

**Job No.:** Daniel C - 483-205326

**Address:** 39 Sagewood Rd, Whakamarama, New Zealand

**Date:** 22/02/2024

**Latitude:** -37.683595

**Longitude:** 175.975359

**Elevation:** 132.5 m

### General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4 m
Wind Region	NZ1	Terrain Category	2.11	Design Wind Speed	38.65 m/s
Wind Pressure	0.9 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

### Pressure Coefficients and Pressures

Shed Type = Mono Open

For roof  $C_{p,i} = 0.6515$

For roof  $C_{p,e}$  from 0 m To 3.65 m  $C_{p,e} = -0.9$   $p_e = -0.73$  KPa  $p_{net} = -1.36$  KPa

For roof  $C_{p,e}$  from 3.65 m To 7.30 m  $C_{p,e} = -0.5$   $p_e = -0.40$  KPa  $p_{net} = -1.03$  KPa

For wall Windward  $C_{p,i} = 0.6515$  side Wall  $C_{p,i} = -0.5599$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 17 m  $C_{p,e} = 0.7$   $p_e = 0.56$  KPa  $p_{net} = 1.10$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.65 m  $C_{p,e} =$   $p_e = -0.52$  KPa  $p_{net} = 0.02$  KPa

Maximum Upward pressure used in roof member Design = 1.36 KPa

Maximum Downward pressure used in roof member Design = 0.70 KPa

Maximum Wall pressure used in Design = 1.10 KPa

Maximum Racking pressure used in Design = 0.96 KPa

### Design Summary

#### Rafter Design Internal

Internal Rafter Load Width = 3400 mm Internal Rafter Span = 5252.298850574713 mm Try Rafter 2x290x45 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 7.47 S1 Upward = 7.47

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>1.35D</sub>	3.96 Kn-m	Capacity	8.48 Kn-m	Passing Percentage	<b>214.14 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	11.72 Kn-m	Capacity	11.3 Kn-m	Passing Percentage	<b>96.42 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-13.31 Kn-m	Capacity	-14.12 Kn-m	Passing Percentage	<b>106.09 %</b>

Pole Shed App Ver 01 2022

V <sub>1.35D</sub>	3.01 Kn	Capacity	25.18 Kn	Passing Percentage	<b>836.54 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	8.93 Kn	Capacity	33.58 Kn	Passing Percentage	<b>376.04 %</b>
V <sub>0.9D-WnUp</sub>	-10.13 Kn	Capacity	-41.96 Kn	Passing Percentage	<b>414.22 %</b>

**Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 10.305 mm

Limit by Woolcock et al, 1999 Span/240 = 22.51 mm

Deflection under Dead and Service Wind = 16.225 mm

Limit by Woolcock et al, 1999 Span/100 = 54.02 mm

**Reactions**

Maximum downward = 8.93 kn Maximum upward = -10.13 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 14.9 f<sub>pj</sub> = 12.9 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 19.50 Kn > -10.13 Kn

**Intermediate Design Sides**

Intermediate Spacing = 2350 mm

Intermediate Span = 3674 mm

Try Intermediate 2x190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1 K<sub>4</sub> = 1 K<sub>5</sub> = 1 K<sub>8</sub> Downward = 0.98

K<sub>8</sub> Upward = 1.00 S<sub>1</sub> Downward = 12.23 S<sub>1</sub> Upward = 0.78

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>Wind+Snow</sub>	2.18 Kn-m	Capacity	6.06 Kn-m	Passing Percentage	<b>277.98 %</b>
V <sub>0.9D-WnUp</sub>	2.37 Kn-m	Capacity	27.5 Kn-m	Passing Percentage	<b>1160.34 %</b>

**Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 22.08 mm

Limit by Woolcock et al, 1999 Span/100 = 36.74 mm

#### Reactions

Maximum = 2.37 kn

#### Girt Design Front and Back

Girt's Spacing = 600 mm

Girt's Span = 3400 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.66 S1 Downward =10.36 S1 Upward =20.14

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>Wind+Snow</sub>	0.95 Kn-m	Capacity	1.09 Kn-m	Passing Percentage	<b>114.74 %</b>
V <sub>0.9D-WnUp</sub>	1.12 Kn-m	Capacity	10.13 Kn-m	Passing Percentage	<b>904.46 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 16.66 mm

Limit by Woolcock et al, 1999 Span/100 = 34.00 mm

Sag during installation = 10.00 mm

#### Reactions

Maximum = 1.12 kn

#### Girt Design Sides

Girt's Spacing = 900 mm

Girt's Span = 2350 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.83 S1 Downward =10.36 S1 Upward =16.74

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>Wind+Snow</sub>	0.68 Kn-m	Capacity	1.36 Kn-m	Passing Percentage	<b>200.00 %</b>
V <sub>0.9D-WnUp</sub>	1.16 Kn-m	Capacity	10.13 Kn-m	Passing Percentage	<b>873.28 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 5.70 mm

Limit by Woolcock et al. 1999 Span/100 = 23.50 mm

Sag during installation =2.28 mm

#### Reactions

Maximum = 1.16 kn

### **Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1500) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1500)

Skin Friction = 18.17 Kn

Weight of Pile + Pile Skin Friction = 22.31 Kn

Uplift on one Pile = 20.85 Kn

Uplift is ok