

Pole Shed App Ver 01 2022

Job No.: Ormond Nurseries
Enclosed Lean To

Address: 13 Rowley Crescent Grovetown,
Grovetown, New Zealand

Date: 10/11/2023

Latitude: -41.491377

Longitude: 173.958501

Elevation: 6 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N3	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	6 m
Wind Region	NZ2	Terrain Category	2.0	Design Wind Speed	38.64 m/s
Wind Pressure	0.9 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 5.55 m $C_{p,e} = -0.9$ $p_e = -0.73$ KPa $p_{net} = -0.73$ KPa

For roof $C_{p,e}$ from 5.55 m To 11.10 m $C_{p,e} = -0.5$ $p_e = -0.40$ KPa $p_{net} = -0.40$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 11.90 m $C_{p,e} = 0.7$ $p_e = 0.56$ KPa $p_{net} = 0.83$ KPa

For side wall $C_{p,e}$ from 0 m To 5.55 m $C_{p,e} =$ $p_e = -0.52$ KPa $p_{net} = -0.52$ KPa

Maximum Upward pressure used in roof member Design = 0.73 KPa

Maximum Downward pressure used in roof member Design = 0.43 KPa

Maximum Wall pressure used in Design = 0.83 KPa

Maximum Racking pressure used in Design = 0.96 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 5250 mm

Try Purlin 250x50 SG8 Dry

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Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.97

K8 Upward = 0.31 S1 Downward = 12.68 S1 Upward = 30.47

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.05 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	323.81 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.37 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	191.14 %
M _{0.9D-W_nUp}	-1.57 Kn-m	Capacity	-1.83 Kn-m	Passing Percentage	116.56 %
V _{1.35D}	0.80 Kn	Capacity	12.06 Kn	Passing Percentage	1507.50 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.72 Kn	Capacity	16.08 Kn	Passing Percentage	934.88 %
V _{0.9D-W_nUp}	-1.19 Kn	Capacity	-20.10 Kn	Passing Percentage	1689.08 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 11.79 mm Limit by Woolcock et al, 1999 Span/240 = 21.67 mm

Deflection under Dead and Service Wind = 14.04 mm Limit by Woolcock et al, 1999 Span/100 = 52.00 mm

Reactions

Maximum downward = 1.72 kn Maximum upward = -1.19 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 5400 mm Internal Rafter Span = 11750 mm Try Rafter 2x400x90 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 4.25 S1 Upward = 4.25

Shear Capacity of timber = 5 MPa Bending Capacity of timber = 38 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M1.35D	31.45 Kn-m	Capacity	83.44 Kn-m	Passing Percentage	265.31 %
M1.2D+1.5L 1.2D+Sn 1.2D+WnDn	68.03 Kn-m	Capacity	111.26 Kn-m	Passing Percentage	163.55 %
M0.9D-WnUp	-47.06 Kn-m	Capacity	-139.08 Kn-m	Passing Percentage	295.54 %
V1.35D	10.71 Kn	Capacity	115.78 Kn	Passing Percentage	1081.05 %
V1.2D+1.5L 1.2D+Sn 1.2D+WnDn	23.16 Kn	Capacity	154.36 Kn	Passing Percentage	666.49 %
V0.9D-WnUp	-16.02 Kn	Capacity	-192.96 Kn	Passing Percentage	1204.49 %

Deflections

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 40.055 mm Limit by Woolcock et al, 1999 Span/240 = 49.58 mm

Deflection under Dead and Service Wind = 53.04 mm Limit by Woolcock et al, 1999 Span/100 = 119.00 mm

Reactions

Maximum downward = 23.16 kn Maximum upward = -16.02 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 180 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 43.67 Kn > -16.02 Kn

Rafter Design External

External Rafter Load Width = 2700 mm External Rafter Span = 5767 mm Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet

condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.94

K8 Upward = 0.94 S1 Downward = 13.93 S1 Upward = 13.93

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	3.79 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	124.54 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	8.19 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	76.92 %
M _{0.9D-W_nUp}	-5.67 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	138.80 %
V _{1.35D}	2.63 Kn	Capacity	14.47 Kn	Passing Percentage	550.19 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	5.68 Kn	Capacity	19.30 Kn	Passing Percentage	339.79 %
V _{0.9D-W_nUp}	-3.93 Kn	Capacity	-24.12 Kn	Passing Percentage	613.74 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 21.76 mm Limit by Woolcock et al, 1999 Span/240 = 24.79 mm
Deflection under Dead and Service Wind = 25.93 mm Limit by Woolcock et al, 1999 Span/100 = 59.50 mm

Reactions

Maximum downward = 5.68 kn Maximum upward = -3.93 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -25.20 kn > -3.93 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -3.93 Kn

Girt Design Front and Back

Girt's Spacing = 900 mm

Girt's Span = 5400 mm

Try Girt 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.97

K8 Upward =0.30 S1 Downward =12.68 S1 Upward =31.05

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	2.72 Kn-m	Capacity	1.77 Kn-m	Passing Percentage	65.07 %
$V_{0.9D-WnUp}$	2.02 Kn-m	Capacity	20.10 Kn-m	Passing Percentage	995.05 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 18.96 mm Limit by Woolcock et al, 1999 Span/100 = 54.00 mm

Sag during installation = 51.56 mm

Reactions

Maximum = 2.02 kn

Girt Design Sides

Girt's Spacing = 900 mm

Girt's Span = 5950 mm

Try Girt 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.97

K8 Upward =0.28 S1 Downward =12.68 S1 Upward =32.60

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

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M _{Wind+Snow}	3.31 Kn-m	Capacity	1.61 Kn-m	Passing Percentage	48.64 %
V _{0.9D-WnUp}	2.22 Kn-m	Capacity	20.10 Kn-m	Passing Percentage	905.41 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 27.95 mm Limit by Woolcock et al. 1999 Span/100 = 59.50 mm
Sag during installation = 76.00 mm

Reactions

Maximum = 2.22 kn

Middle Pole Design

Geometry

275 SED H5 (Minimum 300 dia. at Floor Level)	Dry Use	Height	5600 mm
Area	64885 mm ²	As	48663.8671875 mm ²
I _x	335197731 mm ⁴	Z _x	2331810 mm ³
I _y	335197731 mm ⁴	Z _y	2331810 mm ³
Lateral Restraint	3400 mm c/c		

Loads

Total Area over Pole = 32.13 m²

Dead	8.03 Kn	Live	8.03 Kn
Wind Down	13.82 Kn	Snow	0.00 Kn
Moment wind	34.90 Kn-m		
Phi	0.8	K ₈	0.99
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

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PhiNcx Wind	923.16 Kn	PhiMnx Wind	66.90 Kn-m	PhiVnx Wind	115.24 Kn
PhiNcx Dead	553.89 Kn	PhiMnx Dead	40.14 Kn-m	PhiVnx Dead	69.14 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.55 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.30 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 53.11 \text{ mm} < 56.00 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K ₀ =	$(1 - \sin(30)) / (1 + \sin(30))$				
K _p =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

D _s =	0.6 mm	Pile Diameter
L =	2150 mm	Pile embedment length
f ₁ =	4500 mm	Distance at which the shear force is applied
f ₂ =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	34.90 Kn-m
Shear Wind =	7.76 Kn

Pile Properties

Safety Factory	0.55	
H _u =	13.32 Kn	Ultimate Lateral Strength of the Pile, Short pile
M _u =	35.53 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.98 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

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Geometry

225 SED H5 (Minimum 250 dia. at Floor Level)	Dry Use	Height	5700 mm
Area	44279 mm ²	As	33209.1796875 mm ²
Ix	156100441 mm ⁴	Zx	1314530 mm ³
Iy	156100441 mm ⁴	Zy	1314530 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 16.065 m²

Dead	4.02 Kn	Live	4.02 Kn
Wind Down	6.91 Kn	Snow	0.00 Kn
Moment Wind	11.63 Kn-m		
Phi	0.8	K8	0.49
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	314.62 Kn	PhiMnx Wind	18.84 Kn-m	PhiVnx Wind	78.64 Kn
PhiNcx Dead	188.77 Kn	PhiMnx Dead	11.30 Kn-m	PhiVnx Dead	47.18 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.67 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.43 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 40.63 \text{ mm} < 59.85 \text{ mm}$$

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	4500 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 16.065 m²

Moment Wind = 11.63 Kn-m

Shear Wind = 2.59 Kn

Pile Properties

Safety Factory 0.55

Hu = 5.01 Kn Ultimate Lateral Strength of the Pile, Short pile

Mu = 13.00 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.90 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma 18 Kn/m³ Friction angle 30 deg Cohesion 0 Kn/m³

K0 = $(1 - \sin(30)) / (1 + \sin(30))$

Kp = $(1 + \sin(30)) / (1 - \sin(30))$

Geometry For End Bay Pole

Ds = 0.6 mm Pile Diameter

L = 1500 mm Pile embedment length

f1 = 4500 mm Distance at which the shear force is applied

f2 = 0 mm Distance of top soil at rest pressure

Loads

Moment Wind = 11.63 Kn-m

Shear Wind = 2.59 Kn

Pile Properties

Safety Factory 0.55

Hu = 5.01 Kn Ultimate Lateral Strength of the Pile, Short pile

Mu = 13.00 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.90 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m^3

Density of Timber Pole = 5 Kn/m^3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) \times Density of Soil(18) \times Height of Pile(2150) \times K_s (1.5) \times $0.5 \times \tan(30) \times \pi \times \text{Dia of Pile}(0.6) \times \text{Height of Pile}(2150)$

Skin Friction = 37.33 Kn

Weight of Pile + Pile Skin Friction = 41.09 Kn

Uplift on one Pile = 16.23 Kn

Uplift is ok