

Job No.: 2409020**Address:** 4 Collinson Street, Pākawau,, Collingwood, New Zealand**Date:** 12/09/2024**Latitude:** -40.579639**Longitude:** 172.688896**Elevation:** 8.5 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	2	Subsoil Category	D	Exposure Zone	D
Importance Level	1	Ultimate wind & Earthquake ARI	100 Years	Max Height	4.4 m
Wind Region	NZ2	Terrain Category	1.32	Design Wind Speed	38.32 m/s
Wind Pressure	0.88 KPa	Lee Zone	NO	Ultimate Snow ARI	50 Years
Wind Category	High	Earthquake ARI	100		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressures

Shed Type = Mono Enclosed

For roof $C_{p,i} = -0.3$

For roof $C_{p,e}$ from 0 m To 3.95 m $C_{p,e} = -0.9$ $p_e = -0.71$ KPa $p_{net} = -0.71$ KPa

For roof $C_{p,e}$ from 3.95 m To 7.9 m $C_{p,e} = -0.5$ $p_e = -0.4$ KPa $p_{net} = -0.4$ KPa

For wall Windward $C_{p,i} = -0.3$ side Wall $C_{p,i} = -0.3$

For wall Windward and Leeward $C_{p,e}$ from 0 m To 15 m $C_{p,e} = 0.7$ $p_e = 0.56$ KPa $p_{net} = 0.82$ KPa

For side wall $C_{p,e}$ from 0 m To 3.95 m $C_{p,e} =$ $p_e = -0.52$ KPa $p_{net} = -0.52$ KPa

Maximum Upward pressure used in roof member Design = 0.71 KPa

Maximum Downward pressure used in roof member Design = 0.42 KPa

Maximum Wall pressure used in Design = 0.82 KPa

Maximum Racking pressure used in Design = 0.89 KPa

Design Summary**Purlin Design**

Purlin Spacing = 650 mm

Purlin Span = 4850 mm

Try Purlin 190x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 0.98

K8 Upward = 0.36 S1 Downward = 12.23 S1 Upward = 28.24

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.65 Kn-m	Capacity	1.79 Kn-m	Passing Percentage	275.38 %
$M_{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}$	1.91 Kn-m	Capacity	2.38 Kn-m	Passing Percentage	124.61 %
$M_{0.9D-W_nUp}$	-0.93 Kn-m	Capacity	-1.11 Kn-m	Passing Percentage	79.86 %
$V_{1.35D}$	0.53 Kn	Capacity	8.25 Kn	Passing Percentage	1556.60 %

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V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	1.13 Kn	Capacity	11.00 Kn	Passing Percentage	973.45 %
V _{0.9D-W_nUp}	-0.76 Kn	Capacity	-13.75 Kn	Passing Percentage	1809.21 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 15.64 mm Limit by Woolcock et al, 1999 Span/240 = 20.00 mm

Deflection under Dead and Service Wind = 18.51 mm Limit by Woolcock et al, 1999 Span/100 = 48.00 mm

Reactions

Maximum downward = 1.13 kn Maximum upward = -0.76 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 5000 mm Internal Rafter Span = 3516.6666666666665 mm Try Rafter 2x240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K₁ Short term = 1 K₁ Medium term = 0.8 K₁ Long term = 0.6 K₄ = 1 K₅ = 1 K₈ Downward = 1.00

K₈ Upward = 1.00 S₁ Downward = 6.71 S₁ Upward = 6.71

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	2.61 Kn-m	Capacity	5.8 Kn-m	Passing Percentage	222.22 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	5.57 Kn-m	Capacity	7.74 Kn-m	Passing Percentage	138.96 %
M _{0.9D-W_nUp}	-3.75 Kn-m	Capacity	-9.68 Kn-m	Passing Percentage	258.13 %
V _{1.35D}	2.97 Kn	Capacity	20.84 Kn	Passing Percentage	701.68 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	6.33 Kn	Capacity	27.78 Kn	Passing Percentage	438.86 %
V _{0.9D-W_nUp}	-4.26 Kn	Capacity	-34.74 Kn	Passing Percentage	815.49 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 5.675 mm Limit by Woolcock et al, 1999 Span/240 = 15.28 mm

Deflection under Dead and Service Wind = 7.46 mm Limit by Woolcock et al, 1999 Span/100 = 36.67 mm

Reactions

Maximum downward = 6.33 kn Maximum upward = -4.26 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 19.50 Kn > -4.26 Kn

Rafter Design External

External Rafter Load Width = 2500 mm

External Rafter Span = 3481 mm

Try Rafter 240x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =0.94

K8 Upward =0.94 S1 Downward =13.82 S1 Upward =13.82

Shear Capacity of timber =3 MPa Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

M _{1.35D}	1.28 Kn-m	Capacity	2.73 Kn-m	Passing Percentage	213.28 %
M _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	2.73 Kn-m	Capacity	3.64 Kn-m	Passing Percentage	133.33 %
M _{0.9D-W_nUp}	-1.84 Kn-m	Capacity	-4.55 Kn-m	Passing Percentage	247.28 %
V _{1.35D}	1.47 Kn	Capacity	10.42 Kn	Passing Percentage	708.84 %
V _{1.2D+1.5L 1.2D+S_n 1.2D+W_nD_n}	3.13 Kn	Capacity	13.89 Kn	Passing Percentage	443.77 %
V _{0.9D-W_nUp}	-2.11 Kn	Capacity	-17.37 Kn	Passing Percentage	823.22 %

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k₂ for Long Term Loads = 2

Deflection under Dead and Live Load = 6.31 mm

Limit by Woolcock et al, 1999 Span/240= 15.28 mm

Deflection under Dead and Service Wind = 7.46 mm

Limit by Woolcock et al, 1999 Span/100 = 36.67 mm

Reactions

Maximum downward =3.13 kn Maximum upward = -2.11 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 45 mm

For Parallel to grain loading

$K_{11} = 2.0$ $f_{c,j} = 36.1$ Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$ (Eq 4.12) = -17.01 kn > -2.11 Kn

Single Shear Capacity under short term loads = -9.75 Kn > -2.11 Kn

Girt Design Front and Back

Girt's Spacing = 1300 mm

Girt's Span = 2500 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.80 S_1 Downward = 10.36 S_1 Upward = 17.27

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.83 Kn-m	Capacity	1.32 Kn-m	Passing Percentage	159.04 %
$V_{0.9D-WnUp}$	1.33 Kn	Capacity	10.13 Kn	Passing Percentage	761.65 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 7.86 mm

Limit by Woolcock et al, 1999 Span/100 = 25.00 mm

Sag during installation = 2.92 mm

Reactions

Maximum = 1.33 kn

Girt Design Sides

Girt's Spacing = 700 mm

Girt's Span = 3667 mm

Try Girt 140x45 SG8

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K_1 Short term = 1 $K_4 = 1$ $K_5 = 1$ K_8 Downward = 1.00

K_8 Upward = 0.63 S_1 Downward = 10.36 S_1 Upward = 20.92

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	0.96 Kn-m	Capacity	1.03 Kn-m	Passing Percentage	107.29 %
$V_{0.9D-WnUp}$	1.05 Kn	Capacity	10.13 Kn	Passing Percentage	964.76 %

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 19.60 mm

Limit by Woolcock et al. 1999 Span/100 = 36.67 mm

Sag during installation = 13.53 mm

Reactions

Maximum = 1.05 kn

Middle Pole Design

Geometry

175 SED H5 (Minimum 200 dia. at Floor Level)	Dry Use	Height	4150 mm
Area	27598 mm ²	As	20698.2421875 mm ²
Ix	60639381 mm ⁴	Zx	646820 mm ³
Iy	60639381 mm ⁴	Zx	646820 mm ³
Lateral Restraint	4150 mm c/c		

Loads

Total Area over Pole = 18.33333333333332 m²

Dead	4.58 Kn	Live	4.58 Kn
Wind Down	7.70 Kn	Snow	0.00 Kn
Moment wind	8.06 Kn-m		
Phi	0.8	K8	0.57
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	226.22 Kn	PhiMnx Wind	10.69 Kn-m	PhiVnx Wind	49.01 Kn
PhiNcx Dead	135.73 Kn	PhiMnx Dead	6.42 Kn-m	PhiVnx Dead	29.41 Kn

Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.83 < 1$ OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.64 < 1$ OK

Deflection at top under service lateral loads = 36.82 mm < 41.50 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

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Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3300 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	8.06 Kn-m
Shear Wind =	2.44 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.26 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.19 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.98 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

150 SED H5 (Minimum 175 dia. at Floor Level)	Dry Use	Height	4200 mm
Area	20729 mm ²	As	15546.6796875 mm ²
Ix	34210793 mm ⁴	Zx	421056 mm ³
Iy	34210793 mm ⁴	Zx	421056 mm ³
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 9.166666666666666 m²

Dead	2.29 Kn	Live	2.29 Kn
Wind Down	3.85 Kn	Snow	0.00 Kn
Moment Wind	4.03 Kn-m		
Phi	0.8	K8	0.43
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	36.3 MPa	fs =	2.96 MPa
fc =	18 MPa	fp =	7.2 MPa
ft =	22 MPa	E =	9257 MPa

Capacities

PhiNcx Wind	129.22 Kn	PhiMnx Wind	5.29 Kn-m	PhiVnx Wind	36.81 Kn
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PhiNcx Dead	77.53 Kn	PhiMnx Dead	3.18 Kn-m	PhiVnx Dead	22.09 Kn
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Checks

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.83 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.64 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 34.51 mm < 43.89 mm

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3300 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 9.166666666666666 m²

Moment Wind =	4.03 Kn-m
Shear Wind =	1.22 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.26 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	8.19 Kn-m	Ultimate Moment Capacity of Pile

Checks

$$\text{Applied Forces/Capacities} = 0.49 < 1 \text{ OK}$$

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile**Assumed Soil Properties**

Gamma	18 Kn/m ³	Friction angle	30 deg	Cohesion	0 Kn/m ³
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1300 mm	Pile embedment length
f1 =	3300 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	4.03 Kn-m
Shear Wind =	1.22 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.26 Kn	Ultimate Lateral Strength of the Pile, Short pile

Mu = 8.19 Kn-m Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = $0.49 < 1$ OK

Uplift Check

Density of Concrete = 24 Kn/m³

Density of Timber Pole = 5 Kn/m³

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil (18) x Height of Pile (1300) x Ks (1.5) x $0.5 \times \tan(30)$ x π x Dia of Pile (0.6) x Height of Pile (1300)

Skin Friction = 13.65 Kn

Weight of Pile + Pile Skin Friction = 17.45 Kn

Uplift on one Pile = 8.89 Kn

Uplift is ok