Job No.:	MFB Projects - 1	**Address:**	50 Whitecliffs Drive, Waiau Pa, New Zealand	**Date:**	25/03/2025
Latitude:	-37.153688	**Longitude:**	174.775368	**Elevation:**	21 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	D
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.4 m
Wind Region	NZ1	Terrain Category	2.23	Design Wind Speed	37.97 m/s
Wind Pressure	0.87 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Mono   Enclosed

For roof Cp,i = -0.3

For roof   CP,e   from 0 m   To 4.40 m   Cpe = -0.9   pe = -0.70 KPa   pnet = -0.70 KPa

For roof   CP,e   from 4.40 m   To 8.80 m   Cpe = -0.5   pe = -0.39 KPa   pnet = -0.39 KPa

For wall Windward Cp,i = -0.3   side Wall Cp,i = -0.3

For wall Windward and Leeward   CP,e   from 0 m   To 10 m   Cpe = 0.7   pe = 0.54 KPa   pnet = 0.80 KPa

For side wall   CP,e   from 0 m   To 4.40 m   Cpe =   pe = -0.50 KPa   pnet = -0.50 KPa

Maximum Upward pressure used in roof member Design = 0.70 KPa

Maximum Downward pressure used in roof member Design = 0.38 KPa

Maximum Wall pressure used in Design = 0.80 KPa

Maximum Racking pressure used in Design = 0.93 KPa

Design Summary

Purlin Design

Purlin Spacing = 800 mm   Purlin Span = 5350 mm   Try Purlin 240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =0.94

K8 Upward =0.50   S1 Downward =13.82   S1 Upward =23.71

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.97 Kn-m	Capacity	2.73 Kn-m	Passing Percentage	**281.44 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	3.07 Kn-m	Capacity	3.64 Kn-m	Passing Percentage	**118.57 %**
$M_{0.9D-WnUp}$	-1.36 Kn-m	Capacity	-2.44 Kn-m	Passing Percentage	**179.41 %**
$V_{1.35D}$	0.72 Kn	Capacity	10.42 Kn	Passing Percentage	**1447.22 %**

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$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	1.46 Kn	Capacity	13.89 Kn	Passing Percentage	**951.37 %**
$V_{0.9D-WnUp}$	-1.02 Kn	Capacity	-17.37 Kn	Passing Percentage	**1702.94 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 21.66 mm	Limit by Woolcock et al, 1999 Span/240 = 22.08 mm
Deflection under Dead and Service Wind = 16.33 mm	Limit by Woolcock et al, 1999 Span/100 = 53.00 mm

Reactions

Maximum downward =1.46 kn   Maximum upward = -1.02 kn

Number of Blocking = 1   if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 5500 mm   Internal Rafter Span = 4850 mm   Try Rafter 2x290x45 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =1.00   S1 Downward =7.47   S1 Upward =7.47

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	5.46 Kn-m	Capacity	8.48 Kn-m	Passing Percentage	**155.31 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	11.00 Kn-m	Capacity	11.3 Kn-m	Passing Percentage	**102.73 %**
$M_{0.9D-WnUp}$	-7.68 Kn-m	Capacity	-14.12 Kn-m	Passing Percentage	**183.85 %**
$V_{1.35D}$	4.50 Kn	Capacity	25.18 Kn	Passing Percentage	**559.56 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	9.07 Kn	Capacity	33.58 Kn	Passing Percentage	**370.23 %**
$V_{0.9D-WnUp}$	-6.34 Kn	Capacity	-41.96 Kn	Passing Percentage	**661.83 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 12.235 mm	Limit by Woolcock et al, 1999 Span/240 = 20.83 mm
Deflection under Dead and Service Wind = 15.635 mm	Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

Reactions

Maximum downward =9.07 kn   Maximum upward = -6.34 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 19.50 Kn > -6.34 Kn

Intermediate Design Front and Back

Intermediate Spacing = 2750 mm   Intermediate Span = 4250 mm   Try Intermediate 2x240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =0.94

K8 Upward =1.00   S1 Downward =13.82   S1 Upward =0.95

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	4.97 Kn-m	Capacity	9.68 Kn-m	Passing Percentage	**194.77 %**
$V_{0.9D-WnUp}$	4.67 Kn	Capacity	-34.74 Kn	Passing Percentage	**743.90 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 16.695 mm   Limit byWoolcock et al, 1999 Span/100 = 42.50 mm

Reactions

Maximum = 4.67 kn

Intermediate Design Sides

Intermediate Spacing = 2500 mm   Intermediate Span = 4050 mm   Try Intermediate 2x240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =0.94

K8 Upward =1.00   S1 Downward =13.82   S1 Upward =0.93

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	2.05 Kn-m	Capacity	9.68 Kn-m	Passing Percentage	**472.20 %**
$V_{0.9D-WnUp}$	2.02 Kn	Capacity	34.74 Kn	Passing Percentage	**1719.80 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 12.51 mm Limit by Woolcock et al, 1999 Span/100 = 40.50 mm

Reactions

Maximum = 2.02 kn

Girt Design Front and Back

Girt's Spacing = 1300 mm Girt's Span = 2750 mm Try Girt 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.98

K8 Upward =0.60 S1 Downward =12.23 S1 Upward =21.37

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.98 Kn-m	Capacity	1.83 Kn-m	Passing Percentage	**186.73 %**
$V_{0.9D-WnUp}$	1.43 Kn	Capacity	13.75 Kn	Passing Percentage	**961.54 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 4.49 mm Limit by Woolcock et al, 1999 Span/100 = 27.50 mm
Sag during installation = 4.28 mm

Reactions

Maximum = 1.43 kn

Girt Design Sides

Girt's Spacing = 900 mm Girt's Span = 2500 mm Try Girt 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.98

K8 Upward =0.65 S1 Downward =12.23 S1 Upward =20.38

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.56 Kn-m	Capacity	1.98 Kn-m	Passing Percentage	**353.57 %**
$V_{0.9D-WnUp}$	0.90 Kn	Capacity	13.75 Kn	Passing Percentage	**1527.78 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 2.13 mm Limit by Woolcock et al. 1999 Span/100 = 25.00 mm
Sag during installation =2.92 mm

Reactions

Maximum = 0.90 kn

Middle Pole Design

Geometry

225 UNI H5	Dry Use	Height	4440 mm
Area	39741 mm2	As	29805.46875 mm2
Ix	125741821 mm4	Zx	1117705 mm3
Iy	125741821 mm4	Zx	1117705 mm3
Lateral Restraint	4440 mm c/c		

Loads

Total Area over Pole = 27.5 m2

Dead	6.88 Kn	Live	6.88 Kn
Wind Down	10.45 Kn	Snow	0.00 Kn
Moment wind	12.35 Kn-m		
Phi	0.8	K8	0.68
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Shaving	Steaming	Normal	Dry Use
fb =	34.325 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	20.75 MPa	E =	8793 MPa

Capacities

PhiNcx Wind	390.91 Kn	PhiMnx Wind	20.97 Kn-m	PhiVnx Wind	70.58 Kn
PhiNcx Dead	234.55 Kn	PhiMnx Dead	12.58 Kn-m	PhiVnx Dead	42.35 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.65 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.41 < 1 OK

Deflection at top under service lateral loads = 30.65 mm < 44.40 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1600 mm	Pile embedment length

f1 =	3300 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	12.35 Kn-m
Shear Wind =	3.74 Kn

Pile Properties

Safety Factory	0.55	
Hu =	7.45 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	14.59 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.85 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(1600) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1600)

Skin Friction = 20.68 Kn

Weight of Pile + Pile Skin Friction = 24.58 Kn

Uplift on one Pile = 13.06 Kn

Uplift is ok