Job No.:	TEST J	**Address:**		**Date:**	29/08/2024
Latitude:	-37.685058	**Longitude:**	176.247142	**Elevation:**	5 m

General Input

Roof Live Load	0.25 KPa	Roof Dead Load	0.25 KPa	Roof Live Point Load	1.1 Kn
Snow Zone	N0	Ground Snow Load	0 KPa	Roof Snow Load	0 KPa
Earthquake Zone	1	Subsoil Category	D	Exposure Zone	C
Importance Level	2	Ultimate wind & Earthquake ARI	500 Years	Max Height	6.6 m
Wind Region	NZ1	Terrain Category	3.0	Design Wind Speed	37.35 m/s
Wind Pressure	0.84 KPa	Lee Zone	NO	Ultimate Snow ARI	150 Years
Wind Category	High	Earthquake ARI	500		

Note: Wind lateral loads are governing over Earthquake loads, So only wind loads are considered in calculations

Pressure Coefficients and Pressues

Shed Type = Mono   Enclosed

For roof Cp,i = -0.3

For roof   CP,e   from 0 m   To 3.13 m   Cpe = -0.9167   pe = -0.69 KPa   pnet = -0.69 KPa

For roof   CP,e   from 3.13 m   To 6.25 m   Cpe = -0.8917   pe = -0.67 KPa   pnet = -0.67 KPa

For wall Windward Cp,i = -0.3   side Wall Cp,i = -0.3

For wall Windward and Leeward   CP,e   from 0 m   To 12 m   Cpe = 0.7   pe = 0.53 KPa   pnet = 0.78 KPa

For side wall   CP,e   from 0 m   To 6.25 m   Cpe =   pe = -0.49 KPa   pnet = -0.49 KPa

Maximum Upward pressure used in roof member Design = 0.69 KPa

Maximum Downward pressure used in roof member Design = 0.40 KPa

Maximum Wall pressure used in Design = 0.78 KPa

Maximum Racking pressure used in Design = 0.91 KPa

Design Summary

Purlin Design

Purlin Spacing = 800 mm   Purlin Span = 4350 mm   Try Purlin 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =0.47   S1 Downward =11.27   S1 Upward =24.64

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	0.64 Kn-m	Capacity	2.23 Kn-m	Passing Percentage	**348.44 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	1.76 Kn-m	Capacity	2.97 Kn-m	Passing Percentage	**168.75 %**
$M_{0.9D-WnUp}$	-0.88 Kn-m	Capacity	-1.76 Kn-m	Passing Percentage	**200.00 %**
$V_{1.35D}$	0.59 Kn	Capacity	9.65 Kn	Passing Percentage	**1635.59 %**

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$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	1.22 Kn	Capacity	12.86 Kn	Passing Percentage	**1054.10 %**
$V_{0.9D-WnUp}$	-0.81 Kn	Capacity	-16.08 Kn	Passing Percentage	**1985.19 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3 considering at least 4 members acting together

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 9.57 mm Limit by Woolcock et al, 1999 Span/360 = 11.94 mm

Deflection under Dead and Service Wind = 11.16 mm Limit by Woolcock et al, 1999 Span/250 = 28.67 mm

Reactions

Maximum downward =1.22 kn Maximum upward = -0.81 kn

Number of Blocking = 0 if 0 then no blocking required, if 1 then one midspan blocking required

Rafter Design Internal

Internal Rafter Load Width = 4500 mm Internal Rafter Span = 11850 mm Try Rafter 2x610x45 LVL11

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1 K1 Medium term = 0.8 K1 Long term = 0.6 K4 =1 K5 =1 K8 Downward =1.00

K8 Upward =1.00 S1 Downward =11.05 S1 Upward =11.05

Shear Capacity of timber =5 MPa Bending Capacity of timber =38 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	26.66 Kn-m	Capacity	90.18 Kn-m	Passing Percentage	**338.26 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	55.29 Kn-m	Capacity	120.24 Kn-m	Passing Percentage	**217.47 %**
$M_{0.9D-WnUp}$	-36.73 Kn-m	Capacity	-150.28 Kn-m	Passing Percentage	**409.15 %**
$V_{1.35D}$	9.00 Kn	Capacity	88.28 Kn	Passing Percentage	**980.89 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	18.66 Kn	Capacity	117.7 Kn	Passing Percentage	**630.76 %**
$V_{0.9D-WnUp}$	-12.40 Kn	Capacity	-147.14 Kn	Passing Percentage	**1186.61 %**

Deflections

Modulus of Elasticity = 9900 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 19.465 mm Limit by Woolcock et al, 1999 Span/360 = 33.33 mm

Deflection under Dead and Service Wind = 25.23 mm Limit by Woolcock et al, 1999 Span/250 = 80.00 mm

Reactions

Maximum downward =18.66 kn Maximum upward = -12.40 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J2 Joint Group for Pole = J5

Minimum Bolt edge, end and spacing for Load perpendicular to grains = 60 mm

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 12.6 fpj = 22.7 Mpa for Rafter with effective thickness = 90 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Capacity under short term loads = 29.11 Kn > -12.40 Kn

Rafter Design External

External Rafter Load Width = 2250 mm   External Rafter Span = 3831 mm   Try Rafter 300x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K1 Medium term = 0.8   K1 Long term = 0.6   K4 =1   K5 =1   K8 Downward =0.94

K8 Upward =0.94   S1 Downward =13.93   S1 Upward =13.93

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{1.35D}$	1.39 Kn-m	Capacity	4.72 Kn-m	Passing Percentage	**339.57 %**
$M_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	2.89 Kn-m	Capacity	6.30 Kn-m	Passing Percentage	**217.99 %**
$M_{0.9D-WnUp}$	-1.92 Kn-m	Capacity	-7.87 Kn-m	Passing Percentage	**409.90 %**
$V_{1.35D}$	1.45 Kn	Capacity	14.47 Kn	Passing Percentage	**997.93 %**
$V_{1.2D+1.5L\ 1.2D+Sn\ 1.2D+WnDn}$	3.02 Kn	Capacity	19.30 Kn	Passing Percentage	**639.07 %**
$V_{0.9D-WnUp}$	-2.00 Kn	Capacity	-24.12 Kn	Passing Percentage	**1206.00 %**

Deflections

Modulus of Elasticity = 5400 MPa NZS3603 Amt 4, Table 2.3

k2 for Long Term Loads = 2

Deflection under Dead and Live Load = 3.70 mm	Limit by Woolcock et al, 1999 Span/360= 11.11 mm
Deflection under Dead and Service Wind = 4.32 mm	Limit by Woolcock et al, 1999 Span/250 = 26.67 mm

Reactions

Maximum downward =3.02 kn   Maximum upward = -2.00 kn

Rafter to Pole Connection check

Bolt Size = M12 Number of Bolts = 2

Calculations as per NZS 3603:1993 Amend 2005 clause 4.4

Joint Group for Rafters =J5 Joint Group for Pole = J5

Factor of Safety = 0.7

For Perpendicular to grain loading

K11 = 14.9 fpj = 12.9 Mpa for Rafter with effective thickness = 50 mm

For Parallel to grain loading

K11 = 2.0 fcj = 36.1 Mpa for Pole with effective thickness = 100 mm

Eccentric Load check

V = phi x k1 x k4 x k5 x fs x b x ds .... (Eq 4.12) = -25.20 kn > -2.00 Kn

Single Shear Capacity under short term loads = -10.84 Kn > -2.00 Kn

Girt Design Front and Back

Girt's Spacing = 900 mm   Girt's Span = 4500 mm   Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =0.78   S1 Downward =11.27   S1 Upward =17.82

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.78 Kn-m	Capacity	2.90 Kn-m	Passing Percentage	**162.92 %**
$V_{0.9D-WnUp}$	1.58 Kn	Capacity	16.08 Kn	Passing Percentage	**1017.72 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 16.78 mm   Limit by Woolcock et al, 1999 Span/250 = 18.00 mm
Sag during installation = 24.86 mm

Reactions

Maximum = 1.58 kn

Girt Design Sides

Girt's Spacing = 900 mm   Girt's Span = 4000 mm   Try Girt 200x50 SG8 Dry

Moisture Condition = Dry (Moisture in timber is less than 16% and timber does not remain in continuous wet condition after installation)

K1 Short term = 1   K4 =1   K5 =1   K8 Downward =1.00

K8 Upward =0.50   S1 Downward =11.27   S1 Upward =23.76

Shear Capacity of timber =3 MPa   Bending Capacity of timber =14 MPa NZS3603 Amt 4, table 2.3

Capacity Checks

$M_{Wind+Snow}$	1.40 Kn-m	Capacity	1.87 Kn-m	Passing Percentage	**133.57 %**
$V_{0.9D-WnUp}$	1.40 Kn	Capacity	16.08 Kn	Passing Percentage	**1148.57 %**

Deflections

Modulus of Elasticity = 6700 MPa NZS3603 Amt 4, Table 2.3

Deflection under Snow and Service Wind = 10.48 mm   Limit by Woolcock et al. 1999 Span/100 = 16.00 mm

Sag during installation =15.52 mm

Reactions

Maximum = 1.40 kn

Middle Pole Design

Geometry

350 SED H5 HIGH DENSITY (Minimum 375 dia. at Floor Level)	Dry Use	Height	5640 mm
Area	103154 mm2	As	77365.4296875 mm2
Ix	847191750 mm4	Zx	4674161 mm3
Iy	847191750 mm4	Zx	4674161 mm3
Lateral Restraint	5640 mm c/c		

Loads

Total Area over Pole = 27 m2

Dead	6.75 Kn	Live	6.75 Kn
Wind Down	10.80 Kn	Snow	0.00 Kn
Moment wind	33.36 Kn-m		
Phi	0.8	K8	0.88
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	49.725 MPa	fs =	2.84 MPa
fc =	28.125 MPa	fp =	8.66 MPa
ft =	29.64 MPa	E =	12874 MPa

Capacities

PhiNcx Wind	2037.80 Kn	PhiMnx Wind	163.25 Kn-m	PhiVnx Wind	175.77 Kn
PhiNcx Dead	1222.68 Kn	PhiMnx Dead	97.95 Kn-m	PhiVnx Dead	105.46 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.22 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.05 < 1 OK

Deflection at top under service lateral loads = 16.00 mm < 37.60 mm

Drained Lateral Strength of Middle pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For Middle Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	2200 mm	Pile embedment length
f1 =	4950 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	33.36 Kn-m
Shear Wind =	6.74 Kn

Pile Properties

Safety Factory	0.55	
Hu =	13.27 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	38.68 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.86 < 1 OK

End Pole Design

Geometry For End Bay Pole

Geometry

225 SED H5 HIGH DENSITY (Minimum 250 dia. at Floor Level)	Dry Use	Height	6300 mm
Area	44279 mm2	As	33209.1796875 mm2
Ix	156100441 mm4	Zx	1314530 mm3
Iy	156100441 mm4	Zx	1314530 mm3
Lateral Restraint	mm c/c		

Loads

Total Area over Pole = 9 m2

Dead	2.25 Kn	Live	2.25 Kn
Wind Down	3.60 Kn	Snow	0.00 Kn
Moment Wind	8.34 Kn-m		
Phi	0.8	K8	0.41
K1 snow	0.8	K1 Dead	0.6
K1wind	1		

Material

Peeling	Steaming	Normal	Dry Use
fb =	49.725 MPa	fs =	2.84 MPa
fc =	28.125 MPa	fp =	8.66 MPa
ft =	29.64 MPa	E =	12874 MPa

Capacities

PhiNcx Wind	410.14 Kn	PhiMnx Wind	21.53 Kn-m	PhiVnx Wind	75.45 Kn

PhiNcx Dead	246.08 Kn	PhiMnx Dead	12.92 Kn-m	PhiVnx Dead	45.27 Kn

Checks

(Mx/PhiMnx)+(N/phiNcx) = 0.41 < 1 OK

(Mx/PhiMnx)^2+(N/phiNcx) = 0.17 < 1 OK

Deflection at top under service lateral loads = 25.34 mm < 43.89 mm

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	4950 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Total Area over Pole = 9 m2

Moment Wind =	8.34 Kn-m
Shear Wind =	1.68 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.66 Kn	Ultimate Lateral Strength of the Pile, Short pile
Mu =	13.22 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.63 < 1 OK

Drained Lateral Strength of End pile in cohesionless soils Free Head short pile

Assumed Soil Properties

Gamma	18 Kn/m3	Friction angle	30 deg	Cohesion	0 Kn/m3
K0 =	(1-sin(30)) / (1+sin(30))				
Kp =	(1+sin(30)) / (1-sin(30))				

Geometry For End Bay Pole

Ds =	0.6 mm	Pile Diameter
L =	1500 mm	Pile embedment length
f1 =	4950 mm	Distance at which the shear force is applied
f2 =	0 mm	Distance of top soil at rest pressure

Loads

Moment Wind =	8.34 Kn-m
Shear Wind =	1.68 Kn

Pile Properties

Safety Factory	0.55	
Hu =	4.66 Kn	Ultimate Lateral Strength of the Pile, Short pile

Mu =	13.22 Kn-m	Ultimate Moment Capacity of Pile

Checks

Applied Forces/Capacities = 0.63 < 1 OK

Uplift Check

Density of Concrete = 24 Kn/m3

Density of Timber Pole = 5 Kn/m3

Due to cast in place pile, the surface interaction between soil and pile will be rough thus angle of friction between both is taken equal to soil angle of internal friction

Ks (Lateral Earth Pressure Coefficient)for cast into place concrete piles = 1.5

Formula to calculate Skin Friction = Safecty factor (0.55) x Density of Soil(18) x Height of Pile(2200) x Ks(1.5) x 0.5 x tan(30) x Pi x Dia of Pile(0.6) x Height of Pile(2200)

Skin Friction = 39.09 Kn

Weight of Pile + Pile Skin Friction = 41.52 Kn

Uplift on one Pile = 12.55 Kn

Uplift is ok