**NaN %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm Limit by Woolcock et al, 1999 Span/100 = 50.00 mm
Sag during installation = NaN mm

Reactions

Maximum = 0.00 kn

Girt Design Sides

Girt's Spacing = 0 mm Girt's Span = 4000 mm Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =NaN

K8 Upward =NaN S1 Downward =NaN S1 Upward =NaN

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	**NaN %**

$V_{0.9D\text{-}WnUp}$	0.00 Kn	Capacity	0.00 Kn	Passing Percentage	**NaN %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm   Limit by Woolcock et al. 1999 Span/100 = 40.00 mm
Sag during installation =NaN mm

Reactions

Maximum = 0.00 kn

End Pole Design

Geometry For End Bay Pole

Geometry

275 SED H5 (Minimum 300 dia. at Floor Level)	Dry Use	Height	4800 mm
Area	64885 mm2	As	48663.8671875 mm2
Ix	335197731 mm4	Zx	2331810 mm3
Iy	335197731 mm4	Zx	2331810 mm3
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 40 m2

Dead	10.00 Kn	Live	10.00 Kn
Wind Down	18.40 Kn	Snow	0.00 Kn
Moment Wind	29.69 Kn-m		
Phi	0.8	K8	0.83
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	774.82 Kn	PhiMnx Wind	56.15 Kn-m	PhiVnx Wind	115.24 Kn
PhiNcx Dead	464.89 Kn	PhiMnx Dead	33.69 Kn-m	PhiVnx Dead	69.14 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.58 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.33 < 1 OK

Deflection at top under service lateral loads = 33.53 mm < 49.88 mm

Ds =	0.6 mm	Pile Diameter
L =	2100 mm	Pile embedment length
f1 =	3750 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 40 m2

Moment Wind =	29.69 Kn-m
Shear Wind =	7.92 Kn

Pile Properties

Safety Factory	0.55	
Hu =	14.11 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	31.89 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.93 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	2100 mm	Pile embedment length

f1 =	3750 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	29.69 Kn-m
Shear Wind =	7.92 Kn

Pile Properties

Safety Factory	0.55	
Hu =	14.11 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	31.89 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.93 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(2600) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(2600)

Skin Friction = 54.60 Kn

Weight of Pile + Pile Skin Friction = 59.82 Kn

Uplift on one Pile = 30.20 Kn

Uplift is ok