

Pole Shed App Ver 01 2022

**Job No.:** EHB 850      **Address:** 149 Butson Road, Athol, Southland, New Zealand      **Date:** 11/30/2023  
**Latitude:** -45.487512      **Longitude:** 168.700849      **Elevation:** 387 m

**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	1.58 KPa	Roof Snow Load	1.11 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	2	Ultimate wind & Earthquake ARI	500 Years	Max Height	4.4 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	43.05 m/s
Wind Pressure	1.11 KPa	Lee Zone	NO	Ultimate Snow ARI	150 Years
Wind Category	High	Earthquake ARI	500		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.70 m  $C_{p,e} = -0.9$   $p_e = -0.91$  KPa  $p_{net} = -0.91$  KPa

For roof  $C_{p,e}$  from 3.70 m To 7.40 m  $C_{p,e} = -0.5$   $p_e = -0.51$  KPa  $p_{net} = -0.51$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 3.70 m  $C_{p,e} = 0.7$   $p_e = 0.71$  KPa  $p_{net} = 1.05$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.70 m  $C_{p,e} =$   $p_e = -0.66$  KPa  $p_{net} = -0.66$  KPa

Maximum Upward pressure used in roof member Design = 0.91 KPa

Maximum Downward pressure used in roof member Design = 0.51 KPa

Maximum Wall pressure used in Design = 1.05 KPa

Maximum Racking pressure used in Design = 1.22 KPa

**Design Summary**

**Purlin Design**

Purlin Spacing = 900 mm      Purlin Span = 4650 mm      Try Purlin 250x50 SG8 Dry

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 0.97

K8 Upward = 0.35    S1 Downward = 12.68    S1 Upward = 28.66

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

M <sub>1.35D</sub>	0.82 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	<b>414.63 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	3.43 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	<b>132.07 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-1.67 Kn-m	Capacity	-2.07 Kn-m	Passing Percentage	<b>123.95 %</b>
V <sub>1.35D</sub>	0.71 Kn	Capacity	12.06 Kn	Passing Percentage	<b>1698.59 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	2.95 Kn	Capacity	16.08 Kn	Passing Percentage	<b>545.08 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-1.43 Kn	Capacity	-20.10 Kn	Passing Percentage	<b>1405.59 %</b>

#### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 7.22 mm      Limit by Woolcock et al, 1999 Span/360 = 12.78 mm

Deflection under Dead and Service Wind = 9.08 mm      Limit by Woolcock et al, 1999 Span/250 = 30.67 mm

#### **Reactions**

Maximum downward = 2.95 kn    Maximum upward = -1.43 kn

Number of Blocking = 0    if 0 then no blocking required, if 1 then one midspan blocking required

#### **Rafter Design Internal**

Internal Rafter Load Width = 4800 mm      Internal Rafter Span = 9450 mm      Try Rafter 2x450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 1.00

K8 Upward = 1.00    S1 Downward = 6.68    S1 Upward = 6.68

Shear Capacity of timber = 5.3 MPa    Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

#### **Capacity Checks**

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M1.35D	18.08 Kn-m	Capacity	91.56 Kn-m	Passing Percentage	<b>506.42 %</b>
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	75.55 Kn-m	Capacity	122.08 Kn-m	Passing Percentage	<b>161.59 %</b>
M0.9D-WnUp	-36.70 Kn-m	Capacity	-152.6 Kn-m	Passing Percentage	<b>415.80 %</b>
V1.35D	7.65 Kn	Capacity	96.64 Kn	Passing Percentage	<b>1263.27 %</b>
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	31.98 Kn	Capacity	128.86 Kn	Passing Percentage	<b>402.94 %</b>
V0.9D-WnUp	-15.54 Kn	Capacity	-161.08 Kn	Passing Percentage	<b>1036.55 %</b>

**Deflections**

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 13.62 mm      Limit by Woolcock et al, 1999 Span/360 = 26.67 mm

Deflection under Dead and Service Wind = 19.04 mm      Limit by Woolcock et al, 1999 Span/250 = 64.00 mm

**Reactions**

Maximum downward = 31.98 kn    Maximum upward = -15.54 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 126 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -15.54 Kn

**Rafter Design External**

External Rafter Load Width = 2400 mm      External Rafter Span = 4651 mm      Try Rafter 240x45 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1    K1 Medium term = 0.8    K1 Long term = 0.6    K4 = 1    K5 = 1    K8 Downward = 0.94

K8 Upward = 0.94    S1 Downward = 13.82    S1 Upward = 13.82

Shear Capacity of timber = 5 MPa    Bending Capacity of timber = 38 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>1.35D</sub>	2.19 Kn-m	Capacity	7.41 Kn-m	Passing Percentage	<b>338.36 %</b>
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	9.15 Kn-m	Capacity	9.89 Kn-m	Passing Percentage	<b>108.09 %</b>
M <sub>0.9D-W<sub>n</sub>Up</sub>	-4.45 Kn-m	Capacity	-12.36 Kn-m	Passing Percentage	<b>277.75 %</b>
V <sub>1.35D</sub>	1.88 Kn	Capacity	17.37 Kn	Passing Percentage	<b>923.94 %</b>
V <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	7.87 Kn	Capacity	23.16 Kn	Passing Percentage	<b>294.28 %</b>
V <sub>0.9D-W<sub>n</sub>Up</sub>	-3.82 Kn	Capacity	-28.94 Kn	Passing Percentage	<b>757.59 %</b>

#### Deflections

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 9.70 mm    Limit by Woolcock et al, 1999 Span/360 = 13.33 mm

Deflection under Dead and Service Wind = 12.20 mm    Limit by Woolcock et al, 1999 Span/250 = 32.00 mm

#### Reactions

Maximum downward = 7.87 kn    Maximum upward = -3.82 kn

#### Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 f<sub>pj</sub> = 22.7 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

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$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$  (Eq 4.12) = -28.35 kn > -3.82 Kn

Single Shear Capacity under short term loads = -14.56 Kn > -3.82 Kn

### **Girt Design Front and Back**

Girt's Spacing = 1300 mm

Girt's Span = 2400 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.75    S1 Downward =11.27    S1 Upward =18.41

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

### **Capacity Checks**

$M_{Wind+Snow}$	1.04 Kn-m	Capacity	2.79 Kn-m	Passing Percentage	<b>268.27 %</b>
$V_{0.9D-WnUp}$	1.73 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	<b>929.48 %</b>

### **Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 5.43 mm    Limit by Woolcock et al, 1999 Span/250 = 9.60 mm

Sag during installation = 2.01 mm

### **Reactions**

Maximum = 1.73 kn

### **Girt Design Sides**

Girt's Spacing = 900 mm

Girt's Span = 2400 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.75    S1 Downward =11.27    S1 Upward =18.41

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

### **Capacity Checks**

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M <sub>Wind+Snow</sub>	0.72 Kn-m	Capacity	2.79 Kn-m	Passing Percentage	<b>387.50 %</b>
V <sub>0.9D-WnUp</sub>	1.20 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	<b>1340.00 %</b>

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 3.76 mm      Limit by Woolcock et al. 1999 Span/100 = 9.60 mm  
Sag during installation = 2.01 mm

**Reactions**

Maximum = 1.20 kn

**Middle Pole Design**

**Geometry**

250 SED H5 (Minimum 275 dia. at Floor Level)	Dry Use	Height	3400 mm
Area	54091 mm <sup>2</sup>	As	40568.5546875 mm <sup>2</sup>
I <sub>x</sub>	232952248 mm <sup>4</sup>	Z <sub>x</sub>	1774874 mm <sup>3</sup>
I <sub>y</sub>	232952248 mm <sup>4</sup>	Z <sub>y</sub>	1774874 mm <sup>3</sup>
Lateral Restraint	3400 mm c/c		

**Loads**

Total Area over Pole = 23.04 m<sup>2</sup>

Dead	5.76 Kn	Live	5.76 Kn
Wind Down	11.75 Kn	Snow	25.57 Kn
Moment wind	21.20 Kn-m	Moment snow	8.32 Kn-m
Phi	0.8	K <sub>8</sub>	0.97
K <sub>1</sub> snow	0.8	K <sub>1</sub> Dead	0.6
K <sub>1</sub> wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
f <sub>b</sub> =	36.3 MPa	f <sub>s</sub> =	2.96 MPa
f <sub>c</sub> =	18 MPa	f <sub>p</sub> =	7.2 MPa
f <sub>t</sub> =	22 MPa	E =	9257 MPa

**Capacities**

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PhiNcx Wind	751.80 Kn	PhiMnx Wind	49.75 Kn-m	PhiVnx Wind	96.07 Kn
PhiNcx Dead	451.08 Kn	PhiMnx Dead	29.85 Kn-m	PhiVnx Dead	57.64 Kn
PhiNcx Snow	601.44 Kn	PhiMnx Snow	39.80 Kn-m	PhiVnx Snow	76.85 Kn

**Checks**

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.48 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.24 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 20.67 \text{ mm} < 22.67 \text{ mm}$$

**Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K <sub>0</sub> =	$(1 - \sin(30)) / (1 + \sin(30))$				
K <sub>p</sub> =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For Middle Bay Pole**

D <sub>s</sub> =	0.6 mm	Pile Diameter
L =	1900 mm	Pile embedment length
f <sub>1</sub> =	3300 mm	Distance at which the shear force is applied
f <sub>2</sub> =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	21.20 Kn-m	Moment Snow =	Kn-m
Shear Wind =	6.43 Kn	Shear Snow =	8.32 Kn

**Pile Properties**

Safety Factory	0.55	
H <sub>u</sub> =	11.75 Kn	Ultimate Lateral Strength of the Pile, Short pile
M <sub>u</sub> =	23.46 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

$$\text{Applied Forces/Capacities} = 0.90 < 1 \text{ OK}$$

**End Pole Design**

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**Geometry For End Bay Pole**

**Geometry**

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	4200 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
Ix	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Iy	100042702 mm <sup>4</sup>	Zy	941578 mm <sup>3</sup>
Lateral Restraint	mm c/c		

**Loads**

Total Area over Pole = 11.52 m<sup>2</sup>

Dead	2.88 Kn	Live	2.88 Kn
Wind Down	5.88 Kn	Snow	12.79 Kn
Moment Wind	7.07 Kn-m	Moment snow	2.77 Kn-m
Phi	0.8	K8	0.68
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

**Capacities**

PhiNcx Wind	348.02 Kn	PhiMnx Wind	18.64 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	208.81 Kn	PhiMnx Dead	11.19 Kn-m	PhiVnx Dead	37.77 Kn
PhiNcx Snow	278.42 Kn	PhiMnx Snow	14.91 Kn-m	PhiVnx Snow	50.36 Kn

**Checks**

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.44 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.21 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 20.71 mm < 29.26 mm

Ds =	0.6 mm	Pile Diameter
L =	1900 mm	Pile embedment length



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f1 = 3300 mm Distance at which the shear force is applied  
f2 = 0 mm Distance of top soil at rest pressure

**Loads**

Total Area over Pole = 11.52 m<sup>2</sup>

Moment Wind = 7.07 Kn-m Moment Snow = 2.77 Kn-m  
Shear Wind = 2.14 Kn Shear Snow = 2.77 Kn

**Pile Properties**

Safety Factory 0.55  
Hu = 11.75 Kn Ultimate Lateral Strength of the Pile, Short pile  
Mu = 23.46 Kn-m Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.30 < 1 OK

**Drained Lateral Strength of End pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma 18 Kn/m<sup>3</sup> Friction angle 30 deg Cohesion 0 Kn/m<sup>3</sup>  
K0 =  $(1 - \sin(30)) / (1 + \sin(30))$   
Kp =  $(1 + \sin(30)) / (1 - \sin(30))$

**Geometry For End Bay Pole**

Ds = 0.6 mm Pile Diameter  
L = 1900 mm Pile embedment length  
f1 = 3300 mm Distance at which the shear force is applied  
f2 = 0 mm Distance of top soil at rest pressure

**Loads**

Moment Wind = 7.07 Kn-m Moment Snow = 2.77 Kn-m  
Shear Wind = 2.14 Kn Shear Snow = 2.77 Kn

**Pile Properties**

Safety Factory 0.55  
Hu = 11.75 Kn Ultimate Lateral Strength of the Pile, Short pile

Mu = 23.46 Kn-m Ultimate Moment Capacity of Pile

#### Checks

Applied Forces/Capacities =  $0.30 < 1$  OK

#### Uplift Check

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1900) x Ks(1.5) x  $0.5 \times \tan(30)$  x Pi x Dia of Pile(0.6) x Height of Pile(1900)

Skin Friction = 29.16 Kn

Weight of Pile + Pile Skin Friction = 32.97 Kn

Uplift on one Pile = 15.78 Kn

Uplift is ok