

Pole Shed App Ver 01 2022

Job No.: 2310002 **Address:** 30 Rangihacata Road, Takaka, New Zealand **Date:** 10/11/2023
Latitude: -40.816657 **Longitude:** 172.788278 **Elevation:** 28 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N2	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.3 m
Wind Region	NZ2	Terrain Category	1.82	Design Wind Speed	44.56 m/s
Wind Pressure	1.19 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Very High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 1.98 m $C_{p,e} = -1.0267$ $p_e = -1.10$ KPa $p_{net} = -1.10$ KPa

For roof $C_{p,e}$ from 1.98 m To 3.95 m $C_{p,e} = -0.8367$ $p_e = -0.90$ KPa $p_{net} = -0.90$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 5.70 m $C_{p,e} = 0.7$ $p_e = 0.75$ KPa $p_{net} = 0.75$ KPa

For side wall $C_{p,e}$ from 0 m To 3.95 m $C_{p,e} =$ $p_e = -0.70$ KPa $p_{net} = -0.70$ KPa

Maximum Upward pressure used in roof member Design = 1.10 KPa

Maximum Downward pressure used in roof member Design = 0.22 KPa

Maximum Wall pressure used in Design = 1.11 KPa

Maximum Racking pressure used in Design = 0.64 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm Purlin Span = 5550 mm Try Purlin 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.97

K8 Upward =0.57 S1 Downward =12.68 S1 Upward =22.16

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.17 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	290.60 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.57 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	176.26 %
M _{0.9D-W_nUp}	-3.03 Kn-m	Capacity	-3.31 Kn-m	Passing Percentage	245.19 %
V _{1.35D}	0.84 Kn	Capacity	12.06 Kn	Passing Percentage	1435.71 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.69 Kn	Capacity	16.08 Kn	Passing Percentage	951.48 %
V _{0.9D-W_nUp}	-2.19 Kn	Capacity	-20.10 Kn	Passing Percentage	917.81 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 14.75 mm Limit by Woolcock et al, 1999 Span/240 = 22.92 mm

Deflection under Dead and Service Wind = 15.00 mm Limit by Woolcock et al, 1999 Span/100 = 55.00 mm

Reactions

Maximum downward =1.69 kn Maximum upward = -2.19 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 5700 mm Internal Rafter Span = 5850 mm Try Rafter 2x300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =6.81 S1 Upward =6.81

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{1.35D}	2.72 Kn-m	Capacity	10.08 Kn-m	Passing Percentage	370.59 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	4.92 Kn-m	Capacity	13.44 Kn-m	Passing Percentage	273.17 %
M _{0.9D-WnUp}	7.95 Kn-m	Capacity	-16.8 Kn-m	Passing Percentage	211.32 %
V _{1.35D}	2.34 Kn	Capacity	28.94 Kn	Passing Percentage	1236.75 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	4.25 Kn	Capacity	38.6 Kn	Passing Percentage	908.24 %
V _{0.9D-WnUp}	7.14 Kn	Capacity	-48.24 Kn	Passing Percentage	675.63 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 8 mm Limit by Woolcock et al, 1999 Span/240 = 25.00 mm

Deflection under Dead and Service Wind = 26.5 mm Limit by Woolcock et al, 1999 Span/100 = 60.00 mm

Reactions

Maximum downward = 4.25 kn Maximum upward = 7.14 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > 7.14 Kn

Prop on Sides = 2 2/SG815050Dry 1000mm Reaction Prop = 6.46 Kn down 15.81 Kn Up

Prop Combined axial and bending ratios (M_y/Phi x M_{ny})+(N_c/Phi x N_{cy}) should be less than or equal to 1

For Short Term Load = 0.77 < 1 OK

For Medium Term Load = 0.39 < 1 OK

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For Long Term Load = $0.29 < 1$ OK

Prop Connection check

Effective width of Pole used in Calculations = 150 mm - 20mm (Margin for chamfer)

Bolt Size = M12 Number of Bolts = 2

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Angle of prop = 45 degree

Prop Connection Capacity under Short term loads: 24.85 Kn > 15.81 Kn OK

Prop Connection Capacity under Medium term loads: 19.88 Kn > 6.46 Kn OK

Prop Connection Capacity under Long term loads: 14.91 Kn > 3.60 Kn OK

Girt Design Front and Back

Girt's Spacing = 0 mm

Girt's Span = 5700 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 = 1 K5 = 1 K8 Downward = NaN

K8 Upward = NaN S1 Downward = NaN S1 Upward = NaN

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	NaN %
V _{0.9D-WnUp}	0.00 Kn-m	Capacity	0.00 Kn-m	Passing Percentage	NaN %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm Limit by Woolcock et al, 1999 Span/100 = 57.00 mm

Sag during installation = NaN mm

Reactions

Maximum = 0.00 kn

Girt Design Sides

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Girt's Spacing = 0 mm

Girt's Span = 6000 mm

Try Girt SG8 Dry

Moisture Condition = Wet (Moisture in timber is less than 18% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 = 1 K5 = 1 K8 Downward = NaN

K8 Upward = NaN S1 Downward = NaN S1 Upward = NaN

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.00 Kn-m	Capacity	NaN Kn-m	Passing Percentage	NaN %
V _{0.9D-WnUp}	0.00 Kn-m	Capacity	0.00 Kn-m	Passing Percentage	NaN %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = NaN mm Limit by Woolcock et al. 1999 Span/100 = 60.00 mm
Sag during installation = NaN mm

Reactions

Maximum = 0.00 kn

End Pole Design

Geometry For End Bay Pole

Geometry

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	4000 mm
Area	27598 mm ²	As	20698.2421875 mm ²
I _x	60639381 mm ⁴	Z _x	646820 mm ³
I _y	60639381 mm ⁴	Z _y	646820 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 17.1 m²

Dead	4.28 Kn	Live	4.28 Kn
Wind Down	3.76 Kn	Snow	0.00 Kn
Moment Wind	6.31 Kn-m		

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Phi	0.8	K8	0.61
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	240.88 Kn	PhiMnx Wind	11.39 Kn-m	PhiVnx Wind	49.01 Kn
PhiNcx Dead	144.53 Kn	PhiMnx Dead	6.83 Kn-m	PhiVnx Dead	29.41 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.61 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.36 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 29.12 \text{ mm} < 42.89 \text{ mm}$$

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3225 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

$$\text{Total Area over Pole} = 17.1 \text{ m}^2$$

Moment Wind =	6.31 Kn-m
Shear Wind =	1.96 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.33 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.15 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.77 < 1 \text{ OK}$$

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³
K₀ = $(1 - \sin(30)) / (1 + \sin(30))$
K_p = $(1 + \sin(30)) / (1 - \sin(30))$

Geometry For End Bay Pole

D_s = 0.6 mm Pile Diameter
L = 1300 mm Pile embedment length
f₁ = 3225 mm Distance at which the shear force is applied
f₂ = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 6.31 Kn-m
Shear Wind = 1.96 Kn

Pile Properties

Safety Factory 0.55
H_u = 4.33 Kn Ultimate Lateral Strength of the Pile, Short pile
M_u = 8.15 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.77 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1300) x K_s(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1300)

Skin Friction = 13.65 Kn

Weight of Pile + Pile Skin Friction = 17.91 Kn

Uplift on one Pile = 14.96 Kn

Uplift is ok