



Job No.: 459-788777

Address: 284 Hakarimata Road, Ngāruawāhia 3793, New Zealand

Date: 18/12/2024

Latitude: -37.640673

Longitude: 175.149118

Elevation: 17 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.2 m
Wind Region	NZ1	Terrain Category	2.77	Design Wind Speed	35.65 m/s
Wind Pressure	0.76 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Medium	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.00 m  $C_{p,e} = -1.0973$   $p_e = -1.23$  KPa  $p_{net} = -1.23$  KPa

For roof  $C_{p,e}$  from m To m  $C_{p,e} =$   $p_e =$  KPa  $p_{net} =$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 3 m  $C_{p,e} = 0.7$   $p_e = 0.79$  KPa  $p_{net} = 1.16$  KPa

For side wall  $C_{p,e}$  from 0 m To 2.50 m  $C_{p,e} =$   $p_e = -0.73$  KPa  $p_{net} = -0.73$  KPa

Maximum Upward pressure used in roof member Design = 1.23 KPa

Maximum Downward pressure used in roof member Design = 0.59 KPa

Maximum Wall pressure used in Design = 1.16 KPa

Maximum Racking pressure used in Design = 1.13 KPa

**Design Summary****Purlin Design**

Purlin Spacing = 800 mm

Purlin Span = 5850 mm

Try Purlin 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.97

K8 Upward = 0.74 S1 Downward = 12.68 S1 Upward = 18.58

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

$M_{1.35D}$	1.16 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	293.10 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nDn}$	3.05 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	148.52 %
$M_{0.9D-W_nUp}$	-3.44 Kn-m	Capacity	-4.32 Kn-m	Passing Percentage	284.21 %
$V_{1.35D}$	0.79 Kn	Capacity	12.06 Kn	Passing Percentage	1526.58 %
$V_{1.2D+1.5L 1.2D+S_n 1.2D+W_nDn}$	2.08 Kn	Capacity	16.08 Kn	Passing Percentage	773.08 %
$V_{0.9D-W_nUp}$	-2.35 Kn	Capacity	-20.10 Kn	Passing Percentage	855.32 %

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 16.21 mm

Limit by Woolcock et al, 1999 Span/240 = 24.17 mm

Deflection under Dead and Service Wind = 21.48 mm

Limit by Woolcock et al, 1999 Span/100 = 58.00 mm

**Reactions**

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Maximum downward = 2.08 kn Maximum upward = -2.35 kn

Number of Blocking = 2 if 0 then no blocking required, if 1 then one midspan blocking required

### Rafter Design Internal

Internal Rafter Load Width = 6000 mm

Internal Rafter Span = 4850 mm

Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.81 S1 Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

M1.35D	5.95 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	<b>169.41 %</b>
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	15.70 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	<b>85.61 %</b>
M0.9D-WnUp	-17.73 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	<b>94.75 %</b>
V1.35D	4.91 Kn	Capacity	28.94 Kn	Passing Percentage	<b>589.41 %</b>
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	12.95 Kn	Capacity	38.6 Kn	Passing Percentage	<b>298.07 %</b>
V0.9D-WnUp	-14.62 Kn	Capacity	-48.24 Kn	Passing Percentage	<b>329.96 %</b>

### Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 10.85 mm

Limit by Woolcock et al, 1999 Span/240 = 20.83 mm

Deflection under Dead and Service Wind = 15.975 mm

Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

### Reactions

Maximum downward = 12.95 kn Maximum upward = -14.62 kn

### Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 32.51 Kn > -14.62 Kn

### Rafter Design External

External Rafter Load Width = 3000 mm

External Rafter Span = 4864 mm

Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.94

K8 Upward = 0.94 S1 Downward = 13.93 S1 Upward = 13.93

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

Pole Shed App Ver 01 2022

M <sub>1.35D</sub>	2.99 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	<b>157.86 %</b>
M <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	7.90 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	<b>79.75 %</b>
M <sub>0.9D-WnUp</sub>	-8.92 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	<b>88.23 %</b>
V <sub>1.35D</sub>	2.46 Kn	Capacity	14.47 Kn	Passing Percentage	<b>588.21 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	6.49 Kn	Capacity	19.30 Kn	Passing Percentage	<b>297.38 %</b>
V <sub>0.9D-WnUp</sub>	-7.33 Kn	Capacity	-24.12 Kn	Passing Percentage	<b>329.06 %</b>

**Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 12.06 mm

Limit by Woolcock et al, 1999 Span/240 = 20.83 mm

Deflection under Dead and Service Wind = 15.97 mm

Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

**Reactions**

Maximum downward = 6.49 kn Maximum upward = -7.33 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 14.9 f<sub>pj</sub> = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V =  $\phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s$  ..... (Eq 4.12) = -25.20 kn > -7.33 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -7.33 Kn

**Intermediate Design Front and Back**

Intermediate Spacing = 3000 mm

Intermediate Span = 4050 mm

Try Intermediate 2x250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1 K<sub>4</sub> = 1 K<sub>5</sub> = 1 K<sub>8</sub> Downward = 0.97

K<sub>8</sub> Upward = 1.00 S<sub>1</sub> Downward = 12.68 S<sub>1</sub> Upward = 0.85

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>Wind+Snow</sub>	7.14 Kn-m	Capacity	11.66 Kn-m	Passing Percentage	<b>163.31 %</b>
V <sub>0.9D-WnUp</sub>	7.05 Kn	Capacity	-40.2 Kn	Passing Percentage	<b>570.21 %</b>

**Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 17.34 mm

Limit by Woolcock et al, 1999 Span/100 = 40.50 mm

**Reactions**

Maximum = 7.05 kn

**Intermediate Design Sides**

Intermediate Spacing = 2500 mm

Intermediate Span = 3650 mm

Try Intermediate 2x200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =11.27 S1 Upward =0.72

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{Wind+Snow}$	2.41 Kn-m	Capacity	7.46 Kn-m	Passing Percentage	<b>309.54 %</b>
$V_{0.9D-WnUp}$	2.65 Kn	Capacity	32.16 Kn	Passing Percentage	<b>1213.58 %</b>

#### Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 18.61 mm

Limit by Woolcock et al, 1999 Span/100 = 36.50 mm

#### Reactions

Maximum = 2.65 kn

#### Girt Design Front and Back

Girt's Spacing = 1200 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.79 S1 Downward =9.63 S1 Upward =17.59

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{Wind+Snow}$	1.57 Kn-m	Capacity	1.65 Kn-m	Passing Percentage	<b>105.10 %</b>
$V_{0.9D-WnUp}$	2.09 Kn	Capacity	12.06 Kn	Passing Percentage	<b>577.03 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 15.58 mm

Limit by Woolcock et al, 1999 Span/100 = 30.00 mm

Sag during installation = 4.91 mm

#### Reactions

Maximum = 2.09 kn

#### Girt Design Sides

Girt's Spacing = 1200 mm

Girt's Span = 2500 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.86 S1 Downward =9.63 S1 Upward =16.05

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{Wind+Snow}$	1.09 Kn-m	Capacity	1.80 Kn-m	Passing Percentage	<b>165.14 %</b>
$V_{0.9D-WnUp}$	1.74 Kn	Capacity	12.06 Kn	Passing Percentage	<b>693.10 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 7.51 mm

Limit by Woolcock et al. 1999 Span/100 = 25.00 mm

Sag during installation = 2.37 mm

#### Reactions

Maximum = 1.74 kn

#### Middle Pole Design

##### Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	4300 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
Ix	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Iy	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Lateral Restraint	4300 mm c/c		

##### Loads

Total Area over Pole = 30 m<sup>2</sup>

Dead	7.50 Kn	Live	7.50 Kn
Wind Down	17.70 Kn	Snow	0.00 Kn
Moment wind	14.91 Kn-m		
Phi	0.8	K8	0.66
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

##### Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

##### Capacities

PhiNcx Wind	336.13 Kn	PhiMnx Wind	18.01 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	201.68 Kn	PhiMnx Dead	10.80 Kn-m	PhiVnx Dead	37.77 Kn

##### Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.93 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.78 < 1$  OK

Deflection at top under service lateral loads = 40.86 mm < 43.00 mm

#### Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

##### Assumed Soil Properties

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

##### Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1900 mm	Pile embedment length
f1 =	3150 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

##### Loads

Moment Wind = 14.91 Kn-m  
Shear Wind = 4.73 Kn

#### Pile Properties

Safety Factory 0.55  
Hu = 12.10 Kn Ultimate Lateral Strength of the Pile, Short pile  
Mu = 23.19 Kn-m Ultimate Moment Capacity of Pile

#### Checks

Applied Forces/Capacities = 0.64 < 1 OK

#### End Pole Design

##### Geometry For End Bay Pole

##### Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3900 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
Ix	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Iy	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Lateral Restraint	mm c/c		

#### Loads

Total Area over Pole = 15 m<sup>2</sup>

Dead	3.75 Kn	Live	3.75 Kn
Wind Down	8.85 Kn	Snow	0.00 Kn
Moment Wind	7.46 Kn-m		
Phi	0.8	K8	0.75
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

#### Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

#### Capacities

PhiNcx Wind	383.42 Kn	PhiMnx Wind	20.54 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	230.05 Kn	PhiMnx Dead	12.32 Kn-m	PhiVnx Dead	37.77 Kn

#### Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.41 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.17 < 1 OK

Deflection at top under service lateral loads = 19.91 mm < 41.90 mm

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	3150 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

#### Loads

Total Area over Pole = 15 m<sup>2</sup>

Moment Wind = 7.46 Kn-m  
Shear Wind = 2.37 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	6.47 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	12.07 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.62 < 1 OK

**Drained Lateral Strength of End pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For End Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	3150 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	7.46 Kn-m
Shear Wind =	2.37 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	6.47 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	12.07 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.62 < 1 OK

**Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1900) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1900)

Skin Friction = 29.16 Kn

Weight of Pile + Pile Skin Friction = 34.09 Kn

Uplift on one Pile = 30.15 Kn

Uplift is ok