

Pole Shed App Ver 01 2022

Job No.: Steve Maley-Canopy **Address:** 48 Morven Lane, Fairhall, New Zealand **Date:** 3/11/2025
Latitude: -41.547677 **Longitude:** 173.886444 **Elevation:** 49.5 m

General Input

| | | | | | |
|------------------|----------|--------------------------------|-----------|----------------------|-----------|
| Roof Live Load | 0.25 KPa | Roof Dead Load | 0.25 KPa | Roof Live Point Load | 1.1 Kn |
| Snow Zone | N3 | Ground Snow Load | 0 KPa | Roof Snow Load | 0 KPa |
| Earthquake Zone | 3 | Subsoil Category | D | Exposure Zone | B |
| Importance Level | 1 | Ultimate wind & Earthquake ARI | 100 Years | Max Height | 2.4 m |
| Wind Region | NZ2 | Terrain Category | 2.57 | Design Wind Speed | 39.18 m/s |
| Wind Pressure | 0.92 KPa | Lee Zone | NO | Ultimate Snow ARI | 50 Years |
| Wind Category | High | Earthquake ARI | 100 | | |

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Open

For roof $C_{p,i} = 0.49$

For roof $C_{p,e}$ from 0 m To 1.2 m $C_{p,e} = -1.3$ $p_e = -1.03$ KPa $p_{net} = -1.46$ KPa

For roof $C_{p,e}$ from 1.20 m To 1.50 m $C_{p,e} = -0.7$ $p_e = -0.55$ KPa $p_{net} = -0.98$ KPa

For wall Windward $C_{p,i} = 0.49$ side Wall $C_{p,i} = -0.65$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 3.50 m $C_{p,e} = 0.7$ $p_e = 0.57$ KPa $p_{net} = 1.16$ KPa

For side wall $C_{p,e}$ from 0 m To 2.40 m $C_{p,e} =$ $p_e = -0.53$ KPa $p_{net} = 0.06$ KPa

Maximum Upward pressure used in roof member Design = 1.46 KPa

Maximum Downward pressure used in roof member Design = 0.50 KPa

Maximum Wall pressure used in Design = 1.16 KPa

Maximum Racking pressure used in Design = 0.99 KPa

Design Summary

Purlin Design

Purlin Spacing = 700 mm Purlin Span = 3350 mm Try Purlin 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

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K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.98

K8 Upward = 0.52 S1 Downward = 12.23 S1 Upward = 23.41

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|---|------------|----------|------------|--------------------|------------------|
| M _{1.35D} | 0.33 Kn-m | Capacity | 1.79 Kn-m | Passing Percentage | 542.42 % |
| M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n} | 1.68 Kn-m | Capacity | 2.38 Kn-m | Passing Percentage | 141.67 % |
| M _{0.9D-W_nUp} | -1.21 Kn-m | Capacity | -1.56 Kn-m | Passing Percentage | 128.93 % |
| V _{1.35D} | 0.40 Kn | Capacity | 8.25 Kn | Passing Percentage | 2062.50 % |
| V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n} | 0.94 Kn | Capacity | 11.00 Kn | Passing Percentage | 1170.21 % |
| V _{0.9D-W_nUp} | -1.45 Kn | Capacity | -13.75 Kn | Passing Percentage | 948.28 % |

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 7.92 mm Limit by Woolcock et al, 1999 Span/240 = 13.75 mm

Deflection under Dead and Service Wind = 4.70 mm Limit by Woolcock et al, 1999 Span/100 = 33.00 mm

Reactions

Maximum downward = 0.94 kn Maximum upward = -1.45 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design External

External Rafter Load Width = 1750 mm External Rafter Span = 1313 mm Try Rafter 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.98

K8 Upward = 0.98 S1 Downward = 12.23 S1 Upward = 12.23

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|---|-----------|----------|-----------|--------------------|------------------|
| M _{1.35D} | 0.13 Kn-m | Capacity | 1.79 Kn-m | Passing Percentage | 1376.92 % |
| M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n} | 0.47 Kn-m | Capacity | 2.38 Kn-m | Passing Percentage | 506.38 % |

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| | | | | | |
|--|------------|----------|------------|--------------------|------------------|
| M _{0.9D-WnUp} | -0.47 Kn-m | Capacity | -2.98 Kn-m | Passing Percentage | 634.04 % |
| V _{1.35D} | 0.39 Kn | Capacity | 8.25 Kn | Passing Percentage | 2115.38 % |
| V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn} | 0.92 Kn | Capacity | 11.00 Kn | Passing Percentage | 1195.65 % |
| V _{0.9D-WnUp} | -1.42 Kn | Capacity | -13.75 Kn | Passing Percentage | 968.31 % |

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 0.25 mm Limit by Woolcock et al, 1999 Span/240= 6.25 mm

Deflection under Dead and Service Wind = 0.31 mm Limit by Woolcock et al, 1999 Span/100 = 15.00 mm

Reactions

Maximum downward = 0.92 kn Maximum upward = -1.42 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K₁₁ = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = $\phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s$ (Eq 4.12) = -12.28 kn > -1.42 Kn

Single Shear Capacity under short term loads = -9.75 Kn > -1.42 Kn

Girt Design Front and Back

Girt's Spacing = 0 mm

Girt's Span = 3500 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

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K1 Short term = 1 K4 =1 K5 =1 K8 Downward =NaN

K8 Upward =NaN S1 Downward =NaN S1 Upward =NaN

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|------------------------|-----------|----------|----------|--------------------|-------|
| M _{Wind+Snow} | 0.00 Kn-m | Capacity | NaN Kn-m | Passing Percentage | NaN % |
| V _{0.9D-WnUp} | 0.00 Kn | Capacity | 0.00 Kn | Passing Percentage | NaN % |

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

Sag during installation = NaN mm

Reactions

Maximum = 0.00 kn

Girt Design Sides

Girt's Spacing = 0 mm

Girt's Span = 1500 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =NaN

K8 Upward =NaN S1 Downward =NaN S1 Upward =NaN

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|------------------------|-----------|----------|----------|--------------------|-------|
| M _{Wind+Snow} | 0.00 Kn-m | Capacity | NaN Kn-m | Passing Percentage | NaN % |
| V _{0.9D-WnUp} | 0.00 Kn | Capacity | 0.00 Kn | Passing Percentage | NaN % |

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm Limit by Woolcock et al. 1999 Span/100 = 15.00 mm

Sag during installation =NaN mm

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Reactions

Maximum = 0.00 kn

End Pole Design

Geometry For End Bay Pole

Geometry

| | | | |
|-------------------|--------------------------|--------|--------------------------|
| 125x125 SG8 Dry | Dry Use | Height | 2200 mm |
| Area | 15625 mm ² | As | 11718.75 mm ² |
| Ix | 20345052 mm ⁴ | Zx | 325521 mm ³ |
| Iy | 20345052 mm ⁴ | Zy | 325521 mm ³ |
| Lateral Restraint | mm c/c | | |

Loads

Total Area over Pole = 2.625 m²

| | | | |
|-------------|-----------|---------|---------|
| Dead | 0.66 Kn | Live | 0.66 Kn |
| Wind Down | 1.31 Kn | Snow | 0.00 Kn |
| Moment Wind | 1.87 Kn-m | | |
| Phi | 0.8 | K8 | 0.79 |
| K1 snow | 0.8 | K1 Dead | 0.6 |
| K1 wind | 1 | | |

Material

| | | | |
|---------|----------|--------|----------|
| Shaving | Steaming | Normal | Dry Use |
| fb = | 36.3 MPa | fs = | 2.96 MPa |
| fc = | 18 MPa | fp = | 7.2 MPa |
| ft = | 22 MPa | E = | 9257 MPa |

Capacities

| | | | | | |
|-------------|-----------|-------------|-----------|-------------|----------|
| PhiNcx Wind | 177.16 Kn | PhiMnx Wind | 7.44 Kn-m | PhiVnx Wind | 27.75 Kn |
| PhiNcx Dead | 106.29 Kn | PhiMnx Dead | 4.47 Kn-m | PhiVnx Dead | 16.65 Kn |

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.27 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.08 < 1$ OK

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Deflection at top under service lateral loads = 8.00 mm < 23.94 mm

| | | |
|------|---------|--|
| Ds = | 0.6 mm | Pile Diameter |
| L = | 850 mm | Pile embedment length |
| f1 = | 1800 mm | Distance at which the shear force is applied |
| f2 = | 0 mm | Distance of top soil at rest pressure |

Loads

Total Area over Pole = 2.625 m²

| | |
|---------------|-----------|
| Moment Wind = | 1.87 Kn-m |
| Shear Wind = | 1.04 Kn |

Pile Properties

| | | |
|----------------|-----------|---|
| Safety Factory | 0.55 | |
| Hu = | 2.06 Kn | Ultimate Lateral Strength of the Pile, Short pile |
| Mu = | 2.20 Kn-m | Ultimate Moment Capacity of Pile |

Checks

Applied Forces/Capacities = 0.85 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

| | | | | | |
|-------|-----------------------------------|----------------|--------|----------|---------------------|
| Gamma | 18 Kn/m ³ | Friction angle | 30 deg | Cohesion | 0 Kn/m ³ |
| K0 = | $(1 - \sin(30)) / (1 + \sin(30))$ | | | | |
| Kp = | $(1 + \sin(30)) / (1 - \sin(30))$ | | | | |

Geometry For End Bay Pole

| | | |
|------|---------|--|
| Ds = | 0.6 mm | Pile Diameter |
| L = | 850 mm | Pile embedment length |
| f1 = | 1800 mm | Distance at which the shear force is applied |
| f2 = | 0 mm | Distance of top soil at rest pressure |

Loads

| | |
|---------------|-----------|
| Moment Wind = | 1.87 Kn-m |
| Shear Wind = | 1.04 Kn |

Pile Properties

| | | |
|---------------|-----------|---|
| Safety Factor | 0.55 | |
| Hu = | 2.06 Kn | Ultimate Lateral Strength of the Pile, Short pile |
| Mu = | 2.20 Kn-m | Ultimate Moment Capacity of Pile |

Checks

Applied Forces/Capacities = $0.85 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1400) x Ks(1.5) x $0.5 \times \tan(30)$ x π x Dia of Pile(0.6) x Height of Pile(1400)

Skin Friction = 15.83 Kn

Weight of Pile + Pile Skin Friction = 20.41 Kn

Uplift on one Pile = 3.24 Kn

Uplift is ok