

Job No.: Gray Cross
Latitude: -36.94595

Address: 67 Point View Drive, East Tamaki, New Zealand
Longitude: 174.919247

Date: 23/07/2024
Elevation: 55 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.1 m
Wind Region	NZ1	Terrain Category	3.0	Design Wind Speed	38.82 m/s
Wind Pressure	0.9 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 1.90 m $C_{p,e} = -1.1909$ $p_e = -0.97$ KPa $p_{net} = -0.97$ KPa

For roof $C_{p,e}$ from 1.90 m To 3.80 m $C_{p,e} = -0.7545$ $p_e = -0.61$ KPa $p_{net} = -0.61$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 10 m $C_{p,e} = 0.7$ $p_e = 0.57$ KPa $p_{net} = 0.84$ KPa

For side wall $C_{p,e}$ from 0 m To 3.80 m $C_{p,e} =$ $p_e = -0.53$ KPa $p_{net} = -0.53$ KPa

Maximum Upward pressure used in roof member Design = 0.97 KPa

Maximum Downward pressure used in roof member Design = 0.35 KPa

Maximum Wall pressure used in Design = 0.84 KPa

Maximum Racking pressure used in Design = 0.8 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 4250 mm

Try Purlin 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.98

K8 Upward = 0.73 S1 Downward = 12.23 S1 Upward = 18.68

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.69 Kn-m	Capacity	1.79 Kn-m	Passing Percentage	259.42 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	1.78 Kn-m	Capacity	2.38 Kn-m	Passing Percentage	133.71 %
$M_{0.9D-W_nUp}$	-1.51 Kn-m	Capacity	-2.23 Kn-m	Passing Percentage	147.68 %
$V_{1.35D}$	0.65 Kn	Capacity	8.25 Kn	Passing Percentage	1269.23 %

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V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.29 Kn	Capacity	11.00 Kn	Passing Percentage	852.71 %
V _{0.9D-W_nUp}	-1.42 Kn	Capacity	-13.75 Kn	Passing Percentage	968.31 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 12.70 mm Limit by Woolcock et al, 1999 Span/240 = 17.50 mm

Deflection under Dead and Service Wind = 14.28 mm Limit by Woolcock et al, 1999 Span/100 = 42.00 mm

Reactions

Maximum downward = 1.29 kn Maximum upward = -1.42 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design External

External Rafter Load Width = 2200 mm External Rafter Span = 4809 mm Try Rafter 290x45 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 0.89

K₈ Upward = 0.89 S₁ Downward = 15.23 S₁ Upward = 15.23

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	2.15 Kn-m	Capacity	3.78 Kn-m	Passing Percentage	175.81 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	4.29 Kn-m	Capacity	5.04 Kn-m	Passing Percentage	117.48 %
M _{0.9D-W_nUp}	-4.74 Kn-m	Capacity	-6.29 Kn-m	Passing Percentage	132.70 %
V _{1.35D}	1.79 Kn	Capacity	12.59 Kn	Passing Percentage	703.35 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	3.57 Kn	Capacity	16.79 Kn	Passing Percentage	470.31 %
V _{0.9D-W_nUp}	-3.94 Kn	Capacity	-20.98 Kn	Passing Percentage	532.49 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 10.88 mm Limit by Woolcock et al, 1999 Span/240 = 20.83 mm

Deflection under Dead and Service Wind = 12.23 mm Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

Reactions

Maximum downward = 3.57 kn Maximum upward = -3.94 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9 \text{ f}_{pj} = 12.9 \text{ Mpa}$ for Rafter with effective thickness = 45 mm

For Parallel to grain loading

$K_{11} = 2.0 \text{ f}_{cj} = 36.1 \text{ Mpa}$ for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots (\text{Eq 4.12}) = -21.73 \text{ kn} > -3.94 \text{ Kn}$

Single Shear Capacity under short term loads = -9.75 Kn > -3.94 Kn

Intermediate Design Sides

Intermediate Spacing = 2500 mm

Intermediate Span = 3800 mm

Try Intermediate 2x190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.98

K_8 Upward = 1.00 S_1 Downward = 12.23 S_1 Upward = 0.79

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{\text{Wind+Snow}}$	1.89 Kn-m	Capacity	6.06 Kn-m	Passing Percentage	320.63 %
$V_{0.9D-WnUp}$	1.99 Kn	Capacity	27.5 Kn	Passing Percentage	1381.91 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 20.52 mm

Limit by Woolcock et al, 1999 Span/100 = 38.00 mm

Reactions

Maximum = 1.99 kn

Girt Design Front and Back

Girt's Spacing = 600 mm

Girt's Span = 4400 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.85 S_1 Downward = 10.36 S_1 Upward = 16.20

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{\text{Wind+Snow}}$	1.22 Kn-m	Capacity	1.40 Kn-m	Passing Percentage	114.75 %
$V_{0.9D-WnUp}$	1.11 Kn	Capacity	10.13 Kn	Passing Percentage	912.61 %

Deflections

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Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 35.68 mm

Limit by Woolcock et al, 1999 Span/100 = 44.00 mm

Sag during installation = 28.06 mm

Reactions

Maximum = 1.11 kn

Girt Design Sides

Girt's Spacing = 1300 mm

Girt's Span = 2500 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.80 S1 Downward =10.36 S1 Upward =17.27

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.85 Kn-m	Capacity	1.32 Kn-m	Passing Percentage	155.29 %
$V_{0.9D-WnUp}$	1.36 Kn	Capacity	10.13 Kn	Passing Percentage	744.85 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 8.06 mm

Limit by Woolcock et al. 1999 Span/100 = 25.00 mm

Sag during installation =2.92 mm

Reactions

Maximum = 1.36 kn

End Pole Design

Geometry For End Bay Pole

Geometry

175 UNI H5	Dry Use	Height	3900 mm
Area	24041 mm ²	As	18030.46875 mm ²
I _x	46015259 mm ⁴	Z _x	525889 mm ³
I _y	46015259 mm ⁴	Z _y	525889 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 11 m²

Dead	2.75 Kn	Live	2.75 Kn
Wind Down	3.85 Kn	Snow	0.00 Kn
Moment Wind	3.69 Kn-m		
Phi	0.8	K8	0.56

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K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Shaving	Steaming	Normal	Dry Use
fb =	34.325 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	20.75 MPa	E =	8793 MPa

Capacities

PhiNcx Wind	194.81 Kn	PhiMnx Wind	8.13 Kn-m	PhiVnx Wind	42.70 Kn
PhiNcx Dead	116.89 Kn	PhiMnx Dead	4.88 Kn-m	PhiVnx Dead	25.62 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.50 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.25 < 1$ OK

Deflection at top under service lateral loads = 21.48 mm < 40.90 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3075 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 11 m²

Moment Wind =	3.69 Kn-m
Shear Wind =	1.20 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.47 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.07 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.46 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
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L =	1300 mm	Pile embedment length
f1 =	3075 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	3.69 Kn-m
Shear Wind =	1.20 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.47 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.07 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.46 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile() x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile()

Skin Friction = 0.00 Kn

Weight of Pile + Pile Skin Friction = 0.00 Kn

Uplift on one Pile = 16.39 Kn

Uplift is ok