

AI24BTECH11019-KOTHA PRATHEEK REDDY

- 1) Water emerges from an ogree spillway with velocity =  $13.72 \text{ m/s}$  and depth =  $0.3\text{m}$  at its toe. The tail water depth required to form a hydraulic jump at the toe is [Gate 2008]
 

a) $6.48 \text{ m}$	c) $3.24 \text{ m}$
b) $5.24 \text{ m}$	d) $2.24 \text{ m}$
  
- 2) The flow of water (mass density =  $1000 \text{ kg/m}^3$  and kinematic viscosity =  $10^{-6} \text{ m}^2/\text{s}$ ) in commercial pipe, having equivalent roughness  $k_s$ , as  $0.12\text{mm}$ , yields an average shear stress at the pipe boundary =  $600 \text{ N/m}^2$ . The value of  $k_s/\delta$  ( $\delta$  being the thickness of laminar sub-layer) for this pipe is [Gate 2008]
 

a) 0.25	c) 6.0
b) 0.50	d) 8.0
  
- 3) A river reach of  $2.0 \text{ km}$  long with maximum flood discharge of  $1000 \text{ m}^3/\text{s}$  is to be physically modeled in the laboratory where maximum available discharge is  $0.20 \text{ m}^3/\text{s}$ . For a geometrically similar model based on equality of Froude number, the length of the river reach ( $m$ ) in the model is [Gate 2008]
 

a) 26.4	c) 20.5
b) 25.0	d) 18.0
  
- 4) An outlet irrigates an area of  $20 \text{ ha}$ . The discharge ( $\text{l/s}$ ) required at this outlet to meet the evaporation transpiration requirement of  $20 \text{ mm}$  occurring uniformly in 20 days neglecting other field losses is [Gate 2008]
 

a) 2.52	c) 2.01
b) 2.31	d) 1.52
  
- 5) A wastewater sample contains  $10^{-5.6} \text{ mmol/l}$  of  $\text{OH}^-$  ions at  $25^\circ\text{C}$ . The  $\text{pH}$  of this sample is [Gate 2008]
 

a) 8.6	c) 5.6
b) 8.4	d) 5.4
  
- 6) Group I lists estimation methods of some of the water and wastewater quality parameters. Group II lists the indicators used in the estimation methods. Match

the estimation method (Group *I*) with the corresponding indicator (Group *II*) [Gate 2008]

Group <i>I</i>	Group <i>II</i>
P Azide modified Winkler method for dissolved oxygen	1 Erichrome Black T
Q Dichromate method for chemical oxygen demand	2 Ferrion
R EDTA titrimetric method for hardness	3 Potassium chromate
S Mohr or Argentometric method for chlorides	4 Starch

- a) P-3,Q-2,R-1,S-4                      c) P-4,Q-1,R-2,S-3  
b) P-4,Q-2,R-1,S-3                      d) P-4,Q-2,R-3,S-1

7) Determine the correctness or otherwise of the following **Assertion** [*a*] and the **Reason** [*r*]

**Assertion:** The crown of the outgoing larger diameter sewer is always matched with the crown of incoming smaller diameter sewer.

**Reason:** It eliminates backing up of sewage in the incoming smaller diameter sewer. [Gate 2008]

- a) Both [*a*] and [*r*] are true and [*r*] is not the correct reason for [*a*].  
the correct reason for [*a*].                      c) Both [*a*] and [*r*] are false  
b) Both [*a*] and [*r*] are true and [*r*] is d) [*a*] is true but [*r*] is false

8) The 5-day BOD of a wastewater sample is obtained as 190 mg/l (with  $k = 0.01 \text{ h}^{-1}$ ). The ultimate oxygen demand (mg/l) of the sample will be [Gate 2008]

- a) 3800                      c) 271  
b) 475                      d) 190

9) A water treatment plant is required to process 28800  $\text{m}^3/\text{d}$  of raw water (density = 1000  $\text{kg}/\text{m}^3$ , kinematic viscosity =  $10^{-6} \text{ m}^2/\text{s}$ ). The rapid mixing tank imparts a velocity gradient of  $900 \text{ s}^{-1}$  to blend 35 mg/l of alum with the flow for a detention time for 2 minutes. The power input (W) required for rapid mixing is [Gate 2008]

- a) 32.4                      c) 324  
b) 36                      d) 32400

10) Match the Group *I* (Terminology) with Group *II* (Definition/Brief Description) for wastewater treatment systems [Gate 2008]

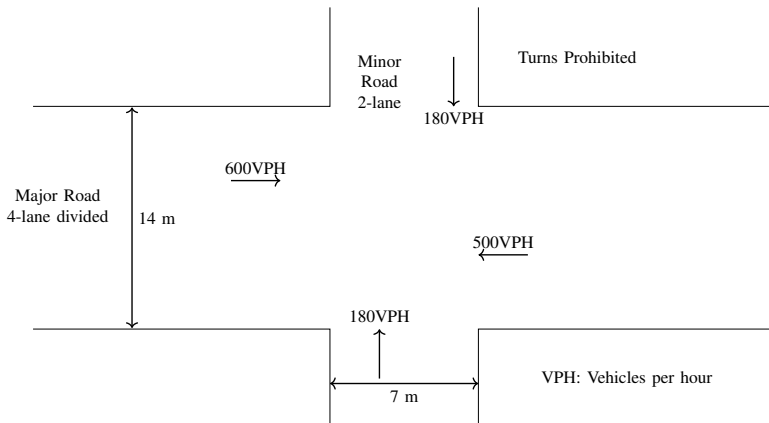
Group <i>I</i>	Group <i>II</i>
P Primary treatment	1 Contaminant removal by physical forces
Q Secondary treatment	2 Involving biological and/or chemical reaction
R Unit operation	3 Conversion of soluble organic matter to biomass
S Unit process	4 Removal of solid materials from incoming wastewater

- a) P-4,Q-3,R-1,S-2                      c) P-3,Q-4,R-2,S-1  
b) P-4,Q-3,R-2,S-1                      d) P-1,Q-2,R-3,S-4

- 11) A roundabout is provided with an average entry width of 8.4 m, width of weaving section as 14 m, and length of the weaving section between channelizing islands as 35 m. The crossing traffic and the total traffinc on the weaving section are 1000 and 2000 PCU per hour respectively. The nearest rounded capacity of the roundabout(in PCU per hour) is [Gate 2008]

- a) 3300                      c) 4500  
b) 3700                      d) 5200

- 12) Design parameters for a signalized intersection are shown in the figure below. The green time calculated for major and minor roads are 34 and 18 s, respectively



The critical lane volume on the major road changes to 440 vehicles per hour per lane and the critical lane volume on the minor road remains unchanged. The green time will [Gate 2008]

- a) increase for the major road and remain same for the minor road                      crease for the minor road  
b) increase for the major road and decrease for the minor road                      c) decrease for both the roads  
d) remain unchanged for both the roads
- 13) It is proposed to widen and strengthen an existing 2-lane NH section as a divided highway. The existing traffic in one direction is 2500 commercial vehicles(CV) per day. The construction will take 1 year. The design CBR of soil subgrade is found to be 5 percent. Given: traffic growth rate for CV = 8 percent, vehicle damage factor = 3.5 (standard axels per CV), design life = 10 years and traffic distribution factors = 0.75. The cumulative standard axels (msa) computed are [Gate 2008]

