

Job No.: 230702**Address:** 45A McArthur Road, Alexandra, New Zealand**Date:** 23/08/2024**Latitude:** -45.194957**Longitude:** 169.372822**Elevation:** 239.5 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3 m
Wind Region	NZ2	Terrain Category	2.06	Design Wind Speed	41.1 m/s
Wind Pressure	1.01 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 2.7 m $C_{p,e} = -0.9$ $p_e = -0.82$ KPa $p_{net} = -0.82$ KPa

For roof $C_{p,e}$ from 2.7 m To 5.4 m $C_{p,e} = -0.5$ $p_e = -0.46$ KPa $p_{net} = -0.46$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 7.2 m $C_{p,e} = 0.64$ $p_e = 0.64$ KPa $p_{net} = 0.94$ KPa

For side wall $C_{p,e}$ from 0 m To 2.7 m $C_{p,e} =$ $p_e = -0.59$ KPa $p_{net} = -0.59$ KPa

Maximum Upward pressure used in roof member Design = 0.82 KPa

Maximum Downward pressure used in roof member Design = 0.39 KPa

Maximum Wall pressure used in Design = 0.94 KPa

Maximum Racking pressure used in Design = 1.07 KPa

Design Summary**Purlin Design**

Purlin Spacing = 900 mm

Purlin Span = 2950 mm

Try Purlin 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.97

K8 Upward = 0.54 S1 Downward = 12.68 S1 Upward = 22.76

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.33 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	1030.30 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	1.1 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	411.82 %
$M_{0.9D-W_nUp}$	-0.58 Kn-m	Capacity	-3.16 Kn-m	Passing Percentage	339.78 %
$V_{1.35D}$	0.45 Kn	Capacity	12.06 Kn	Passing Percentage	2680.00 %

Pole Shed App Ver 01 2022

V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	1.23 Kn	Capacity	16.08 Kn	Passing Percentage	1307.32 %
V _{0.9D-WnUp}	-0.79 Kn	Capacity	-20.10 Kn	Passing Percentage	2544.30 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 1.14 mm Limit by Woolcock et al, 1999 Span/240 = 12.08 mm

Deflection under Dead and Service Wind = 1.32 mm Limit by Woolcock et al, 1999 Span/100 = 29.00 mm

Reactions

Maximum downward = 1.23 kn Maximum upward = -0.79 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 3100 mm Internal Rafter Span = 7050 mm Try Rafter 2x300x45 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 7.61 S₁ Upward = 7.61

Shear Capacity of timber = 5 MPa Bending Capacity of timber = 38 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	6.50 Kn-m	Capacity	24.62 Kn-m	Passing Percentage	378.77 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	17.91 Kn-m	Capacity	32.84 Kn-m	Passing Percentage	183.36 %
M _{0.9D-WnUp}	-11.46 Kn-m	Capacity	-41.04 Kn-m	Passing Percentage	358.12 %
V _{1.35D}	3.69 Kn	Capacity	43.42 Kn	Passing Percentage	1176.69 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	10.16 Kn	Capacity	57.88 Kn	Passing Percentage	569.69 %
V _{0.9D-WnUp}	-6.50 Kn	Capacity	-72.36 Kn	Passing Percentage	1113.23 %

Deflections

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 14.61 mm Limit by Woolcock et al, 1999 Span/240 = 30.00 mm

Deflection under Dead and Service Wind = 18.805 mm Limit by Woolcock et al, 1999 Span/100 = 72.00 mm

Reactions

Maximum downward = 10.16 kn Maximum upward = -6.50 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 12.6 \text{ fpj} = 22.7 \text{ Mpa}$ for Rafter with effective thickness = 90 mm

For Parallel to grain loading

$K_{11} = 2.0 \text{ fcj} = 36.1 \text{ Mpa}$ for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -6.50 Kn

Rafter Design External

External Rafter Load Width = 1550 mm

External Rafter Span = 3412 mm

Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 K_1 Medium term = 0.8 K_1 Long term = 0.6 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.94

K_8 Upward = 0.94 S_1 Downward = 13.93 S_1 Upward = 13.93

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.76 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	621.05 %
$M_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	2.10 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	300.00 %
$M_{0.9D-W_nUp}$	-1.34 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	587.31 %
$V_{1.35D}$	0.89 Kn	Capacity	14.47 Kn	Passing Percentage	1625.84 %
$V_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	2.46 Kn	Capacity	19.30 Kn	Passing Percentage	784.55 %
$V_{0.9D-W_nUp}$	-1.57 Kn	Capacity	-24.12 Kn	Passing Percentage	1536.31 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k_2 for Long Term Loads = 2

Deflection under Dead and Live Load = 1.67 mm

Limit by Woolcock et al, 1999 Span/240 = 15.00 mm

Deflection under Dead and Service Wind = 1.94 mm

Limit by Woolcock et al, 1999 Span/100 = 36.00 mm

Reactions

Maximum downward = 2.46 kn Maximum upward = -1.57 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9 \text{ fpj} = 12.9 \text{ Mpa}$ for Rafter with effective thickness = 50 mm

For Parallel to grain loading

$K_{11} = 2.0$ $f_{c,j} = 36.1$ Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi_i \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -25.20 kn > -1.57 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -1.57 Kn

Girt Design Front and Back

Girt's Spacing = 1300 mm

Girt's Span = 3100 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.63 S_1 Downward = 11.27 S_1 Upward = 20.92

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.47 Kn-m	Capacity	2.33 Kn-m	Passing Percentage	158.50 %
$V_{0.9D-WnUp}$	1.89 Kn	Capacity	16.08 Kn	Passing Percentage	850.79 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 10.99 mm

Limit by Woolcock et al, 1999 Span/100 = 31.00 mm

Sag during installation = 5.60 mm

Reactions

Maximum = 1.89 kn

Girt Design Sides

Girt's Spacing = 800 mm

Girt's Span = 3600 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.55 S_1 Downward = 11.27 S_1 Upward = 22.54

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.22 Kn-m	Capacity	2.06 Kn-m	Passing Percentage	168.85 %
$V_{0.9D-WnUp}$	1.35 Kn	Capacity	16.08 Kn	Passing Percentage	1191.11 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 12.30 mm

Limit by Woolcock et al. 1999 Span/100 = 36.00 mm

Sag during installation = 10.18 mm

Reactions

Maximum = 1.35 kn

Middle Pole Design

Geometry

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	2700 mm
Area	27598 mm ²	As	20698.2421875 mm ²
Ix	60639381 mm ⁴	Zx	646820 mm ³
Iy	60639381 mm ⁴	Zx	646820 mm ³
Lateral Restraint	3400 mm c/c		

Loads

Total Area over Pole = 11.16 m²

Dead	2.79 Kn	Live	2.79 Kn
Wind Down	4.35 Kn	Snow	7.03 Kn
Moment wind	5.58 Kn-m	Moment snow	2.09 Kn-m
Phi	0.8	K8	0.76
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	302.65 Kn	PhiMnx Wind	14.30 Kn-m	PhiVnx Wind	49.01 Kn
PhiNcx Dead	181.59 Kn	PhiMnx Dead	8.58 Kn-m	PhiVnx Dead	29.41 Kn
PhiNcx Snow	242.12 Kn	PhiMnx Snow	11.44 Kn-m	PhiVnx Snow	39.21 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.44 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.20 < 1$ OK

Deflection at top under service lateral loads = 11.32 mm < 27.00 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Pole Shed App Ver 01 2022

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2250 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	5.58 Kn-m	Moment Snow =	Kn-m
Shear Wind =	2.48 Kn	Shear Snow =	2.09 Kn

Pile Properties

Safety Factory	0.55	
Hu =	5.51 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.51 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.74 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	2700 mm
Area	27598 mm ²	As	20698.2421875 mm ²
Ix	60639381 mm ⁴	Zx	646820 mm ³
Iy	60639381 mm ⁴	Zx	646820 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 5.58 m²

Dead	1.40 Kn	Live	1.40 Kn
Wind Down	2.18 Kn	Snow	3.52 Kn
Moment Wind	1.86 Kn-m	Moment snow	0.70 Kn-m
Phi	0.8	K8	0.92
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	366.39 Kn	PhiMnx Wind	17.32 Kn-m	PhiVnx Wind	49.01 Kn
-------------	-----------	-------------	------------	-------------	----------

Pole Shed App Ver 01 2022

PhiNcx Dead	219.84 Kn	PhiMnx Dead	10.39 Kn-m	PhiVnx Dead	29.41 Kn
PhiNcx Snow	293.11 Kn	PhiMnx Snow	13.85 Kn-m	PhiVnx Snow	39.21 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.13 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.03 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 4.18 \text{ mm} < 29.93 \text{ mm}$$

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2250 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

$$\text{Total Area over Pole} = 5.58 \text{ m}^2$$

Moment Wind =	1.86 Kn-m	Moment Snow =	0.70 Kn-m
Shear Wind =	0.83 Kn	Shear Snow =	0.70 Kn

Pile Properties

Safety Factory	0.55	
Hu =	5.51 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.51 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.25 < 1 \text{ OK}$$

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2250 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	1.86 Kn-m	Moment Snow =	0.70 Kn-m
Shear Wind =	0.83 Kn	Shear Snow =	0.70 Kn

Pile Properties

Safety Factory	0.55
----------------	------

Hu =	5.51 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.51 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.25 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1300) x Ks(1.5) x 0.5 x tan(30) x π x Dia of Pile(0.6) x Height of Pile(1300)

Skin Friction = 13.65 Kn

Weight of Pile + Pile Skin Friction = 17.45 Kn

Uplift on one Pile = 6.64 Kn

Uplift is ok