



**Job No.:** 652551 - 1  
**Latitude:** -34.972616

**Address:** 721b Taupo Bay Rd, Taupo Bay, New Zealand  
**Longitude:** 173.697678

**Date:** 13/04/2024  
**Elevation:** 43.5 m

### General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	3.6 m
Wind Region	NZ1	Terrain Category	2.34	Design Wind Speed	43 m/s
Wind Pressure	1.11 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

### Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.30 m  $C_{p,e} = -0.9$   $p_e = -0.90$  KPa  $p_{net} = -0.90$  KPa

For roof  $C_{p,e}$  from 3.30 m To 6.60 m  $C_{p,e} = -0.5$   $p_e = -0.50$  KPa  $p_{net} = -0.50$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 8 m  $C_{p,e} = 0.7$   $p_e = 0.70$  KPa  $p_{net} = 1.03$  KPa

For side wall  $C_{p,e}$  from 0 m To 3.30 m  $C_{p,e} =$   $p_e = -0.65$  KPa  $p_{net} = -0.65$  KPa

Maximum Upward pressure used in roof member Design = 0.90 KPa

Maximum Downward pressure used in roof member Design = 0.43 KPa

Maximum Wall pressure used in Design = 1.03 KPa

Maximum Racking pressure used in Design = 1.2 KPa

### Design Summary

#### Rafter Design Internal

Internal Rafter Load Width = 4000 mm Internal Rafter Span = 7850 mm Try Rafter 2x360x45 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 1.00 S1 Downward = 8.40 S1 Upward = 8.40

Shear Capacity of timber = 5 MPa Bending Capacity of timber = 38 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

M <sub>1.35D</sub>	10.40 Kn-m	Capacity	34.4 Kn-m	Passing Percentage	330.77 %
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>n</sub>D<sub>n</sub></sub>	22.49 Kn-m	Capacity	45.86 Kn-m	Passing Percentage	203.91 %
M <sub>0.9D-W<sub>n</sub>Up</sub>	-20.80 Kn-m	Capacity	-57.32 Kn-m	Passing Percentage	275.58 %
V <sub>1.35D</sub>	5.30 Kn	Capacity	52.1 Kn	Passing Percentage	983.02 %

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V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	11.46 Kn	Capacity	69.46 Kn	Passing Percentage	<b>606.11 %</b>
V <sub>0.9D-WnUp</sub>	-10.60 Kn	Capacity	-86.84 Kn	Passing Percentage	<b>819.25 %</b>

**Deflections**

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 16.625 mm Limit by Woolcock et al, 1999 Span/240 = 33.33 mm

Deflection under Dead and Service Wind = 22.015 mm Limit by Woolcock et al, 1999 Span/100 = 80.00 mm

**Reactions**

Maximum downward = 11.46 kn Maximum upward = -10.60 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K<sub>11</sub> = 12.6 f<sub>pj</sub> = 22.7 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K<sub>11</sub> = 2.0 f<sub>cj</sub> = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -10.60 Kn

**Girt Design Front and Back**

Girt's Spacing = 1300 mm

Girt's Span = 2000 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1 K<sub>4</sub> = 1 K<sub>5</sub> = 1 K<sub>8</sub> Downward = 1.00

K<sub>8</sub> Upward = 0.88 S<sub>1</sub> Downward = 10.36 S<sub>1</sub> Upward = 15.45

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>Wind+Snow</sub>	0.67 Kn-m	Capacity	1.45 Kn-m	Passing Percentage	<b>216.42 %</b>
V <sub>0.9D-WnUp</sub>	1.34 Kn	Capacity	10.13 Kn	Passing Percentage	<b>755.97 %</b>

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 4.05 mm Limit by Woolcock et al, 1999 Span/100 = 20.00 mm

Sag during installation = 1.20 mm

#### Reactions

Maximum = 1.34 kn

#### Girt Design Sides

Girt's Spacing = 1300 mm

Girt's Span = 2000 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1    K4 =1    K5 =1    K8 Downward =1.00

K8 Upward =0.88    S1 Downward =10.36    S1 Upward =15.45

Shear Capacity of timber =3 MPa    Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

#### Capacity Checks

$M_{Wind+Snow}$	0.67 Kn-m	Capacity	1.45 Kn-m	Passing Percentage	<b>216.42 %</b>
$V_{0.9D-WnUp}$	1.34 Kn	Capacity	10.13 Kn	Passing Percentage	<b>755.97 %</b>

#### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 4.05 mm

Limit by Woolcock et al. 1999 Span/100 = 20.00 mm

Sag during installation =1.20 mm

#### Reactions

Maximum = 1.34 kn

#### Middle Pole Design

##### Geometry

200 SED H5 (Minimum 225 dia. at Floor Level)	Dry Use	Height	3300 mm
Area	35448 mm <sup>2</sup>	As	26585.7421875 mm <sup>2</sup>
I <sub>x</sub>	100042702 mm <sup>4</sup>	Z <sub>x</sub>	941578 mm <sup>3</sup>
I <sub>y</sub>	100042702 mm <sup>4</sup>	Z <sub>y</sub>	941578 mm <sup>3</sup>
Lateral Restraint	3400 mm c/c		

#### Loads

Total Area over Pole = 16 m<sup>2</sup>

Dead	4.00 Kn	Live	4.00 Kn
Wind Down	6.88 Kn	Snow	0.00 Kn
Moment wind	11.63 Kn-m		
Phi	0.8	K8	0.86
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

#### Material

Peeling	Steaming	Normal	Dry Use
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fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

**Capacities**

PhiNcx Wind	438.78 Kn	PhiMnx Wind	23.50 Kn-m	PhiVnx Wind	62.96 Kn
PhiNcx Dead	263.27 Kn	PhiMnx Dead	14.10 Kn-m	PhiVnx Dead	37.77 Kn

**Checks**

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.53 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.28 < 1 \text{ OK}$$

$$\text{Deflection at top under service lateral loads} = 20.97 \text{ mm} < 33.00 \text{ mm}$$

**Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile**

**Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For Middle Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	1550 mm	Pile embedment length
f1 =	2700 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	11.63 Kn-m
Shear Wind =	4.31 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	7.81 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	12.75 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

$$\text{Applied Forces/Capacities} = 0.91 < 1 \text{ OK}$$

**Uplift Check**

$$\text{Density of Concrete} = 24 \text{ Kn/m}^3$$

$$\text{Density of Timber Pole} = 5 \text{ Kn/m}^3$$

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

$$K_s (\text{Lateral Earth Pressure Coefficient}) \text{ for cast into place concrete piles} = 1.5$$

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Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil(18) x Height of Pile(1550) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(1550)

Skin Friction = 19.40 Kn

Weight of Pile + Pile Skin Friction = 23.43 Kn

Uplift on one Pile = 10.80 Kn

Uplift is ok