Job No.: Kieran Pierce - 1 **Address:** 254B Crane Rd, Kauri, New Zealand **Date:** 30/01/2024
Latitude: -35.654249 **Longitude:** 174.267084 **Elevation:** 150.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	5.975 m
Wind Region	NZ1	Terrain Category	2.0	Design Wind Speed	41.5 m/s
Wind Pressure	1.03 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Gable Open

For roof Cp,i = -0.3

For roof CP,e from 0 m To 5.98 m Cpe = -0.9 pe = -0.69 KPa pnet = -0.86 KPa

For roof CP,e from 5.98 m To 11.96 m Cpe = -0.5 pe = -0.38 KPa pnet = -0.55 KPa

For wall Windward Cp,i = -0.3 side Wall Cp,i = -0.3

For wall Windward and Leeward CP,e from 0 m To 10 m Cpe = 0.7 pe = 0.65 KPa pnet = 0.96 KPa

For side wall CP,e from 0 m To 5.98 m Cpe = pe = -0.60 KPa pnet = -0.60 KPa

Maximum Upward pressure used in roof member Design = 0.86 KPa

Maximum Downward pressure used in roof member Design = 0.50 KPa

Maximum Wall pressure used in Design = 0.96 KPa

Maximum Racking pressure used in Design = 0.93 KPa

Design Summary

Rafter Design Internal

Internal Rafter Load Width = 6000 mm Internal Rafter Span = 9850 mm Try Rafter 2x400x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =6.26 S1 Upward =6.26

Shear Capacity of timber =5.3 MPa Bending Capacity of timber =48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	24.56 Kn-m	Capacity	73.78 Kn-m	Passing Percentage	**300.41 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	58.21 Kn-m	Capacity	98.38 Kn-m	Passing Percentage	**169.01 %**
$M_{0.9D-WnUp}$	-46.21 Kn-m	Capacity	-122.98 Kn-m	Passing Percentage	**266.13 %**

$V_{1.35D}$	9.97 Kn	Capacity	85.9 Kn	Passing Percentage	**861.58 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	23.64 Kn	Capacity	114.54 Kn	Passing Percentage	**484.52 %**
$V_{0.9D-WnUp}$	-18.76 Kn	Capacity	-143.18 Kn	Passing Percentage	**763.22 %**

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 28.535 mm — Limit by Woolcock et al, 1999 Span/240 = 41.67 mm

Deflection under Dead and Service Wind = 39.635 mm — Limit by Woolcock et al, 1999 Span/100 = 100.00 mm

Reactions

Maximum downward =23.64 kn   Maximum upward = -18.76 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 4

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 58.22 Kn > -18.76 Kn

Girt Design Front and Back

Girt's Spacing = 800 mm   Girt's Span = 6000 mm   Try Girt 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =0.97

K8 Upward =0.72   S1 Downward =12.68   S1 Upward =18.90

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	3.46 Kn-m	Capacity	4.22 Kn-m	Passing Percentage	**121.97 %**
$V_{0.9D-WnUp}$	2.30 Kn-m	Capacity	20.10 Kn-m	Passing Percentage	**873.91 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 29.71 mm — Limit by Woolcock et al, 1999 Span/100 = 60.00 mm

Second page

Sag during installation = 78.58 mm

Reactions

Maximum = 2.30 kn

Girt Design Sides

Girt's Spacing = 800 mm Girt's Span = 3640 mm Try Girt 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.98

K8 Upward =0.47 S1 Downward =12.23 S1 Upward =24.59

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.27 Kn-m	Capacity	1.43 Kn-m	Passing Percentage	**112.60 %**
$V_{0.9D-WnUp}$	1.40 Kn-m	Capacity	13.75 Kn-m	Passing Percentage	**982.14 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 10.19 mm Limit by Woolcock et al. 1999 Span/100 = 36.40 mm
Sag during installation =13.14 mm

Reactions

Maximum = 1.40 kn

Middle Pole Design

Geometry

250 SED H5 HIGH DENSITY (Minimum 275 dia. at Floor Level)	Dry Use	Height	5685 mm
Area	54091 mm2	As	40568.5546875 mm2
Ix	232952248 mm4	Zx	1774874 mm3
Iy	232952248 mm4	Zx	1774874 mm3
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 30 m2

Dead	7.50 Kn	Live	7.50 Kn
Wind Down	15.00 Kn	Snow	0.00 Kn
Moment wind	37.26 Kn-m		
Phi	0.8	K8	1.00
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	49.725 MPa	fs =	2.84 MPa
fc =	28.125 MPa	fp =	8.66 MPa
ft =	29.64 MPa	E =	12874 MPa

Capacities

PhiNcx Wind	1217.06 Kn	PhiMnx Wind	70.60 Kn-m	PhiVnx Wind	92.17 Kn
PhiNcx Dead	730.23 Kn	PhiMnx Dead	42.36 Kn-m	PhiVnx Dead	55.30 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.55 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.30 < 1 OK

Deflection at top under service lateral loads = 59.29 mm < 56.85 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	2200 mm	Pile embedment length
f1 =	4481 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	37.26 Kn-m
Shear Wind =	8.31 Kn

Pile Properties

Safety Factory	0.55	
Hu =	14.20 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	37.83 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.98 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(2200) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(2200)

Skin Friction = 39.09 Kn

Weight of Pile + Pile Skin Friction = 43.51 Kn

Uplift on one Pile = 19.05 Kn

Uplift is ok