Pole Shed App Ver 01 2022

 Job No.:
 EHB 248-1
 Address:
 94 Brookdale Road, Kaka Point 9271, New Zealand
 Date:
 03/09/2024

 Latitude:
 -46.374119
 Longitude:
 169.76568
 Elevation:
 12.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	2	Ultimate wind & Earthquake ARI	500 Years	Max Height	5 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	41.82 m/s
Wind Pressure	1.05 KPa	Lee Zone	NO	Ultimate Snow ARI	150 Years
Wind Category	High	Earthquake ARI	500		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Gable Open

For roof Cp, i = 0.5616

For roof CP,e from 0 m To 3.47 m Cpe = -0.9 pe = -0.73 KPa pnet = -1.23 KPa

For roof CP,e from 3.47 m To 6.94 m Cpe = -0.5 pe = -0.41 KPa pnet = -0.90 KPa

For wall Windward Cp, i = 0.5616 side Wall Cp, i = -0.5771

For wall Windward and Leeward CP,e from 0 m To 12 m Cpe = 0.7 pe = 0.66 KPa pnet = 1.27 KPa

For side wall CP,e from 0 m To 3.47 m Cpe = pe = -0.61 KPa pnet = 0.00 KPa

Maximum Upward pressure used in roof member Design = 1.23 KPa

Maximum Downward pressure used in roof member Design = 0.80 KPa

Maximum Wall pressure used in Design = 1.27 KPa

Maximum Racking pressure used in Design = 1.07 KPa

Design Summary

Intermediate Design Front and Back

Intermediate Spacing = 3000 mm Intermediate Span = 2959 mm Try Intermediate 2x150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 9.63 S1 Upward = 0.55

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	4.17 Kn-m	Capacity	4.2 Kn-m	Passing Percentage	100.72 %
V _{0.9D-WnUp}	5.64 Kn	Capacity	-24.12 Kn	Passing Percentage	427.66 %

Deflections

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Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 42.785 mm

Limit byWoolcock et al, 1999 Span/250 = 11.84 mm

Reactions

Maximum = 5.64 kn

Girt Design Front and Back

Girt's Spacing = 600 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward =0.79 S1 Downward =9.63 S1 Upward =17.59

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

 $M_{Wind+Snow}$

0.86 Kn-m

Capacity

1.65 Kn-m

Passing Percentage

191.86 %

 $V_{0.9D\text{-}WnUp}$

1.14 Kn

Capacity

12.06 Kn

Passing Percentage

1057.89 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 12.76 mm

Limit by Woolcock et al, 1999 Span/250 = 12.00 mm

Sag during installation = 4.91 mm

Reactions

Maximum = 1.14 kn

Girt Design Sides

Girt's Spacing = 600 mm

Girt's Span = 2325 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.88 S1 Downward = 9.63 S1 Upward = 15.48

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

 $M_{Wind+Snow}$

0.51 Kn-m

Capacity

1.85 Kn-m

Passing Percentage

362.75 %

 $V_{0.9D\text{-W}nUp}$

0.89 Kn

Capacity

12.06 Kn

Passing Percentage

1355.06 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 4.60 mm

Limit by Woolcock et al. 1999 Span/100 = 9.30 mm

Sag during installation =1.77 mm

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Reactions

Maximum = 0.89 kn

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(2000) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(2000)

Skin Friction = 32.31 Kn

Weight of Pile + Pile Skin Friction = 35.33 Kn

Uplift on one Pile = 28.04 Kn

Uplift is ok