

Job No.: 2312001 - 1

Address: 49 Martin Loop, Mariri, New Zealand

Date: 30/01/2024

Latitude: -41.168836

Longitude: 173.03198

Elevation: 2.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N3	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	D
Importance Level	2	Ultimate wind & Earthquake ARI	500 Years	Max Height	5.4 m
Wind Region	NZ2	Terrain Category	1.0	Design Wind Speed	43.31 m/s
Wind Pressure	1.13 KPa	Lee Zone	NO	Ultimate Snow ARI	150 Years
Wind Category	High	Earthquake ARI	500		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Gable Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 2.61 m $C_{p,e} = -0.891$ $p_e = -0.90$ KPa $p_{net} = -0.90$ KPa

For roof $C_{p,e}$ from 2.61 m To 5.23 m $C_{p,e} = -0.891$ $p_e = -0.90$ KPa $p_{net} = -0.90$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 10 m $C_{p,e} = 0.7$ $p_e = 0.71$ KPa $p_{net} = 1.05$ KPa

For side wall $C_{p,e}$ from 0 m To 5.23 m $C_{p,e} =$ $p_e = -0.66$ KPa $p_{net} = -0.66$ KPa

Maximum Upward pressure used in roof member Design = 0.90 KPa

Maximum Downward pressure used in roof member Design = 0.33 KPa

Maximum Wall pressure used in Design = 1.05 KPa

Maximum Racking pressure used in Design = 0.91 KPa

Design Summary

Purlin Design

Purlin Spacing = 650 mm

Purlin Span = 4850 mm

Try Purlin 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.43 S1 Downward = 11.27 S1 Upward = 26.03

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.65 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	343.08 %
$M_{1.2D+1.5L\ 1.2D+S_n\ 1.2D+W_nD_n}$	1.91 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	155.50 %
$M_{0.9D-W_nUp}$	-1.29 Kn-m	Capacity	-1.59 Kn-m	Passing Percentage	150.00 %

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V _{1.35D}	0.53 Kn	Capacity	9.65 Kn	Passing Percentage	1820.75 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.06 Kn	Capacity	12.86 Kn	Passing Percentage	1213.21 %
V _{0.9D-W_nUp}	-1.06 Kn	Capacity	-16.08 Kn	Passing Percentage	1516.98 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 12.07 mm

Limit by Woolcock et al, 1999 Span/360 = 13.33 mm

Deflection under Dead and Service Wind = 13.38 mm

Limit by Woolcock et al, 1999 Span/250 = 32.00 mm

Reactions

Maximum downward = 1.06 kn Maximum upward = -1.06 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 5000 mm

Internal Rafter Span = 4850 mm

Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 6.81 S₁ Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	4.96 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	203.23 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	9.92 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	135.48 %
M _{0.9D-W_nUp}	-9.92 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	169.35 %
V _{1.35D}	4.09 Kn	Capacity	28.94 Kn	Passing Percentage	707.58 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	8.18 Kn	Capacity	38.6 Kn	Passing Percentage	471.88 %
V _{0.9D-W_nUp}	-8.18 Kn	Capacity	-48.24 Kn	Passing Percentage	589.73 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 9.04 mm

Limit by Woolcock et al, 1999 Span/360 = 13.89 mm

Deflection under Dead and Service Wind = 11.135 mm

Limit by Woolcock et al, 1999 Span/250 = 33.33 mm

Reactions

Maximum downward = 8.18 kn Maximum upward = -8.18 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Second page

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$ $f_{pj} = 12.9$ Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

$K_{11} = 2.0$ $f_{cj} = 36.1$ Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -8.18 Kn

Rafter Design External

External Rafter Load Width = 2500 mm

External Rafter Span = 4808 mm

Try Rafter 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 K_1 Medium term = 0.8 K_1 Long term = 0.6 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 1.00 S_1 Downward = 11.27 S_1 Upward = 11.27

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	2.44 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	91.39 %
$M_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	4.88 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	60.86 %
$M_{0.9D-W_nUp}$	-4.88 Kn-m	Capacity	-3.72 Kn-m	Passing Percentage	76.23 %
$V_{1.35D}$	2.03 Kn	Capacity	9.65 Kn	Passing Percentage	475.37 %
$V_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	4.06 Kn	Capacity	12.86 Kn	Passing Percentage	316.75 %
$V_{0.9D-W_nUp}$	-4.06 Kn	Capacity	-16.08 Kn	Passing Percentage	396.06 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k_2 for Long Term Loads = 2

Deflection under Dead and Live Load = 33.91 mm

Limit by Woolcock et al, 1999 Span/360 = 13.89 mm

Deflection under Dead and Service Wind = 37.58 mm

Limit by Woolcock et al, 1999 Span/250 = 33.33 mm

Reactions

Maximum downward = 4.06 kn Maximum upward = -4.06 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9 \text{ fpj} = 12.9 \text{ Mpa}$ for Rafter with effective thickness = 50 mm

For Parallel to grain loading

$K_{11} = 2.0 \text{ fcj} = 36.1 \text{ Mpa}$ for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -14.70 kn > -4.06 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -4.06 Kn

Girt Design Front and Back

Girt's Spacing = 600 mm

Girt's Span = 5000 mm

Try Girt 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.98

K_8 Upward = 0.65 S_1 Downward = 12.23 S_1 Upward = 20.38

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{\text{Wind+Snow}}$	1.97 Kn-m	Capacity	1.98 Kn-m	Passing Percentage	100.51 %
$V_{0.9D-WnUp}$	1.57 Kn-m	Capacity	13.75 Kn-m	Passing Percentage	875.80 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 29.75 mm

Limit by Woolcock et al, 1999 Span/250 = 20.00 mm

Sag during installation = 46.79 mm

Reactions

Maximum = 1.57 kn

Girt Design Sides

Girt's Spacing = 600 mm

Girt's Span = 5000 mm

Try Girt 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.98

K_8 Upward = 0.65 S_1 Downward = 12.23 S_1 Upward = 20.38

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{\text{Wind+Snow}}$	1.97 Kn-m	Capacity	1.98 Kn-m	Passing Percentage	100.51 %
$V_{0.9D-WnUp}$	1.57 Kn-m	Capacity	13.75 Kn-m	Passing Percentage	875.80 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

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Deflection under Snow and Service Wind = 29.75 mm

Limit by Woolcock et al. 1999 Span/100 = 20.00 mm

Sag during installation = 46.79 mm

Reactions

Maximum = 1.57 kn

Middle Pole Design

Geometry

275 SED H5 (Minimum 300 dia. at Floor Level)	Dry Use	Height	5100 mm
Area	64885 mm ²	As	48663.8671875 mm ²
Ix	335197731 mm ⁴	Zx	2331810 mm ³
Iy	335197731 mm ⁴	Zy	2331810 mm ³
Lateral Restraint	5100 mm c/c		

Loads

Total Area over Pole = 25 m²

Dead	6.25 Kn	Live	6.25 Kn
Wind Down	8.25 Kn	Snow	0.00 Kn
Moment wind	16.54 Kn-m		
Phi	0.8	K8	0.78
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	729.27 Kn	PhiMnx Wind	52.85 Kn-m	PhiVnx Wind	115.24 Kn
PhiNcx Dead	437.56 Kn	PhiMnx Dead	31.71 Kn-m	PhiVnx Dead	69.14 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.34 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.13 < 1$ OK

Deflection at top under service lateral loads = 20.63 mm < 34.00 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

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Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1700 mm	Pile embedment length
f1 =	4050 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	16.54 Kn-m
Shear Wind =	4.08 Kn

Pile Properties

Safety Factory	0.55	
Hu =	7.61 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	18.07 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.92 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	5200 mm
Area	35448 mm ²	As	26585.7421875 mm ²
Ix	100042702 mm ⁴	Zx	941578 mm ³
Iy	100042702 mm ⁴	Zy	941578 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 12.5 m²

Dead	3.13 Kn	Live	3.13 Kn
Wind Down	4.13 Kn	Snow	0.00 Kn
Moment Wind	8.27 Kn-m		
Phi	0.8	K8	0.48
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	243.49 Kn	PhiMnx Wind	13.04 Kn-m	PhiVnx Wind	62.96 Kn
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PhiNcx Dead	146.10 Kn	PhiMnx Dead	7.83 Kn-m	PhiVnx Dead	37.77 Kn
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Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.68 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.44 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 36.50 \text{ mm} < 35.91 \text{ mm}$$

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	4050 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

$$\text{Total Area over Pole} = 12.5 \text{ m}^2$$

Moment Wind =	8.27 Kn-m
Shear Wind =	2.04 Kn

Pile Properties

Safety Factory	0.55	
Hu =	3.66 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.52 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.97 < 1 \text{ OK}$$

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile**Assumed Soil Properties**

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	4050 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	8.27 Kn-m
Shear Wind =	2.04 Kn

Pile Properties

Safety Factory	0.55	
Hu =	3.66 Kn	Ultimate Lateral Strength of the Pile, Short pile

Mu = 8.52 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.97 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil (18) x Height of Pile (1700) x Ks (1.5) x $0.5 \times \tan(30)$ x π x Dia of Pile (0.6) x Height of Pile (1700)

Skin Friction = 23.34 Kn

Weight of Pile + Pile Skin Friction = 26.31 Kn

Uplift on one Pile = 16.88 Kn

Uplift is ok