



**Job No.:** 2409031  
**Latitude:** -40.884582

**Address:** 21 Hill View Road, Motupipi, New Zealand  
**Longitude:** 172.839328

**Date:** 18/12/2024  
**Elevation:** 113.5 m

### General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N2	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.1 m
Wind Region	NZ2	Terrain Category	2.24	Design Wind Speed	44.74 m/s
Wind Pressure	1.2 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	Very High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

### Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.10 m  $C_{p,e} = -0.9$   $p_e = -0.97$  KPa  $p_{net} = -0.97$  KPa

For roof  $C_{p,e}$  from 3.10 m To 6.20 m  $C_{p,e} = -0.5$   $p_e = -0.54$  KPa  $p_{net} = -0.54$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 8 m  $C_{p,e} = 0.7$   $p_e = 0.76$  KPa  $p_{net} = 1.12$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.10 m  $C_{p,e} =$   $p_e = -0.70$  KPa  $p_{net} = -0.70$  KPa

Maximum Upward pressure used in roof member Design = 0.97 KPa

Maximum Downward pressure used in roof member Design = 0.58 KPa

Maximum Wall pressure used in Design = 1.12 KPa

Maximum Racking pressure used in Design = 1.30 KPa

### Design Summary

#### Rafter Design External

External Rafter Load Width = 5000 mm

External Rafter Span = 7922 mm

Try Rafter 450x63 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.95

K8 Upward = 0.95 S1 Downward = 13.57 S1 Upward = 13.57

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{1.35D}$	13.24 Kn-m	Capacity	43.42 Kn-m	Passing Percentage	327.95 %
$M_{1.2D+1.5L\ 1.2D+S_n\ 1.2D+W_nDn}$	34.52 Kn-m	Capacity	57.89 Kn-m	Passing Percentage	167.70 %
$M_{0.9D-W_nUp}$	-29.22 Kn-m	Capacity	-72.37 Kn-m	Passing Percentage	247.67 %
$V_{1.35D}$	6.68 Kn	Capacity	48.32 Kn	Passing Percentage	723.35 %
$V_{1.2D+1.5L\ 1.2D+S_n\ 1.2D+W_nDn}$	17.43 Kn	Capacity	64.43 Kn	Passing Percentage	369.65 %
$V_{0.9D-W_nUp}$	-14.75 Kn	Capacity	-80.54 Kn	Passing Percentage	546.03 %

#### Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 15.20 mm

Limit by Woolcock et al, 1999 Span/240 = 33.33 mm

Deflection under Dead and Service Wind = 20.02 mm

Limit by Woolcock et al, 1999 Span/100 = 80.00 mm

#### Reactions

Second page

Maximum downward = 17.43 kn Maximum upward = -14.75 kn

#### Rafter to Pole Connection check

Bolt Size = M16 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafter = J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 12.6 \text{ fpi} = 22.7 \text{ Mpa}$  for Rafter with effective thickness = 63 mm

For Parallel to grain loading

$K_{11} = 2.0 \text{ fcj} = 36.1 \text{ Mpa}$  for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$  (Eq 4.12) = -86.48 kn > -14.75 Kn

Single Shear Capacity under short term loads = -38.81 Kn > -14.75 Kn

#### Girt Design Front and Back

Girt's Spacing = 700 mm

Girt's Span = 5000 mm

Try Girt 240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1  $K_4 = 1$   $K_5 = 1$   $K_8$  Downward = 0.94

$K_8$  Upward = 0.53  $S_1$  Downward = 13.82  $S_1$  Upward = 23.03

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{\text{Wind+Snow}}$	2.45 Kn-m	Capacity	2.57 Kn-m	Passing Percentage	<b>104.90 %</b>
$V_{0.9D-WnUp}$	1.96 Kn	Capacity	17.37 Kn	Passing Percentage	<b>886.22 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 18.37 mm

Limit by Woolcock et al, 1999 Span/100 = 50.00 mm

Sag during installation = 46.79 mm

#### Reactions

Maximum = 1.96 kn

#### Girt Design Sides

Girt's Spacing = 700 mm

Girt's Span = 4000 mm

Try Girt 240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1  $K_4 = 1$   $K_5 = 1$   $K_8$  Downward = 0.94

$K_8$  Upward = 0.34  $S_1$  Downward = 13.82  $S_1$  Upward = 29.13

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{\text{Wind+Snow}}$	1.57 Kn-m	Capacity	1.66 Kn-m	Passing Percentage	<b>105.73 %</b>
$V_{0.9D-WnUp}$	1.57 Kn	Capacity	17.37 Kn	Passing Percentage	<b>1106.37 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 7.52 mm

Limit by Woolcock et al. 1999 Span/100 = 40.00 mm

Sag during installation = 19.16 mm

#### Reactions

Maximum = 1.57 kn

#### End Pole Design

##### Geometry For End Bay Pole

##### Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	2900 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
Ix	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Iy	100042702 mm <sup>4</sup>	Zx	941578 mm <sup>3</sup>
Lateral Restraint	mm c/c		

##### Loads

Total Area over Pole = 40 m<sup>2</sup>

Dead	10.00 Kn	Live	10.00 Kn
Wind Down	23.20 Kn	Snow	0.00 Kn
Moment Wind	11.68 Kn-m		
Phi	0.8	K8	0.95
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

##### Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

##### Capacities

PhiNcx Wind	483.02 Kn	PhiMnx Wind	25.87 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	289.81 Kn	PhiMnx Dead	15.52 Kn-m	PhiVnx Dead	37.77 Kn

##### Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.54 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.29 < 1$  OK

Deflection at top under service lateral loads = 16.99 mm < 30.92 mm

Ds =	0.6 mm	Pile Diameter
L =	1600 mm	Pile embedment length
f1 =	2325 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

##### Loads

Total Area over Pole = 40 m<sup>2</sup>

Moment Wind =	11.68 Kn-m
Shear Wind =	5.02 Kn

##### Pile Properties

Safety Factory	0.55	
Hu =	9.30 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	13.39 Kn-m	Ultimate Moment Capacity of Pile

#### Checks

Applied Forces/Capacities =  $0.87 < 1$  OK

### Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

#### Assumed Soil Properties

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

#### Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1600 mm	Pile embedment length
f1 =	2325 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

#### Loads

Moment Wind =	11.68 Kn-m
Shear Wind =	5.02 Kn

#### Pile Properties

Safety Factory	0.55	
Hu =	9.30 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	13.39 Kn-m	Ultimate Moment Capacity of Pile

#### Checks

Applied Forces/Capacities =  $0.87 < 1$  OK

### Uplift Check

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(2100) x Ks(1.5) x  $0.5 \times \tan(30)$  x Pi x Dia of Pile(0.6) x Height of Pile(2100)

Skin Friction = 35.62 Kn

Weight of Pile + Pile Skin Friction = 39.84 Kn

Uplift on one Pile = 29.80 Kn

Uplift is ok