



Revealing aeroelastic effects on low-rise roof structures in turbulent winds via isogeometric fluid–structure interaction

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Abstract

Aeroelastic effects, which affect the dynamic responses of low-rise roof structures to extreme wind conditions, are often neglected or oversimplified in current wind engineering design standards and applications. However, it is crucial to understand the details of those aeroelastic effects for performance-based wind engineering. This paper presents an isogeometric fluid–structure interaction (FSI) tool to investigate the aeroelastic effects of wind pressure distributions on roof structures under different turbulent wind conditions. A representative low-rise roof structure is simulated with an FSI model using an Arbitrary Lagrange-Eulerian-based variational multi-scale formulation coupled with isogeometric Kirchhoff-Love shells. The simulation results are compared to the quasi-steady approach and wind load provisions from ASCE 7-22. It shows that the quasi-steady approach and the design standard underestimate the pressure fluctuations, indicating the necessity of using FSI simulations to capture the aeroelastic effect for the roof of low-rise structures. This paper also studies the impacts of different roof configurations, e.g., the number of roof panels and inflow turbulent intensity, on the distribution of pressure coefficients and roof deflections. For the given mean wind speed, the mean pressure coefficient remains almost the same regardless of the turbulent intensity and roof configuration. However, the pressure fluctuation (standard deviation) varies significantly with the turbulence intensity and roof configuration. The aeroelastic effect also leads to complicated roof deflections at the crucial location having the maximum pressure coefficient. The paper first describes the mathematical details of the FSI model and simulation setup. Then, the pressure coefficients by the FSI simulation and design code are compared. Finally, the roof deflection with different inlet turbulence intensities and roof configurations are presented and discussed.

Keywords FSI · Civil structures · IGA

1 Introduction

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Widely adopted in coastal areas of the USA, low-rise structures represent one of the most vulnerable structures under extreme wind events and are the largest source of direct and indirect damage and fatality. Nowadays, the wind engineering community is moving towards performance-based design to increase urban resilience [1]. Design standards, such as ASCE 7-22 [2], provide prescriptive guidelines for calculating the pressure on the roof of low-rise structures. However, more accurate predictions on the uplifting pressure are required for performance-based wind engineering. Research into low-rise structures and their interaction with turbulent winds is vital for predicting, responding to, and mitigating threats of extreme wind events to our community. Due to the systematic, repeatable, controllable boundary conditions of the low-rise structures, numerical models based on

computational fluid dynamics (CFD) are frequently used to estimate wind pressure loads for more effective designs.

Many wind tunnel experiments have indicated the importance of structure's aeroelastic behaviors [3] on wind pressure distributions. A recent study in Florida International University's Wall of Wind experimental facility [4] found that the membrane deformation of flexible roofs can result in a significant reduction of peak pressure coefficients as compared to those of rigid roofs. However, these aeroelastic effects are neither thoroughly investigated nor accounted for in most of the current wind engineering models to determine the wind-induced pressure distributions. These CFD models, including Reynolds-averaged Navier-Stokes (RANS) and large-eddy simulations (LES), often neglect the aeroelastic effects or employ oversimplified approaches. High-fidelity fluid-structure interaction (FSI) simulations of low-rise roof structures are still limited due to the large Reynolds number of wind flows and complicated interaction between wind and structures.

This paper presents a general high-fidelity FSI model and deploys it to representative low-rise roof structures to investigate the aeroelastic effects on wind pressure distribution and roof deflections. For fluid mechanics, the FSI model uses a residual-based variational multi-scale formulation (VMS) to capture high Reynolds number wind flows. RBVMS, as a variationally-consistent LES turbulence model, aims to capture the spatiotemporal details of large Reynolds number wind turbulence [5]. A boundary-fitted moving domain CFD technique based on arbitrary Lagrangian–Eulerian (ALE) is utilized to handle the deformation of the fluid–structure interface. For structural mechanics, the FSI model utilizes isogeometric analysis (IGA)-based Kirchhoff-Love shells to discretize the roof structure. IGA aims to integrate design and analysis by directly employing the spline functions of computer-aided design (CAD) models to perform engineering analysis [6, 7]. Besides, due to the higher-order basis functions, IGA attains exact geometry descriptions of structures and has been proven to have higher accuracy per degree of freedom than traditional finite element methods using Lagrangian polynomial basis functions. The integration of ALE, VMS, and IGA (isogeometric ALE-VMS) leads to a high-fidelity predictive FSI model suitable for wind engineering applications considered in this paper. The isogeometric ALE-VMS FSI model has been widely validated by a set of benchmark problems and realistic problems in wind energy [8–11], which share several similarities with the wind engineering problem considered here from the perspective of aerodynamics. A set of parametric studies is carried out to demonstrate the aeroelastic effects on roofs with different stiffness under various inflow turbulence conditions.

The paper is structured as follows. Section 2 introduces the high-fidelity FSI model, including governing equations of fluid and structural dynamics and the isogeometric

ALE-VMS methodology. The details of the setup of FSI simulations of roof structures are shown in Sect. 3. Then, the FSI simulation results are presented and compared to the pressure coefficients suggested by ASCE 7-22 in Sect. 4. Lastly, Sect. 5 draws the conclusions.

2 FSI formulation

2.1 Governing equations

Let Ω_f , Ω_s , and Γ_I denote the fluid domain, structural domain, and fluid–structure interface, respectively. The strong form governing equations of the FSI model are given as

$$\rho_f \left[\frac{\partial \mathbf{u}}{\partial t} + (\mathbf{u} - \hat{\mathbf{u}}) \cdot \nabla \mathbf{u} - \mathbf{f}_f \right] - \nabla \cdot \boldsymbol{\sigma}_f = \mathbf{0} \quad \text{in } \Omega_f \quad (1)$$

$$\nabla \cdot \mathbf{u} = 0 \quad \text{in } \Omega_f \quad (2)$$

$$\rho_s \left(\frac{d^2 \mathbf{y}}{dt^2} - \mathbf{f}_s \right) - \nabla \cdot \boldsymbol{\sigma}_s = \mathbf{0} \quad \text{in } \Omega_s \quad (3)$$

$$\mathbf{u} = \frac{d\mathbf{y}}{dt} \quad \text{in } \Gamma_I \quad (4)$$

$$\boldsymbol{\sigma}_f \mathbf{n}_f + \boldsymbol{\sigma}_s \mathbf{n}_s = \mathbf{0} \quad \text{in } \Gamma_I \quad (5)$$

where Eq. 1 and Eq. 2 are the Navier-Stokes equations of incompressible flows, in which ρ_f is the fluid density, \mathbf{u} is the fluid velocity, \mathbf{f}_f is the fluid body force per unit mass, $\boldsymbol{\sigma}_f = -p\mathbf{I} + 2\mu\nabla^s\mathbf{u}$ is the fluid Cauchy stress tensor, where p is the fluid pressure, μ is the dynamics viscosity, ∇^s is the symmetric gradient operator, and \mathbf{I} is a 3×3 identity matrix. Equation 3 is the conversation law of linear momentum of structural mechanics, in which ρ_s is the structural density, \mathbf{y} is the structural displacement, \mathbf{f}_s is the structural body force per unit mass, and $\boldsymbol{\sigma}_s$ is the structural Cauchy stress tensor. At last, Eq. 4 and Eq. 5 are the kinematics and traction compatibility on the fluid–structure interface, in which \mathbf{n}_f and \mathbf{n}_s represent the unit normal vectors of on the fluid–structure interface ($\mathbf{n}_f = -\mathbf{n}_s$). One should note that the structural equation is imposed on a Lagrangian description, while the fluid equations are imposed on a moving domain through an ALE technique with $\hat{\mathbf{u}}$ representing the mesh velocity.

2.2 Isogeometric thin shell theory

Since the roof structure considered in this paper is essentially a thin shell, the Kirchhoff-Love shell kinematics theory is introduced into the weak formulation (or Galerkin formulation) of the structural mechanics equation (Eq. 3) to save computational cost. Furthermore, the structure is assumed to obey the St. Venant-Kirchhoff material law. As a result, a thin shell formulation, written purely in terms of the roof's

mid-surface displacements, is obtained. The shell formulation is also referred to as “rotation-free” [12] compared with Reissner-Mindlin shell theory. The shell formulation is discretized by isogeometric analysis (IGA) using second-order Non-Uniform Rational B-Splines (NURBS) [6]. The weak formulation of the rotation-free isogeometric shell is stated as: Find the shell mid-surface displacement $\mathbf{y} \in \mathcal{V}_s$ such that for $\forall \mathbf{w}_s \in \mathcal{W}_s$

$$B^s(\mathbf{w}_s, \mathbf{y}) - F^s(\mathbf{w}_s) = 0 \quad (6)$$

where

$$B^s(\mathbf{w}_s, \mathbf{y}) = \int_{A_s} \mathbf{w}_s \cdot \bar{\rho}_s h \frac{\partial^2 \mathbf{y}}{\partial t^2} d\Omega \quad (7)$$

$$+ \int_{A_s} \delta \boldsymbol{\epsilon}(\mathbf{w}_s) [\mathbf{K}_{mem} \boldsymbol{\epsilon}(\mathbf{y}) + \mathbf{K}_{coup} \boldsymbol{\kappa}(\mathbf{y})] dA \quad (8)$$

$$+ \int_{A_s} \delta \boldsymbol{\kappa}(\mathbf{w}_s) [\mathbf{K}_{coup} \boldsymbol{\epsilon}(\mathbf{y}) + \mathbf{K}_{bend} \boldsymbol{\kappa}(\mathbf{y})] dA \quad (9)$$

and

$$F^s(\mathbf{w}_s) = \int_{A_s} \mathbf{w}_s \cdot \bar{\rho}_s h \mathbf{f}_s d\Omega + \int_{\Gamma_s^N} \mathbf{w}_s \cdot \mathbf{h}_s d\Gamma \quad (10)$$

In the above formulation, \mathcal{W}_s and \mathcal{V}_s denote the test and trial function sets for the structural mechanics equations. h is the thickness of the roof, A_s denotes the middle surface, $\boldsymbol{\epsilon}(\mathbf{y})$ and $\boldsymbol{\kappa}(\mathbf{y})$ are the tensors of the roof membrane strains and curvature changes expressed in Voigt notation and written with respect to the local basis oriented on the first covariant basis vector of the roof mid-surface. $\delta \boldsymbol{\epsilon}(\mathbf{w}_s)$ and $\delta \boldsymbol{\kappa}(\mathbf{w}_s)$ are the corresponding variations. \mathbf{h}_s is the applied traction on the roof surface. \mathbf{K}_{mem} , \mathbf{K}_{coup} , and \mathbf{K}_{bend} are the membrane, coupling, and bending stiffness, respectively, which are defined as

$$\mathbf{K}_{mem} = \int_{-h/2}^{h/2} \mathbf{C} dz \quad (11)$$

$$\mathbf{K}_{coup} = \int_{-h/2}^{h/2} \mathbf{C} z dz \quad (12)$$

$$\mathbf{K}_{bend} = \int_{-h/2}^{h/2} \mathbf{C} z^2 dz \quad (13)$$

where \mathbf{C} is defined as

$$\mathbf{C} = \begin{bmatrix} \frac{E}{1-\nu^2} & \frac{\nu E}{1-\nu^2} & 0 \\ \frac{\nu E}{1-\nu^2} & \frac{E}{1-\nu^2} & 0 \\ 0 & 0 & G \end{bmatrix} \quad (14)$$

where E , G , and ν are Young’s modulus, shear modulus, and Poisson’s ratio of the structural material, respectively.

2.3 ALE-VMS for fluid mechanics

This paper adopts an ALE-VMS approach to solve fluid mechanics equations. The weak formulation is stated as: Find $\{\mathbf{u}, p\} \in \mathcal{V}_f$ such that $\forall \{\mathbf{w}_f, q\} \in \mathcal{W}_f$

$$B^f(\{\mathbf{w}_f, q\}, \{\mathbf{u}, p\}) - F^f(\{\mathbf{w}_f, q\}) = 0 \quad (15)$$

where $B^f(\{\mathbf{w}_f, q\}, \{\mathbf{u}, p\})$ and $F^f(\{\mathbf{w}_f, q\})$ are defined as

$$\begin{aligned} B^f(\{\mathbf{w}_f, q\}, \{\mathbf{u}, p\}) &= \int_{\Omega_f} \mathbf{w}_f \cdot \rho_f \left[\frac{\partial \mathbf{u}}{\partial t} + (\mathbf{u} - \hat{\mathbf{u}}) \cdot \nabla \mathbf{u} \right] d\Omega_f \\ &+ \int_{\Omega_f} q \nabla \cdot \mathbf{u} d\Omega_f \\ &+ \int_{\Omega_f} \nabla \mathbf{w}_f^S : \boldsymbol{\sigma}_f(\mathbf{u}, p) d\Omega_f \\ &- \sum_{e=1}^{N_{el}} \int_{\Omega_f^e} \left[(\mathbf{u} - \hat{\mathbf{u}}) \cdot \nabla \mathbf{w}_f + \frac{\nabla q}{\rho_f} \right] \cdot \mathbf{u}' d\Omega_f \\ &- \sum_{e=1}^{N_{el}} \int_{\Omega_f^e} p' \nabla \cdot \mathbf{w}_f d\Omega_f \\ &+ \sum_{e=1}^{N_{el}} \int_{\Omega_f^e} \mathbf{w}_f \cdot (\mathbf{u}' \cdot \nabla \mathbf{u}) d\Omega_f \\ &- \sum_{e=1}^{N_{el}} \int_{\Omega_f^e} \frac{\nabla \mathbf{w}_f}{\rho_f} : (\mathbf{u}' \otimes \mathbf{u}') d\Omega_f \end{aligned} \quad (16)$$

and

$$F^f(\{\mathbf{w}_f, q\}) = \int_{\Omega_f} \mathbf{w}_f \cdot \rho_f \mathbf{f}_f d\Omega_f + \int_{\Gamma_f^N} \mathbf{w}_f \cdot \mathbf{h}_f d\Gamma \quad (17)$$

In the above formulation, \mathcal{W}_f and \mathcal{V}_f denote the test and trial function sets for the fluid mechanics equations. \mathbf{h}_f is the fluid traction applied on Γ_f^N ($\Gamma_f^N \subset \partial \Omega_f$). All the integrals of B^f computed in an element-wise fashion correspond to the fine-scale terms stemming from the residual-based variational multi-scale formulation (VMS). In these terms, e is the element count and N_{el} is the total number of elements. The fine-scale velocity \mathbf{u}' and fine-scale pressure p' are defined as

$$\mathbf{u}' = -\tau_M \left\{ \rho_f \left[\frac{\partial \mathbf{u}}{\partial t} + (\mathbf{u} - \hat{\mathbf{u}}) \cdot \nabla \mathbf{u} - \mathbf{f}_f \right] - \nabla \cdot \boldsymbol{\sigma}_f(\mathbf{u}, p) \right\} \quad (18)$$

$$p' = -\rho_f \tau_C \nabla \cdot \mathbf{u} \quad (19)$$

where τ_M and τ_C are corresponding stabilization parameters based on streamline upwind Petrov-Galerkin (SUPG), pressure-stabilizing Petrov-Galerkin (PSPG), and Least Square on Incompressibility constraint (LSIC). More details about their definitions can be found in [5, 13–15].

2.4 FSI formulation

The above finite element and IGA formulations for fluid and structural mechanics feature a non-matching FSI approach, similar to those applied to wind turbine simulations [16]. The final FSI formulation can be stated as: Find $\{\mathbf{u}, p\} \in \mathcal{V}_f$ and $\mathbf{y} \in \mathcal{V}_s$ such that $\forall \{\mathbf{w}_f, q\} \in \mathcal{W}_f$ and $\mathbf{w}_s \in \mathcal{W}_s$

$$\begin{aligned} & B^f(\{\mathbf{w}_f, q\}, \{\mathbf{u}, p\}) - F^f(\{\mathbf{w}_f, q\}) + B^s(\mathbf{w}_s, \mathbf{y}) - F^s(\mathbf{w}_s) \\ & - \int_{\Gamma_I} (\mathbf{w}_f - \mathbf{w}_s) \cdot \boldsymbol{\sigma}_f(\mathbf{u}, p) \mathbf{n}_f d\Gamma \\ & - \int_{\Gamma_I} [\delta_{\{\mathbf{u}, p\}} \boldsymbol{\sigma}_f \mathbf{n}_f(\{\mathbf{w}_f, q\})] \cdot \left(\mathbf{u}_f - \frac{d\mathbf{y}}{dt} \right) d\Gamma \\ & + \int_{\Gamma_I} (\mathbf{w}_f - \mathbf{w}_s) \cdot \beta \left(\mathbf{u}_f - \frac{d\mathbf{y}}{dt} \right) d\Gamma = 0 \end{aligned} \quad (20)$$

The above formulation belongs to a family of an augmented Lagrangian approach for coupled problems parameterized by β .

2.5 Coupling strategy and other numerical details

The above FSI formulation is solved in a block-iterative approach, with equations as given by or similar to those in Section 5.1 in [17] and Section 5.1 in [18]. Besides, a weakly-enforced no-slip boundary condition [19, 20] is applied on the fluid–structure interface to relax the boundary-layer resolution without losing the accuracy of aerodynamic load prediction. The generalized- α method is used for the time integration. The Newton-Raphson method is used to linearize the nonlinear nodal equations. The resulting linear systems are solved by a generalized minimal residual method (GMRES) solver with a block-diagonal preconditioning. All the simulations presented next are carried out in parallel using Message Passing Interface (MPI) on the Stampede2 at the Texas Advanced Computing Center (TACC).

The FSI model presented in the paper comprises of a set of advanced numerical techniques for fluid and structural dynamics and their coupling. The IGA-based Kirchhoff-Love shell model plays a significant role in modeling thin-wall structures [21–25]. We should point out that IGA also shows great advantages in flow simulations [26–31]. VMS and its combinations with ALE (ALE-VMS) [32–38] and Space-Time (ST-VMS) technique [39–45], have success-

fully been employed as large eddy simulation (LES) models in simulating of a wide range of challenging CFD and FSI problems. These methods show significant advantages when being deployed to flow problems with moving interfaces and boundaries. Several recent validations and applications include environmental flows [20, 46–48], wind energy [8, 9, 11, 35, 49–65], tidal energy [63, 66–70], cavitation flows [71, 72], supersonic flows [73], bio-mechanics [74–80], gas turbine [81–83], and transportation engineering [84–89].

3 Computational FSI setup

This section introduces the computational FSI setup of the parametric study. The details of inflow turbulence generation are presented first, followed by the roof structure configurations and material definitions.

3.1 Inflow turbulence generation

Inflow turbulence is an important factor in wind engineering simulations. Despite its wide usage in many CFD/FSI simulations, uniform inflows are hardly seen in reality. In ASCE 7-22, the turbulence intensity is defined as a function of the equivalent height of the structure, \bar{z} , and an exposure type-based turbulence intensity factor, c , as:

$$I_{\bar{z}} = c \left(\frac{10}{\bar{z}} \right)^{1/6} \quad (21)$$

However, the effects caused by turbulence with this approach may be not sufficiently accurate for performance-based wind engineering designs. Thus, two numerical approaches, recycling/rescaling methods [90–93] and synthetic methods [94–96], are often adopted.

The recycling/rescaling methods need to perform auxiliary LES simulations, while the synthetic methods do not need and thus are computationally efficient. Because of this advantage, several synthetic methods, including synthetic random Fourier method [97], synthetic digital filtering method [98], and synthetic-eddy-method [96, 99, 100], have been proposed to generate artificial turbulent inflow conditions for wind engineering applications. Leveraging the authors’ previous inflow model developed in [70], this paper employs a synthetic-eddy-method (SEM) based on a Lagrangian treatment of vortices to generate turbulent inflow conditions for the FSI simulations of roof structures in this paper. The inflow is assumed to consist of randomly distributed turbulent spots, and the spots are assumed to be convected downstream with provided velocity profile using Taylor’s frozen turbulence hypothesis. The SEM approach is based on a superposition of different moving vortices. The velocity fluctuation at a point x that is influenced by N vor-

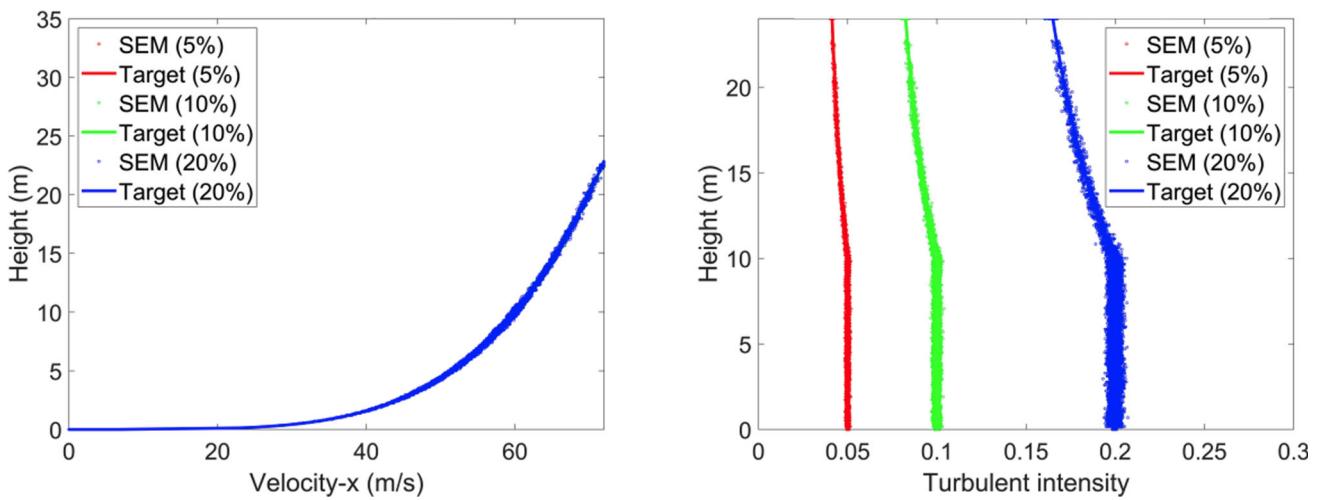


Fig. 1 Inflow wind profiles. *Left:* Mean flow. *Right:* Turbulent intensity

tices (or eddies) can be calculated by

$$\mathbf{u}'_i(\mathbf{x}) = \frac{1}{\sqrt{N}} \sum_{k=1}^N a_{ij} \sigma_j^k f_\sigma^k \left(\frac{\mathbf{x} - \mathbf{x}^k}{s^k} \right) \quad (22)$$

where \mathbf{x}^k is the location of the k^{th} eddy, s^k is the length scale of the eddy obtained from an empirical formula, and $f_\sigma^k(\mathbf{x})$ is a so-called shape function. σ_j^k contains randomly assigned eddy intensities, which obey a Gaussian distribution with zero mean value and a standard deviation of 1. a_{ij} are the Lund coefficients related to the Reynolds stresses $R_{ij} = \langle \mathbf{u}'_i \mathbf{u}'_j \rangle$, defined as

$$\mathbf{a} = \begin{bmatrix} \sqrt{R_{11}} & 0 & 0 \\ \frac{R_{21}}{a_{11}} & \sqrt{R_{22} - a_{21}^2} & 0 \\ \frac{R_{31}}{a_{11}} & \frac{R_{32} - a_{22} \cdot a_{31}}{a_{22}} & \sqrt{R_{33} - a_{31}^2 - a_{32}^2} \end{bmatrix} \quad (23)$$

In this paper, the mean profile and turbulent intensity of the wind profile are functions of height and are given as

$$u = u_r \left(\frac{z}{z_r} \right)^\alpha \quad (24)$$

$$I_u = \begin{cases} I_r & (z < z_r) \\ I_r \left(\frac{z}{z_r} \right)^{-\alpha} & (z \geq z_r) \end{cases} \quad (25)$$

where z is the height coordinate, z_r is a reference height, u_r and I_r are the mean wind velocity and turbulent intensity at z_r , and α is an empirical coefficient depending on the ground roughness and atmosphere stability. In this paper, u_r , z_r , and α are fixed to 60 m/s, 10 m, and 0.22, respectively. Three different turbulent intensities $I_r = 5\%$, 10%, and 20% are generated by the above SEM approach. The generated mean

and turbulent intensity profiles of the inflow wind conditions at several inlet locations are shown in Fig. 1.

3.2 Structural model

Figure 2 shows the geometry and dimensions of the roof structures considered in this paper. The roofs make use of homogeneous material, whose properties are summarized in Table 1. The roof structures are divided into multiple small panels to represent different roof-supporting systems. Shear connections (i.e., pinned boundary conditions) are assumed between the roof panels and the supporting truss structure. Due to the large stiffness of the supporting truss, it is assumed that there is no displacement on the boundary of each panel in the translational directions. Three different roof configurations of 4×4 , 4×6 , and 4×8 panels are considered in this paper. The panel stiffness in each configuration increases with the number of panels, hence increasing the overall roof stiffness. Due to the pinned boundary condition and high stiffness of the supporting truss, the displacement of a panel is not coupled with adjacent ones. The roof structures are discretized by second-order NURBS elements with 4,761 for the 4×4 case and 5,037 elements for the 4×6 and 4×8 cases. The fluid domain in the FSI simulations is a box with dimensions 55 m \times 40 m \times 24 m, and a refinement box with dimensions 45 m \times 30 m \times 10 m around the building is employed to better capture the bluff aerodynamics. The computational domain is discretized by tetrahedral elements in the interior and triangular prism elements in boundary layers around the structure surface and ground. We mesh the structure with a uniform grid with a length of 0.02 m. The height of the first boundary layer prism mesh is 0.002 m. The total number of nodes and elements of the mesh is 3,657,285 and 17,036,174, respectively. Figure 3 shows the fluid mesh on the x - z plane, y - z plane, and prism boundary layer near

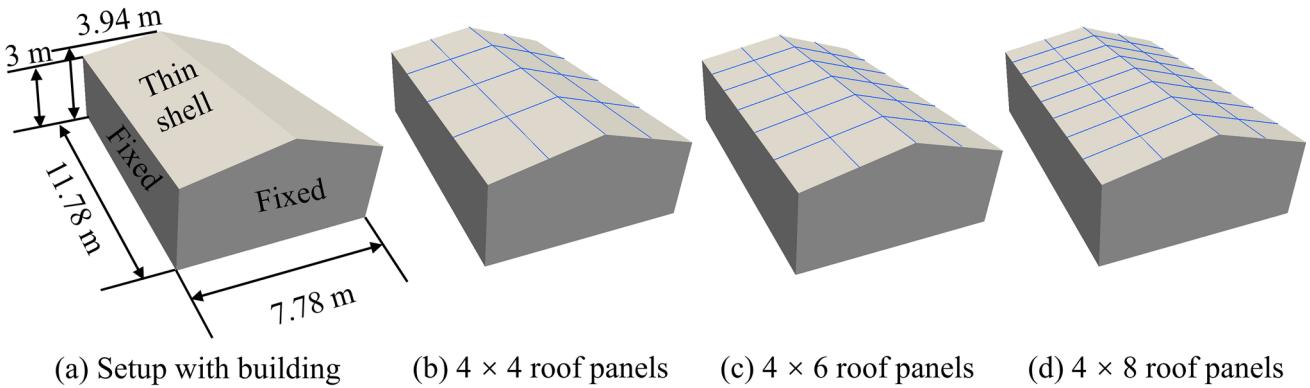


Fig. 2 Geometry and dimensions of the roof

Table 1 Material properties of the roof

Name	Notation (units)	Value
Structure density	ρ_s ($\text{kg} \cdot \text{m}^{-3}$)	600.0
Young's modulus	E ($\text{kg} \cdot \text{m}^{-2}$)	3.0×10^9
Poisson's ratio	ν	0.08
Thickness of shell	h (m)	1.111×10^{-2}

the fluid–structure interface. The following boundary conditions (BCs) are employed in the simulations (also see Fig. 4). Dirichlet flow BC based on the wind profile specified in the previous section is used for the inlet, traction-free is used for the outflow, no penetration BC is used for the side and top walls, and no-slip BC is enforced weakly for the fluid–structure interface and the ground. Each roof is simulated with three inflow conditions with turbulent intensity $I_r = 5\%$, 10% , and 20% described above.

Linear eigenvalue analyses are carried out for panels in each roof configuration. Because all the panels are uncoupled in a configuration, the overall roof frequencies are the same as the panel frequencies. The natural frequency of different roof structures changes from 4.28 Hz to 8.20 Hz . Table 2 shows the first 9 mode frequencies of the panels/roofs, while Figs. 5–7 exhibit the mode shapes of a signal panel in each configura-

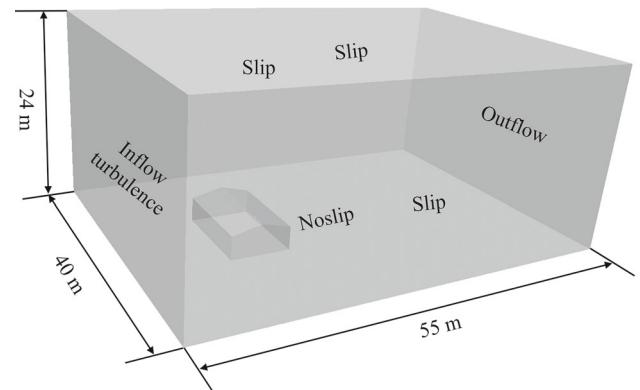


Fig. 4 FSI setup and boundary conditions

tion. The low order mode shape, e.g., the mode shape of the first mode, is similar for panels in different configurations, but different mode shapes are observed for the high order modes.

4 Results and discussions

The results and discussions are focused on the wind flow field and the aeroelastic effects on pressure coefficients under the

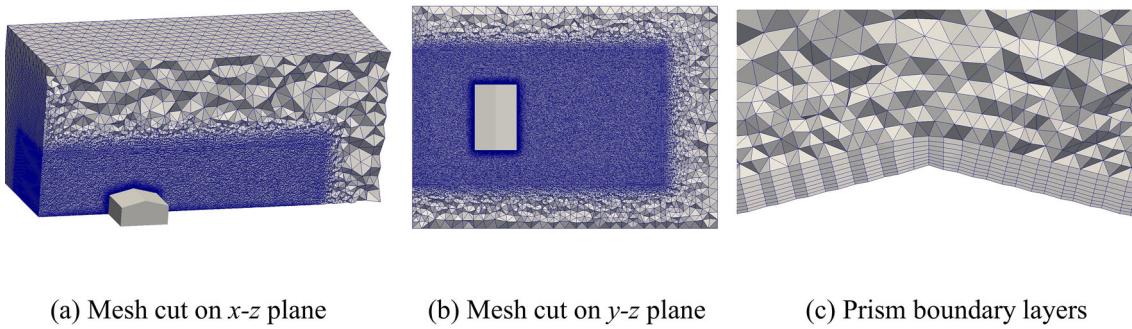


Fig. 3 Fluid mesh of the FSI simulations

Table 2 Mode frequency of roof panels (Hz)

Mode	Configuration		
	4×4	4×6	4×8
1_{st}	4.28	5.91	8.20
2_{nd}	8.21	14.79	17.17
3_{rd}	13.27	14.89	24.14
4_{th}	14.81	23.73	32.28
5_{th}	17.17	29.93	33.02
6_{th}	23.74	30.01	47.99
7_{th}	24.22	38.76	52.05
8_{th}	28.40	38.78	53.79
9_{th}	32.28	51.55	60.70

inflow turbulence intensities and panel configurations specified in the previous sections. The pressure coefficient in this study is defined as

$$C_p = (p - p_\infty)/(\rho U_\infty^2/2) \quad (26)$$

where p_∞ and U_∞ are the characteristic wind pressure and wind speed at the inlet. \overline{C}_p and C'_p denote the time-averaged value of C_p and its standard deviation.

4.1 Wind flow field

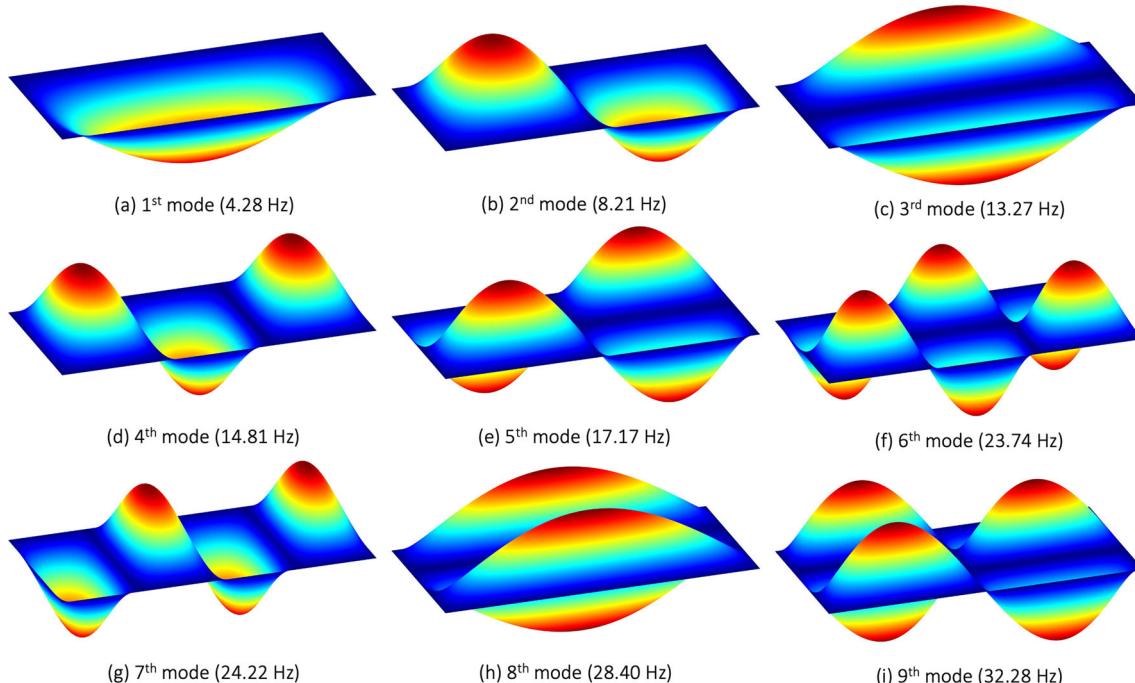
Figure 8 shows the time-averaged streamlines and mean velocity magnitude on the plane that 1.0 m above the ground ($z = 1.0$ m plane) for all analysis cases. Four flow re-

circulation regions are observed around the corners of the building envelope due to the flow separation, while additional irregular flow re-circulations are formed behind the building's leeward wall. By comparing the plots in Fig. 8, the turbulence intensity and roof configurations have no significant influence on the size of re-circulation bubbles. However, the variations in turbulence intensity and roof configuration have large impacts on the roof pressure coefficient, which is discussed in the following section.

4.2 Aeroelastic effects

Instantaneous flow fields and roof deflections predicted by the FSI simulations at the fully developed stage are plotted in Fig. 9. As the turbulence intensity increases at the inlet, larger turbulence around the building envelope and in the wake is seen. The turbulence leads to complicated wind-roof interaction and noticeable structural responses. For all three analyzed configurations, the instantaneous displacement magnitude grows with the magnitude of turbulence intensity.

While the time-averaged pressure coefficient, \overline{C}_p , provides a baseline for determining the wind load on the roof structure, the pressure fluctuation due to the aeroelastic effects should also be considered in the performance-based-wind engineering. The standard deviation of the pressure coefficient, C'_p , reflects the pressure fluctuation over time.

**Fig. 5** Mode shapes for a panel from the 4×4 roof configuration

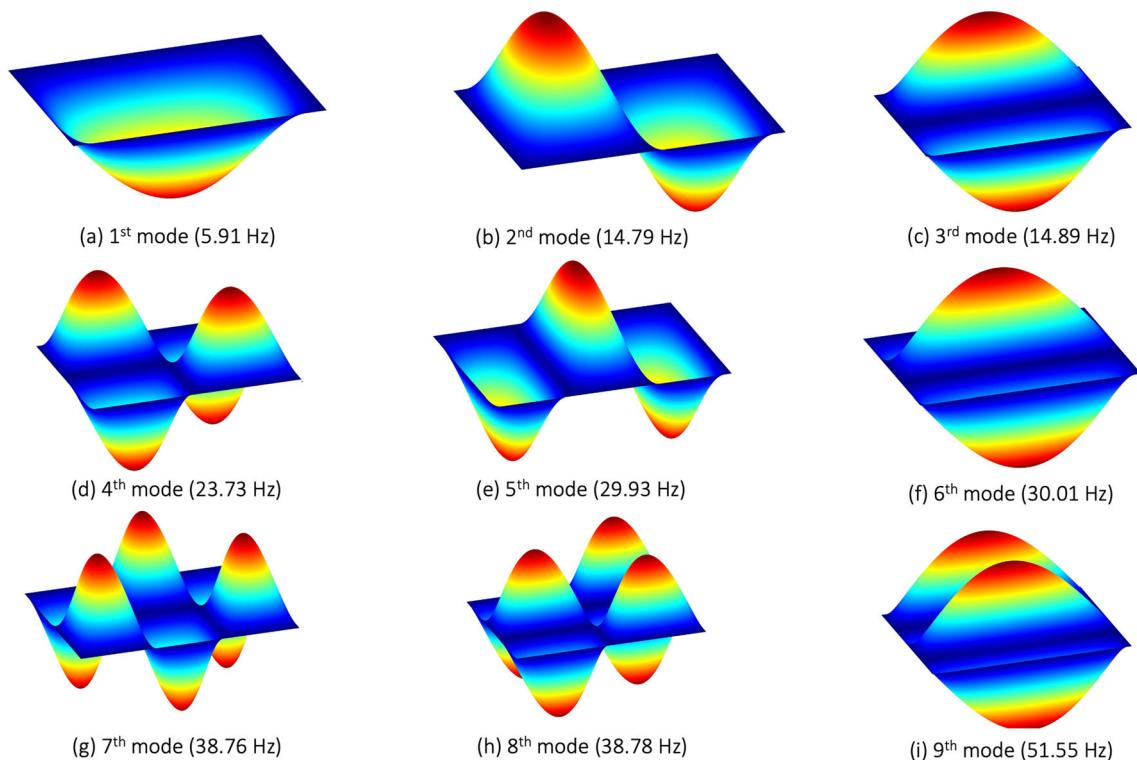


Fig. 6 Mode shapes for a panel from the 4×6 roof configuration

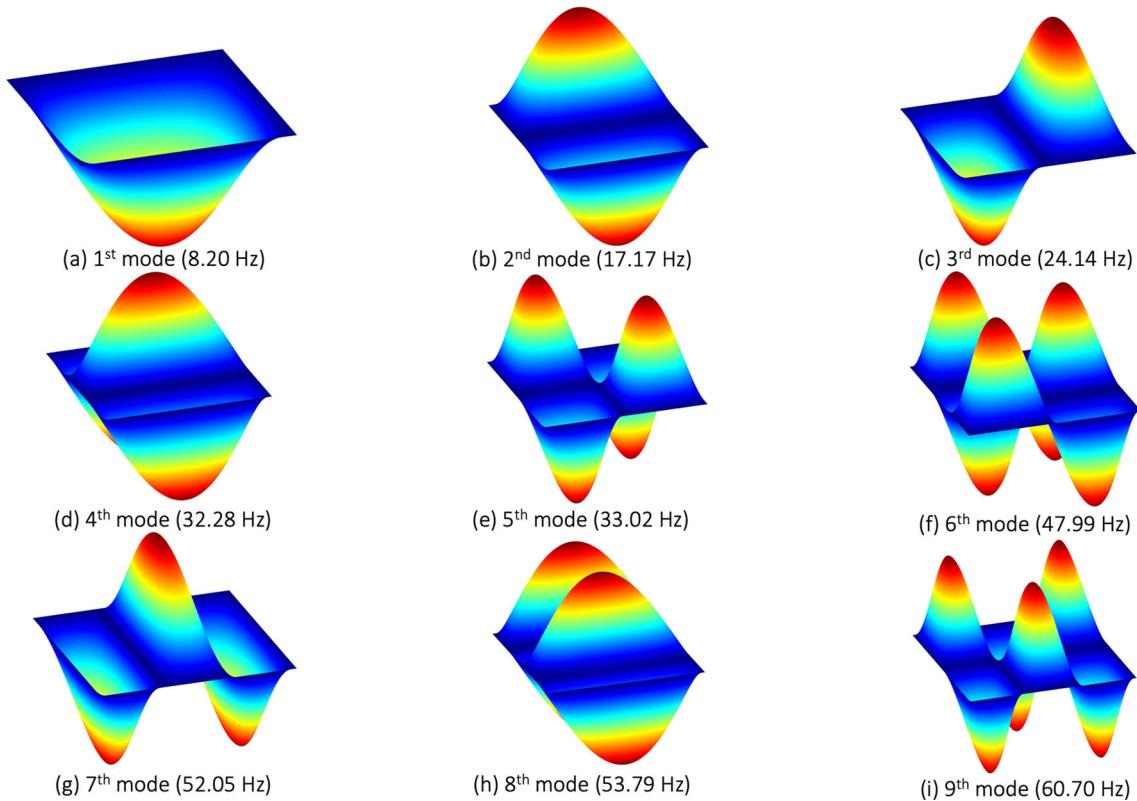


Fig. 7 Mode shapes for a panel from the 4×8 roof configuration

Fig. 8 Time-averaged streamlines and wind speed on the $z = 1.0\text{ m}$ plane (unit: m/s) $t = 7\text{ s}$

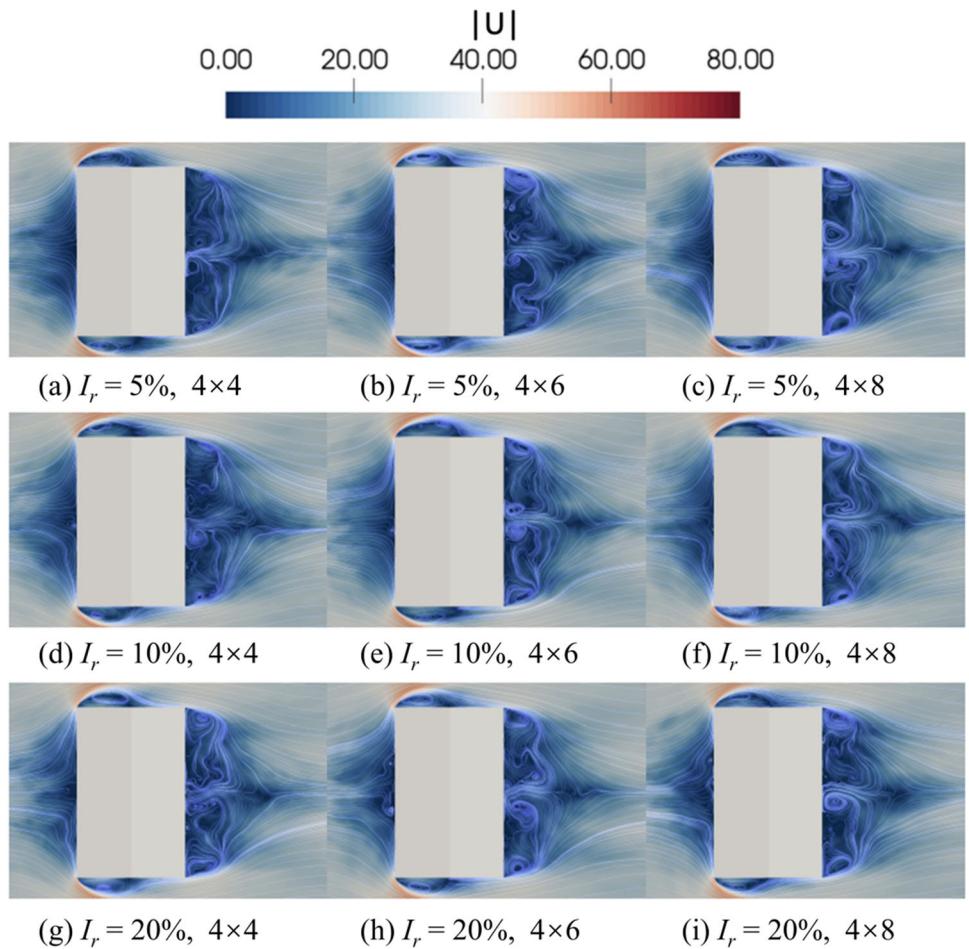


Fig. 9 Instantaneous wind speed (unit: m/s) and roof deformation magnitude (unit: m) at $t = 7\text{ s}$

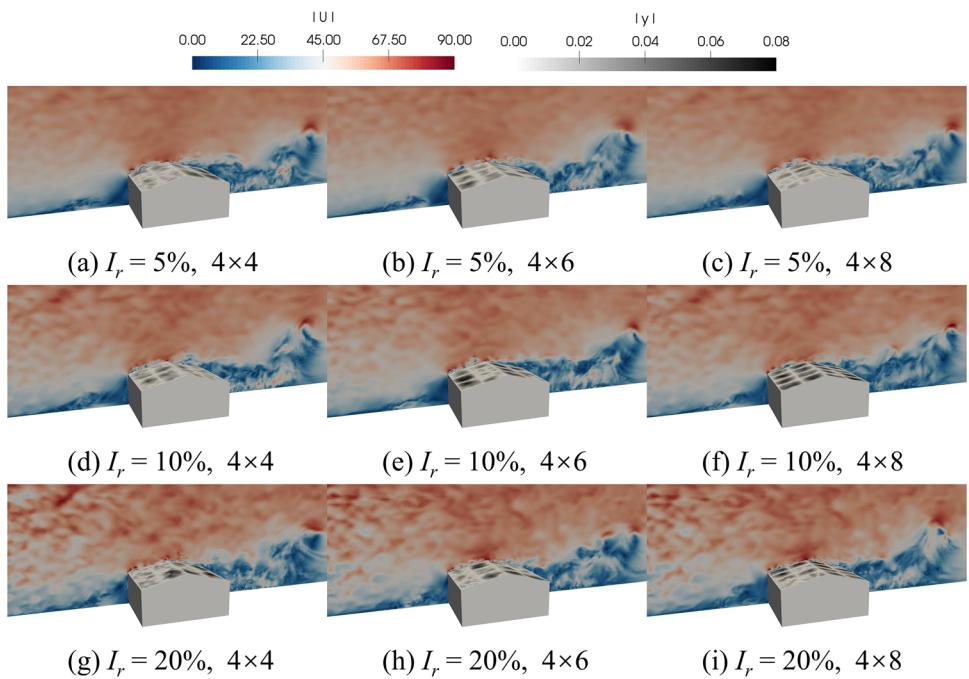


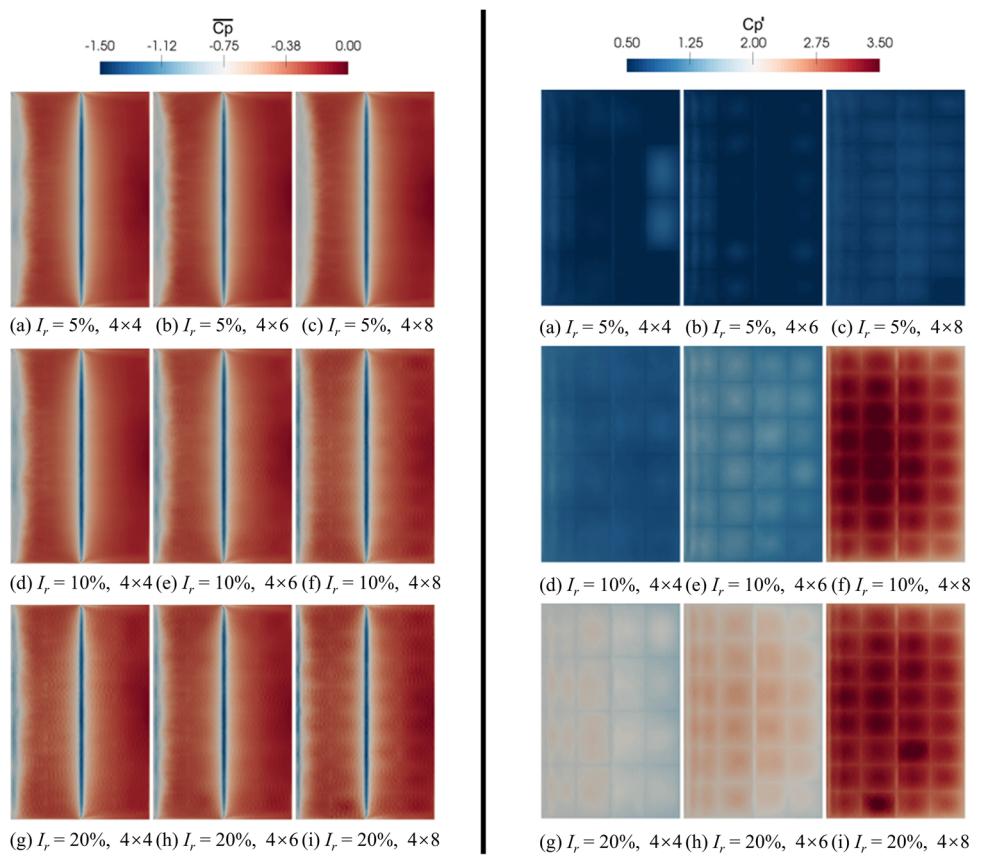
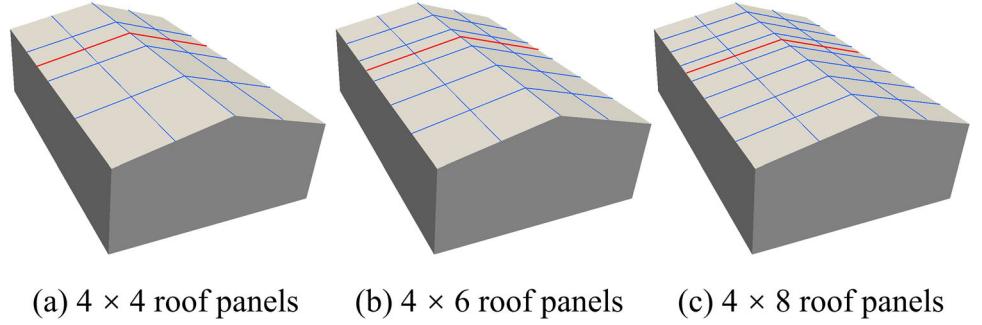
Fig. 10 $\overline{C_p}$ and C'_p distribution**Fig. 11** Diagram of panel center line (marked as red line)

Figure 10 shows distributions of $\overline{C_p}$ and C'_p on the roof structures. The $\overline{C_p}$ and C'_p along the center line of a panel in each configuration (see Fig. 11 for the location) are plotted in Fig. 12 and Fig. 13. All the roof surfaces are subject to high uplift pressure (pressure suction) since the roof slope is relatively small. In particular, the roof windward edge and roof ridge experience larger pressure suction due to the flow separations in these regions, as shown in Fig. 8 and Fig. 9. The distribution patterns of C_p are similar in all cases, as indicated in Fig. 10 (left) and Fig. 12. In contrast, Fig. 10 (right) and Fig. 13 show that the distributions of C'_p over roof panels are significantly different for each case. The value of C'_p increases with the turbulence intensity, I_r , and the overall stiffness of the roof. With the same I_r , the stiffer roof con-

figuration tends to have a larger C'_p , which indicates that the stiffer roof system experiences higher pressure fluctuation due to wind. This effect is also observed in experimental work by [4, 101]. Moreover, it is worth noticing that, for the cases with low I_r (5 %), the difference in pressure fluctuation is not as pronounceable as the cases with higher I_r (10 % and 20 %). For the roof configurations with lower stiffness (4×4 and 4×6), the increase in pressure fluctuation is proportional to the increment of I_r . However, the increment of C'_p for the roof with the 4×8 panel configuration (stiffest configuration in this study) is negligible when I_r goes from 10 % to 20 %. Such behaviors are essential to be considered in the performance based-design process and are difficult to capture without conducting FSI simulations.

Fig. 12 Comparison of \bar{C}_p over panel center line ($I_r = 5\%, 10\%, 20\%$ from left to right)

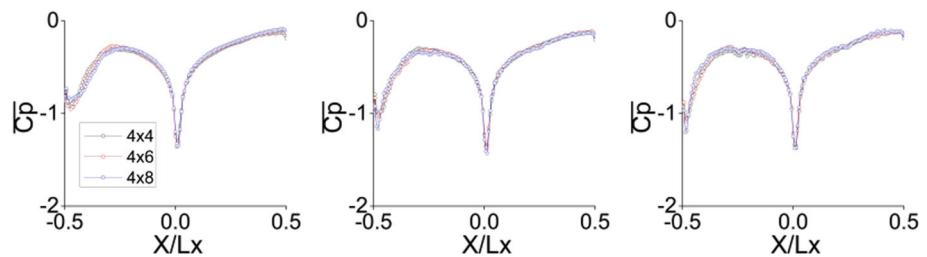


Fig. 13 Comparison of C'_p over panel center line ($I_r = 5\%, 10\%, 20\%$ from left to right)

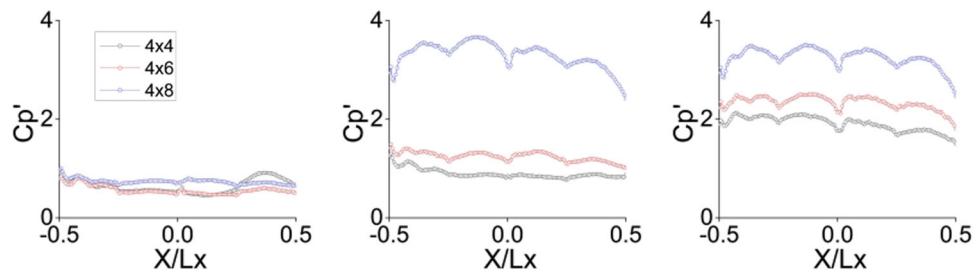


Fig. 14 Time history of wind pressure at a representative location

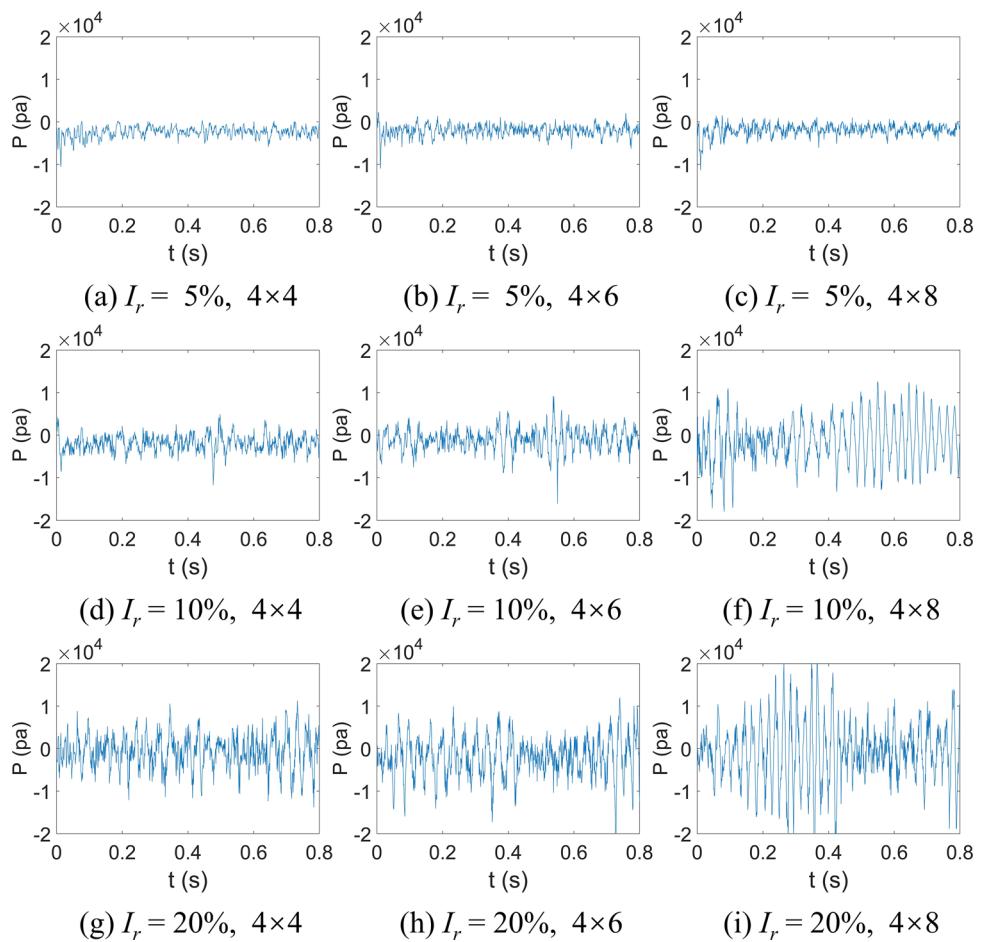
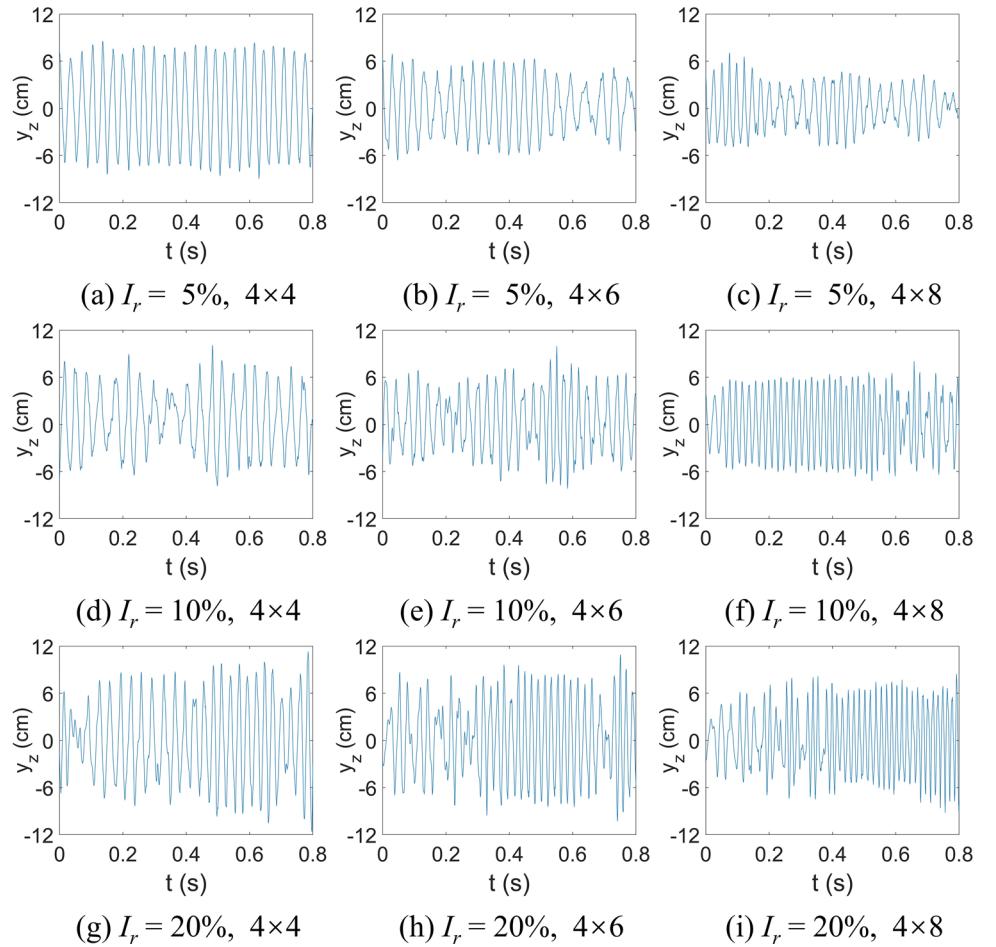


Fig. 15 Time history of deflection at a representative location



To further demonstrate the necessity of FSI simulations, the quasi-steady approach, which is widely used in wind engineering [102–104], is also used for estimating the pressure fluctuations (e.g., standard deviation) and compared with the FSI predictions. The quasi-steady approach is a simplified reduced-order model by ignoring the aeroelastic effects and correlating pressure fluctuation and turbulence intensity based on the following empirical relationship.

$$C'_p \approx 2I_u |\bar{C}_p| \quad (27)$$

where \bar{C}_p is the mean pressure coefficient, which is often estimated based on design codes and standards, for example, ASCE 7-22. The values of \bar{C}_p in ASCE 7-22 are tabulated and can be determined based on the wind load direction, roof angle, and mean roof height to base ratio of the building. For the studied cases, in which the roof angle is 13.58° and the mean roof height to base ratio is 0.446, the value of \bar{C}_p is -0.76 . By substituting this value into Eq. 27, values of C'_p for $I_r = 5$, 10, and 20% are 0.076, 0.152, and 0.304, respectively. However, the value of C'_p ranges from 0.5 to 3.5 in the FSI simulation, as shown in Fig. 10 (right) and Fig. 13. In comparison, the pressure fluctuations due to the aeroelastic effects

are largely underestimated by the quasi-static approach with the code-based \bar{C}_p value. This discrepancy indicates that the turbulence and aeroelastic behaviors, which lead to large flow separations and structure resonant displacement, have non-negligible effects on pressure fluctuations and should be carefully accounted for in practice.

To illustrate the dynamic process of the wind-roof interaction, a representative location that has the maximum \bar{C}_p is chosen for further analysis. The time history of pressure and roof deflection at this location are plotted in Figs. 14 and 15, respectively. For the pressure coefficient, on the one hand, larger inflow turbulence intensity leads to higher fluctuations for roofs with the same configuration. On the other hand, the pressure fluctuations increase with the number of roof panels (stiffness) under higher turbulence intensity, while the pressure fluctuations stay almost the same in lower turbulence intensity ($I_r = 5\%$). This confirms with the C'_p plots in Fig. 13. For the deflection, for the same roof configuration, increasing inflow turbulence intensity can lead to slightly larger amplitudes in deflection fluctuations. This behavior corresponds to the increasing trend of pressure amplitude presented in Fig. 14. Although, the pressure amplitudes under a I_r condition remain the same or slightly increase when

the number of panels increases, the roof deflection amplitudes reduce due to the increasing overall roof stiffness. It is noteworthy that while the deflection amplitude reduces, the frequencies of deflection fluctuation increase with roof stiffness. This suggests that cyclic fatigue of the roof supporting systems should be carefully considered for structures exposed to high I_r conditions.

5 Conclusion

The paper presented a high-fidelity isogeometric FSI model and its application for investigating the aeroelastic effects on pressure coefficient distributions and deflections of representative roof structures under different wind conditions and roof configurations. The FSI model integrates a set of advanced numerical methods, including moving domain CFD technique-based ALE-VMS and IGA-based Kirchhoff-Love thin shell theory. The simulated results indicated the necessity of FSI simulations under turbulent wind conditions by comparing them with the quasi-steady approach, which significantly underestimates pressure fluctuation. The paper also found that the time-averaged pressure coefficient C_p remains almost the same regardless of the turbulent intensity and number of roof panels for a given mean wind profile. At last, the FSI simulations showed that increasing the panel stiffness leads to higher fluctuation in pressure coefficients, which has been evidenced by several experimental investigations. Because increasing the number of roof panels increases the roof stiffness and pressure fluctuation simultaneously, the roof deflection fluctuation seems unchanged for a given inflow turbulence intensity. In addition, the risk of cyclic fatigue due to the increase of deflection fluctuation frequency by increasing overall roof stiffness is demonstrated in the FSI simulation results. Through this case study, we believe that this isogeometric ALE-VMS approach is a valuable FSI tool for wind engineering and structural engineering applications.

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