

Job No.: EHB 128 - 3

Address: 57 Blackmore Road, Garston, New Zealand

Date: 30/01/2024

Latitude: -45.474083

Longitude: 168.68633

Elevation: 319 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.93 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	7.9 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	40.29 m/s
Wind Pressure	0.97 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Gable Open

For roof $C_{p,i} = -0.564$

For roof $C_{p,e}$ from 0 m To 3.48 m $C_{p,e} = -1.3$ $p_e = -1.14$ KPa $p_{net} = -1.59$ KPa

For roof $C_{p,e}$ from 3.48 m To 5.0 m $C_{p,e} = -0.7$ $p_e = -0.61$ KPa $p_{net} = -1.06$ KPa

For wall Windward $C_{p,i} = 0.4576$ side Wall $C_{p,i} = -0.5984$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 17 m $C_{p,e} = 0.7$ $p_e = 0.61$ KPa $p_{net} = 1.16$ KPa

For side wall $C_{p,e}$ from 0 m To 6.95 m $C_{p,e} =$ $p_e = -0.57$ KPa $p_{net} = -0.02$ KPa

Maximum Upward pressure used in roof member Design = 1.59 KPa

Maximum Downward pressure used in roof member Design = 0.32 KPa

Maximum Wall pressure used in Design = 1.16 KPa

Maximum Racking pressure used in Design = 0.92 KPa

Design Summary

Girt Design Front and Back

Girt's Spacing = 700 mm

Girt's Span = 3000 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.92 S1 Downward = 11.27 S1 Upward = 14.55

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.91 Kn-m	Capacity	3.42 Kn-m	Passing Percentage	375.82 %
$V_{0.9D-WnUp}$	1.22 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	1318.03 %

Deflections

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Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 5.92 mm

Limit by Woolcock et al, 1999 Span/100 = 30.00 mm

Sag during installation = 4.91 mm

Reactions

Maximum = 1.22 kn

Girt Design Sides

Girt's Spacing = 700 mm

Girt's Span = 5000 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.41 S1 Downward = 11.27 S1 Upward = 26.57

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	2.54 Kn-m	Capacity	1.53 Kn-m	Passing Percentage	60.24 %
$V_{0.9D-WnUp}$	2.03 Kn-m	Capacity	16.08 Kn-m	Passing Percentage	792.12 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 45.66 mm

Limit by Woolcock et al. 1999 Span/100 = 50.00 mm

Sag during installation = 37.90 mm

Reactions

Maximum = 2.03 kn

Middle Pole Design

Geometry

300 SED H5 (Minimum 325 dia. at Floor Level)	Dry Use	Height	8600 mm
Area	76660 mm ²	As	57495.1171875 mm ²
I _x	467896461 mm ⁴	Z _x	2994537 mm ³
I _y	467896461 mm ⁴	Z _y	2994537 mm ³
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 7.5 m²

Dead	1.88 Kn	Live	1.88 Kn
Wind Down	2.40 Kn	Snow	4.72 Kn
Moment wind	32.22 Kn-m	Moment snow	5.50 Kn-m
Phi	0.8	K8	1.00
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	1103.91 Kn	PhiMnx Wind	86.96 Kn-m	PhiVnx Wind	136.15 Kn
PhiNcx Dead	662.34 Kn	PhiMnx Dead	52.18 Kn-m	PhiVnx Dead	81.69 Kn
PhiNcx Snow	883.13 Kn	PhiMnx Snow	69.57 Kn-m	PhiVnx Snow	108.92 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.38 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.15 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 71.00 \text{ mm} < 86.00 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	2100 mm	Pile embedment length
f1 =	5925 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	32.22 Kn-m	Moment Snow =	Kn-m
Shear Wind =	5.44 Kn	Shear Snow =	5.50 Kn

Pile Properties

Safety Factory	0.55	
Hu =	10.28 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	35.25 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.91 < 1 \text{ OK}$$

Uplift Check

$$\text{Density of Concrete} = 24 \text{ Kn/m}^3$$

$$\text{Density of Timber Pole} = 5 \text{ Kn/m}^3$$

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Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil (18) x Height of Pile (2100) x K_s (1.5) x 0.5 x $\tan(30)$ x π x Dia of Pile (0.6) x Height of Pile (2100)

Skin Friction = 35.62 Kn

Weight of Pile + Pile Skin Friction = 38.79 Kn

Uplift on one Pile = 10.24 Kn

Uplift is ok