

Job No.: Anthony Loomes
Latitude: -35.730989

Address: 739 Owhiwa Rd. Parua Bay, New Zealand
Longitude: 174.427491

Date: 28/03/2024
Elevation: 214.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4 m
Wind Region	NZ1	Terrain Category	2.91	Design Wind Speed	45.78 m/s
Wind Pressure	1.26 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Very High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Open

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.5 m $C_{p,e} = -0.9$ $p_e = -1.02$ KPa $p_{net} = -1.02$ KPa

For roof $C_{p,e}$ from 3.5 m To 7.00 m $C_{p,e} = -0.5$ $p_e = -0.57$ KPa $p_{net} = -0.57$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 7.0 m $C_{p,e} = 0.7$ $p_e = 0.79$ KPa $p_{net} = 1.17$ KPa

For side wall $C_{p,e}$ from 0 m To 3.5 m $C_{p,e} =$ $p_e = -0.74$ KPa $p_{net} = -0.74$ KPa

Maximum Upward pressure used in roof member Design = 1.02 KPa

Maximum Downward pressure used in roof member Design = 0.49 KPa

Maximum Wall pressure used in Design = 1.17 KPa

Maximum Racking pressure used in Design = 1.36 KPa

Design Summary

Purlin Design

Purlin Spacing = 800 mm

Purlin Span = 3350 mm

Try Purlin 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.68 S1 Downward = 10.36 S1 Upward = 19.84

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.38 Kn-m	Capacity	0.99 Kn-m	Passing Percentage	260.53 %
$M_{1.2D+1.5L, 1.2D+S_n, 1.2D+W_nDn}$	1.26 Kn-m	Capacity	1.32 Kn-m	Passing Percentage	104.76 %
$M_{0.9D-W_nUp}$	-0.89 Kn-m	Capacity	-1.12 Kn-m	Passing Percentage	125.84 %
$V_{1.35D}$	0.45 Kn	Capacity	6.08 Kn	Passing Percentage	1351.11 %
$V_{1.2D+1.5L, 1.2D+S_n, 1.2D+W_nDn}$	1.06 Kn	Capacity	8.10 Kn	Passing Percentage	764.15 %
$V_{0.9D-W_nUp}$	-1.07 Kn	Capacity	-10.13 Kn	Passing Percentage	946.73 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 10.75 mm

Limit by Woolcock et al, 1999 Span/240 = 13.75 mm

Deflection under Dead and Service Wind = 13.35 mm

Limit by Woolcock et al, 1999 Span/100 = 33.00 mm

Reactions

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Maximum downward = 1.06 kn Maximum upward = -1.07 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 3500 mm

Internal Rafter Span = 3350 mm

Try Rafter 2x240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.71 S1 Upward = 6.71

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M1.35D	1.66 Kn-m	Capacity	5.8 Kn-m	Passing Percentage	349.40 %
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	3.88 Kn-m	Capacity	7.74 Kn-m	Passing Percentage	199.48 %
M0.9D-WnUp	-3.90 Kn-m	Capacity	-9.68 Kn-m	Passing Percentage	248.21 %
V1.35D	1.98 Kn	Capacity	20.84 Kn	Passing Percentage	1052.53 %
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	4.63 Kn	Capacity	27.78 Kn	Passing Percentage	600.00 %
V0.9D-WnUp	-4.66 Kn	Capacity	-34.74 Kn	Passing Percentage	745.49 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 3.3 mm

Limit by Woolcock et al, 1999 Span/240 = 14.58 mm

Deflection under Dead and Service Wind = 4.55 mm

Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

Reactions

Maximum downward = 4.63 kn Maximum upward = -4.66 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 19.50 Kn > -4.66 Kn

Rafter Design External

External Rafter Load Width = 1750 mm

External Rafter Span = 1861 mm

Try Rafter 240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.94

K8 Upward = 0.94 S1 Downward = 13.82 S1 Upward = 13.82

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

Pole Shed App Ver 01 2022

M1.35D	0.26 Kn-m	Capacity	2.73 Kn-m	Passing Percentage	1050.00 %
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	0.74 Kn-m	Capacity	3.64 Kn-m	Passing Percentage	491.89 %
M0.9D-WnUp	-0.60 Kn-m	Capacity	-4.55 Kn-m	Passing Percentage	758.33 %
V1.35D	0.55 Kn	Capacity	10.42 Kn	Passing Percentage	1894.55 %
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	1.29 Kn	Capacity	13.89 Kn	Passing Percentage	1076.74 %
V0.9D-WnUp	-1.29 Kn	Capacity	-17.37 Kn	Passing Percentage	1346.51 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 0.42 mm

Limit by Woolcock et al, 1999 Span/240= 8.50 mm

Deflection under Dead and Service Wind = 0.53 mm

Limit by Woolcock et al, 1999 Span/100 = 20.40 mm

Reactions

Maximum downward =1.29 kn Maximum upward = -1.29 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds (Eq 4.12) = -17.01 kn > -1.29 Kn

Single Shear Capacity under short term loads = -9.75 Kn > -1.29 Kn

Girt Design Front and Back

Girt's Spacing = 800 mm

Girt's Span = 3500 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.99 S1 Downward =10.36 S1 Upward =11.80

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

MWind+Snow	1.43 Kn-m	Capacity	1.63 Kn-m	Passing Percentage	113.99 %
V0.9D-WnUp	1.64 Kn	Capacity	10.13 Kn	Passing Percentage	617.68 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 26.53 mm

Limit by Woolcock et al, 1999 Span/100 = 35.00 mm

Sag during installation = 11.23 mm

Reactions

Maximum = 1.64 kn

Girt Design Sides

Girt's Spacing = 800 mm

Girt's Span = 2040 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.88 S1 Downward =10.36 S1 Upward =15.60

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.49 Kn-m	Capacity	1.44 Kn-m	Passing Percentage	293.88 %
$V_{0.9D-WnUp}$	0.95 Kn	Capacity	10.13 Kn	Passing Percentage	1066.32 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 3.06 mm

Limit by Woolcock et al. 1999 Span/100 = 20.40 mm

Sag during installation =1.30 mm

Reactions

Maximum = 0.95 kn

Middle Pole Design

Geometry

175 SED H5 HIGH DENSITY (Minimum 200 dia. at Floor Level)	Dry Use	Height	3760 mm
Area	27598 mm ²	As	20698.2421875 mm ²
I _x	60639381 mm ⁴	Z _x	646820 mm ³
I _y	60639381 mm ⁴	Z _y	646820 mm ³
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 12.25 m²

Dead	3.06 Kn	Live	3.06 Kn
Wind Down	6.00 Kn	Snow	0.00 Kn
Moment wind	9.50 Kn-m		
Phi	0.8	K8	1.00
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	49.725 MPa	f _s =	2.84 MPa
f _c =	28.125 MPa	f _p =	8.66 MPa
f _t =	29.64 MPa	E =	12874 MPa

Capacities

PhiN _{cx} Wind	620.95 Kn	PhiM _{nx} Wind	25.73 Kn-m	PhiV _{nx} Wind	47.03 Kn
PhiN _{cx} Dead	372.57 Kn	PhiM _{nx} Dead	15.44 Kn-m	PhiV _{nx} Dead	28.22 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.39 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.16 < 1$ OK

Deflection at top under service lateral loads = 25.71 mm < 37.60 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1400 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	9.50 Kn-m
Shear Wind =	3.17 Kn

Pile Properties

Safety Factory	0.55	
Hu =	5.56 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	9.86 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.96 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

150 SED H5 HIGH DENSITY (Minimum 175 dia. at Floor Level)	Dry Use	Height	3800 mm
Area	20729 mm2	As	15546.6796875 mm2
Ix	34210793 mm4	Zx	421056 mm3
Iy	34210793 mm4	Zy	421056 mm3
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 3.570075481595897 m2

Dead	0.89 Kn	Live	0.89 Kn
Wind Down	1.75 Kn	Snow	0.00 Kn
Moment Wind	3.21 Kn-m		
Phi	0.8	K8	0.52
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	49.725 MPa	fs =	2.84 MPa
fc =	28.125 MPa	fp =	8.66 MPa
ft =	29.64 MPa	E =	12874 MPa

Capacities

PhiNcx Wind	241.00 Kn	PhiMnx Wind	8.66 Kn-m	PhiVnx Wind	35.32 Kn
PhiNcx Dead	144.60 Kn	PhiMnx Dead	5.19 Kn-m	PhiVnx Dead	21.19 Kn

Checks

$$(M_x/\phi M_{nx}) + (N/\phi N_{cx}) = 0.39 < 1 \text{ OK}$$

$$(M_x/\phi M_{nx})^2 + (N/\phi N_{cx}) = 0.15 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 16.37 mm < 39.90 mm

Ds =	0.6 mm	Pile Diameter
L =	1400 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 3.570075481595897 m²

Moment Wind =	3.21 Kn-m
Shear Wind =	1.07 Kn

Pile Properties

Safety Factor	0.55	
Hu =	5.56 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	9.86 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.33 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1400 mm	Pile embedment length
f1 =	3000 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	3.21 Kn-m
Shear Wind =	1.07 Kn

Pile Properties

Safety Factor	0.55	
Hu =	5.56 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	9.86 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.33 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil (18) x Height of Pile (1400) x Ks (1.5) x 0.5 x tan(30) x Pi x Dia of Pile (0.6) x Height of Pile (1400)

Skin Friction = 15.83 Kn

Weight of Pile + Pile Skin Friction = 19.92 Kn

Uplift on one Pile = 9.74 Kn

Uplift is ok