

**Job No.:** 2401016 - 1

**Address:** Thorpe Orinoco Road, Tasman, New Zealand

**Date:** 21/02/2024

**Latitude:** -41.251821

**Longitude:** 172.881289

**Elevation:** 196.5 m

### General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4 m
Wind Region	NZ2	Terrain Category	3.0	Design Wind Speed	55.84 m/s
Wind Pressure	1.87 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	extra High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

### Pressure Coefficients and Pressures

Shed Type = Mono Open

For roof  $C_{p,i} = -0.6027$

For roof  $C_{p,e}$  from 0 m To 8 m  $C_{p,e} = -0.434$   $p_e = -0.54$  KPa  $p_{net} = -1.20$  KPa

For roof  $C_{p,e}$  from m To m  $C_{p,e} =$   $p_e =$  KPa  $p_{net} =$  KPa

For wall Windward  $C_{p,i} = 0.4722$  side Wall  $C_{p,i} = -0.6027$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 9 m  $C_{p,e} = 0.7$   $p_e = 1.18$  KPa  $p_{net} = 2.19$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.25 m  $C_{p,e} =$   $p_e = -1.09$  KPa  $p_{net} = -0.08$  KPa

Maximum Upward pressure used in roof member Design = 1.20 KPa

Maximum Downward pressure used in roof member Design = 1.18 KPa

Maximum Wall pressure used in Design = 2.19 KPa

Maximum Racking pressure used in Design = 1.69 KPa

### Design Summary

#### Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 2850 mm

Try Purlin 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.82 S1 Downward = 9.63 S1 Upward = 16.99

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{1.35D}$	0.31 Kn-m	Capacity	1.26 Kn-m	Passing Percentage	<b>406.45 %</b>
$M_{1.2D+1.5L\ 1.2D+S_n\ 1.2D+W_nD_n}$	1.35 Kn-m	Capacity	1.68 Kn-m	Passing Percentage	<b>124.44 %</b>
$M_{0.9D-W_nUp}$	-0.89 Kn-m	Capacity	-1.71 Kn-m	Passing Percentage	<b>192.13 %</b>

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V <sub>1.35D</sub>	0.43 Kn	Capacity	7.24 Kn	Passing Percentage	<b>1683.72 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	1.90 Kn	Capacity	9.65 Kn	Passing Percentage	<b>507.89 %</b>
V <sub>0.9D-WnUp</sub>	-1.25 Kn	Capacity	-12.06 Kn	Passing Percentage	<b>964.80 %</b>

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 4.59 mm

Limit by Woolcock et al, 1999 Span/240 = 11.67 mm

Deflection under Dead and Service Wind = 8.33 mm

Limit by Woolcock et al, 1999 Span/100 = 28.00 mm

**Reactions**

Maximum downward = 1.90 kn Maximum upward = -1.25 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

**Girt Design Front and Back**

Girt's Spacing = 600 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1 K<sub>4</sub> = 1 K<sub>5</sub> = 1 K<sub>8</sub> Downward = 1.00

K<sub>8</sub> Upward = 0.79 S<sub>1</sub> Downward = 9.63 S<sub>1</sub> Upward = 17.59

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>Wind+Snow</sub>	1.48 Kn-m	Capacity	1.65 Kn-m	Passing Percentage	<b>111.49 %</b>
V <sub>0.9D-WnUp</sub>	1.97 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	<b>612.18 %</b>

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 14.71 mm

Limit by Woolcock et al, 1999 Span/100 = 30.00 mm

Sag during installation = 4.91 mm

**Reactions**

Maximum = 1.97 kn

**Girt Design Sides**

Girt's Spacing = 900 mm

Girt's Span = 2000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1 K<sub>4</sub> = 1 K<sub>5</sub> = 1 K<sub>8</sub> Downward = 1.00

K<sub>8</sub> Upward = 0.92 S<sub>1</sub> Downward = 9.63 S<sub>1</sub> Upward = 14.36

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

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### Capacity Checks

$M_{Wind+Snow}$	0.99 Kn-m	Capacity	1.94 Kn-m	Passing Percentage	<b>195.96 %</b>
$V_{0.9D-WnUp}$	1.97 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	<b>612.18 %</b>

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 4.36 mm

Limit by Woolcock et al. 1999 Span/100 = 20.00 mm

Sag during installation = 0.97 mm

### Reactions

Maximum = 1.97 kn

### End Pole Design

#### Geometry For End Bay Pole

#### Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3850 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
$I_x$	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
$I_y$	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Lateral Restraint	mm c/c		

### Loads

Total Area over Pole = 6 m<sup>2</sup>

Dead	1.50 Kn	Live	1.50 Kn
Wind Down	7.08 Kn	Snow	0.00 Kn
Moment Wind	5.06 Kn-m		
Phi	0.8	K8	0.76
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

### Material

Peeling	Steaming	Normal	Dry Use
$f_b =$	36.3 MPa	$f_s =$	2.96 MPa
$f_c =$	18 MPa	$f_p =$	7.2 MPa
$f_t =$	22 MPa	E =	9257 MPa

### Capacities

$\Phi N_{cx}$ Wind	389.24 Kn	$\Phi M_{nx}$ Wind	20.85 Kn-m	$\Phi V_{nx}$ Wind	62.96 Kn
$\Phi N_{cx}$ Dead	233.54 Kn	$\Phi M_{nx}$ Dead	12.51 Kn-m	$\Phi V_{nx}$ Dead	37.77 Kn

### Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.27 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.08 < 1$  OK

Deflection at top under service lateral loads = 12.25 mm < 39.90 mm

Ds =	0.6 mm	Pile Diameter
L =	1450 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

#### Loads

Total Area over Pole = 6 m<sup>2</sup>

Moment Wind =	5.06 Kn-m
Shear Wind =	1.69 Kn

#### Pile Properties

Safety Factory	0.55	
Hu =	6.10 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	10.87 Kn-m	Ultimate Moment Capacity of Pile

#### Checks

Applied Forces/Capacities = 0.47 < 1 OK

### Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

#### Assumed Soil Properties

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

#### Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1450 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

#### Loads

Moment Wind =	5.06 Kn-m
Shear Wind =	1.69 Kn

#### Pile Properties

Safety Factory	0.55	
Hu =	6.10 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	10.87 Kn-m	Ultimate Moment Capacity of Pile

#### Checks

Applied Forces/Capacities = 0.47 < 1 OK

### Uplift Check

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1600) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1600)

Skin Friction = 20.68 Kn

Weight of Pile + Pile Skin Friction = 25.36 Kn

Uplift on one Pile = 11.70 Kn

Uplift is ok