

Job No.: Heath Forsyth
Latitude: -41.497228

Address: 8 Waipuna Street, Mayfield, Blenheim 7201, New Zealand
Longitude: 173.952182

Date: 27/11/2024
Elevation: 6 m

General Input

| | | | | | |
|------------------|----------|--------------------------------|-----------|----------------------|-----------|
| Roof Live Load | 0.25 KPa | Roof Dead Load | 0.25 KPa | Roof Live Point Load | 1.1 Kn |
| Snow Zone | N3 | Ground Snow Load | 0 KPa | Roof Snow Load | 0 KPa |
| Earthquake Zone | 3 | Subsoil Category | D | Exposure Zone | B |
| Importance Level | 1 | Ultimate wind & Earthquake ARI | 100 Years | Max Height | 4.2 m |
| Wind Region | NZ2 | Terrain Category | 2.31 | Design Wind Speed | 34.91 m/s |
| Wind Pressure | 0.73 KPa | Lee Zone | NO | Ultimate Snow ARI | 50 Years |
| Wind Category | Medium | Earthquake ARI | 100 | | |

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.93 m $C_{p,e} = -0.9$ $p_e = -0.59$ KPa $p_{net} = -0.59$ KPa

For roof $C_{p,e}$ from 3.93 m To 7.85 m $C_{p,e} = -0.5$ $p_e = -0.33$ KPa $p_{net} = -0.33$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 13.6 m $C_{p,e} = 0.7$ $p_e = 0.46$ KPa $p_{net} = 0.68$ KPa

For side wall $C_{p,e}$ from 0 m To 3.93 m $C_{p,e} =$ $p_e = -0.43$ KPa $p_{net} = -0.43$ KPa

Maximum Upward pressure used in roof member Design = 0.59 KPa

Maximum Downward pressure used in roof member Design = 0.35 KPa

Maximum Wall pressure used in Design = 0.68 KPa

Maximum Racking pressure used in Design = 0.79 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 4650 mm

Try Purlin 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.44 S1 Downward = 11.27 S1 Upward = 25.48

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|--|------------|----------|------------|--------------------|------------------|
| M _{1.35D} | 0.82 Kn-m | Capacity | 2.23 Kn-m | Passing Percentage | 271.95 % |
| M _{1.2D+1.5L 1.2D+S_n 1.2D+W_{nDn}} | 2.01 Kn-m | Capacity | 2.97 Kn-m | Passing Percentage | 147.76 % |
| M _{0.9D-W_{nUp}} | -0.89 Kn-m | Capacity | -1.66 Kn-m | Passing Percentage | 186.52 % |
| V _{1.35D} | 0.71 Kn | Capacity | 9.65 Kn | Passing Percentage | 1359.15 % |

Pole Shed App Ver 01 2022

| | | | | | |
|--|----------|----------|-----------|--------------------|------------------|
| V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn} | 1.41 Kn | Capacity | 12.86 Kn | Passing Percentage | 912.06 % |
| V _{0.9D-WnUp} | -0.76 Kn | Capacity | -16.08 Kn | Passing Percentage | 2115.79 % |

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 14.10 mm Limit by Woolcock et al, 1999 Span/240 = 19.17 mm

Deflection under Dead and Service Wind = 15.86 mm Limit by Woolcock et al, 1999 Span/100 = 46.00 mm

Reactions

Maximum downward = 1.41 kn Maximum upward = -0.76 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4800 mm Internal Rafter Span = 4650 mm Try Rafter 2x250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 6.13 S₁ Upward = 6.13

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|--|------------|----------|-------------|--------------------|-----------------|
| M _{1.35D} | 4.38 Kn-m | Capacity | 7 Kn-m | Passing Percentage | 159.82 % |
| M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn} | 8.76 Kn-m | Capacity | 9.34 Kn-m | Passing Percentage | 106.62 % |
| M _{0.9D-WnUp} | -4.74 Kn-m | Capacity | -11.66 Kn-m | Passing Percentage | 245.99 % |
| V _{1.35D} | 3.77 Kn | Capacity | 24.12 Kn | Passing Percentage | 639.79 % |
| V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn} | 7.53 Kn | Capacity | 32.16 Kn | Passing Percentage | 427.09 % |
| V _{0.9D-WnUp} | -4.07 Kn | Capacity | -40.2 Kn | Passing Percentage | 987.71 % |

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 12.74 mm Limit by Woolcock et al, 1999 Span/240 = 20.00 mm

Deflection under Dead and Service Wind = 15.925 mm Limit by Woolcock et al, 1999 Span/100 = 48.00 mm

Reactions

Maximum downward = 7.53 kn Maximum upward = -4.07 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$ fpj = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

$K_{11} = 2.0$ fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -4.07 Kn

Rafter Design External

External Rafter Load Width = 2400 mm

External Rafter Span = 4608 mm

Try Rafter 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 K_1 Medium term = 0.8 K_1 Long term = 0.6 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.97

K_8 Upward = 0.97 S_1 Downward = 12.68 S_1 Upward = 12.68

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|--|------------|----------|------------|--------------------|-----------------|
| $M_{1.35D}$ | 2.15 Kn-m | Capacity | 3.40 Kn-m | Passing Percentage | 158.14 % |
| $M_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$ | 4.30 Kn-m | Capacity | 4.53 Kn-m | Passing Percentage | 105.35 % |
| $M_{0.9D-W_nUp}$ | -2.33 Kn-m | Capacity | -5.67 Kn-m | Passing Percentage | 243.35 % |
| $V_{1.35D}$ | 1.87 Kn | Capacity | 12.06 Kn | Passing Percentage | 644.92 % |
| $V_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$ | 3.73 Kn | Capacity | 16.08 Kn | Passing Percentage | 431.10 % |
| $V_{0.9D-W_nUp}$ | -2.02 Kn | Capacity | -20.10 Kn | Passing Percentage | 995.05 % |

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k_2 for Long Term Loads = 2

Deflection under Dead and Live Load = 14.16 mm

Limit by Woolcock et al, 1999 Span/240 = 20.00 mm

Deflection under Dead and Service Wind = 15.93 mm

Limit by Woolcock et al, 1999 Span/100 = 48.00 mm

Reactions

Maximum downward = 3.73 kn Maximum upward = -2.02 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$ fpj = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

$K_{11} = 2.0$ $f_{c,j} = 36.1$ Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi_i \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -19.95 kn > -2.02 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.02 Kn

Girt Design Front and Back

Girt's Spacing = 1200 mm

Girt's Span = 4800 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.75 S_1 Downward = 11.27 S_1 Upward = 18.41

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|-----------------|-----------|----------|-----------|--------------------|-----------------|
| $M_{Wind+Snow}$ | 2.35 Kn-m | Capacity | 2.79 Kn-m | Passing Percentage | 118.72 % |
| $V_{0.9D-WnUp}$ | 1.96 Kn | Capacity | 16.08 Kn | Passing Percentage | 820.41 % |

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 25.25 mm

Limit by Woolcock et al, 1999 Span/100 = 48.00 mm

Sag during installation = 32.19 mm

Reactions

Maximum = 1.96 kn

Girt Design Sides

Girt's Spacing = 1200 mm

Girt's Span = 4800 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.75 S_1 Downward = 11.27 S_1 Upward = 18.41

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

| | | | | | |
|-----------------|-----------|----------|-----------|--------------------|-----------------|
| $M_{Wind+Snow}$ | 2.35 Kn-m | Capacity | 2.79 Kn-m | Passing Percentage | 118.72 % |
| $V_{0.9D-WnUp}$ | 1.96 Kn | Capacity | 16.08 Kn | Passing Percentage | 820.41 % |

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 25.25 mm

Limit by Woolcock et al. 1999 Span/100 = 48.00 mm

Sag during installation = 32.19 mm

Reactions

Maximum = 1.96 kn

Middle Pole Design

Geometry

| | | | |
|--|--------------------------|--------|-------------------------------|
| 175 SED H5 (Minimum 200 dia. at Floor Level) | Dry Use | Height | 3900 mm |
| Area | 27598 mm ² | As | 20698.2421875 mm ² |
| Ix | 60639381 mm ⁴ | Zx | 646820 mm ³ |
| Iy | 60639381 mm ⁴ | Zx | 646820 mm ³ |
| Lateral Restraint | 3900 mm c/c | | |

Loads

Total Area over Pole = 23.04 m²

| | | | |
|-------------|-----------|---------|---------|
| Dead | 5.76 Kn | Live | 5.76 Kn |
| Wind Down | 8.06 Kn | Snow | 0.00 Kn |
| Moment wind | 8.34 Kn-m | | |
| Phi | 0.8 | K8 | 0.63 |
| K1 snow | 0.8 | K1 Dead | 0.6 |
| K1 wind | 1 | | |

Material

| | | | |
|---------|----------|--------|----------|
| Peeling | Steaming | Normal | Dry Use |
| fb = | 36.3 MPa | fs = | 2.96 MPa |
| fc = | 18 MPa | fp = | 7.2 MPa |
| ft = | 22 MPa | E = | 9257 MPa |

Capacities

| | | | | | |
|-------------|-----------|-------------|------------|-------------|----------|
| PhiNcx Wind | 250.83 Kn | PhiMnx Wind | 11.86 Kn-m | PhiVnx Wind | 49.01 Kn |
| PhiNcx Dead | 150.50 Kn | PhiMnx Dead | 7.11 Kn-m | PhiVnx Dead | 29.41 Kn |

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.78 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.57 < 1$ OK

Deflection at top under service lateral loads = 34.20 mm < 39.00 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

| | | | | | |
|-------|-----------------------------------|----------------|--------|----------|---------------------|
| Gamma | 18 Kn/m ³ | Friction angle | 30 deg | Cohesion | 0 Kn/m ³ |
| K0 = | $(1 - \sin(30)) / (1 + \sin(30))$ | | | | |
| Kp = | $(1 + \sin(30)) / (1 - \sin(30))$ | | | | |

Pole Shed App Ver 01 2022

Geometry For Middle Bay Pole

| | | |
|------|---------|--|
| Ds = | 0.6 mm | Pile Diameter |
| L = | 1600 mm | Pile embedment length |
| f1 = | 3150 mm | Distance at which the shear force is applied |
| f2 = | 0 mm | Distance of top soil at rest pressure |

Loads

| | |
|---------------|-----------|
| Moment Wind = | 8.34 Kn-m |
| Shear Wind = | 2.65 Kn |

Pile Properties

| | | |
|----------------|------------|---|
| Safety Factory | 0.55 | |
| Hu = | 7.68 Kn | Ultimate Lateral Strength of the Pile, Short pile |
| Mu = | 14.44 Kn-m | Ultimate Moment Capacity of Pile |

Checks

Applied Forces/Capacities = 0.58 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

| | | | |
|--|--------------------------|--------|-------------------------------|
| 150 SED H5 (Minimum 175 dia. at Floor Level) | Dry Use | Height | 3950 mm |
| Area | 20729 mm ² | As | 15546.6796875 mm ² |
| Ix | 34210793 mm ⁴ | Zx | 421056 mm ³ |
| Iy | 34210793 mm ⁴ | Zx | 421056 mm ³ |
| Lateral Restraint | mm c/c | | |

Loads

Total Area over Pole = 11.52 m²

| | | | |
|-------------|-----------|---------|---------|
| Dead | 2.88 Kn | Live | 2.88 Kn |
| Wind Down | 4.03 Kn | Snow | 0.00 Kn |
| Moment Wind | 4.17 Kn-m | | |
| Phi | 0.8 | K8 | 0.48 |
| K1 snow | 0.8 | K1 Dead | 0.6 |
| K1wind | 1 | | |

Material

| | | | |
|---------|----------|--------|----------|
| Peeling | Steaming | Normal | Dry Use |
| fb = | 36.3 MPa | fs = | 2.96 MPa |
| fc = | 18 MPa | fp = | 7.2 MPa |
| ft = | 22 MPa | E = | 9257 MPa |

Capacities

| | | | | | |
|-------------|-----------|-------------|-----------|-------------|----------|
| PhiNcx Wind | 144.04 Kn | PhiMnx Wind | 5.90 Kn-m | PhiVnx Wind | 36.81 Kn |
|-------------|-----------|-------------|-----------|-------------|----------|

Pole Shed App Ver 01 2022

| | | | | | |
|-------------|----------|-------------|-----------|-------------|----------|
| PhiNcx Dead | 86.42 Kn | PhiMnx Dead | 3.54 Kn-m | PhiVnx Dead | 22.09 Kn |
|-------------|----------|-------------|-----------|-------------|----------|

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.77 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.57 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 32.56 mm < 41.90 mm

| | | |
|------|---------|--|
| Ds = | 0.6 mm | Pile Diameter |
| L = | 1300 mm | Pile embedment length |
| f1 = | 3150 mm | Distance at which the shear force is applied |
| f2 = | 0 mm | Distance of top soil at rest pressure |

Loads

Total Area over Pole = 11.52 m²

| | |
|---------------|-----------|
| Moment Wind = | 4.17 Kn-m |
| Shear Wind = | 1.32 Kn |

Pile Properties

| | | |
|----------------|-----------|---|
| Safety Factory | 0.55 | |
| Hu = | 4.40 Kn | Ultimate Lateral Strength of the Pile, Short pile |
| Mu = | 8.11 Kn-m | Ultimate Moment Capacity of Pile |

Checks

Applied Forces/Capacities = 0.51 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

| | | | | | |
|-------|-----------------------------------|----------------|--------|----------|---------------------|
| Gamma | 18 Kn/m ³ | Friction angle | 30 deg | Cohesion | 0 Kn/m ³ |
| K0 = | $(1 - \sin(30)) / (1 + \sin(30))$ | | | | |
| Kp = | $(1 + \sin(30)) / (1 - \sin(30))$ | | | | |

Geometry For End Bay Pole

| | | |
|------|---------|--|
| Ds = | 0.6 mm | Pile Diameter |
| L = | 1300 mm | Pile embedment length |
| f1 = | 3150 mm | Distance at which the shear force is applied |
| f2 = | 0 mm | Distance of top soil at rest pressure |

Loads

| | |
|---------------|-----------|
| Moment Wind = | 4.17 Kn-m |
| Shear Wind = | 1.32 Kn |

Pile Properties

| | | |
|----------------|---------|---|
| Safety Factory | 0.55 | |
| Hu = | 4.40 Kn | Ultimate Lateral Strength of the Pile, Short pile |

Mu = 8.11 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.51 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil (18) x Height of Pile (1600) x Ks (1.5) x $0.5 \times \tan(30)$ x π x Dia of Pile (0.6) x Height of Pile (1600)

Skin Friction = 20.68 Kn

Weight of Pile + Pile Skin Friction = 25.36 Kn

Uplift on one Pile = 8.41 Kn

Uplift is ok