

Job No.: 240410**Address:** 355 Golden Road, Springvale, Alexandra, New Zealand**Date:** 21/05/2024**Latitude:** -45.16889**Longitude:** 169.399416**Elevation:** 295 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.2 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	41.9 m/s
Wind Pressure	1.05 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.7 m $C_{p,e} = -0.9$ $p_e = -0.85$ KPa $p_{net} = -0.85$ KPa

For roof $C_{p,e}$ from 3.7 m To 7.4 m $C_{p,e} = -.05$ $p_e = -0.47$ KPa $p_{net} = -0.47$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 9 m $C_{p,e} = 0.7$ $p_e = 0.66$ KPa $p_{net} = 0.98$ KPa

For side wall $C_{p,e}$ from 0 m To 13.6 m $C_{p,e} =$ $p_e = -0.62$ KPa $p_{net} = -0.62$ KPa

Maximum Upward pressure used in roof member Design = 0.84 KPa

Maximum Downward pressure used in roof member Design = 0.5 KPa

Maximum Wall pressure used in Design = 0.98 KPa

Maximum Racking pressure used in Design = 1.13 KPa

Design Summary**Purlin Design**

Purlin Spacing = 900 mm

Purlin Span = 5050 mm

Try Purlin 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.97

K8 Upward = 0.62 S1 Downward = 12.68 S1 Upward = 21.13

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.97 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	350.52 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.67 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	169.66 %
M _{0.9D-W_nUp}	-1.76 Kn-m	Capacity	-3.59 Kn-m	Passing Percentage	203.98 %
V _{1.35D}	0.77 Kn	Capacity	12.06 Kn	Passing Percentage	1566.23 %

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V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	2.11 Kn	Capacity	16.08 Kn	Passing Percentage	762.09 %
V _{0.9D-WnUp}	-1.40 Kn	Capacity	-20.10 Kn	Passing Percentage	1435.71 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 10.07 mm Limit by Woolcock et al, 1999 Span/240 = 20.83 mm

Deflection under Dead and Service Wind = 12.59 mm Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

Reactions

Maximum downward = 2.11 kn Maximum upward = -1.40 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Intermediate Design Front and Back

Intermediate Spacing = 2600 mm Intermediate Span = 3050 mm Try Intermediate 2x200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 11.27 S₁ Upward = 0.66

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	2.96 Kn-m	Capacity	7.46 Kn-m	Passing Percentage	252.03 %
V _{0.9D-WnUp}	3.89 Kn	Capacity	-32.16 Kn	Passing Percentage	826.74 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 15.3 mm Limit by Woolcock et al, 1999 Span/100 = 30.50 mm

Reactions

Maximum = 3.89 kn

Intermediate Design Sides

Intermediate Spacing = 2250 mm Intermediate Span = 3800 mm Try Intermediate 2x200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 11.27 S₁ Upward = 0.73

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	Capacity
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	1.99 Kn-m		7.46 Kn-m	Passing Percentage	374.87 %
V _{0.9D-WnUp}	2.09 Kn	Capacity	32.16 Kn	Passing Percentage	1538.76 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 31.9 mm

Limit by Woolcock et al, 1999 Span/100 = 38.00 mm

Reactions

Maximum = 2.09 kn

Girt Design Front and Back

Girt's Spacing = 1300 mm

Girt's Span = 2600 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.84 S1 Downward =9.63 S1 Upward =16.37

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.08 Kn-m	Capacity	1.77 Kn-m	Passing Percentage	163.89 %
V _{0.9D-WnUp}	1.66 Kn	Capacity	12.06 Kn	Passing Percentage	726.51 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 13.22 mm

Limit by Woolcock et al, 1999 Span/100 = 26.00 mm

Sag during installation = 2.77 mm

Reactions

Maximum = 1.66 kn

Girt Design Sides

Girt's Spacing = 1300 mm

Girt's Span = 2250 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.89 S1 Downward =9.63 S1 Upward =15.23

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.81 Kn-m	Capacity	1.87 Kn-m	Passing Percentage	230.86 %
V _{0.9D-WnUp}	1.43 Kn	Capacity	12.06 Kn	Passing Percentage	843.36 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 7.41 mm

Limit by Woolcock et al. 1999 Span/100 = 22.50 mm

Sag during installation = 1.55 mm

Reactions

Maximum = 1.43 kn

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1500) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1500)

Skin Friction = 18.17 Kn

Weight of Pile + Pile Skin Friction = 22.07 Kn

Uplift on one Pile = 14.39 Kn

Uplift is ok