

Job No.: 618411 - 2
Latitude: -35.268603

Address: 1263A Bulls Rd, Kerikeri, New Zealand
Longitude: 173.938025

Date: 30/01/2024
Elevation: 172.5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.4 m
Wind Region	NZ1	Terrain Category	2.96	Design Wind Speed	43.23 m/s
Wind Pressure	1.12 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 4.0 m $C_{p,e} = -0.9$ $p_e = -0.91$ KPa $p_{net} = -0.91$ KPa

For roof $C_{p,e}$ from 4.0 m To 8.0 m $C_{p,e} = -0.5$ $p_e = -0.50$ KPa $p_{net} = -0.50$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 11.0 m $C_{p,e} = 0.7$ $p_e = 0.71$ KPa $p_{net} = 1.05$ KPa

For side wall $C_{p,e}$ from 0 m To 4.0 m $C_{p,e} =$ $p_e = -0.66$ KPa $p_{net} = -0.66$ KPa

Maximum Upward pressure used in roof member Design = 0.91 KPa

Maximum Downward pressure used in roof member Design = 0.44 KPa

Maximum Wall pressure used in Design = 1.05 KPa

Maximum Racking pressure used in Design = 1.14 KPa

Design Summary

Purlin Design

Purlin Spacing = 900 mm

Purlin Span = 4410 mm

Try Purlin 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.98

K8 Upward = 0.72 S1 Downward = 12.23 S1 Upward = 19.03

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.74 Kn-m	Capacity	1.79 Kn-m	Passing Percentage	241.89 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	1.87 Kn-m	Capacity	2.38 Kn-m	Passing Percentage	127.27 %
$M_{0.9D-W_nUp}$	-1.5 Kn-m	Capacity	-2.18 Kn-m	Passing Percentage	189.57 %

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V _{1.35D}	0.67 Kn	Capacity	8.25 Kn	Passing Percentage	1231.34 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	1.47 Kn	Capacity	11.00 Kn	Passing Percentage	748.30 %
V _{0.9D-WnUp}	-1.36 Kn	Capacity	-13.75 Kn	Passing Percentage	1011.03 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 14.74 mm

Limit by Woolcock et al, 1999 Span/240 = 18.17 mm

Deflection under Dead and Service Wind = 17.69 mm

Limit by Woolcock et al, 1999 Span/100 = 43.60 mm

Reactions

Maximum downward = 1.47 kn Maximum upward = -1.36 kn

Number of Blocking = 1 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4560 mm

Internal Rafter Span = 5350 mm

Try Rafter 2x290x45 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 7.47 S₁ Upward = 7.47

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	5.51 Kn-m	Capacity	8.48 Kn-m	Passing Percentage	153.90 %
M _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	12.07 Kn-m	Capacity	11.3 Kn-m	Passing Percentage	93.62 %
M _{0.9D-WnUp}	-11.18 Kn-m	Capacity	-14.12 Kn-m	Passing Percentage	126.30 %
V _{1.35D}	4.12 Kn	Capacity	25.18 Kn	Passing Percentage	611.17 %
V _{1.2D+1.5L 1.2D+Sn 1.2D+WnDn}	9.03 Kn	Capacity	33.58 Kn	Passing Percentage	371.87 %
V _{0.9D-WnUp}	-8.36 Kn	Capacity	-41.96 Kn	Passing Percentage	501.91 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 14.85 mm

Limit by Woolcock et al, 1999 Span/240 = 22.92 mm

Deflection under Dead and Service Wind = 19.8 mm

Limit by Woolcock et al, 1999 Span/100 = 55.00 mm

Reactions

Maximum downward = 9.03 kn Maximum upward = -8.36 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

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Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$ $f_{pj} = 12.9$ Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

$K_{11} = 2.0$ $f_{cj} = 36.1$ Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 19.50 Kn > -8.36 Kn

Rafter Design External

External Rafter Load Width = 2280 mm

External Rafter Span = 10829 mm

Try Rafter 450x45 LVL13

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 K_1 Medium term = 0.8 K_1 Long term = 0.6 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.72

K_8 Upward = 0.72 S_1 Downward = 19.04 S_1 Upward = 19.04

Shear Capacity of timber = 5.3 MPa Bending Capacity of timber = 48 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	11.28 Kn-m	Capacity	23.45 Kn-m	Passing Percentage	207.89 %
$M_{1.2D+1.5L\ 1.2D+S_n\ 1.2D+W_nD_n}$	24.73 Kn-m	Capacity	31.26 Kn-m	Passing Percentage	126.41 %
$M_{0.9D-W_nUp}$	-22.89 Kn-m	Capacity	-39.08 Kn-m	Passing Percentage	170.73 %
$V_{1.35D}$	4.17 Kn	Capacity	34.52 Kn	Passing Percentage	827.82 %
$V_{1.2D+1.5L\ 1.2D+S_n\ 1.2D+W_nD_n}$	9.14 Kn	Capacity	46.02 Kn	Passing Percentage	503.50 %
$V_{0.9D-W_nUp}$	-8.46 Kn	Capacity	-57.53 Kn	Passing Percentage	680.02 %

Deflections

Modulus of Elasticity = 11000 MPa NZS3603 Amt 4, Table 2.3

k_2 for Long Term Loads = 2

Deflection under Dead and Live Load = 34.69 mm

Limit by Woolcock et al, 1999 Span/240 = 45.83 mm

Deflection under Dead and Service Wind = 41.63 mm

Limit by Woolcock et al, 1999 Span/100 = 110.00 mm

Reactions

Maximum downward = 9.14 kn Maximum upward = -8.46 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 3

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 12.6 \text{ fpj} = 22.7 \text{ Mpa}$ for Rafter with effective thickness = 45 mm

For Parallel to grain loading

$K_{11} = 2.0 \text{ fcj} = 36.1 \text{ Mpa}$ for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi_i \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -65.11 kn > -8.46 Kn

Single Shear Capacity under short term loads = -21.83 Kn > -8.46 Kn

Intermediate Design Sides

Intermediate Spacing = 5500 mm

Intermediate Span = 3850 mm

Try Intermediate 2x250x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.97

K_8 Upward = 1.00 S_1 Downward = 12.68 S_1 Upward = 0.83

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{\text{Wind+Snow}}$	5.35 Kn-m	Capacity	11.66 Kn-m	Passing Percentage	217.94 %
$V_{0.9D-WnUp}$	5.56 Kn-m	Capacity	40.2 Kn-m	Passing Percentage	723.02 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 23.495 mm

Limit by Woolcock et al, 1999 Span/100 = 38.50 mm

Reactions

Maximum = 5.56 kn

Girt Design Front and Back

Girt's Spacing = 650 mm

Girt's Span = 4560 mm

Try Girt 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 0.98

K_8 Upward = 0.70 S_1 Downward = 12.23 S_1 Upward = 19.46

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{\text{Wind+Snow}}$	1.77 Kn-m	Capacity	2.11 Kn-m	Passing Percentage	119.21 %
$V_{0.9D-WnUp}$	1.56 Kn-m	Capacity	13.75 Kn-m	Passing Percentage	881.41 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 22.30 mm

Limit by Woolcock et al, 1999 Span/100 = 45.60 mm

Sag during installation = 32.37 mm

Reactions

Maximum = 1.56 kn

Girt Design Sides

Girt's Spacing = 1300 mm

Girt's Span = 5500 mm

Try Girt 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K4 =1 K5 =1 K8 Downward =0.98

K8 Upward =0.32 S1 Downward =12.23 S1 Upward =30.23

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{Wind+Snow}	5.16 Kn-m	Capacity	0.97 Kn-m	Passing Percentage	18.80 %
V _{0.9D-WnUp}	3.75 Kn-m	Capacity	13.75 Kn-m	Passing Percentage	366.67 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 94.37 mm

Limit by Woolcock et al. 1999 Span/100 = 55.00 mm

Sag during installation =68.50 mm

Reactions

Maximum = 3.75 kn

Middle Pole Design

Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3950 mm
Area	35448 mm ²	As	26585.7421875 mm ²
I _x	100042702 mm ⁴	Z _x	941578 mm ³
I _y	100042702 mm ⁴	Z _y	941578 mm ³
Lateral Restraint	1300 mm c/c		

Loads

Total Area over Pole = 25.08 m²

Dead	6.27 Kn	Live	6.27 Kn
Wind Down	11.04 Kn	Snow	0.00 Kn
Moment wind	12.55 Kn-m		
Phi	0.8	K8	1.00
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

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Peeling	Steaming	Normal	Dry Use
$f_b =$	36.3 MPa	$f_s =$	2.96 MPa
$f_c =$	18 MPa	$f_p =$	7.2 MPa
$f_t =$	22 MPa	$E =$	9257 MPa

Capacities

PhiNcx Wind	510.45 Kn	PhiMnx Wind	27.34 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	306.27 Kn	PhiMnx Dead	16.41 Kn-m	PhiVnx Dead	37.77 Kn

Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.51 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.26 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 33.09 \text{ mm} < 39.50 \text{ mm}$$

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
$K_0 =$	$(1 - \sin(30)) / (1 + \sin(30))$				
$K_p =$	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For Middle Bay Pole

$D_s =$	0.6 mm	Pile Diameter
$L =$	1550 mm	Pile embedment length
$f_1 =$	3300 mm	Distance at which the shear force is applied
$f_2 =$	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	12.55 Kn-m
Shear Wind =	3.80 Kn

Pile Properties

Safety Factory	0.55	
$H_u =$	6.84 Kn	Ultimate Lateral Strength of the Pile, Short pile
$M_u =$	13.36 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.94 < 1 \text{ OK}$$

End Pole Design

Geometry For End Bay Pole

Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3950 mm
Area	35448 mm ²	As	26585.7421875 mm ²

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Ix	100042702 mm ⁴	Zx	941578 mm ³
Iy	100042702 mm ⁴	Zy	941578 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 25.08 m²

Dead	6.27 Kn	Live	6.27 Kn
Wind Down	11.04 Kn	Snow	0.00 Kn
Moment Wind	9.41 Kn-m		
Phi	0.8	K8	0.74
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
f _b =	36.3 MPa	f _s =	2.96 MPa
f _c =	18 MPa	f _p =	7.2 MPa
f _t =	22 MPa	E =	9257 MPa

Capacities

PhiN _{cx} Wind	377.56 Kn	PhiM _{nx} Wind	20.22 Kn-m	PhiV _{nx} Wind	62.96 Kn
PhiN _{cx} Dead	226.54 Kn	PhiM _{nx} Dead	12.13 Kn-m	PhiV _{nx} Dead	37.77 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.53 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.28 < 1$ OK

Deflection at top under service lateral loads = 27.58 mm < 43.89 mm

D _s =	0.6 mm	Pile Diameter
L =	1550 mm	Pile embedment length
f ₁ =	3300 mm	Distance at which the shear force is applied
f ₂ =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 25.08 m²

Moment Wind =	9.41 Kn-m
Shear Wind =	2.85 Kn

Pile Properties

Safety Factory	0.55	
H _u =	6.84 Kn	Ultimate Lateral Strength of the Pile, Short pile
M _u =	13.36 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.70 < 1$ OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K ₀ =	$(1 - \sin(30)) / (1 + \sin(30))$				
K _p =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

D _s =	0.6 mm	Pile Diameter
L =	1550 mm	Pile embedment length
f ₁ =	3300 mm	Distance at which the shear force is applied
f ₂ =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	9.41 Kn-m
Shear Wind =	2.85 Kn

Pile Properties

Safety Factory	0.55	
H _u =	6.84 Kn	Ultimate Lateral Strength of the Pile, Short pile
M _u =	13.36 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.70 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

K_s (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1550) x K_s(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1550)

Skin Friction = 19.40 Kn

Weight of Pile + Pile Skin Friction = 23.43 Kn

Uplift on one Pile = 17.18 Kn

Uplift is ok