Pole Shed App Ver 01 2022

Job No.:	EHB 850	**Address:**	149 Butson Road, Athol, Southland, New Zealand	**Date:**	11/30/2023
Latitude:	-45.487512	**Longitude:**	168.700849	**Elevation:**	387 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	1.58 KPa	Roof Snow Load	1.11 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	2	Ultimate wind & Earthquake ARI	500 Years	Max Height	4.4 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	43.05 m/s
Wind Pressure	1.11 KPa	Lee Zone	NO	Ultimate Snow ARI	150 Years
Wind Category	High	Earthquake ARI	500		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Mono Enclosed

For roof Cp,i = -0.3

For roof CP,e from 0 m To 3.70 m Cpe = -0.9 pe = -0.91 KPa pnet = -0.91 KPa

For roof CP,e from 3.70 m To 7.40 m Cpe = -0.5 pe = -0.51 KPa pnet = -0.51 KPa

For wall Windward Cp,i = -0.3 side Wall Cp,i = -0.3

For wall Windward and Leeward CP,e from 0 m To 3.70 m Cpe = 0.7 pe = 0.71 KPa pnet = 1.05 KPa

For side wall CP,e from 0 m To 3.70 m Cpe = pe = -0.66 KPa pnet = -0.66 KPa

Maximum Upward pressure used in roof member Design = 0.91 KPa

Maximum Downward pressure used in roof member Design = 0.51 KPa

Maximum Wall pressure used in Design = 1.05 KPa

Maximum Racking pressure used in Design = 1.22 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm Purlin Span = 4650 mm Try Purlin 250x50 SG8 Dry

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =0.97

K8 Upward =0.35   S1 Downward =12.68   S1 Upward =28.66

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.82 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	**414.63 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	3.43 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	**132.07 %**
$M_{0.9D-WnUp}$	-1.67 Kn-m	Capacity	-2.07 Kn-m	Passing Percentage	**123.95 %**
$V_{1.35D}$	0.71 Kn	Capacity	12.06 Kn	Passing Percentage	**1698.59 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	2.95 Kn	Capacity	16.08 Kn	Passing Percentage	**545.08 %**
$V_{0.9D-WnUp}$	-1.43 Kn	Capacity	-20.10 Kn	Passing Percentage	**1405.59 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 7.22 mm	Limit by Woolcock et al, 1999 Span/360 = 12.78 mm
Deflection under Dead and Service Wind = 9.08 mm	Limit by Woolcock et al, 1999 Span/250 = 30.67 mm

Reactions

Maximum downward =2.95 kn   Maximum upward = -1.43 kn

Number of Blocking = 0   if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4800 mm   Internal Rafter Span = 9450 mm   Try Rafter 2x450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =1.00   S1 Downward =6.68   S1 Upward =6.68

Shear Capacity of timber =5.3 MPa   Bending Capacity of timber =48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

Second page

$M_{1.35D}$	18.08 Kn-m	Capacity	91.56 Kn-m	Passing Percentage	**506.42 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	75.55 Kn-m	Capacity	122.08 Kn-m	Passing Percentage	**161.59 %**
$M_{0.9D-WnUp}$	-36.70 Kn-m	Capacity	-152.6 Kn-m	Passing Percentage	**415.80 %**
$V_{1.35D}$	7.65 Kn	Capacity	96.64 Kn	Passing Percentage	**1263.27 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	31.98 Kn	Capacity	128.86 Kn	Passing Percentage	**402.94 %**
$V_{0.9D-WnUp}$	-15.54 Kn	Capacity	-161.08 Kn	Passing Percentage	**1036.55 %**

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 13.62 mm Limit by Woolcock et al, 1999 Span/360 = 26.67 mm
Deflection under Dead and Service Wind = 19.04 mm Limit by Woolcock et al, 1999 Span/250 = 64.00 mm

Reactions

Maximum downward =31.98 kn Maximum upward = -15.54 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -15.54 Kn

Rafter Design External

External Rafter Load Width = 2400 mm External Rafter Span = 4651 mm Try Rafter 300x45 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =0.88

K8 Upward =0.88   S1 Downward =15.50   S1 Upward =15.50

Shear Capacity of timber =5 MPa   Bending Capacity of timber =38 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	2.19 Kn-m	Capacity	10.84 Kn-m	Passing Percentage	**494.98 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	9.15 Kn-m	Capacity	14.45 Kn-m	Passing Percentage	**157.92 %**
$M_{0.9D-WnUp}$	-4.45 Kn-m	Capacity	-18.07 Kn-m	Passing Percentage	**406.07 %**
$V_{1.35D}$	1.88 Kn	Capacity	21.71 Kn	Passing Percentage	**1154.79 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	7.87 Kn	Capacity	28.94 Kn	Passing Percentage	**367.73 %**
$V_{0.9D-WnUp}$	-3.82 Kn	Capacity	-36.18 Kn	Passing Percentage	**947.12 %**

Deflections

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 4.96 mm	Limit by Woolcock et al, 1999 Span/360= 13.33 mm
Deflection under Dead and Service Wind = 6.25 mm	Limit by Woolcock et al, 1999 Span/250 = 32.00 mm

Reactions

Maximum downward =7.87 kn   Maximum upward = -3.82 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds .... (Eq 4.12) = -37.80 kn > -3.82 Kn

Single Shear Capacity under short term loads = -14.56 Kn > -3.82 Kn

Girt Design Front and Back

Girt's Spacing = 1300 mm   Girt's Span = 2400 mm   Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =0.75   S1 Downward =11.27   S1 Upward =18.41

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.04 Kn-m	Capacity	2.79 Kn-m	Passing Percentage	**268.27 %**
$V_{0.9D-WnUp}$	1.73 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	**929.48 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 5.43 mm   Limit by Woolcock et al, 1999 Span/250 = 9.60 mm
Sag during installation = 2.01 mm

Reactions

Maximum = 1.73 kn

Girt Design Sides

Girt's Spacing = 900 mm   Girt's Span = 2400 mm   Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =0.75   S1 Downward =11.27   S1 Upward =18.41

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.72 Kn-m	Capacity	2.79 Kn-m	Passing Percentage	**387.50 %**
$V_{0.9D-WnUp}$	1.20 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	**1340.00 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 3.76 mm Limit by Woolcock et al. 1999 Span/100 = 9.60 mm
Sag during installation =2.01 mm

Reactions

Maximum = 1.20 kn

Middle Pole Design

Geometry

250 SED H5 (Minimum 275 dia. at Floor Level)	Dry Use	Height	3400 mm
Area	54091 mm2	As	40568.5546875 mm2
Ix	232952248 mm4	Zx	1774874 mm3
Iy	232952248 mm4	Zx	1774874 mm3
Lateral Restraint	3400 mm c/c		

Loads

Total Area over Pole = 23.04 m2

Dead	5.76 Kn	Live	5.76 Kn
Wind Down	11.75 Kn	Snow	25.57 Kn
Moment wind	21.20 Kn-m	Moment snow	8.32 Kn-m
Phi	0.8	K8	0.97
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	751.80 Kn	PhiMnx Wind	49.75 Kn-m	PhiVnx Wind	96.07 Kn
PhiNcx Dead	451.08 Kn	PhiMnx Dead	29.85 Kn-m	PhiVnx Dead	57.64 Kn
PhiNcx Snow	601.44 Kn	PhiMnx Snow	39.80 Kn-m	PhiVnx Snow	76.85 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.48 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.24 < 1 OK

Deflection at top under service lateral loads = 20.67 mm < 22.67 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1900 mm	Pile embedment length
f1 =	3300 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	21.20 Kn-m	Moment Snow =	Kn-m
Shear Wind =	6.43 Kn	Shear Snow =	8.32 Kn

Pile Properties

Safety Factory	0.55	
Hu =	11.75 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	23.46 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.90 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	4200 mm
Area	35448 mm2	As	26585.7421875 mm2
Ix	100042702 mm4	Zx	941578 mm3
Iy	100042702 mm4	Zx	941578 mm3
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 11.52 m2

Dead	2.88 Kn	Live	2.88 Kn
Wind Down	5.88 Kn	Snow	12.79 Kn
Moment Wind	7.07 Kn-m	Moment snow	2.77 Kn-m
Phi	0.8	K8	0.68
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	348.02 Kn	PhiMnx Wind	18.64 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	208.81 Kn	PhiMnx Dead	11.19 Kn-m	PhiVnx Dead	37.77 Kn
PhiNcx Snow	278.42 Kn	PhiMnx Snow	14.91 Kn-m	PhiVnx Snow	50.36 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.44 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.21 < 1 OK

Deflection at top under service lateral loads = 20.71 mm < 29.26 mm

Ds =	0.6 mm	Pile Diameter
L =	1900 mm	Pile embedment length

f1 =	3300 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 11.52 m2

Moment Wind =	7.07 Kn-m	Moment Snow =	2.77 Kn-m
Shear Wind =	2.14 Kn	Shear Snow =	2.77 Kn

Pile Properties

Safety Factory	0.55	
Hu =	11.75 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	23.46 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.30 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1900 mm	Pile embedment length
f1 =	3300 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	7.07 Kn-m	Moment Snow =	2.77 Kn-m
Shear Wind =	2.14 Kn	Shear Snow =	2.77 Kn

Pile Properties

Safety Factory	0.55	
Hu =	11.75 Kn	Ultimate Lateral Strength of the Pile, Short pile

Mu = 23.46 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.30 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(1900) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1900)

Skin Friction = 29.16 Kn

Weight of Pile + Pile Skin Friction = 32.97 Kn

Uplift on one Pile = 15.78 Kn

Uplift is ok