

Pole Shed App Ver 01 2022

Job No.: SB 93 Slone Shed - 2 **Address:** 20 Paton Place, Te Anau, New Zealand **Date:** 12/05/2025
Latitude: -45.410188 **Longitude:** 167.733113 **Elevation:** 219 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N5	Ground Snow Load	0.9 KPa	Roof Snow Load	0.63 KPa
Earthquake Zone	3	Subsoil Category	D	Exposure Zone	B
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.5 m
Wind Region	NZ2	Terrain Category	3.0	Design Wind Speed	34.86 m/s
Wind Pressure	0.73 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Medium	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = 0.6718$

For roof $C_{p,e}$ from 0 m To 3.50 m $C_{p,e} = -0.9$ $p_e = -0.55$ KPa $p_{net} = -1.04$ KPa

For roof $C_{p,e}$ from 3.5 m To 7 m $C_{p,e} = -0.5$ $p_e = -0.30$ KPa $p_{net} = -0.30$ KPa

For wall Windward $C_{p,i} = 0.6718$ side Wall $C_{p,i} = -0.5976$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 8 m $C_{p,e} = 0.7$ $p_e = 0.46$ KPa $p_{net} = 0.93$ KPa

For side wall $C_{p,e}$ from 0 m To 3.50 m $C_{p,e} =$ $p_e = -0.43$ KPa $p_{net} = -0.43$ KPa

Maximum Upward pressure used in roof member Design = 1.04 KPa

Maximum Downward pressure used in roof member Design = 0.54 KPa

Maximum Wall pressure used in Design = 0.93 KPa

Maximum Racking pressure used in Design = 0.79 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm Purlin Span = 3850 mm Try Purlin 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

Pole Shed App Ver 01 2022

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.53 S1 Downward = 11.27 S1 Upward = 23.16

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.56 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	398.21 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.09 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	142.11 %
M _{0.9D-W_nUp}	-1.36 Kn-m	Capacity	-1.96 Kn-m	Passing Percentage	144.12 %
V _{1.35D}	0.58 Kn	Capacity	9.65 Kn	Passing Percentage	1663.79 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.61 Kn	Capacity	12.86 Kn	Passing Percentage	798.76 %
V _{0.9D-W_nUp}	-1.41 Kn	Capacity	-16.08 Kn	Passing Percentage	1140.43 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 11.10 mm Limit by Woolcock et al, 1999 Span/240 = 15.83 mm

Deflection under Dead and Service Wind = 8.42 mm Limit by Woolcock et al, 1999 Span/100 = 38.00 mm

Reactions

Maximum downward = 1.61 kn Maximum upward = -1.41 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4000 mm Internal Rafter Span = 3453.6036036036035 mm Try Rafter 2x250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 6.13 S1 Upward = 6.13

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	2.01 Kn-m	Capacity	7 Kn-m	Passing Percentage	348.26 %
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Pole Shed App Ver 01 2022

M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	5.55 Kn-m	Capacity	9.34 Kn-m	Passing Percentage	168.29 %
M _{0.9D-WnUp}	-4.86 Kn-m	Capacity	-11.66 Kn-m	Passing Percentage	239.92 %
V _{1.35D}	2.33 Kn	Capacity	24.12 Kn	Passing Percentage	1035.19 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	6.42 Kn	Capacity	32.16 Kn	Passing Percentage	500.93 %
V _{0.9D-WnUp}	-5.63 Kn	Capacity	-40.2 Kn	Passing Percentage	714.03 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 3.375 mm Limit by Woolcock et al, 1999 Span/240 = 15.02 mm

Deflection under Dead and Service Wind = 4.81 mm Limit by Woolcock et al, 1999 Span/100 = 36.04 mm

Reactions

Maximum downward = 6.42 kn Maximum upward = -5.63 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 100 mm

For Parallel to grain loading

K₁₁ = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 21.67 Kn > -5.63 Kn

Rafter Design External

External Rafter Load Width = 2000 mm External Rafter Span = 3432 mm Try Rafter 250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

Pole Shed App Ver 01 2022

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.97

K8 Upward = 0.97 S1 Downward = 12.68 S1 Upward = 12.68

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	0.99 Kn-m	Capacity	3.40 Kn-m	Passing Percentage	343.43 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.74 Kn-m	Capacity	4.53 Kn-m	Passing Percentage	165.33 %
M _{0.9D-W_nUp}	-2.40 Kn-m	Capacity	-5.67 Kn-m	Passing Percentage	236.25 %
V _{1.35D}	1.16 Kn	Capacity	12.06 Kn	Passing Percentage	1039.66 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	3.19 Kn	Capacity	16.08 Kn	Passing Percentage	504.08 %
V _{0.9D-W_nUp}	-2.80 Kn	Capacity	-20.10 Kn	Passing Percentage	717.86 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 3.75 mm Limit by Woolcock et al, 1999 Span/240 = 15.02 mm

Deflection under Dead and Service Wind = 4.81 mm Limit by Woolcock et al, 1999 Span/100 = 36.04 mm

Reactions

Maximum downward = 3.19 kn Maximum upward = -2.80 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K₁₁ = 14.9 f_{pj} = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K₁₁ = 2.0 f_{cj} = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = $\phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s$ (Eq 4.12) = -19.95 kn > -2.80 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.80 Kn

Girt Design Front and Back

Girt's Spacing = 900 mm

Girt's Span = 2000 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.92 S1 Downward =9.63 S1 Upward =14.36

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	0.42 Kn-m	Capacity	1.94 Kn-m	Passing Percentage	461.90 %
V _{0.9D-WnUp}	0.84 Kn	Capacity	12.06 Kn	Passing Percentage	1435.71 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 3.10 mm Limit by Woolcock et al, 1999 Span/100 = 20.00 mm

Sag during installation = 0.97 mm

Reactions

Maximum = 0.84 kn

Girt Design Sides

Girt's Spacing = 1200 mm

Girt's Span = 3604 mm

Try Girt 150x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =0.71 S1 Downward =9.63 S1 Upward =19.27

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	1.81 Kn-m	Capacity	1.48 Kn-m	Passing Percentage	81.77 %
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Pole Shed App Ver 01 2022

V_{0.9D-WnUp} 2.01 Kn Capacity 12.06 Kn Passing Percentage **600.00 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 43.63 mm Limit by Woolcock et al. 1999 Span/100 = 36.04 mm
Sag during installation = 10.23 mm

Reactions

Maximum = 2.01 kn

Middle Pole Design

Geometry

150 SED H5 (Minimum 175 dia. at Floor Level)	Dry Use	Height	3750 mm
Area	20729 mm ²	As	15546.6796875 mm ²
I _x	34210793 mm ⁴	Z _x	421056 mm ³
I _y	34210793 mm ⁴	Z _y	421056 mm ³
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 14.414414414414413 m²

Dead	3.60 Kn	Live	3.60 Kn
Wind Down	7.78 Kn	Snow	9.08 Kn
Moment wind	4.50 Kn-m	Moment snow	1.95 Kn-m
Phi	0.8	K ₈	1.00
K ₁ snow	0.8	K ₁ Dead	0.6
K ₁ wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

PhiN _{Cx} Wind	298.50 Kn	PhiM _{Nx} Wind	12.23 Kn-m	PhiV _{Nx} Wind	36.81 Kn
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Pole Shed App Ver 01 2022

PhiNcx Dead	179.10 Kn	PhiMnx Dead	7.34 Kn-m	PhiVnx Dead	22.09 Kn
PhiNcx Snow	238.80 Kn	PhiMnx Snow	9.78 Kn-m	PhiVnx Snow	29.45 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.43 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.20 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 26.19 \text{ mm} < 37.50 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2625 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	4.50 Kn-m	Moment Snow =	Kn-m
Shear Wind =	1.71 Kn	Shear Snow =	1.95 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.99 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.79 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.58 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

Pole Shed App Ver 01 2022

Geometry

150 SED H5 (Minimum 175 dia. at Floor Level)	Dry Use	Height	3250 mm
Area	20729 mm ²	As	15546.6796875 mm ²
Ix	34210793 mm ⁴	Zx	421056 mm ³
Iy	34210793 mm ⁴	Zy	421056 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 7.207207207207206 m²

Dead	1.80 Kn	Live	1.80 Kn
Wind Down	3.89 Kn	Snow	4.54 Kn
Moment Wind	2.25 Kn-m	Moment snow	0.98 Kn-m
Phi	0.8	K8	0.67
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	200.07 Kn	PhiMnx Wind	8.20 Kn-m	PhiVnx Wind	36.81 Kn
PhiNcx Dead	120.04 Kn	PhiMnx Dead	4.92 Kn-m	PhiVnx Dead	22.09 Kn
PhiNcx Snow	160.06 Kn	PhiMnx Snow	6.56 Kn-m	PhiVnx Snow	29.45 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.32 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.12 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 12.19 mm < 34.91 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2625 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 7.207207207207206 m²

Moment Wind =	2.25 Kn-m	Moment Snow =	0.98 Kn-m
Shear Wind =	0.86 Kn	Shear Snow =	0.98 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.99 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.79 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.29 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	2625 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	2.25 Kn-m	Moment Snow =	0.98 Kn-m
Shear Wind =	0.86 Kn	Shear Snow =	0.98 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.99 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	7.79 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.29 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m^3

Density of Timber Pole = 5 Kn/m^3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1300) x K_s (1.5) x $0.5 \times \tan(30) \times \pi \times \text{Dia of Pile}(0.6) \times \text{Height of Pile}(1300)$

Skin Friction = 13.65 Kn

Weight of Pile + Pile Skin Friction = 17.91 Kn

Uplift on one Pile = 11.75 Kn

Uplift is ok