

Pole Shed App Ver 01 2022

**Job No.:** Tod Thompson - 1      **Address:** 271 Ody Rd, Whangarei Heads, New Zealand      **Date:** 9/14/2023  
**Latitude:** -35.805435      **Longitude:** 174.531314      **Elevation:** 25.5 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4 m
Wind Region	NZ1	Terrain Category	2.0	Design Wind Speed	39.2 m/s
Wind Pressure	0.92 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Open

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.79 m  $C_{p,e} = -0.9$   $p_e = -0.75$  KPa  $p_{net} = -0.93$  KPa

For roof  $C_{p,e}$  from 3.79 m To 7.58 m  $C_{p,e} = -0.5$   $p_e = -0.41$  KPa  $p_{net} = -0.59$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 8 m  $C_{p,e} = 0.7$   $p_e = 0.58$  KPa  $p_{net} = 0.86$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.79 m  $C_{p,e} =$   $p_e = -0.54$  KPa  $p_{net} = -0.54$  KPa

Maximum Upward pressure used in roof member Design = 0.93 KPa

Maximum Downward pressure used in roof member Design = 0.43 KPa

Maximum Wall pressure used in Design = 0.86 KPa

Maximum Racking pressure used in Design = 0.99 KPa

**Design Summary**

**Rafter Design Internal**

Internal Rafter Load Width = 4000 mm      Internal Rafter Span = 7850 mm      Try Rafter 2x300x63 LVL11

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 1.00    S1 Downward = 5.30    S1 Upward = 5.30

Shear Capacity of timber = 5 MPa    Bending Capacity of timber = 38 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>1.35D</sub>	10.40 Kn-m	Capacity	34.48 Kn-m	Passing Percentage	<b>331.54 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	22.49 Kn-m	Capacity	45.96 Kn-m	Passing Percentage	<b>204.36 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-21.72 Kn-m	Capacity	-57.46 Kn-m	Passing Percentage	<b>264.55 %</b>
V <sub>1.35D</sub>	5.30 Kn	Capacity	60.78 Kn	Passing Percentage	<b>1146.79 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	11.46 Kn	Capacity	81.04 Kn	Passing Percentage	<b>707.16 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-11.07 Kn	Capacity	-101.3 Kn	Passing Percentage	<b>915.09 %</b>

#### **Deflections**

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 20.525 mm    Limit by Woolcock et al, 1999 Span/240 = 33.33 mm

Deflection under Dead and Service Wind = 27.175 mm    Limit by Woolcock et al, 1999 Span/100 = 80.00 mm

#### **Reactions**

Maximum downward = 11.46 kn    Maximum upward = -11.07 kn

#### **Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 f<sub>pj</sub> = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

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$K_{11} = 2.0$  fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -11.07 Kn

#### **Girt Design Front and Back**

Girt's Spacing = 900 mm

Girt's Span = 4000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 1.00

$K_8$  Upward = 0.92     $S_1$  Downward = 9.63     $S_1$  Upward = 14.36

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

$M_{Wind+Snow}$	1.55 Kn-m	Capacity	1.94 Kn-m	Passing Percentage	<b>125.16 %</b>
$V_{0.9D-WnUp}$	1.55 Kn-m	Capacity	12.06 Kn-m	Passing Percentage	<b>778.06 %</b>

#### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 27.38 mm    Limit by Woolcock et al, 1999 Span/100 = 40.00 mm

Sag during installation = 15.52 mm

#### **Reactions**

Maximum = 1.55 kn

#### **Girt Design Sides**

Girt's Spacing = 900 mm

Girt's Span = 4000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 1.00

$K_8$  Upward = 0.92     $S_1$  Downward = 9.63     $S_1$  Upward = 14.36

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

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**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

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Sag during installation = 15.52 mm

**Reactions**

Maximum = 1.55 kn

**Middle Pole Design**

**Geometry**

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3700 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
I <sub>x</sub>	100042702 mm <sup>4</sup>	Z <sub>x</sub>	941578 mm <sup>3</sup>
I <sub>y</sub>	100042702 mm <sup>4</sup>	Z <sub>y</sub>	941578 mm <sup>3</sup>
Lateral Restraint	3700 mm c/c		

**Loads**

Total Area over Pole = 16 m<sup>2</sup>

Dead	4.00 Kn	Live	4.00 Kn
Wind Down	6.88 Kn	Snow	0.00 Kn
Moment wind	11.85 Kn-m		
Phi	0.8	K <sub>8</sub>	0.80
K <sub>1</sub> snow	0.8	K <sub>1</sub> Dead	0.6
K <sub>1</sub> wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
f <sub>b</sub> =	36.3 MPa	f <sub>s</sub> =	2.96 MPa
f <sub>c</sub> =	18 MPa	f <sub>p</sub> =	7.2 MPa
f <sub>t</sub> =	22 MPa	E =	9257 MPa

**Capacities**

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PhiNcx Wind	406.33 Kn	PhiMnx Wind	21.77 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	243.80 Kn	PhiMnx Dead	13.06 Kn-m	PhiVnx Dead	37.77 Kn

**Checks**

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.58 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.33 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 26.61 \text{ mm} < 37.00 \text{ mm}$$

**Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K <sub>0</sub> =	$(1 - \sin(30)) / (1 + \sin(30))$				
K <sub>p</sub> =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For Middle Bay Pole**

D <sub>s</sub> =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f <sub>1</sub> =	3000 mm	Distance at which the shear force is applied
f <sub>2</sub> =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	11.85 Kn-m
Shear Wind =	3.95 Kn

**Pile Properties**

Safety Factory	0.55	
H <sub>u</sub> =	6.68 Kn	Ultimate Lateral Strength of the Pile, Short pile
M <sub>u</sub> =	11.94 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

$$\text{Applied Forces/Capacities} = 0.99 < 1 \text{ OK}$$

**Uplift Check**

$$\text{Density of Concrete} = 24 \text{ Kn/m}^3$$

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Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1500) x Ks(1.5) x  $0.5 \times \tan(30)$  x  $\pi$  x Dia of Pile(0.6) x Height of Pile(1500)

Skin Friction = 18.17 Kn

Weight of Pile + Pile Skin Friction = 22.07 Kn

Uplift on one Pile = 11.28 Kn

Uplift is ok