



**Job No.:** Kelly Stoneman**Address:** 185 Mangapai Road, Mangapai, New Zealand**Date:** 30/05/2024**Latitude:** -35.841495**Longitude:** 174.299375**Elevation:** 32 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.2 m
Wind Region	NZ1	Terrain Category	2.05	Design Wind Speed	42.81 m/s
Wind Pressure	1.1 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Open

For roof  $C_{p,i} = 0.6894$

For roof  $C_{p,e}$  from 0 m To 2.95 m  $C_{p,e} = -0.9$   $p_e = -0.77$  KPa  $p_{net} = -1.49$  KPa

For roof  $C_{p,e}$  from 2.95 m To 5.90 m  $C_{p,e} = -0.5$   $p_e = -0.43$  KPa  $p_{net} = -1.15$  KPa

For wall Windward  $C_{p,i} = 0.6894$  side Wall  $C_{p,i} = -0.6304$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 6 m  $C_{p,e} = 0.7$   $p_e = 0.69$  KPa  $p_{net} = 1.29$  KPa

For side wall  $C_{p,e}$  from 0 m To 2.95 m  $C_{p,e} =$   $p_e = -0.64$  KPa  $p_{net} = -0.04$  KPa

Maximum Upward pressure used in roof member Design = 1.49 KPa

Maximum Downward pressure used in roof member Design = 0.76 KPa

Maximum Wall pressure used in Design = 1.29 KPa

Maximum Racking pressure used in Design = 1.18 KPa

**Design Summary****Purlin Design**

Purlin Spacing = 900 mm

Purlin Span = 5850 mm

Try Purlin 240x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.94

K8 Upward = 0.47 S1 Downward = 13.82 S1 Upward = 24.81

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>1.35D</sub>	1.3 Kn-m	Capacity	9.37 Kn-m	Passing Percentage	720.77 %
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	4.08 Kn-m	Capacity	12.49 Kn-m	Passing Percentage	306.13 %
M <sub>0.9D-W<sub>n</sub>Up</sub>	-4.87 Kn-m	Capacity	-7.73 Kn-m	Passing Percentage	1288.33 %
V <sub>1.35D</sub>	0.89 Kn	Capacity	18.41 Kn	Passing Percentage	2068.54 %

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V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	2.79 Kn	Capacity	24.54 Kn	Passing Percentage	<b>879.57 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-3.33 Kn	Capacity	-30.68 Kn	Passing Percentage	<b>921.32 %</b>

#### **Deflections**

Modulus of Elasticity = 12100 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 12.69 mm                      Limit by Woolcock et al, 1999 Span/240 = 24.17 mm

Deflection under Dead and Service Wind = 18.60 mm                      Limit by Woolcock et al, 1999 Span/100 = 58.00 mm

#### **Reactions**

Maximum downward = 2.79 kn    Maximum upward = -3.33 kn

Number of Blocking = 1    if 0 then no blocking required, if 1 then one midspan blocking required

#### **Rafter Design External**

External Rafter Load Width = 3000 mm                      External Rafter Span = 2810 mm                      Try Rafter 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1    K<sub>1</sub> Medium term = 0.8    K<sub>1</sub> Long term = 0.6    K<sub>4</sub> = 1    K<sub>5</sub> = 1    K<sub>8</sub> Downward = 0.97

K<sub>8</sub> Upward = 0.97    S<sub>1</sub> Downward = 12.68    S<sub>1</sub> Upward = 12.68

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>1.35D</sub>	1.00 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	<b>340.00 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	3.14 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	<b>144.27 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-3.75 Kn-m	Capacity	-5.67 Kn-m	Passing Percentage	<b>151.20 %</b>
V <sub>1.35D</sub>	1.42 Kn	Capacity	12.06 Kn	Passing Percentage	<b>849.30 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	4.47 Kn	Capacity	16.08 Kn	Passing Percentage	<b>359.73 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-5.33 Kn	Capacity	-20.10 Kn	Passing Percentage	<b>377.11 %</b>

#### **Deflections**

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 2.70 mm                      Limit by Woolcock et al, 1999 Span/240 = 12.50 mm

Deflection under Dead and Service Wind = 3.96 mm                      Limit by Woolcock et al, 1999 Span/100 = 30.00 mm

#### **Reactions**

Maximum downward = 4.47 kn    Maximum upward = -5.33 kn

#### **Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9 \text{ f}_{pj} = 12.9 \text{ Mpa}$  for Rafter with effective thickness = 50 mm

For Parallel to grain loading

$K_{11} = 2.0 \text{ f}_{cj} = 36.1 \text{ Mpa}$  for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots (\text{Eq 4.12}) = -19.95 \text{ kn} > -5.33 \text{ Kn}$

Single Shear Capacity under short term loads = -10.84 Kn > -5.33 Kn

### Intermediate Design Front and Back

Intermediate Spacing = 3000 mm

Intermediate Span = 2549 mm

Try Intermediate 2x150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 1.00

$K_8$  Upward = 1.00     $S_1$  Downward = 9.63     $S_1$  Upward = 0.51

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

$M_{\text{Wind+Snow}}$	3.14 Kn-m	Capacity	4.2 Kn-m	Passing Percentage	<b>133.76 %</b>
$V_{0.9D-WnUp}$	4.93 Kn	Capacity	-24.12 Kn	Passing Percentage	<b>489.25 %</b>

### Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 14.015 mm

Limit by Woolcock et al, 1999 Span/100 = 25.49 mm

### Reactions

Maximum = 4.93 kn

### Girt Design Front and Back

Girt's Spacing = 1100 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 1.00

$K_8$  Upward = 0.79     $S_1$  Downward = 9.63     $S_1$  Upward = 17.59

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

$M_{\text{Wind+Snow}}$	1.60 Kn-m	Capacity	1.65 Kn-m	Passing Percentage	<b>103.13 %</b>
$V_{0.9D-WnUp}$	2.13 Kn	Capacity	12.06 Kn	Passing Percentage	<b>566.20 %</b>

### Deflections

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Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 15.88 mm

Limit by Woolcock et al, 1999 Span/100 = 30.00 mm

Sag during installation = 4.91 mm

#### Reactions

Maximum = 2.13 kn

#### Girt Design Sides

Girt's Spacing = 1100 mm

Girt's Span = 3000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.79    S1 Downward =9.63    S1 Upward =17.59

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>Wind+Snow</sub>	1.60 Kn-m	Capacity	1.65 Kn-m	Passing Percentage	<b>103.13 %</b>
V <sub>0.9D-WnUp</sub>	2.13 Kn	Capacity	12.06 Kn	Passing Percentage	<b>566.20 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 15.88 mm

Limit by Woolcock et al. 1999 Span/100 = 30.00 mm

Sag during installation =4.91 mm

#### Reactions

Maximum = 2.13 kn

#### End Pole Design

##### Geometry For End Bay Pole

##### Geometry

150 SED H5 HIGH DENSITY (Minimum 175 dia. at Floor Level)	Dry Use	Height	2950 mm
Area	20729 mm <sup>2</sup>	As	15546.6796875 mm <sup>2</sup>
I <sub>x</sub>	34210793 mm <sup>4</sup>	Z <sub>x</sub>	421056 mm <sup>3</sup>
I <sub>y</sub>	34210793 mm <sup>4</sup>	Z <sub>y</sub>	421056 mm <sup>3</sup>
Lateral Restraint	mm c/c		

#### Loads

Total Area over Pole = 9 m<sup>2</sup>

Dead	2.25 Kn	Live	2.25 Kn
Wind Down	6.84 Kn	Snow	0.00 Kn
Moment Wind	4.52 Kn-m		
Phi	0.8	K8	0.76

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K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
fb =	49.725 MPa	fs =	2.84 MPa
fc =	28.125 MPa	fp =	8.66 MPa
ft =	29.64 MPa	E =	12874 MPa

**Capacities**

PhiNcx Wind	354.84 Kn	PhiMnx Wind	12.74 Kn-m	PhiVnx Wind	35.32 Kn
PhiNcx Dead	212.90 Kn	PhiMnx Dead	7.65 Kn-m	PhiVnx Dead	21.19 Kn

**Checks**

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.39 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.16 < 1$  OK

Deflection at top under service lateral loads = 14.73 mm < 31.92 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2400 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Total Area over Pole = 9 m<sup>2</sup>

Moment Wind =	4.52 Kn-m
Shear Wind =	1.88 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	5.29 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.63 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.59 < 1 OK

**Drained Lateral Strength of End pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For End Bay Pole**

Ds =	0.6 mm	Pile Diameter
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L =	1300 mm	Pile embedment length
f1 =	2400 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	4.52 Kn-m
Shear Wind =	1.88 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	5.29 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.63 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities =  $0.59 < 1$  OK

**Uplift Check**

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1600) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1600)

Skin Friction = 20.68 Kn

Weight of Pile + Pile Skin Friction = 25.92 Kn

Uplift on one Pile = 22.77 Kn

Uplift is ok