



**Job No.:** TEST J**Address:****Date:** 29/08/2024**Latitude:** -37.685058**Longitude:** 176.247142**Elevation:** 5 m**General Input**

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	2	Ultimate wind & Earthquake ARI	500 Years	Max Height	6.6 m
Wind Region	NZ1	Terrain Category	3.0	Design Wind Speed	37.35 m/s
Wind Pressure	0.84 KPa	Lee Zone	NO	Ultimate Snow ARI	150 Years
Wind Category	High	Earthquake ARI	500		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

**Pressure Coefficients and Pressures**

Shed Type = Mono Enclosed

For roof  $C_{p,i} = -0.3$

For roof  $C_{p,e}$  from 0 m To 3.13 m  $C_{p,e} = -0.9167$   $p_e = -0.69$  KPa  $p_{net} = -0.69$  KPa

For roof  $C_{p,e}$  from 3.13 m To 6.25 m  $C_{p,e} = -0.8917$   $p_e = -0.67$  KPa  $p_{net} = -0.67$  KPa

For wall Windward  $C_{p,i} = -0.3$  side Wall  $C_{p,i} = -0.3$

For wall Windward and Leeward  $C_{p,e}$  from 0 m To 12 m  $C_{p,e} = 0.7$   $p_e = 0.53$  KPa  $p_{net} = 0.78$  KPa

For side wall  $C_{p,e}$  from 0 m To 6.25 m  $C_{p,e} =$   $p_e = -0.49$  KPa  $p_{net} = -0.49$  KPa

Maximum Upward pressure used in roof member Design = 0.69 KPa

Maximum Downward pressure used in roof member Design = 0.40 KPa

Maximum Wall pressure used in Design = 0.78 KPa

Maximum Racking pressure used in Design = 0.91 KPa

**Design Summary****Purlin Design**

Purlin Spacing = 800 mm

Purlin Span = 4350 mm

Try Purlin 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 = 1 K5 = 1 K8 Downward = 1.00

K8 Upward = 0.47 S1 Downward = 11.27 S1 Upward = 24.64

Shear Capacity of timber = 3 MPa Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>1.35D</sub>	0.64 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	348.44 %
M <sub>1.2D+1.5L 1.2D+S<sub>n</sub> 1.2D+W<sub>nDn</sub></sub>	1.76 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	168.75 %
M <sub>0.9D-W<sub>nUp</sub></sub>	-0.88 Kn-m	Capacity	-1.76 Kn-m	Passing Percentage	200.00 %
V <sub>1.35D</sub>	0.59 Kn	Capacity	9.65 Kn	Passing Percentage	1635.59 %

Pole Shed App Ver 01 2022

V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	1.22 Kn	Capacity	12.86 Kn	Passing Percentage	<b>1054.10 %</b>
V <sub>0.9D-WnUp</sub>	-0.81 Kn	Capacity	-16.08 Kn	Passing Percentage	<b>1985.19 %</b>

**Deflections**

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 9.57 mm Limit by Woolcock et al, 1999 Span/360 = 11.94 mm

Deflection under Dead and Service Wind = 11.16 mm Limit by Woolcock et al, 1999 Span/250 = 28.67 mm

**Reactions**

Maximum downward = 1.22 kn Maximum upward = -0.81 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

**Rafter Design Internal**

Internal Rafter Load Width = 4500 mm Internal Rafter Span = 11850 mm Try Rafter 2x610x45 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K<sub>1</sub> Short term = 1 K<sub>1</sub> Medium term = 0.8 K<sub>1</sub> Long term = 0.6 K<sub>4</sub> = 1 K<sub>5</sub> = 1 K<sub>8</sub> Downward = 1.00

K<sub>8</sub> Upward = 1.00 S<sub>1</sub> Downward = 11.05 S<sub>1</sub> Upward = 11.05

Shear Capacity of timber = 5 MPa Bending Capacity of timber = 38 MPa NZS3603 Amt 4, table 2.3

**Capacity Checks**

M <sub>1.35D</sub>	26.66 Kn-m	Capacity	90.18 Kn-m	Passing Percentage	<b>338.26 %</b>
M <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	55.29 Kn-m	Capacity	120.24 Kn-m	Passing Percentage	<b>217.47 %</b>
M <sub>0.9D-WnUp</sub>	-36.73 Kn-m	Capacity	-150.28 Kn-m	Passing Percentage	<b>409.15 %</b>
V <sub>1.35D</sub>	9.00 Kn	Capacity	88.28 Kn	Passing Percentage	<b>980.89 %</b>
V <sub>1.2D+1.5L 1.2D+Sn 1.2D+WnDn</sub>	18.66 Kn	Capacity	117.7 Kn	Passing Percentage	<b>630.76 %</b>
V <sub>0.9D-WnUp</sub>	-12.40 Kn	Capacity	-147.14 Kn	Passing Percentage	<b>1186.61 %</b>

**Deflections**

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k<sub>2</sub> for Long Term Loads = 2

Deflection under Dead and Live Load = 19.465 mm Limit by Woolcock et al, 1999 Span/360 = 33.33 mm

Deflection under Dead and Service Wind = 25.23 mm Limit by Woolcock et al, 1999 Span/250 = 80.00 mm

**Reactions**

Maximum downward = 18.66 kn Maximum upward = -12.40 kn

**Rafter to Pole Connection check**

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 12.6$  fpj = 22.7 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

$K_{11} = 2.0$  fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -12.40 Kn

### Rafter Design External

External Rafter Load Width = 2250 mm

External Rafter Span = 3831 mm

Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_1$  Medium term = 0.8     $K_1$  Long term = 0.6     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 0.94

$K_8$  Upward = 0.94     $S_1$  Downward = 13.93     $S_1$  Upward = 13.93

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

$M_{1.35D}$	1.39 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	<b>339.57 %</b>
$M_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	2.89 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	<b>217.99 %</b>
$M_{0.9D-W_nUp}$	-1.92 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	<b>409.90 %</b>
$V_{1.35D}$	1.45 Kn	Capacity	14.47 Kn	Passing Percentage	<b>997.93 %</b>
$V_{1.2D+1.5L \ 1.2D+S_n \ 1.2D+W_nD_n}$	3.02 Kn	Capacity	19.30 Kn	Passing Percentage	<b>639.07 %</b>
$V_{0.9D-W_nUp}$	-2.00 Kn	Capacity	-24.12 Kn	Passing Percentage	<b>1206.00 %</b>

### Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

$k_2$  for Long Term Loads = 2

Deflection under Dead and Live Load = 3.70 mm

Limit by Woolcock et al, 1999 Span/360 = 11.11 mm

Deflection under Dead and Service Wind = 4.32 mm

Limit by Woolcock et al, 1999 Span/250 = 26.67 mm

### Reactions

Maximum downward = 3.02 kn    Maximum upward = -2.00 kn

### Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters = J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

$K_{11} = 14.9$  fpj = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

$K_{11} = 2.0$   $f_{c,j} = 36.1$  Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

$V = \phi_i \times k_1 \times k_4 \times k_5 \times f_s \times b \times d_s \dots\dots\dots$  (Eq 4.12) = -25.20 kn > -2.00 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.00 Kn

### Girt Design Front and Back

Girt's Spacing = 900 mm

Girt's Span = 4500 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 1.00

$K_8$  Upward = 0.78     $S_1$  Downward = 11.27     $S_1$  Upward = 17.82

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

$M_{Wind+Snow}$	1.78 Kn-m	Capacity	2.90 Kn-m	Passing Percentage	<b>162.92 %</b>
$V_{0.9D-WnUp}$	1.58 Kn	Capacity	16.08 Kn	Passing Percentage	<b>1017.72 %</b>

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 16.78 mm

Limit by Woolcock et al, 1999 Span/250 = 18.00 mm

Sag during installation = 24.86 mm

### Reactions

Maximum = 1.58 kn

### Girt Design Sides

Girt's Spacing = 900 mm

Girt's Span = 4000 mm

Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

$K_1$  Short term = 1     $K_4 = 1$      $K_5 = 1$      $K_8$  Downward = 1.00

$K_8$  Upward = 0.50     $S_1$  Downward = 11.27     $S_1$  Upward = 23.76

Shear Capacity of timber = 3 MPa    Bending Capacity of timber = 14 MPa NZS3603 Amt 4, table 2.3

### Capacity Checks

$M_{Wind+Snow}$	1.40 Kn-m	Capacity	1.87 Kn-m	Passing Percentage	<b>133.57 %</b>
$V_{0.9D-WnUp}$	1.40 Kn	Capacity	16.08 Kn	Passing Percentage	<b>1148.57 %</b>

### Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 10.48 mm

Limit by Woolcock et al. 1999 Span/100 = 16.00 mm

Sag during installation = 15.52 mm

#### Reactions

Maximum = 1.40 kn

#### Middle Pole Design

##### Geometry

350 SED H5 HIGH DENSITY (Minimum 375 dia. at Floor Level)	Dry Use	Height	5640 mm
Area	103154 mm <sup>2</sup>	As	77365.4296875 mm <sup>2</sup>
Ix	847191750 mm <sup>4</sup>	Zx	4674161 mm <sup>3</sup>
Iy	847191750 mm <sup>4</sup>	Zy	4674161 mm <sup>3</sup>
Lateral Restraint	5640 mm c/c		

##### Loads

Total Area over Pole = 27 m<sup>2</sup>

Dead	6.75 Kn	Live	6.75 Kn
Wind Down	10.80 Kn	Snow	0.00 Kn
Moment wind	33.36 Kn-m		
Phi	0.8	K8	0.88
K1 snow	0.8	K1 Dead	0.6
K1 wind	1		

##### Material

Peeling	Steaming	Normal	Dry Use
fb =	49.725 MPa	fs =	2.84 MPa
fc =	28.125 MPa	fp =	8.66 MPa
ft =	29.64 MPa	E =	12874 MPa

##### Capacities

PhiNcx Wind	2037.80 Kn	PhiMnx Wind	163.25 Kn-m	PhiVnx Wind	175.77 Kn
PhiNcx Dead	1222.68 Kn	PhiMnx Dead	97.95 Kn-m	PhiVnx Dead	105.46 Kn

##### Checks

$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.22 < 1$  OK

$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.05 < 1$  OK

Deflection at top under service lateral loads = 16.00 mm < 37.60 mm

#### Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

##### Assumed Soil Properties

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

Pole Shed App Ver 01 2022

**Geometry For Middle Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	2200 mm	Pile embedment length
f1 =	4950 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	33.36 Kn-m
Shear Wind =	6.74 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	13.27 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	38.68 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.86 < 1 OK

**End Pole Design**

**Geometry For End Bay Pole**

**Geometry**

225 SED H5 HIGH DENSITY (Minimum 250 dia. at Floor Level)	Dry Use	Height	6300 mm
Area	44279 mm <sup>2</sup>	As	33209.1796875 mm <sup>2</sup>
Ix	156100441 mm <sup>4</sup>	Zx	1314530 mm <sup>3</sup>
Iy	156100441 mm <sup>4</sup>	Zx	1314530 mm <sup>3</sup>
Lateral Restraint	mm c/c		

**Loads**

Total Area over Pole = 9 m<sup>2</sup>

Dead	2.25 Kn	Live	2.25 Kn
Wind Down	3.60 Kn	Snow	0.00 Kn
Moment Wind	8.34 Kn-m		
Phi	0.8	K8	0.41
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

**Material**

Peeling	Steaming	Normal	Dry Use
fb =	49.725 MPa	fs =	2.84 MPa
fc =	28.125 MPa	fp =	8.66 MPa
ft =	29.64 MPa	E =	12874 MPa

**Capacities**

PhiNcx Wind	410.14 Kn	PhiMnx Wind	21.53 Kn-m	PhiVnx Wind	75.45 Kn
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PhiNcx Dead	246.08 Kn	PhiMnx Dead	12.92 Kn-m	PhiVnx Dead	45.27 Kn
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**Checks**

$$(M_x/\Phi M_{nx}) + (N/\Phi N_{cx}) = 0.41 < 1 \text{ OK}$$

$$(M_x/\Phi M_{nx})^2 + (N/\Phi N_{cx}) = 0.17 < 1 \text{ OK}$$

Deflection at top under service lateral loads = 25.34 mm < 43.89 mm

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	4950 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Total Area over Pole = 9 m<sup>2</sup>

Moment Wind =	8.34 Kn-m
Shear Wind =	1.68 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	4.66 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	13.22 Kn-m	Ultimate Moment Capacity of Pile

**Checks**

Applied Forces/Capacities = 0.63 < 1 OK

**Drained Lateral Strength of End pile in cohesionless soils Free Head short pile****Assumed Soil Properties**

Gamma	18 Kn/m <sup>3</sup>	Friction angle	30 deg	Cohesion	0 Kn/m <sup>3</sup>
K0 =	$(1 - \sin(30)) / (1 + \sin(30))$				
Kp =	$(1 + \sin(30)) / (1 - \sin(30))$				

**Geometry For End Bay Pole**

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	4950 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

**Loads**

Moment Wind =	8.34 Kn-m
Shear Wind =	1.68 Kn

**Pile Properties**

Safety Factory	0.55	
Hu =	4.66 Kn	Ultimate Lateral Strength of the Pile, Short pile



Mu = 13.22 Kn-m Ultimate Moment Capacity of Pile

#### Checks

Applied Forces/Capacities =  $0.63 < 1$  OK

#### Uplift Check

Density of Concrete = 24 Kn/m<sup>3</sup>

Density of Timber Pole = 5 Kn/m<sup>3</sup>

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient) for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safety factor (0.55) x Density of Soil (18) x Height of Pile (2200) x Ks (1.5) x  $0.5 \times \tan(30)$  x  $\pi$  x Dia of Pile (0.6) x Height of Pile (2200)

Skin Friction = 39.09 Kn

Weight of Pile + Pile Skin Friction = 41.52 Kn

Uplift on one Pile = 12.55 Kn

Uplift is ok