



**Job No.:** 119 Owhiwa Road Parua Bay **Address:** 119 Owhiwa Road, Parua Bay, New Zealand**Date:** 16/10/2024**Latitude:** -35.7608**Longitude:** 174.452808**Elevation:** 110 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4 m
Wind Region	NZ1	Terrain Category	3.0	Design Wind Speed	53.06 m/s
Wind Pressure	1.69 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	extra High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.6 m  $C_{p,e} = -0.9$   $p_e = -1.37$  KPa  $p_{net} = -1.37$  KPa

For roof  $C_{p,e}$  from 3.6 m To 7.2 m  $C_{p,e} = -0.5$   $p_e = -0.76$  KPa  $p_{net} = -0.76$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 9 m  $C_{p,e} = 0.7$   $p_e = 1.06$  KPa  $p_{net} = 1.57$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.6 m  $C_{p,e} =$   $p_e = -0.99$  KPa  $p_{net} = -0.99$  KPa

Maximum Upward pressure used in roof member Design = 1.37 KPa

Maximum Downward pressure used in roof member Design = 0.66 KPa

Maximum Wall pressure used in Design = 1.57 KPa

Maximum Racking pressure used in Design = 1.75 KPa

**Design Summary****Purlin Design**

Purlin Spacing = 900 mm

Purlin Span = 3550 mm

Try Purlin 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.98

K8 Upward = 0.81 S1 Downward = 12.23 S1 Upward = 17.05

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

$M_{1.35D}$	0.48 Kn-m	Capacity	1.79 Kn-m	Passing Percentage	<b>372.92 %</b>
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	1.4 Kn-m	Capacity	2.38 Kn-m	Passing Percentage	<b>170.00 %</b>
$M_{0.9D-W_nUp}$	-1.62 Kn-m	Capacity	-2.46 Kn-m	Passing Percentage	<b>424.14 %</b>
$V_{1.35D}$	0.54 Kn	Capacity	8.25 Kn	Passing Percentage	<b>1527.78 %</b>

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V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	1.53 Kn	Capacity	11.00 Kn	Passing Percentage	<b>718.95 %</b>
V <sub>0.9D-WnUp</sub>	-1.83 Kn	Capacity	-13.75 Kn	Passing Percentage	<b>751.37 %</b>

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 6.12 mm                      Limit by Woolcock et al, 1999 Span/240 = 14.58 mm

Deflection under Dead and Service Wind = 8.47 mm                      Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

**Reactions**

Maximum downward = 1.53 kn    Maximum upward = -1.83 kn

Number of Blocking = 1    if 0 then no blocking required, if 1 then one midspan blocking required

**Rafter Design Internal**

Internal Rafter Load Width = 3700 mm                      Internal Rafter Span = 8850 mm                      Try Rafter 2x360x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1    K<sub>1</sub> Medium term = 0.8    K<sub>1</sub> Long term = 0.6    K<sub>4</sub> = 1    K<sub>5</sub> = 1    K<sub>8</sub> Downward = 1.00

K<sub>8</sub> Upward = 1.00    S<sub>1</sub> Downward = 5.90    S<sub>1</sub> Upward = 5.90

Shear Capacity of timber = 5.3 MPa    Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>1.35D</sub>	12.23 Kn-m	Capacity	60.82 Kn-m	Passing Percentage	<b>497.30 %</b>
M <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	34.78 Kn-m	Capacity	81.1 Kn-m	Passing Percentage	<b>233.18 %</b>
M <sub>0.9D-WnUp</sub>	-41.48 Kn-m	Capacity	-101.38 Kn-m	Passing Percentage	<b>244.41 %</b>
V <sub>1.35D</sub>	5.53 Kn	Capacity	77.32 Kn	Passing Percentage	<b>1398.19 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	15.72 Kn	Capacity	103.08 Kn	Passing Percentage	<b>655.73 %</b>
V <sub>0.9D-WnUp</sub>	-18.75 Kn	Capacity	-128.86 Kn	Passing Percentage	<b>687.25 %</b>

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 15.835 mm                      Limit by Woolcock et al, 1999 Span/240 = 37.50 mm

Deflection under Dead and Service Wind = 24.345 mm                      Limit by Woolcock et al, 1999 Span/100 = 90.00 mm

**Reactions**

Maximum downward = 15.72 kn    Maximum upward = -18.75 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 12.6$  fpj = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

$K_{11} = 2.0$  fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 43.67 Kn > -18.75 Kn

## Rafter Design External

External Rafter Load Width = 1850 mm

External Rafter Span = 4318 mm

Try Rafter 290x45 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_1$  Medium term = 0.8     $K_1$  Long term = 0.6     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 0.89

$K_8$  Upward = 0.89     $S_1$  Downward = 15.23     $S_1$  Upward = 15.23

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

## Capacity Checks

$M_{1.35D}$	1.46 Kn-m	Capacity	3.78 Kn-m	Passing Percentage	<b>258.90 %</b>
$M_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	4.14 Kn-m	Capacity	5.04 Kn-m	Passing Percentage	<b>121.74 %</b>
$M_{0.9D-W_nUp}$	-4.94 Kn-m	Capacity	-6.29 Kn-m	Passing Percentage	<b>127.33 %</b>
$V_{1.35D}$	1.35 Kn	Capacity	12.59 Kn	Passing Percentage	<b>932.59 %</b>
$V_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	3.83 Kn	Capacity	16.79 Kn	Passing Percentage	<b>438.38 %</b>
$V_{0.9D-W_nUp}$	-4.57 Kn	Capacity	-20.98 Kn	Passing Percentage	<b>459.08 %</b>

## Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

$k_2$  for Long Term Loads = 2

Deflection under Dead and Live Load = 6.00 mm

Limit by Woolcock et al, 1999 Span/240 = 18.75 mm

Deflection under Dead and Service Wind = 8.30 mm

Limit by Woolcock et al, 1999 Span/100 = 45.00 mm

## Reactions

Maximum downward = 3.83 kn    Maximum upward = -4.57 kn

## Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$  fpj = 12.9 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

$K_{11} = 2.0$   $f_{c,j} = 36.1$  Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$  (Eq 4.12) = -21.73 kn > -4.57 Kn

Single Shear Capacity under short term loads = -9.75 Kn > -4.57 Kn

### Intermediate Design Sides

Intermediate Spacing = 2250 mm

Intermediate Span = 3650 mm

Try Intermediate 2x190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 0.98

$K_8$  Upward = 1.00     $S_1$  Downward = 12.23     $S_1$  Upward = 0.78

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

$M_{Wind+Snow}$	2.94 Kn-m	Capacity	6.06 Kn-m	Passing Percentage	<b>206.12 %</b>
$V_{0.9D-WnUp}$	3.22 Kn	Capacity	27.5 Kn	Passing Percentage	<b>854.04 %</b>

### Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 29.39 mm

Limit by Woolcock et al, 1999 Span/100 = 36.50 mm

### Reactions

Maximum = 3.22 kn

### Girt Design Front and Back

Girt's Spacing = 850 mm

Girt's Span = 3700 mm

Try Girt 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 0.98

$K_8$  Upward = 0.79     $S_1$  Downward = 12.23     $S_1$  Upward = 17.53

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

$M_{Wind+Snow}$	2.28 Kn-m	Capacity	2.40 Kn-m	Passing Percentage	<b>105.26 %</b>
$V_{0.9D-WnUp}$	2.47 Kn	Capacity	13.75 Kn	Passing Percentage	<b>556.68 %</b>

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 18.90 mm

Limit by Woolcock et al, 1999 Span/100 = 37.00 mm

Sag during installation = 14.03 mm

#### Reactions

Maximum = 2.47 kn

#### Girt Design Sides

Girt's Spacing = 850 mm

Girt's Span = 2250 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.84    S1 Downward =10.36    S1 Upward =16.38

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>Wind+Snow</sub>	0.84 Kn-m	Capacity	1.39 Kn-m	Passing Percentage	<b>165.48 %</b>
V <sub>0.9D-WnUp</sub>	1.50 Kn	Capacity	10.13 Kn	Passing Percentage	<b>675.33 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 6.46 mm

Limit by Woolcock et al. 1999 Span/100 = 22.50 mm

Sag during installation =1.92 mm

#### Reactions

Maximum = 1.50 kn

#### Middle Pole Design

##### Geometry

225 SED H5 (Minimum 250 dia. at Floor Level)	Dry Use	Height	3710 mm
Area	44279 mm <sup>2</sup>	As	33209.1796875 mm <sup>2</sup>
I <sub>x</sub>	156100441 mm <sup>4</sup>	Z <sub>x</sub>	1314530 mm <sup>3</sup>
I <sub>y</sub>	156100441 mm <sup>4</sup>	Z <sub>y</sub>	1314530 mm <sup>3</sup>
Lateral Restraint	3710 mm c/c		

#### Loads

Total Area over Pole = 16.65 m<sup>2</sup>

Dead	4.16 Kn	Live	4.16 Kn
Wind Down	10.99 Kn	Snow	0.00 Kn
Moment wind	19.38 Kn-m		
Phi	0.8	K8	0.88
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

#### Material

Peeling	Steaming	Normal	Dry Use
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fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

**Capacities**

PhiNcx Wind	558.20 Kn	PhiMnx Wind	33.42 Kn-m	PhiVnx Wind	78.64 Kn
PhiNcx Dead	334.92 Kn	PhiMnx Dead	20.05 Kn-m	PhiVnx Dead	47.18 Kn

**Checks**

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.61 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.37 < 1$  OK

Deflection at top under service lateral loads = 27.96 mm < 37.10 mm

**Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For Middle Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	1800 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	19.38 Kn-m
Shear Wind =	6.46 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	10.83 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	19.75 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.98 < 1 OK

**End Pole Design**

**Geometry For End Bay Pole**

**Geometry**

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	3800 mm
Area	27598 mm <sup>2</sup>	As	20698.2421875 mm <sup>2</sup>
Ix	60639381 mm <sup>4</sup>	Zx	646820 mm <sup>3</sup>

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Iy	60639381 mm <sup>4</sup>	Zx	646820 mm <sup>3</sup>
Lateral Restraint	mm c/c		

**Loads**

Total Area over Pole = 8.325 m<sup>2</sup>

Dead	2.08 Kn	Live	2.08 Kn
Wind Down	5.49 Kn	Snow	0.00 Kn
Moment Wind	6.46 Kn-m		
Phi	0.8	K8	0.66
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

**Capacities**

PhiNcx Wind	261.19 Kn	PhiMnx Wind	12.35 Kn-m	PhiVnx Wind	49.01 Kn
PhiNcx Dead	156.71 Kn	PhiMnx Dead	7.41 Kn-m	PhiVnx Dead	29.41 Kn

**Checks**

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.56 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.31 < 1$  OK

Deflection at top under service lateral loads = 25.80 mm < 39.90 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Total Area over Pole = 8.325 m<sup>2</sup>

Moment Wind =	6.46 Kn-m
Shear Wind =	2.15 Kn

**Pile Properties**

Safety Factor	0.55	
Hu =	4.55 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.02 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.80 < 1 OK



## Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

### Assumed Soil Properties

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

### Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

### Loads

Moment Wind =	6.46 Kn-m
Shear Wind =	2.15 Kn

### Pile Properties

Safety Factory	0.55	
Hu =	4.55 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.02 Kn-m	Ultimate Moment Capacity of Pile

### Checks

Applied Forces/Capacities = 0.80 < 1 OK

## Uplift Check

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1800) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1800)

Skin Friction = 26.17 Kn

Weight of Pile + Pile Skin Friction = 30.29 Kn

Uplift on one Pile = 19.06 Kn

Uplift is ok