



**Job No.:** Mark Galvin - 1**Address:** 295 Te Rakehou Rd, Feilding, New Zealand**Date:** 14/11/2024**Latitude:** -40.198765**Longitude:** 175.494393**Elevation:** 79 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N1	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.8 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	38.52 m/s
Wind Pressure	0.89 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Gable Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 5.40 m  $C_{p,e} = -0.9$   $p_e = -0.72$  KPa  $p_{net} = -0.72$  KPa

For roof  $C_{p,e}$  from 5.40 m To 10.80 m  $C_{p,e} = -0.5$   $p_e = -0.40$  KPa  $p_{net} = -0.40$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 12 m  $C_{p,e} = 0.7$   $p_e = 0.56$  KPa  $p_{net} = 0.83$  KPa

For side wall  $C_{p,e}$  from 0 m To 5.40 m  $C_{p,e} =$   $p_e = -0.52$  KPa  $p_{net} = -0.52$  KPa

Maximum Upward pressure used in roof member Design = 0.72 KPa

Maximum Downward pressure used in roof member Design = 0.35 KPa

Maximum Wall pressure used in Design = 0.83 KPa

Maximum Racking pressure used in Design = 0.80 KPa

**Design Summary****Rafter Design Internal**

Internal Rafter Load Width = 4000 mm

Internal Rafter Span = 11850 mm

Try Rafter 2x400x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.26 S1 Upward = 6.26

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

$M_{1.35D}$	23.70 Kn-m	Capacity	73.78 Kn-m	Passing Percentage	311.31 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	47.39 Kn-m	Capacity	98.38 Kn-m	Passing Percentage	207.60 %
$M_{0.9D-W_nUp}$	-34.75 Kn-m	Capacity	-122.98 Kn-m	Passing Percentage	353.90 %
$V_{1.35D}$	8.00 Kn	Capacity	85.9 Kn	Passing Percentage	1073.75 %

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V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	16.00 Kn	Capacity	114.54 Kn	Passing Percentage	715.88 %
V <sub>0.9D-WnUp</sub>	-11.73 Kn	Capacity	-143.18 Kn	Passing Percentage	1220.63 %

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 39.45 mm

Limit by Woolcock et al, 1999 Span/240 = 50.00 mm

Deflection under Dead and Service Wind = 49.31 mm

Limit by Woolcock et al, 1999 Span/100 = 120.00 mm

**Reactions**

Maximum downward = 16.00 kn Maximum upward = -11.73 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 f<sub>pj</sub> = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 43.67 Kn > -11.73 Kn

**Girt Design Front and Back**

Girt's Spacing = 900 mm

Girt's Span = 4000 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1 K<sub>4</sub> = 1 K<sub>5</sub> = 1 K<sub>8</sub> Downward = 1.00

K<sub>8</sub> Upward = 0.50 S<sub>1</sub> Downward = 11.27 S<sub>1</sub> Upward = 23.76

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>Wind+Snow</sub>	1.49 Kn-m	Capacity	1.87 Kn-m	Passing Percentage	125.50 %
V <sub>0.9D-WnUp</sub>	1.49 Kn	Capacity	16.08 Kn	Passing Percentage	1079.19 %

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 11.15 mm

Limit by Woolcock et al, 1999 Span/100 = 40.00 mm

Sag during installation = 15.52 mm

### Reactions

Maximum = 1.49 kn

### Girt Design Sides

Girt's Spacing = 900 mm

Girt's Span = 4000 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.50    S1 Downward =11.27    S1 Upward =23.76

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

M <sub>Wind+Snow</sub>	1.49 Kn-m	Capacity	1.87 Kn-m	Passing Percentage	<b>125.50 %</b>
V <sub>0.9D-WnUp</sub>	1.49 Kn	Capacity	16.08 Kn	Passing Percentage	<b>1079.19 %</b>

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

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Sag during installation =15.52 mm

### Reactions

Maximum = 1.49 kn

### Middle Pole Design

#### Geometry

225 SED H5 (Minimum 250 dia. at Floor Level)	Dry Use	Height	4500 mm
Area	44279 mm <sup>2</sup>	As	33209.1796875 mm <sup>2</sup>
I <sub>x</sub>	156100441 mm <sup>4</sup>	Z <sub>x</sub>	1314530 mm <sup>3</sup>
I <sub>y</sub>	156100441 mm <sup>4</sup>	Z <sub>y</sub>	1314530 mm <sup>3</sup>
Lateral Restraint	4500 mm c/c		

### Loads

Total Area over Pole = 24 m<sup>2</sup>

Dead	6.00 Kn	Live	6.00 Kn
Wind Down	8.40 Kn	Snow	0.00 Kn
Moment wind	13.79 Kn-m		
Phi	0.8	K8	0.72
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

### Material

Peeling	Steaming	Normal	Dry Use
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fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

**Capacities**

PhiNcx Wind	460.22 Kn	PhiMnx Wind	27.55 Kn-m	PhiVnx Wind	78.64 Kn
PhiNcx Dead	276.13 Kn	PhiMnx Dead	16.53 Kn-m	PhiVnx Dead	47.18 Kn

**Checks**

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.54 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.29 < 1$  OK

Deflection at top under service lateral loads = 28.96 mm < 45.00 mm

**Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For Middle Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	1600 mm	Pile embedment length
f1 =	3600 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	13.79 Kn-m
Shear Wind =	3.83 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	7.02 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	14.88 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.93 < 1 OK

**Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

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Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1600) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1600)

Skin Friction = 20.68 Kn

Weight of Pile + Pile Skin Friction = 24.34 Kn

Uplift on one Pile = 11.88 Kn

Uplift is ok