

Job No.: 412miranda-rob**Address:** 43 Baigent Road, Miranda, New Zealand**Date:** 09/07/2024**Latitude:** -37.209371**Longitude:** 175.318714**Elevation:** 49 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.6 m
Wind Region	NZ1	Terrain Category	2.0	Design Wind Speed	41.57 m/s
Wind Pressure	1.04 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.65 m $C_{p,e} = -0.9$ $p_e = -0.84$ KPa $p_{net} = -0.84$ KPa

For roof $C_{p,e}$ from 3.65 m To 7.30 m $C_{p,e} = -0.5$ $p_e = -0.47$ KPa $p_{net} = -0.47$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 7.50 m $C_{p,e} = 0.7$ $p_e = 0.65$ KPa $p_{net} = 0.96$ KPa

For side wall $C_{p,e}$ from 0 m To 3.65 m $C_{p,e} =$ $p_e = -0.61$ KPa $p_{net} = -0.61$ KPa

Maximum Upward pressure used in roof member Design = 0.84 KPa

Maximum Downward pressure used in roof member Design = 0.50 KPa

Maximum Wall pressure used in Design = 0.96 KPa

Maximum Racking pressure used in Design = 1.12 KPa

Design Summary**Rafter Design Internal**

Internal Rafter Load Width = 5000 mm Internal Rafter Span = 4200.348027842228 mm Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.81 S1 Upward = 6.81

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	3.72 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	270.97 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	8.82 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	152.38 %
M _{0.9D-W_nUp}	-6.78 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	247.79 %
V _{1.35D}	3.54 Kn	Capacity	28.94 Kn	Passing Percentage	817.51 %

Pole Shed App Ver 01 2022

V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	8.40 Kn	Capacity	38.6 Kn	Passing Percentage	459.52 %
V _{0.9D-WnUp}	-6.46 Kn	Capacity	-48.24 Kn	Passing Percentage	746.75 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 5.18 mm

Limit by Woolcock et al, 1999 Span/240 = 18.13 mm

Deflection under Dead and Service Wind = 7.195 mm

Limit by Woolcock et al, 1999 Span/100 = 43.50 mm

Reactions

Maximum downward = 8.40 kn Maximum upward = -6.46 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 32.51 Kn > -6.46 Kn

Rafter Design External

External Rafter Load Width = 2500 mm

External Rafter Span = 2964 mm

Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 0.94

K₈ Upward = 0.94 S₁ Downward = 13.93 S₁ Upward = 13.93

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.93 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	507.53 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	2.20 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	286.36 %
M _{0.9D-WnUp}	-1.69 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	465.68 %
V _{1.35D}	1.25 Kn	Capacity	14.47 Kn	Passing Percentage	1157.60 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	2.96 Kn	Capacity	19.30 Kn	Passing Percentage	652.03 %
V _{0.9D-WnUp}	-2.28 Kn	Capacity	-24.12 Kn	Passing Percentage	1057.89 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 1.58 mm

Limit by Woolcock et al, 1999 Span/240= 13.13 mm

Deflection under Dead and Service Wind = 1.98 mm

Limit by Woolcock et al, 1999 Span/100 = 31.50 mm

Reactions

Maximum downward =2.96 kn Maximum upward = -2.28 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds (Eq 4.12) = -25.20 kn > -2.28 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.28 Kn

Intermediate Design Front and Back

Intermediate Spacing = 2500 mm

Intermediate Span = 2750 mm

Try Intermediate 2x150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =9.63 S1 Upward =0.53

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	2.27 Kn-m	Capacity	4.2 Kn-m	Passing Percentage	185.02 %
V _{0.9D-WnUp}	3.30 Kn	Capacity	-24.12 Kn	Passing Percentage	730.91 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 11.775 mm

Limit by Woolcock et al, 1999 Span/100 = 27.50 mm

Reactions

Maximum = 3.30 kn

Girt Design Front and Back

Girt's Spacing = 1300 mm

Girt's Span = 2500 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.86 S1 Downward =9.63 S1 Upward =16.05

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.97 Kn-m	Capacity	1.80 Kn-m	Passing Percentage	185.57 %
$V_{0.9D-WnUp}$	1.56 Kn	Capacity	12.06 Kn	Passing Percentage	773.08 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 6.74 mm

Limit by Woolcock et al, 1999 Span/100 = 25.00 mm

Sag during installation = 2.37 mm

Reactions

Maximum = 1.56 kn

Girt Design Sides

Girt's Spacing = 1300 mm

Girt's Span = 3150 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.77 S1 Downward =9.63 S1 Upward =18.02

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.55 Kn-m	Capacity	1.61 Kn-m	Passing Percentage	103.87 %
$V_{0.9D-WnUp}$	1.97 Kn	Capacity	12.06 Kn	Passing Percentage	612.18 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 16.98 mm

Limit by Woolcock et al, 1999 Span/100 = 31.50 mm

Sag during installation =5.97 mm

Reactions

Maximum = 1.97 kn

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1600) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1600)

Skin Friction = 20.68 Kn

Weight of Pile + Pile Skin Friction = 25.09 Kn

Uplift on one Pile = 13.38 Kn

Uplift is ok