Job No.:	5115024437-1	**Address:**	29 Westview Dr, Ashburton,New Zealand	**Date:**	23/07/2024
Latitude:	-43.91125	**Longitude:**	171.714367	**Elevation:**	97.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N4	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.08 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	38.22 m/s
Wind Pressure	0.88 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Mono Open

For roof Cp,i = 0.4613

For roof CP,e from 0 m To 5 m Cpe = -0.8153 pe = -0.64 KPa pnet = -1.04 KPa

For roof CP,e from m To m Cpe = pe = KPa pnet = KPa

For wall Windward Cp,i = 0.4613 side Wall Cp,i = -0.5738

For wall Windward and Leeward CP,e from 0 m To 14.40 m Cpe = 0.7 pe = 0.55 KPa pnet = 1.05 KPa

For side wall CP,e from 0 m To 3.42 m Cpe = pe = -0.51 KPa pnet = -0.01 KPa

Maximum Upward pressure used in roof member Design = 1.04 KPa

Maximum Downward pressure used in roof member Design = 0.66 KPa

Maximum Wall pressure used in Design = 1.05 KPa

Maximum Racking pressure used in Design = 0.395 KPa

Design Summary

Purlin Design

Purlin Spacing = 850 mm	Purlin Span = 4650 mm	Try Purlin 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.77 S1 Downward =11.27 S1 Upward =18.02

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.78 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	**285.90 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	2.21 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	**134.39 %**
$M_{0.9D-WnUp}$	-1.87 Kn-m	Capacity	-2.86 Kn-m	Passing Percentage	**257.66 %**
$V_{1.35D}$	0.67 Kn	Capacity	9.65 Kn	Passing Percentage	**1440.30 %**

Second page

$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	1.90 Kn	Capacity	12.86 Kn	Passing Percentage	**676.84 %**
$V_{0.9D-WnUp}$	-1.61 Kn	Capacity	-16.08 Kn	Passing Percentage	**998.76 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 13.31 mm	Limit by Woolcock et al, 1999 Span/240 = 19.17 mm
Deflection under Dead and Service Wind = 18.42 mm	Limit by Woolcock et al, 1999 Span/100 = 46.00 mm

Reactions

Maximum downward =1.90 kn   Maximum upward = -1.61 kn

Number of Blocking = 1   if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4800 mm   Internal Rafter Span = 4850 mm   Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =1.00   S1 Downward =6.81   S1 Upward =6.81

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	4.76 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	**211.76 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	13.55 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	**99.19 %**
$M_{0.9D-WnUp}$	-11.50 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	**146.09 %**
$V_{1.35D}$	3.93 Kn	Capacity	28.94 Kn	Passing Percentage	**736.39 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	11.17 Kn	Capacity	38.6 Kn	Passing Percentage	**345.57 %**
$V_{0.9D-WnUp}$	-9.49 Kn	Capacity	-48.24 Kn	Passing Percentage	**508.32 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 8.68 mm	Limit by Woolcock et al, 1999 Span/240 = 20.83 mm
Deflection under Dead and Service Wind = 13.34 mm	Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

Reactions

Maximum downward =11.17 kn   Maximum upward = -9.49 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -9.49 Kn

Rafter Design External

External Rafter Load Width = 2400 mm External Rafter Span = 4975 mm Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.94

K8 Upward =0.94 S1 Downward =13.93 S1 Upward =13.93

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	2.51 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	**188.05 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	7.13 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	**88.36 %**
$M_{0.9D-WnUp}$	-6.05 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	**130.08 %**
$V_{1.35D}$	2.01 Kn	Capacity	14.47 Kn	Passing Percentage	**719.90 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	5.73 Kn	Capacity	19.30 Kn	Passing Percentage	**336.82 %**
$V_{0.9D-WnUp}$	-4.87 Kn	Capacity	-24.12 Kn	Passing Percentage	**495.28 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 9.65 mm	Limit by Woolcock et al, 1999 Span/240= 20.83 mm
Deflection under Dead and Service Wind = 13.34 mm	Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

Reactions

Maximum downward =5.73 kn Maximum upward = -4.87 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds .... (Eq 4.12) = -25.20 kn > -4.87 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -4.87 Kn

Girt Design Front and Back

Girt's Spacing = 0 mm   Girt's Span = 2400 mm   Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =NaN

K8 Upward =NaN   S1 Downward =NaN   S1 Upward =NaN

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	**NaN %**
$V_{0.9D-WnUp}$	0.00 Kn	Capacity	0.00 Kn	Passing Percentage	**NaN %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm   Limit by Woolcock et al, 1999 Span/100 = 24.00 mm

Sag during installation = NaN mm

Reactions

Maximum = 0.00 kn

Girt Design Sides

Girt's Spacing = 0 mm   Girt's Span = 2500 mm   Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =NaN

K8 Upward =NaN   S1 Downward =NaN   S1 Upward =NaN

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	**NaN %**
$V_{0.9D-WnUp}$	0.00 Kn	Capacity	0.00 Kn	Passing Percentage	**NaN %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm   Limit by Woolcock et al. 1999 Span/100 = 25.00 mm

Sag during installation =NaN mm

Reactions

Maximum = 0.00 kn

Middle Pole Design

Geometry

175x175 SG8 Dry	Dry Use	Height	3888 mm
Area	30625 mm2	As	22968.75 mm2
Ix	78157552 mm4	Zx	893229 mm3
Iy	78157552 mm4	Zx	893229 mm3
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 12 m2

Dead	3.00 Kn	Live	3.00 Kn
Wind Down	7.92 Kn	Snow	7.56 Kn
Moment wind	5.90 Kn-m	Moment snow	4.40 Kn-m
Phi	0.8	K8	1.00
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Shaving	Steaming	Normal	Dry Use
fb =	14 MPa	fs =	3 MPa
fc =	18 MPa	fp =	8.9 MPa
ft =	6 MPa	E =	8000 MPa

Capacities

PhiNcx Wind	441.00 Kn	PhiMnx Wind	10.00 Kn-m	PhiVnx Wind	55.13 Kn
PhiNcx Dead	264.60 Kn	PhiMnx Dead	6.00 Kn-m	PhiVnx Dead	33.08 Kn
PhiNcx Snow	352.80 Kn	PhiMnx Snow	8.00 Kn-m	PhiVnx Snow	44.10 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.63 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.38 < 1 OK

Deflection at top under service lateral loads = 21.04 mm < 38.88 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	3060 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	5.90 Kn-m	Moment Snow =	Kn-m
Shear Wind =	1.93 Kn	Shear Snow =	4.40 Kn

Pile Properties

Safety Factory	0.55	
Hu =	6.59 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	11.99 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.67 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

175x175 SG8 Dry	Dry Use	Height	3780 mm
Area	30625 mm2	As	22968.75 mm2
Ix	78157552 mm4	Zx	893229 mm3
Iy	78157552 mm4	Zx	893229 mm3
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 12 m2

Dead	3.00 Kn	Live	3.00 Kn
Wind Down	7.92 Kn	Snow	7.56 Kn
Moment Wind	2.95 Kn-m	Moment snow	2.20 Kn-m
Phi	0.8	K8	0.59
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Shaving	Steaming	Normal	Dry Use
fb =	14 MPa	fs =	3 MPa
fc =	18 MPa	fp =	8.9 MPa
ft =	6 MPa	E =	8000 MPa

Capacities

PhiNcx Wind	261.83 Kn	PhiMnx Wind	5.94 Kn-m	PhiVnx Wind	55.13 Kn

PhiNcx Dead	157.10 Kn	PhiMnx Dead	3.56 Kn-m	PhiVnx Dead	33.08 Kn
PhiNcx Snow	209.46 Kn	PhiMnx Snow	4.75 Kn-m	PhiVnx Snow	44.10 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.56 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.31 < 1 OK

Deflection at top under service lateral loads = 11.01 mm < 40.70 mm

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	3060 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 12 m2

Moment Wind =	2.95 Kn-m	Moment Snow =	2.20 Kn-m
Shear Wind =	0.96 Kn	Shear Snow =	2.20 Kn

Pile Properties

Safety Factory	0.55	
Hu =	6.59 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	11.99 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.33 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	3060 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	2.95 Kn-m	Moment Snow =	2.20 Kn-m
Shear Wind =	0.96 Kn	Shear Snow =	2.20 Kn

Pile Properties

Safety Factory	0.55

Hu =	6.59 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	11.99 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.33 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(1500) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1500)

Skin Friction = 18.17 Kn

Weight of Pile + Pile Skin Friction = 22.82 Kn

Uplift on one Pile = 9.78 Kn

Uplift is ok