

Job No.: 5127036734**Address:** 122 Oldfield Road New Job, Kimbell, New Zealand**Date:** 04/07/2024**Latitude:** -44.0772**Longitude:** 170.776688**Elevation:** 382.5 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N4	Ground Snow Load	1.74 KPa	Roof Snow Load	0.84 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.694 m
Wind Region	NZ2	Terrain Category	1.78	Design Wind Speed	49.09 m/s
Wind Pressure	1.45 KPa	Lee Zone	YES	Ultimate Snow ARI	50 Years
Wind Category	Very High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Gable Open

For roof $C_{p,i} = 0.6885$

For roof $C_{p,e}$ from 0 m To 2.50 m $C_{p,e} = -0.3707$ $p_e = -0.26$ KPa $p_{net} = -0.84$ KPa

For roof $C_{p,e}$ from 2.50 m To 5 m $C_{p,e} = -0.6$ $p_e = -0.42$ KPa $p_{net} = -1.00$ KPa

For wall Windward $C_{p,i} = 0.6885$ side Wall $C_{p,i} = -0.6286$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 8 m $C_{p,e} = 0.7$ $p_e = 0.91$ KPa $p_{net} = 1.80$ KPa

For side wall $C_{p,e}$ from 0 m To 3.60 m $C_{p,e} =$ $p_e = -0.85$ KPa $p_{net} = 0.04$ KPa

Maximum Upward pressure used in roof member Design = 1.0 KPa

Maximum Downward pressure used in roof member Design = 1.02 KPa

Maximum Wall pressure used in Design = 1.80 KPa

Maximum Racking pressure used in Design = 1.56 KPa

Design Summary**Purlin Design**

Purlin Spacing = 600 mm

Purlin Span = 4850 mm

Try Purlin 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.75 S1 Downward = 11.27 S1 Upward = 18.41

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.6 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	371.67 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	2.33 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	127.47 %
$M_{0.9D-W_nUp}$	-1.37 Kn-m	Capacity	-2.79 Kn-m	Passing Percentage	203.65 %
$V_{1.35D}$	0.49 Kn	Capacity	9.65 Kn	Passing Percentage	1969.39 %

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V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	1.92 Kn	Capacity	12.86 Kn	Passing Percentage	669.79 %
V _{0.9D-WnUp}	-1.13 Kn	Capacity	-16.08 Kn	Passing Percentage	1423.01 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 11.14 mm Limit by Woolcock et al, 1999 Span/240 = 20.00 mm

Deflection under Dead and Service Wind = 18.76 mm Limit by Woolcock et al, 1999 Span/100 = 48.00 mm

Reactions

Maximum downward = 1.92 kn Maximum upward = -1.13 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design External

External Rafter Load Width = 2500 mm External Rafter Span = 2573 mm Try Rafter 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 0.97

K₈ Upward = 0.97 S₁ Downward = 12.68 S₁ Upward = 12.68

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.70 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	485.71 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	2.73 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	165.93 %
M _{0.9D-WnUp}	-1.60 Kn-m	Capacity	-5.67 Kn-m	Passing Percentage	354.38 %
V _{1.35D}	1.09 Kn	Capacity	12.06 Kn	Passing Percentage	1106.42 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	4.25 Kn	Capacity	16.08 Kn	Passing Percentage	378.35 %
V _{0.9D-WnUp}	-2.49 Kn	Capacity	-20.10 Kn	Passing Percentage	807.23 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 1.09 mm Limit by Woolcock et al, 1999 Span/240 = 10.42 mm

Deflection under Dead and Service Wind = 1.83 mm Limit by Woolcock et al, 1999 Span/100 = 25.00 mm

Reactions

Maximum downward = 4.25 kn Maximum upward = -2.49 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9 \text{ f} \cdot \text{p} \cdot \text{j} = 12.9 \text{ Mpa}$ for Rafter with effective thickness = 50 mm

For Parallel to grain loading

$K_{11} = 2.0 \text{ f} \cdot \text{c} \cdot \text{j} = 36.1 \text{ Mpa}$ for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots (\text{Eq 4.12}) = -19.95 \text{ kn} > -2.49 \text{ Kn}$

Single Shear Capacity under short term loads = -10.84 Kn > -2.49 Kn

Girt Design Front and Back

Girt's Spacing = 800 mm

Girt's Span = 5000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.54 S_1 Downward = 9.63 S_1 Upward = 22.70

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{\text{Wind+Snow}}$	4.50 Kn-m	Capacity	1.14 Kn-m	Passing Percentage	25.33 %
$V_{0.9D-WnUp}$	3.60 Kn	Capacity	12.06 Kn	Passing Percentage	335.00 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 182.42 mm

Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

Sag during installation = 37.90 mm

Reactions

Maximum = 3.60 kn

Girt Design Sides

Girt's Spacing = 800 mm

Girt's Span = 2500 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.86 S_1 Downward = 9.63 S_1 Upward = 16.05

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{\text{Wind+Snow}}$	1.13 Kn-m	Capacity	1.80 Kn-m	Passing Percentage	159.29 %
$V_{0.9D-WnUp}$	1.80 Kn	Capacity	12.06 Kn	Passing Percentage	670.00 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 11.40 mm

Limit by Woolcock et al. 1999 Span/100 = 25.00 mm

Sag during installation = 2.37 mm

Reactions

Maximum = 1.80 kn

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1300) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1300)

Skin Friction = 13.65 Kn

Weight of Pile + Pile Skin Friction = 17.02 Kn

Uplift on one Pile = 9.69 Kn

Uplift is ok