

Pole Shed App Ver 01 2022

Job No.: Tod Thompson - 1 **Address:** 271 Ody Rd, Whangarei Heads, New Zealand **Date:** 10/11/2023
Latitude: -35.805435 **Longitude:** 174.531314 **Elevation:** 25.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4 m
Wind Region	NZ1	Terrain Category	2.0	Design Wind Speed	39.2 m/s
Wind Pressure	0.92 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Open

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.79 m $C_{p,e} = -0.9$ $p_e = -0.75$ KPa $p_{net} = -0.93$ KPa

For roof $C_{p,e}$ from 3.79 m To 7.58 m $C_{p,e} = -0.5$ $p_e = -0.41$ KPa $p_{net} = -0.59$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 8 m $C_{p,e} = 0.7$ $p_e = 0.58$ KPa $p_{net} = 0.86$ KPa

For side wall $C_{p,e}$ from 0 m To 3.79 m $C_{p,e} =$ $p_e = -0.54$ KPa $p_{net} = -0.54$ KPa

Maximum Upward pressure used in roof member Design = 0.93 KPa

Maximum Downward pressure used in roof member Design = 0.43 KPa

Maximum Wall pressure used in Design = 0.86 KPa

Maximum Racking pressure used in Design = 0.99 KPa

Design Summary

Purlin Design

Purlin Spacing = 600 mm Purlin Span = 5350 mm Try Purlin 200x50 SG8 Dry

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.70 S1 Downward = 11.27 S1 Upward = 19.34

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.72 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	309.72 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.12 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	140.09 %
M _{0.9D-W_nUp}	-1.51 Kn-m	Capacity	-2.62 Kn-m	Passing Percentage	173.51 %
V _{1.35D}	0.54 Kn	Capacity	9.65 Kn	Passing Percentage	1787.04 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.17 Kn	Capacity	12.86 Kn	Passing Percentage	1099.15 %
V _{0.9D-W_nUp}	-1.13 Kn	Capacity	-16.08 Kn	Passing Percentage	1423.01 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 16.56 mm Limit by Woolcock et al, 1999 Span/240 = 22.08 mm

Deflection under Dead and Service Wind = 19.74 mm Limit by Woolcock et al, 1999 Span/100 = 53.00 mm

Reactions

Maximum downward = 1.17 kn Maximum upward = -1.13 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4000 mm Internal Rafter Span = 7850 mm Try Rafter 2x300x63 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 5.30 S1 Upward = 5.30

Shear Capacity of timber = 5 MPa Bending Capacity of timber = 38 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M1.35D	10.40 Kn-m	Capacity	34.48 Kn-m	Passing Percentage	331.54 %
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	22.49 Kn-m	Capacity	45.96 Kn-m	Passing Percentage	204.36 %
M0.9D-WnUp	-21.72 Kn-m	Capacity	-57.46 Kn-m	Passing Percentage	264.55 %
V1.35D	5.30 Kn	Capacity	60.78 Kn	Passing Percentage	1146.79 %
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	11.46 Kn	Capacity	81.04 Kn	Passing Percentage	707.16 %
V0.9D-WnUp	-11.07 Kn	Capacity	-101.3 Kn	Passing Percentage	915.09 %

Deflections

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 20.525 mm Limit by Woolcock et al, 1999 Span/240 = 33.33 mm

Deflection under Dead and Service Wind = 27.175 mm Limit by Woolcock et al, 1999 Span/100 = 80.00 mm

Reactions

Maximum downward = 11.46 kn Maximum upward = -11.07 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -11.07 Kn

Girt Design Front and Back

Girt's Spacing = 900 mm

Girt's Span = 2750 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after

installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.99 S1 Downward =9.63 S1 Upward =11.91

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.73 Kn-m	Capacity	2.07 Kn-m	Passing Percentage	283.56 %
V _{0.9D-WnUp}	1.06 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	1137.74 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 6.12 mm Limit by Woolcock et al, 1999 Span/100 = 27.50 mm
Sag during installation = 3.47 mm

Reactions

Maximum = 1.06 kn

Girt Design Sides

Girt's Spacing = 900 mm Girt's Span = 4000 mm Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.92 S1 Downward =9.63 S1 Upward =14.36

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.55 Kn-m	Capacity	1.94 Kn-m	Passing Percentage	125.16 %
V _{0.9D-WnUp}	1.55 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	778.06 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 27.38 mm Limit by Woolcock et al. 1999 Span/100 = 40.00 mm

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Sag during installation = 15.52 mm

Reactions

Maximum = 1.55 kn

Middle Pole Design

Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3700 mm
Area	35448 mm ²	As	26585.7421875 mm ²
Ix	100042702 mm ⁴	Zx	941578 mm ³
Iy	100042702 mm ⁴	Zx	941578 mm ³
Lateral Restraint	3700 mm c/c		

Loads

Total Area over Pole = 16 m²

Dead	4.00 Kn	Live	4.00 Kn
Wind Down	6.88 Kn	Snow	0.00 Kn
Moment wind	11.85 Kn-m		
Phi	0.8	K8	0.80
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	406.33 Kn	PhiMnx Wind	21.77 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	243.80 Kn	PhiMnx Dead	13.06 Kn-m	PhiVnx Dead	37.77 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\phi N_{cx}) = 0.58 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\phi N_{cx}) = 0.33 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 26.61 mm < 37.00 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³
K₀ = $(1 - \sin(30)) / (1 + \sin(30))$
K_p = $(1 + \sin(30)) / (1 - \sin(30))$

Geometry For Middle Bay Pole

D_s = 0.6 mm Pile Diameter
L = 1500 mm Pile embedment length
f₁ = 3000 mm Distance at which the shear force is applied
f₂ = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 11.85 Kn-m
Shear Wind = 3.95 Kn

Pile Properties

Safety Factory 0.55
H_u = 6.68 Kn Ultimate Lateral Strength of the Pile, Short pile
M_u = 11.94 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.99 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1500) x K_s(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1500)

Skin Friction = 18.17 Kn

Weight of Pile + Pile Skin Friction = 22.07 Kn

Uplift on one Pile = 11.28 Kn

Uplift is ok