

Job Number:

**BWhite
Consulting Ltd**

Issue:

PRODUCER STATEMENT-PS1-DESIGN

ISSUED BY: **BWhite Consulting Ltd (Design Engineer: Bevan White)**

TO BE SUPPLIED TO: **Southland District Council** IN RESPECT OF: **Proposed NEW Farm Shed**

AT: **57 Blackmore Road, Garston, New Zealand**

LEGAL DESCRIPTION

We have been engaged by **Ezequote Pty Ltd** to provide **Specific Structural Engineering Design** services in respect of the requirements of Clause(s) **B1** of the Building Code for part only (as specified in the attachment to this statement), of the proposed building work.

☐ ALL ☒ Part only as specified: Purlins, Rafters, Girts, Poles, Columns, Pole embedment and all connections

The design has been prepared in accordance with compliance documents to NZ Building Code issued by Ministry of Business, Innovation & Employment Clauses **B1/VM1 and B1/VM4**

The proposed building work covered by the producer statement is described on **Ezequote** drawings title **EHB 128 A** and numbered **A101-A116 Rev-1** dated **24/01/2024** together with the following specification, and other documents set out in the schedule attached to this statement: **Design Featured Report Dated 24/01/2024 and numbered "Second Page"**

On behalf of BWhite Consulting Ltd, and subject to:

1. Site verification of the following design assumptions: **an Ultimate foundation bearing pressure of 300 kPa in accordance with NZS3604:2011**
2. **The building has a design life of 50 years and an Importance Level 1**
3. **Unless specifically noted, compliance of the drawings to None-Specific codes such as NZS3604 and NZS4229 have not been checked by this practice**
4. **This Certificate does not cover any other building code clause including weather tightness**
5. **Inspections of the building to be completed by Southland District Council. As BWhite Consulting Ltd are not undertaking inspections, we cannot issue a producer Statement-PS4- Construction Review.**
6. **This Producer Statement- Design is valid for a building consent issued within 1 year from the date of issue**
7. All proprietary products meeting their performance specification requirements

I believe on reasonable grounds that a) the building, if constructed in accordance with the drawings, specifications, and other documents provided or listed in the attached schedule, will comply with the relevant provisions of the Building Code and that b), the persons who have undertaken the design have the necessary competency to do so. I also recommend the following level of construction monitoring/observation:

☒ CM1 ☐ CM2 ☐ CM3 ☐ CM4 ☐ CM5 or as per agreement with owner/developer (**stated above**)

I, Bevan White am CPEng **108276** I am Member of Engineering New Zealand and hold the following qualification: **BE.Civil**

BWhite Consulting Ltd holds a current policy of Professional Indemnity Insurance no less than \$200,000.

Signed by **Bevan White** on behalf of **BWhite Consulting Ltd** Dated: **24/01/2024**

Email: bwhitecpeng@gmail.com Phone: 0211-979786

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Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of \$200,000.

This form is to accompany Form 2 of the Building (Forms) Regulations 2004 for the application of a Building Consent

Date: 24/01/2024

**BWhite
Consulting Ltd**

18B Jules Crescent,

Bell Block New Plymouth 4312

New Zealand

File No:

DESIGN FEATURES SUMMARY FOR PROPOSED NEW FARM SHED 57 BLACKMORE ROAD, GARSTON, NEW ZEALAND

Site Specific Loads

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.93 KPa	Roof Snow Load	0.65 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & EQ ARI	100 Years	Max Height	8.4 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	41.41 m/s
Wind Pressure	1.03 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years

Timber

Sawn Timber to be graded to the properties of SG6 and SG8 or better as mentioned on plans, with moisture content of 18% or less for dry and 25% or less for wet.

The following standards have been used in the design of this structure

- NZS 3603:1993 Timber Structures Standard
- NZS 3604:2011 Timber Framed Buildings. Standards New Zealand, 2011
- NZS 3404:1997 Steel Structures
- AS/NZS 1170 2003 Structural Design Actions
- AS/NZS 1170.2 2021 Structural Design Actions- Wind Action
- Branz. "Engineering Basis of NZS 3604". April 2013

Yours Faithfully

BWhite CONSULTING LTD

Bevan White

Director | BE Civil . CMengNZ CPEng

Email: bwhitecpeng@gmail.com Contact: 0211 979 786

Job No.: EHB 128 A**Address:** 57 Blackmore Road, Garston, New Zealand**Date:** 24/01/2024**Latitude:** -45.474105**Longitude:** 168.686346**Elevation:** 319 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.93 KPa	Roof Snow Load	0.65 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	8.4 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	41.41 m/s
Wind Pressure	1.03 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = 0.6583$

For roof $C_{p,e}$ from 0 m To 4.20 m $C_{p,e} = -1.14$ $p_e = -0.83$ KPa $p_{net} = -1.41$ KPa

For roof $C_{p,e}$ from 4.20 m To 8.40 m $C_{p,e} = -0.78$ $p_e = -0.57$ KPa $p_{net} = -1.15$ KPa

For wall Windward $C_{p,i} = 0.6583$ side Wall $C_{p,i} = -0.5725$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 17.4 m $C_{p,e} = 0.7$ $p_e = 0.63$ KPa $p_{net} = 1.25$ KPa

For side wall $C_{p,e}$ from 0 m To 8.40 m $C_{p,e} =$ $p_e = -0.59$ KPa $p_{net} = 0.03$ KPa

Maximum Upward pressure used in roof member Design = 1.41 KPa

Maximum Downward pressure used in roof member Design = 0.71 KPa

Maximum Wall pressure used in Design = 1.25 KPa

Maximum Racking pressure used in Design = 1.11 KPa

Design Summary**Rafter Design Internal**

Internal Rafter Load Width = 4700 mm

Internal Rafter Span = 10350 mm

Try Rafter 2x450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.68 S1 Upward = 6.68

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	21.24 Kn-m	Capacity	91.56 Kn-m	Passing Percentage	431.07 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	63.56 Kn-m	Capacity	122.08 Kn-m	Passing Percentage	192.07 %
$M_{0.9D-W_nUp}$	-74.58 Kn-m	Capacity	-152.6 Kn-m	Passing Percentage	204.61 %
$V_{1.35D}$	8.21 Kn	Capacity	96.64 Kn	Passing Percentage	1177.10 %

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V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	24.57 Kn	Capacity	128.86 Kn	Passing Percentage	524.46 %
V _{0.9D-WnUp}	-28.82 Kn	Capacity	-161.08 Kn	Passing Percentage	558.92 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 19.085 mm

Limit by Woolcock et al, 1999 Span/240 = 43.75 mm

Deflection under Dead and Service Wind = 30.215 mm

Limit by Woolcock et al, 1999 Span/100 = 105.00 mm

Reactions

Maximum downward = 24.57 kn Maximum upward = -28.82 kn

Rafter to Pole Connection check

Bolt Size = M16 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 80 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 12.6 f_{pj} = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 77.63 Kn > -28.82 Kn

Rafter Design External

External Rafter Load Width = 2350 mm

External Rafter Span = 10348 mm

Try Rafter 450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 0.95

K₈ Upward = 0.95 S₁ Downward = 13.57 S₁ Upward = 13.57

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	10.62 Kn-m	Capacity	43.42 Kn-m	Passing Percentage	408.85 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	31.77 Kn-m	Capacity	57.89 Kn-m	Passing Percentage	182.22 %
M _{0.9D-WnUp}	-37.27 Kn-m	Capacity	-72.37 Kn-m	Passing Percentage	194.18 %
V _{1.35D}	4.10 Kn	Capacity	48.32 Kn	Passing Percentage	1178.54 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	12.28 Kn	Capacity	64.43 Kn	Passing Percentage	524.67 %
V _{0.9D-WnUp}	-14.41 Kn	Capacity	-80.54 Kn	Passing Percentage	558.92 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 21.20 mm

Limit by Woolcock et al, 1999 Span/240 = 43.75 mm

Deflection under Dead and Service Wind = 30.21 mm

Limit by Woolcock et al, 1999 Span/100 = 105.00 mm

Reactions

Maximum downward = 12.28 kn Maximum upward = -14.41 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 12.6 f_{pj} = 22.7 Mpa for Rafter with effective thickness = 63 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = $\phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s$ (Eq 4.12) = -91.15 kn > -14.41 Kn

Single Shear Capacity under short term loads = -21.83 Kn > -14.41 Kn

Girt Design Front and Back

Girt's Spacing = 0 mm

Girt's Span = 2350 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₄ = 1 K₅ = 1 K₈ Downward = NaN

K₈ Upward = NaN S₁ Downward = NaN S₁ Upward = NaN

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	NaN %
V _{0.9D-WnUp}	0.00 Kn-m	Capacity	0.00 Kn-m	Passing Percentage	NaN %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm

Limit by Woolcock et al, 1999 Span/100 = 23.50 mm

Sag during installation = NaN mm

Reactions

Maximum = 0.00 kn

Girt Design Sides

Girt's Spacing = 0 mm

Girt's Span = 5250 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =NaN

K8 Upward =NaN S1 Downward =NaN S1 Upward =NaN

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	NaN %
V _{0.9D-WnUp}	0.00 Kn-m	Capacity	0.00 Kn-m	Passing Percentage	NaN %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm

Limit by Woolcock et al. 1999 Span/100 = 52.50 mm

Sag during installation =NaN mm

Reactions

Maximum = 0.00 kn

Middle Pole Design

Geometry

350 SED H5 (Minimum 375 dia. at Floor Level)	Dry Use	Height	8450 mm
Area	103154 mm ²	As	77365.4296875 mm ²
I _x	847191750 mm ⁴	Z _x	4674161 mm ³
I _y	847191750 mm ⁴	Z _y	4674161 mm ³
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 24.675 m²

Dead	6.17 Kn	Live	6.17 Kn
Wind Down	17.52 Kn	Snow	16.04 Kn
Moment wind	68.85 Kn-m	Moment snow	9.16 Kn-m
Phi	0.8	K8	1.00
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

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PhiNcx Wind	1485.42 Kn	PhiMnx Wind	135.74 Kn-m	PhiVnx Wind	183.20 Kn
PhiNcx Dead	891.25 Kn	PhiMnx Dead	81.44 Kn-m	PhiVnx Dead	109.92 Kn
PhiNcx Snow	1188.33 Kn	PhiMnx Snow	108.59 Kn-m	PhiVnx Snow	146.56 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.53 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.28 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 87.55 \text{ mm} < 84.50 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K ₀ =	$(1 - \sin(30)) / (1 + \sin(30))$				
K _p =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

D _s =	0.6 mm	Pile Diameter
L =	2700 mm	Pile embedment length
f ₁ =	6300 mm	Distance at which the shear force is applied
f ₂ =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	68.85 Kn-m	Moment Snow =	Kn-m
Shear Wind =	10.93 Kn	Shear Snow =	9.16 Kn

Pile Properties

Safety Factor	0.55	
H _u =	19.49 Kn	Ultimate Lateral Strength of the Pile, Short pile
M _u =	72.07 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.96 < 1 \text{ OK}$$

Uplift Check

$$\text{Density of Concrete} = 24 \text{ Kn/m}^3$$

$$\text{Density of Timber Pole} = 5 \text{ Kn/m}^3$$

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

$$K_s (\text{Lateral Earth Pressure Coefficient}) \text{ for cast into place concrete piles} = 1.5$$

$$\text{Formula to calculate Skin Friction} = \text{Safety factor (0.55)} \times \text{Density of Soil (18)} \times \text{Height of Pile (2700)} \times K_s (1.5) \times 0.5 \times \tan(30) \times \pi \times \text{Dia of Pile (0.6)} \times \text{Height of Pile (2700)}$$

$$\text{Skin Friction} = 58.88 \text{ Kn}$$

Weight of Pile + Pile Skin Friction = 61.86 Kn

Uplift on one Pile = 29.24 Kn

Uplift is ok