

Job No.: SHEDKIED-4 bay - 1**Address:** 11075 West Coast RD, Lake Pearson, New Zealand**Date:** 14/05/2024**Latitude:** -43.056693**Longitude:** 171.744045**Elevation:** 636.5 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N4	Ground Snow Load	2.37 KPa	Roof Snow Load	1.66 KPa
Earthquake Zone	4	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.3 m
Wind Region	NZ2	Terrain Category	2.39	Design Wind Speed	42.75 m/s
Wind Pressure	1.1 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Gable Open

For roof $C_{p,i} = 0.6503$

For roof $C_{p,e}$ from 0 m To 3.3 m $C_{p,e} = -0.9$ $p_e = -0.81$ KPa $p_{net} = -1.52$ KPa

For roof $C_{p,e}$ from 3.3 m To 6.6 m $C_{p,e} = -0.5$ $p_e = -0.45$ KPa $p_{net} = -1.07$ KPa

For wall Windward $C_{p,i} = 0.6503$ side Wall $C_{p,i} = 0.5577$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 19.5 m $C_{p,e} = 0.7$ $p_e = 0.69$ KPa $p_{net} = 1.35$ KPa

For side wall $C_{p,e}$ from 0 m To 3.3 m $C_{p,e} =$ $p_e = -0.64$ KPa $p_{net} = 0.02$ KPa

Maximum Upward pressure used in roof member Design = 1.52 KPa

Maximum Downward pressure used in roof member Design = 0.86 KPa

Maximum Wall pressure used in Design = 1.35 KPa

Maximum Racking pressure used in Design = 1.08 KPa

Design Summary**Rafter Design Internal**

Internal Rafter Load Width = 4950 mm

Internal Rafter Span = 4650 mm

Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.81 S1 Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.78 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	566.29 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	9.35 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	143.74 %
M _{0.9D-W_nUp}	5.93 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	283.31 %
V _{1.35D}	3.86 Kn	Capacity	28.94 Kn	Passing Percentage	749.74 %

$V_{1.2D+1.5L\ 1.2D+S_n\ 1.2D+W_nD_n}$	20.46 Kn	Capacity	38.6 Kn	Passing Percentage	188.66 %
$V_{0.9D-W_nUp}$	17.3 Kn	Capacity	-48.24 Kn	Passing Percentage	278.84 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k_2 for Long Term Loads = 2

Deflection under Dead and Live Load = 2 mm

Limit by Woolcock et al, 1999 Span/240 = 20.00 mm

Deflection under Dead and Service Wind = 19.5 mm

Limit by Woolcock et al, 1999 Span/100 = 48.00 mm

Reactions

Maximum downward = 20.46 kn Maximum upward = 17.3 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 4

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9\ f_{pj} = 12.9\ \text{Mpa}$ for Rafter with effective thickness = 100 mm

For Parallel to grain loading

$K_{11} = 2.0\ f_{cj} = 36.1\ \text{Mpa}$ for Pole with effective thickness = 100 mm

Capacity under short term loads = 43.34 Kn > 17.3 Kn

Prop on Sides = 2 2/SG830050Dry 1270mm Reaction Prop = 14.94 Kn down 35.00 Kn Up

Prop Combined axial and bending ratios $(M_y/\Phi \times M_{ny}) + (N_c/\Phi \times N_{cy})$ should be less than or equal to 1

For Short Term Load = 0.98 < 1 OK

For Medium Term Load = 0.52 < 1 OK

For Long Term Load = 0.14 < 1 OK

Prop Connection check

Effective width of Pole used in Calculations = 175 mm - 20mm (Margin for chamfer)

Bolt Size = M12 Number of Bolts = 4

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Angle of prop = 45 degree

Prop Connection Capacity under Short term loads: 49.69 Kn > 35 Kn OK

Prop Connection Capacity under Medium term loads: 39.75 Kn > 14.94 Kn OK

Prop Connection Capacity under Long term loads: 29.81 Kn > 2.92 Kn OK

Intermediate Design Front and Back

Intermediate Spacing = 2475 mm

Intermediate Span = 2550 mm

Try Intermediate 2x150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =9.63 S1 Upward =0.51

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	4.77 Kn-m	Capacity	4.2 Kn-m	Passing Percentage	88.05 %
$V_{0.9D-WnUp}$	7.48 Kn	Capacity	-24.12 Kn	Passing Percentage	322.46 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 33.355 mm

Limit by Woolcock et al, 1999 Span/100 = 25.50 mm

Reactions

Maximum = 7.48 kn

Intermediate Design Sides

Intermediate Spacing = 2400 mm

Intermediate Span = 2850 mm

Try Intermediate 2x200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =11.27 S1 Upward =0.63

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	2.89 Kn-m	Capacity	7.46 Kn-m	Passing Percentage	258.13 %
$V_{0.9D-WnUp}$	4.05 Kn	Capacity	32.16 Kn	Passing Percentage	794.07 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 21.3 mm

Limit by Woolcock et al, 1999 Span/100 = 28.50 mm

Reactions

Maximum = 4.05 kn

Girt Design Front and Back

Girt's Spacing = 1300 mm

Girt's Span = 2475 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

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K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.86 S1 Downward =9.63 S1 Upward =15.97

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.65 Kn-m	Capacity	1.81 Kn-m	Passing Percentage	109.70 %
V _{0.9D-WnUp}	2.67 Kn	Capacity	12.06 Kn	Passing Percentage	451.69 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 20.29 mm Limit by Woolcock et al, 1999 Span/100 = 24.75 mm
Sag during installation = 2.28 mm

Reactions

Maximum = 2.67 kn

Girt Design Sides

Girt's Spacing = 1300 mm Girt's Span = 2400 mm Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.87 S1 Downward =9.63 S1 Upward =15.73

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.55 Kn-m	Capacity	1.83 Kn-m	Passing Percentage	118.06 %
V _{0.9D-WnUp}	2.59 Kn	Capacity	12.06 Kn	Passing Percentage	465.64 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 17.94 mm Limit by Woolcock et al. 1999 Span/100 = 24.00 mm
Sag during installation =2.01 mm

Reactions

Maximum = 2.59 kn

Middle Pole Design

Geometry

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	3000 mm
Area	27598 mm ²	As	20698.2421875 mm ²
I _x	60639381 mm ⁴	Z _x	646820 mm ³

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Iy	60639381 mm ⁴	Zx	646820 mm ³
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 23.76 m²

Dead	8.79 Kn	Live	7.19 Kn
Wind Down	24.74 Kn	Snow	47.76 Kn
Moment wind	7.26 Kn-m	Moment snow	4.20 Kn-m
Phi	0.8	K ₈	1.00
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

PhiN _{cx} Wind	397.41 Kn	PhiM _{nx} Wind	18.78 Kn-m	PhiV _{nx} Wind	49.01 Kn
PhiN _{cx} Dead	238.44 Kn	PhiM _{nx} Dead	11.27 Kn-m	PhiV _{nx} Dead	29.41 Kn
PhiN _{cx} Snow	317.93 Kn	PhiM _{nx} Snow	15.03 Kn-m	PhiV _{nx} Snow	39.21 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.72 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.47 < 1$ OK

Deflection at top under service lateral loads = 17.99 mm < 30.00 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K ₀ =	$(1 - \sin(30)) / (1 + \sin(30))$				
K _p =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

D _s =	0.6 mm	Pile Diameter
L =	1800 mm	Pile embedment length
f ₁ =	2475 mm	Distance at which the shear force is applied
f ₂ =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	7.26 Kn-m	Moment Snow =	Kn-m
Shear Wind =	2.93 Kn	Shear Snow =	6.44 Kn

Pile Properties

Safety Factory	0.55	
Hu =	12.16 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	18.79 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.53 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	3000 mm
Area	27598 mm ²	As	20698.2421875 mm ²
Ix	60639381 mm ⁴	Zx	646820 mm ³
Iy	60639381 mm ⁴	Zy	646820 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 11.88 m²

Dead	2.97 Kn	Live	2.97 Kn
Wind Down	10.22 Kn	Snow	19.72 Kn
Moment Wind	3.63 Kn-m	Moment snow	3.22 Kn-m
Phi	0.8	K8	0.86
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	341.68 Kn	PhiMnx Wind	16.15 Kn-m	PhiVnx Wind	49.01 Kn
PhiNcx Dead	205.01 Kn	PhiMnx Dead	9.69 Kn-m	PhiVnx Dead	29.41 Kn
PhiNcx Snow	273.35 Kn	PhiMnx Snow	12.92 Kn-m	PhiVnx Snow	39.21 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.34 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.15 < 1$ OK

Deflection at top under service lateral loads = 9.87 mm < 32.92 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length

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f1 = 2475 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Total Area over Pole = 11.88 m²

Moment Wind = 3.63 Kn-m Moment Snow = 3.22 Kn-m
Shear Wind = 1.47 Kn Shear Snow = 3.22 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.19 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 7.68 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.62 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³
K0 = $(1 - \sin(30)) / (1 + \sin(30))$
Kp = $(1 + \sin(30)) / (1 - \sin(30))$

Geometry For End Bay Pole

Ds = 0.6 mm Pile Diameter
L = 1300 mm Pile embedment length
f1 = 2475 mm Distance at which the shear force is applied
f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 3.63 Kn-m Moment Snow = 3.22 Kn-m
Shear Wind = 1.47 Kn Shear Snow = 3.22 Kn

Pile Properties

Safety Factory 0.55
Hu = 5.19 Kn Ultimate Lateral Strength of the Pile, Short pile
Mu = 7.68 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.62 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

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Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil (18) x Height of Pile (1800) x K_s (1.5) x 0.5 x $\tan(30)$ x π x Dia of Pile (0.6) x Height of Pile (1800)

Skin Friction = 26.17 Kn

Weight of Pile + Pile Skin Friction = 31.43 Kn

Uplift on one Pile = 30.77 Kn

Uplift is ok