

Pole Shed App Ver 01 2022

Job No.: Max Scotchmer -
Gable - 2

Address: 444 Kaituna - Tuamarina Road, Kaituna,
New Zealand

Date: 28/04/2025

Latitude: -41.439276

Longitude: 173.911144

Elevation: 33.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N3	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.3 m
Wind Region	NZ2	Terrain Category	3.0	Design Wind Speed	48.41 m/s
Wind Pressure	1.41 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Very High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Gable Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 5.01 m $C_{p,e} = -0.9$ $p_e = -1.05$ KPa $p_{net} = -1.05$ KPa

For roof $C_{p,e}$ from 5.01 m To 10.01 m $C_{p,e} = -0.5$ $p_e = -0.59$ KPa $p_{net} = -0.59$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 7 m $C_{p,e} = 0.7$ $p_e = 0.89$ KPa $p_{net} = 1.31$ KPa

For side wall $C_{p,e}$ from 0 m To 5.01 m $C_{p,e} =$ $p_e = -0.82$ KPa $p_{net} = -0.82$ KPa

Maximum Upward pressure used in roof member Design = 1.05 KPa

Maximum Downward pressure used in roof member Design = 0.62 KPa

Maximum Wall pressure used in Design = 1.31 KPa

Maximum Racking pressure used in Design = 1.57 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 2850 mm

Try Purlin 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

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K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.98

K8 Upward = 0.59 S1 Downward = 12.23 S1 Upward = 21.57

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.31 Kn-m	Capacity	1.79 Kn-m	Passing Percentage	577.42 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.45 Kn-m	Capacity	2.38 Kn-m	Passing Percentage	164.14 %
M _{0.9D-W_nUp}	-0.75 Kn-m	Capacity	-1.80 Kn-m	Passing Percentage	240.00 %
V _{1.35D}	0.43 Kn	Capacity	8.25 Kn	Passing Percentage	1918.60 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.18 Kn	Capacity	11.00 Kn	Passing Percentage	932.20 %
V _{0.9D-W_nUp}	-1.06 Kn	Capacity	-13.75 Kn	Passing Percentage	1297.17 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 5.01 mm Limit by Woolcock et al, 1999 Span/240 = 11.67 mm

Deflection under Dead and Service Wind = 3.39 mm Limit by Woolcock et al, 1999 Span/100 = 28.00 mm

Reactions

Maximum downward = 1.18 kn Maximum upward = -1.06 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 3000 mm Internal Rafter Span = 3350 mm Try Rafter 2x240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.71 S1 Upward = 6.71

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.42 Kn-m	Capacity	5.8 Kn-m	Passing Percentage	408.45 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	3.87 Kn-m	Capacity	7.74 Kn-m	Passing Percentage	200.00 %

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M _{0.9D-WnUp}	-3.47 Kn-m	Capacity	-9.68 Kn-m	Passing Percentage	278.96 %
V _{1.35D}	1.70 Kn	Capacity	20.84 Kn	Passing Percentage	1225.88 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	4.62 Kn	Capacity	27.78 Kn	Passing Percentage	601.30 %
V _{0.9D-WnUp}	-4.15 Kn	Capacity	-34.74 Kn	Passing Percentage	837.11 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 2.825 mm Limit by Woolcock et al, 1999 Span/240 = 14.58 mm

Deflection under Dead and Service Wind = 4.24 mm Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

Reactions

Maximum downward = 4.62 kn Maximum upward = -4.15 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 19.50 Kn > -4.15 Kn

Rafter Design External

External Rafter Load Width = 1500 mm External Rafter Span = 3575 mm Try Rafter 240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 0.94

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K8 Upward =0.94 S1 Downward =13.82 S1 Upward =13.82

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.81 Kn-m	Capacity	2.73 Kn-m	Passing Percentage	337.04 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.20 Kn-m	Capacity	3.64 Kn-m	Passing Percentage	165.45 %
M _{0.9D-W_nUp}	-1.98 Kn-m	Capacity	-4.55 Kn-m	Passing Percentage	229.80 %
V _{1.35D}	0.90 Kn	Capacity	10.42 Kn	Passing Percentage	1157.78 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.47 Kn	Capacity	13.89 Kn	Passing Percentage	562.35 %
V _{0.9D-W_nUp}	-2.21 Kn	Capacity	-17.37 Kn	Passing Percentage	785.97 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 3.14 mm Limit by Woolcock et al, 1999 Span/240= 14.58 mm

Deflection under Dead and Service Wind = 4.24 mm Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

Reactions

Maximum downward =2.47 kn Maximum upward = -2.21 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k₁ x k₄ x k₅ x f_s x b x d_s (Eq 4.12) = -17.01 kn > -2.21 Kn

Single Shear Capacity under short term loads = -9.75 Kn > -2.21 Kn

Girt Design Front and Back

Girt's Spacing = 0 mm

Girt's Span = 3000 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =NaN

K8 Upward =NaN S1 Downward =NaN S1 Upward =NaN

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	NaN %
V _{0.9D-WnUp}	0.00 Kn	Capacity	0.00 Kn	Passing Percentage	NaN %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm Limit by Woolcock et al, 1999 Span/100 = 30.00 mm

Sag during installation = NaN mm

Reactions

Maximum = 0.00 kn

Girt Design Sides

Girt's Spacing = 0 mm

Girt's Span = 3500 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =NaN

K8 Upward =NaN S1 Downward =NaN S1 Upward =NaN

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	NaN %
V _{0.9D-WnUp}	0.00 Kn	Capacity	0.00 Kn	Passing Percentage	NaN %

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Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm Limit by Woolcock et al. 1999 Span/100 = 35.00 mm

Sag during installation = NaN mm

Reactions

Maximum = 0.00 kn

Middle Pole Design

Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	4060 mm
Area	35448 mm ²	As	26585.7421875 mm ²
Ix	100042702 mm ⁴	Zx	941578 mm ³
Iy	100042702 mm ⁴	Zx	941578 mm ³
Lateral Restraint	4060 mm c/c		

Loads

Total Area over Pole = 10.5 m²

Dead	2.63 Kn	Live	2.63 Kn
Wind Down	6.51 Kn	Snow	0.00 Kn
Moment wind	10.86 Kn-m		
Phi	0.8	K8	0.71
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	364.45 Kn	PhiMnx Wind	19.52 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	218.67 Kn	PhiMnx Dead	11.71 Kn-m	PhiVnx Dead	37.77 Kn

Checks

$$(M_x/\phi M_{nx}) + (N/\phi N_{cx}) = 0.59 < 1 \text{ OK}$$

$$(M_x/\phi M_{nx})^2 + (N/\phi N_{cx}) = 0.34 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 28.76 \text{ mm} < 40.60 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³

$$K_0 = (1 - \sin(30)) / (1 + \sin(30))$$

$$K_p = (1 + \sin(30)) / (1 - \sin(30))$$

Geometry For Middle Bay Pole

Ds = 0.6 mm Pile Diameter
L = 1700 mm Pile embedment length
f1 = 3225 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 10.86 Kn-m
Shear Wind = 3.37 Kn

Pile Properties

Safety Factory 0.55
Hu = 8.89 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 17.17 Kn-m Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.63 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

Geometry

175 SED H5 (Minimum 200 dia. at Floor Level) Dry Use Height 4100 mm

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Area	27598 mm ²	As	20698.2421875 mm ²
Ix	60639381 mm ⁴	Zx	646820 mm ³
Iy	60639381 mm ⁴	Zy	646820 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 5.25 m²

Dead	1.31 Kn	Live	1.31 Kn
Wind Down	3.25 Kn	Snow	0.00 Kn
Moment Wind	5.43 Kn-m		
Phi	0.8	K8	0.58
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	231.09 Kn	PhiMnx Wind	10.92 Kn-m	PhiVnx Wind	49.01 Kn
PhiNcx Dead	138.65 Kn	PhiMnx Dead	6.55 Kn-m	PhiVnx Dead	29.41 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.52 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.27 < 1$ OK

Deflection at top under service lateral loads = 25.07 mm < 42.89 mm

Ds =	0.6 mm	Pile Diameter
L =	1400 mm	Pile embedment length
f1 =	3225 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 5.25 m²

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Moment Wind = 5.43 Kn-m
Shear Wind = 1.68 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.29 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 10.02 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.54 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³
K0 = (1-sin(30)) / (1+sin(30))
Kp = (1+sin(30)) / (1-sin(30))

Geometry For End Bay Pole

Ds = 0.6 mm Pile Diameter
L = 1400 mm Pile embedment length
f1 = 3225 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 5.43 Kn-m
Shear Wind = 1.68 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.29 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 10.02 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.54 < 1 OK

Uplift Check

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Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1700) x Ks(1.5) x $0.5 \times \tan(30) \times \pi \times \text{Dia of Pile}(0.6) \times \text{Height of Pile}(1700)$

Skin Friction = 23.34 Kn

Weight of Pile + Pile Skin Friction = 27.76 Kn

Uplift on one Pile = 8.66 Kn

Uplift is ok