

Job No.: Sara Stringer -2**Address:** 102 Hawker Lane, Koromiko, New Zealand**Date:** 13/04/2024**Latitude:** -41.341529**Longitude:** 173.958099**Elevation:** 24.5 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N3	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.6 m
Wind Region	NZ3	Terrain Category	2.57	Design Wind Speed	43.24 m/s
Wind Pressure	1.12 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.3 m $C_{p,e} = -0.9$ $p_e = -0.91$ KPa $p_{net} = -0.91$ KPa

For roof $C_{p,e}$ from 3.3 m To 6.6 m $C_{p,e} = -0.5$ $p_e = -0.5$ KPa $p_{net} = -0.5$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 7.2 m $C_{p,e} = 0.7$ $p_e = 0.71$ KPa $p_{net} = 1.05$ KPa

For side wall $C_{p,e}$ from 0 m To 3.3 m $C_{p,e} =$ $p_e = -0.66$ KPa $p_{net} = -0.66$ KPa

Maximum Upward pressure used in roof member Design = 0.91 KPa

Maximum Downward pressure used in roof member Design = 0.44 KPa

Maximum Wall pressure used in Design = 1.05 KPa

Maximum Racking pressure used in Design = 1.21 KPa

Design Summary**Rafter Design Internal**

Internal Rafter Load Width = 3625 mm

Internal Rafter Span = 7050 mm

Try Rafter 2x300x45 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 7.61 S1 Upward = 7.61

Shear Capacity of timber = 5 MPa Bending Capacity of timber = 38 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	7.60 Kn-m	Capacity	24.62 Kn-m	Passing Percentage	323.95 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	16.67 Kn-m	Capacity	32.84 Kn-m	Passing Percentage	197.00 %
$M_{0.9D-W_nUp}$	-15.43 Kn-m	Capacity	-41.04 Kn-m	Passing Percentage	265.98 %
$V_{1.35D}$	4.31 Kn	Capacity	43.42 Kn	Passing Percentage	1007.42 %

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V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	9.46 Kn	Capacity	57.88 Kn	Passing Percentage	611.84 %
V _{0.9D-WnUp}	-8.75 Kn	Capacity	-72.36 Kn	Passing Percentage	826.97 %

Deflections

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 17.085 mm

Limit by Woolcock et al, 1999 Span/240 = 30.00 mm

Deflection under Dead and Service Wind = 22.78 mm

Limit by Woolcock et al, 1999 Span/100 = 72.00 mm

Reactions

Maximum downward = 9.46 kn Maximum upward = -8.75 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 12.6 f_{pj} = 22.7 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -8.75 Kn

Girt Design Front and Back

Girt's Spacing = 1300 mm

Girt's Span = 1813 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 0.95 S₁ Downward = 9.63 S₁ Upward = 13.67

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.56 Kn-m	Capacity	1.99 Kn-m	Passing Percentage	355.36 %
V _{0.9D-WnUp}	1.24 Kn	Capacity	12.06 Kn	Passing Percentage	972.58 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 2.04 mm

Limit by Woolcock et al, 1999 Span/100 = 18.13 mm

Sag during installation = 0.65 mm

Reactions

Maximum = 1.24 kn

Girt Design Sides

Girt's Spacing = 900 mm

Girt's Span = 3600 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.95 S1 Downward =9.63 S1 Upward =13.62

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.53 Kn-m	Capacity	1.99 Kn-m	Passing Percentage	130.07 %
V _{0.9D-WnUp}	1.70 Kn	Capacity	12.06 Kn	Passing Percentage	709.41 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 21.94 mm

Limit by Woolcock et al. 1999 Span/100 = 36.00 mm

Sag during installation =10.18 mm

Reactions

Maximum = 1.70 kn

Middle Pole Design

Geometry

200 SED H5 HIGH DENSITY (Minimum 225 dia. at Floor Level)	Dry Use	Height	3300 mm
Area	35448 mm ²	As	26585.7421875 mm ²
I _x	100042702 mm ⁴	Z _x	941578 mm ³
I _y	100042702 mm ⁴	Z _y	941578 mm ³
Lateral Restraint	3400 mm c/c		

Loads

Total Area over Pole = 13.05 m²

Dead	3.26 Kn	Live	3.26 Kn
Wind Down	5.74 Kn	Snow	0.00 Kn
Moment wind	10.63 Kn-m		
Phi	0.8	K8	0.86
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
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fb =	49.725 MPa	fs =	2.84 MPa
fc =	28.125 MPa	fp =	8.66 MPa
ft =	29.64 MPa	E =	12874 MPa

Capacities

PhiNcx Wind	685.59 Kn	PhiMnx Wind	32.20 Kn-m	PhiVnx Wind	60.40 Kn
PhiNcx Dead	411.36 Kn	PhiMnx Dead	19.32 Kn-m	PhiVnx Dead	36.24 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.35 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.13 < 1$ OK

Deflection at top under service lateral loads = 13.78 mm < 33.00 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	2700 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	10.63 Kn-m
Shear Wind =	3.94 Kn

Pile Properties

Safety Factory	0.55	
Hu =	7.16 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	11.65 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.91 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

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Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1500) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1500)

Skin Friction = 18.17 Kn

Weight of Pile + Pile Skin Friction = 22.07 Kn

Uplift on one Pile = 8.94 Kn

Uplift is ok