



Pole Shed App Ver 01 2022

**Job No.:** Wither Hills Farm  
Park Woolshed-Training Bay  
5870

**Address:** Lot 1 DP 8914, Redwood Street, Witherlea, New Zealand  
**Date:** 25/06/2025

**Latitude:** -41.544902

**Longitude:** 173.963463

**Elevation:** 48.5 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N3	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	6.8 m
Wind Region	NZ2	Terrain Category	1.91	Design Wind Speed	40.36 m/s
Wind Pressure	0.98 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Gable Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 4.5 m  $C_{p,e} = -1.0689$   $p_e = -0.94$  KPa  $p_{net} = -0.94$  KPa

For roof  $C_{p,e}$  from 4.5 m To 9 m  $C_{p,e} = -0.5844$   $p_e = -0.51$  KPa  $p_{net} = -0.51$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 25.13 m  $C_{p,e} = 0.7$   $p_e = 0.62$  KPa  $p_{net} = 0.91$  KPa

For side wall  $C_{p,e}$  from 0 m To 6.40 m  $C_{p,e} =$   $p_e = -0.57$  KPa  $p_{net} = -0.57$  KPa

Maximum Upward pressure used in roof member Design = 0.94 KPa

Maximum Downward pressure used in roof member Design = 0.47 KPa

Maximum Wall pressure used in Design = 0.91 KPa

Maximum Racking pressure used in Design = 0.88 KPa

**Design Summary**

**Purlin Design**

Purlin Spacing = 900 mm

Purlin Span = 5720 mm

Try Purlin 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

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condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 0.94

K8 Upward = 0.47    S1 Downward = 13.93    S1 Upward = 24.73

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>1.35D</sub>	1.24 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	<b>380.65 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	3.46 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	<b>182.08 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-2.63 Kn-m	Capacity	-3.94 Kn-m	Passing Percentage	<b>149.81 %</b>
V <sub>1.35D</sub>	0.87 Kn	Capacity	14.47 Kn	Passing Percentage	<b>1663.22 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	1.98 Kn	Capacity	19.30 Kn	Passing Percentage	<b>974.75 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-1.84 Kn	Capacity	-24.12 Kn	Passing Percentage	<b>1310.87 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 13.58 mm    Limit by Woolcock et al, 1999 Span/240 = 23.63 mm

Deflection under Dead and Service Wind = 11.81 mm    Limit by Woolcock et al, 1999 Span/100 = 56.70 mm

#### Reactions

Maximum downward = 1.98 kn    Maximum upward = -1.84 kn

Number of Blocking = 1    if 0 then no blocking required, if 1 then one midspan blocking required

#### Rafter Design Internal

Internal Rafter Load Width = 5145 mm    Internal Rafter Span = 8850 mm    Try Rafter 2x360x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 1.00    S1 Downward = 5.90    S1 Upward = 5.90

Shear Capacity of timber = 5.3 MPa    Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

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M <sub>1.35D</sub>	17.00 Kn-m	Capacity	60.82 Kn-m	Passing Percentage	<b>357.76 %</b>
M <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	38.79 Kn-m	Capacity	81.1 Kn-m	Passing Percentage	<b>209.07 %</b>
M <sub>0.9D-WnUp</sub>	-36.02 Kn-m	Capacity	-101.38 Kn-m	Passing Percentage	<b>281.45 %</b>
V <sub>1.35D</sub>	7.68 Kn	Capacity	77.32 Kn	Passing Percentage	<b>1006.77 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	17.53 Kn	Capacity	103.08 Kn	Passing Percentage	<b>588.02 %</b>
V <sub>0.9D-WnUp</sub>	-16.28 Kn	Capacity	-128.86 Kn	Passing Percentage	<b>791.52 %</b>

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 22.025 mm      Limit by Woolcock et al, 1999 Span/240 = 37.50 mm

Deflection under Dead and Service Wind = 29.975 mm      Limit by Woolcock et al, 1999 Span/100 = 90.00 mm

**Reactions**

Maximum downward = 17.53 kn    Maximum upward = -16.28 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 f<sub>pj</sub> = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 43.67 Kn > -16.28 Kn

**Intermediate Design Front and Back**

Intermediate Spacing = 2935 mm    Intermediate Span = 5849 mm    Try Intermediate 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

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K1 Short term = 1    K4 =1    K5 =1    K8 Downward =0.94

K8 Upward =1.00    S1 Downward =13.93    S1 Upward =1.12

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>Wind+Snow</sub>	11.42 Kn-m	Capacity	16.8 Kn-m	Passing Percentage	<b>147.11 %</b>
V <sub>0.9D-WnUp</sub>	7.81 Kn	Capacity	-48.24 Kn	Passing Percentage	<b>617.67 %</b>

**Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 33.505 mm    Limit by Woolcock et al, 1999 Span/100 = 58.49 mm

**Reactions**

Maximum = 7.81 kn

**Girt Design Front and Back**

Girt's Spacing = 900 mm                      Girt's Span = 2935 mm                      Try Girt 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =0.94

K8 Upward =0.46    S1 Downward =13.93    S1 Upward =25.16

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>Wind+Snow</sub>	0.88 Kn-m	Capacity	3.83 Kn-m	Passing Percentage	<b>435.23 %</b>
V <sub>0.9D-WnUp</sub>	1.20 Kn	Capacity	24.12 Kn	Passing Percentage	<b>2010.00 %</b>

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 1.05 mm    Limit by Woolcock et al, 1999 Span/100 = 29.35 mm

Sag during installation = 4.50 mm

**Reactions**

Maximum = 1.20 kn

### Girt Design Sides

Girt's Spacing = 900 mm

Girt's Span = 2250 mm

Try Girt 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =0.94

K8 Upward =0.57    S1 Downward =13.93    S1 Upward =22.03

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

M <sub>Wind+Snow</sub>	0.52 Kn-m	Capacity	4.82 Kn-m	Passing Percentage	<b>926.92 %</b>
V <sub>0.9D-WnUp</sub>	0.92 Kn	Capacity	24.12 Kn	Passing Percentage	<b>2621.74 %</b>

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 0.36 mm    Limit by Woolcock et al. 1999 Span/100 = 22.50 mm  
Sag during installation =1.55 mm

### Reactions

Maximum = 0.92 kn

### Middle Pole Design

#### Geometry

300 SED H5 (Minimum 325 dia. at Floor Level)	Dry Use	Height	6440 mm
Area	76660 mm <sup>2</sup>	As	57495.1171875 mm <sup>2</sup>
I <sub>x</sub>	467896461 mm <sup>4</sup>	Z <sub>x</sub>	2994537 mm <sup>3</sup>
I <sub>y</sub>	467896461 mm <sup>4</sup>	Z <sub>y</sub>	2994537 mm <sup>3</sup>
Lateral Restraint	6440 mm c/c		

### Loads

Total Area over Pole = 23.1525 m<sup>2</sup>

Dead	5.79 Kn	Live	5.79 Kn
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Wind Down	10.88 Kn	Snow	0.00 Kn
Moment wind	39.16 Kn-m		
Phi	0.8	K8	0.64
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

**Capacities**

PhiNcx Wind	706.94 Kn	PhiMnx Wind	55.69 Kn-m	PhiVnx Wind	136.15 Kn
PhiNcx Dead	424.17 Kn	PhiMnx Dead	33.41 Kn-m	PhiVnx Dead	81.69 Kn

**Checks**

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.73 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.53 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 55.63 \text{ mm} < 64.40 \text{ mm}$$

**Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For Middle Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	2400 mm	Pile embedment length
f1 =	5100 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	39.16 Kn-m
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Shear Wind =

7.68 Kn

#### **Pile Properties**

Safety Factory            0.55

Hu =                        16.42 Kn

Ultimate Lateral Strength of the Pile, Short pile

Mu =                        49.58 Kn-m

Ultimate Moment Capacity of Pile

#### **Checks**

Applied Forces/Capacities = 0.79 < 1 OK

#### **Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(2400) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(2400)

Skin Friction = 46.52 Kn

Weight of Pile + Pile Skin Friction = 50.15 Kn

Uplift on one Pile = 16.55 Kn

Uplift is ok