

Engineering Hydraulics: From Pipe to Outlet

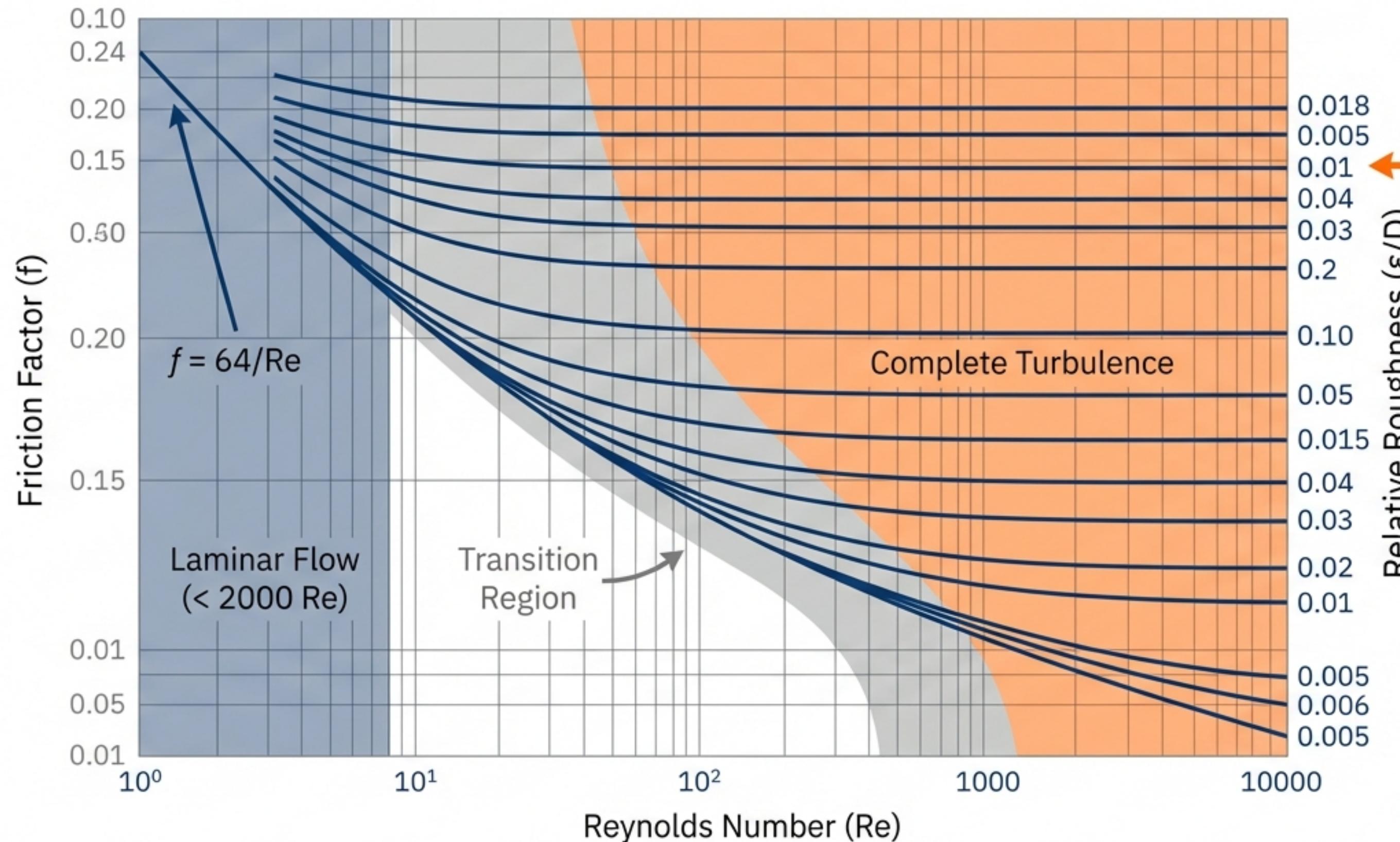
Analysis of Pressurized Flow, Open
Channel Systems, and Outlet Structures

1. Pressurized Systems
2. Gravity Systems
3. Outlet Control

Material	ε (mm)
Concrete, coarse	0.25
Concrete, new smooth	0.025



The Physics of Pipe Flow: Friction & Resistance

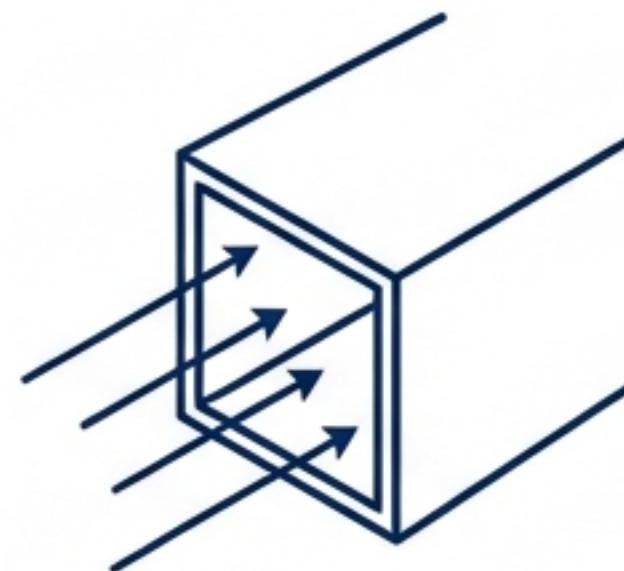


In turbulent flow, friction is driven by material roughness (ϵ), not fluid viscosity.

$$Re = \frac{\rho V d}{\mu}$$

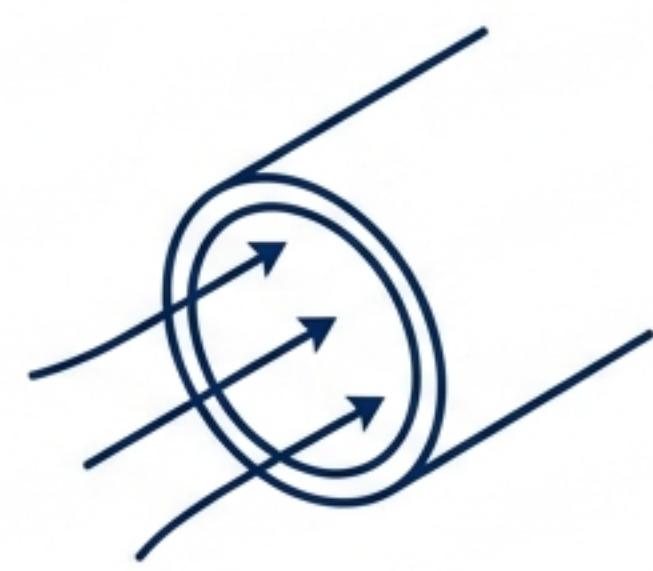
Local Losses: The Cost of Transitions

$$h_L = K_c \frac{V^2}{2g}$$

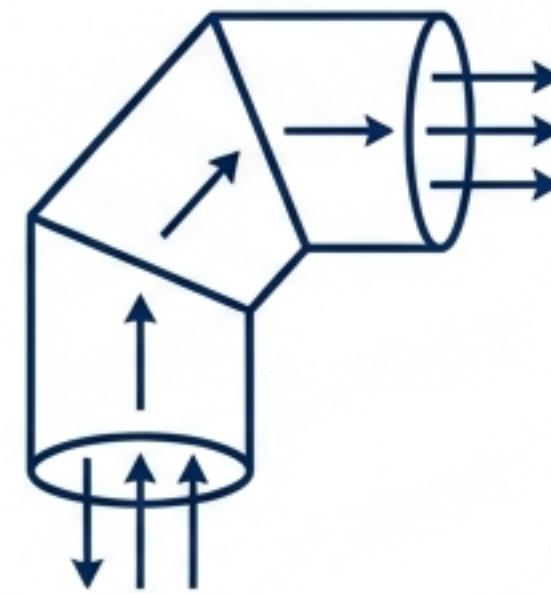


Square Edge Entrance:
 $K = 0.50$

Technical Grey IBM Plex Sans
with IBM Plex Mono



Well-Rounded
Entrance:
 $K = 0.03$



90° Miter Bend:
 $K = 1.1$



Globe Valve:
 $K = 10.0$

Key Insight: Sharp transitions and complex valves act as significant energy sinks compared to smooth fittings.

Adding Energy: Pump Flow Dynamics

$$P = (\gamma Q H) / \eta$$

Technical Grey Power Required (kW)

IBM Plex Mono Dynamic Head (meters) – The energy difference overcoming gravity and friction

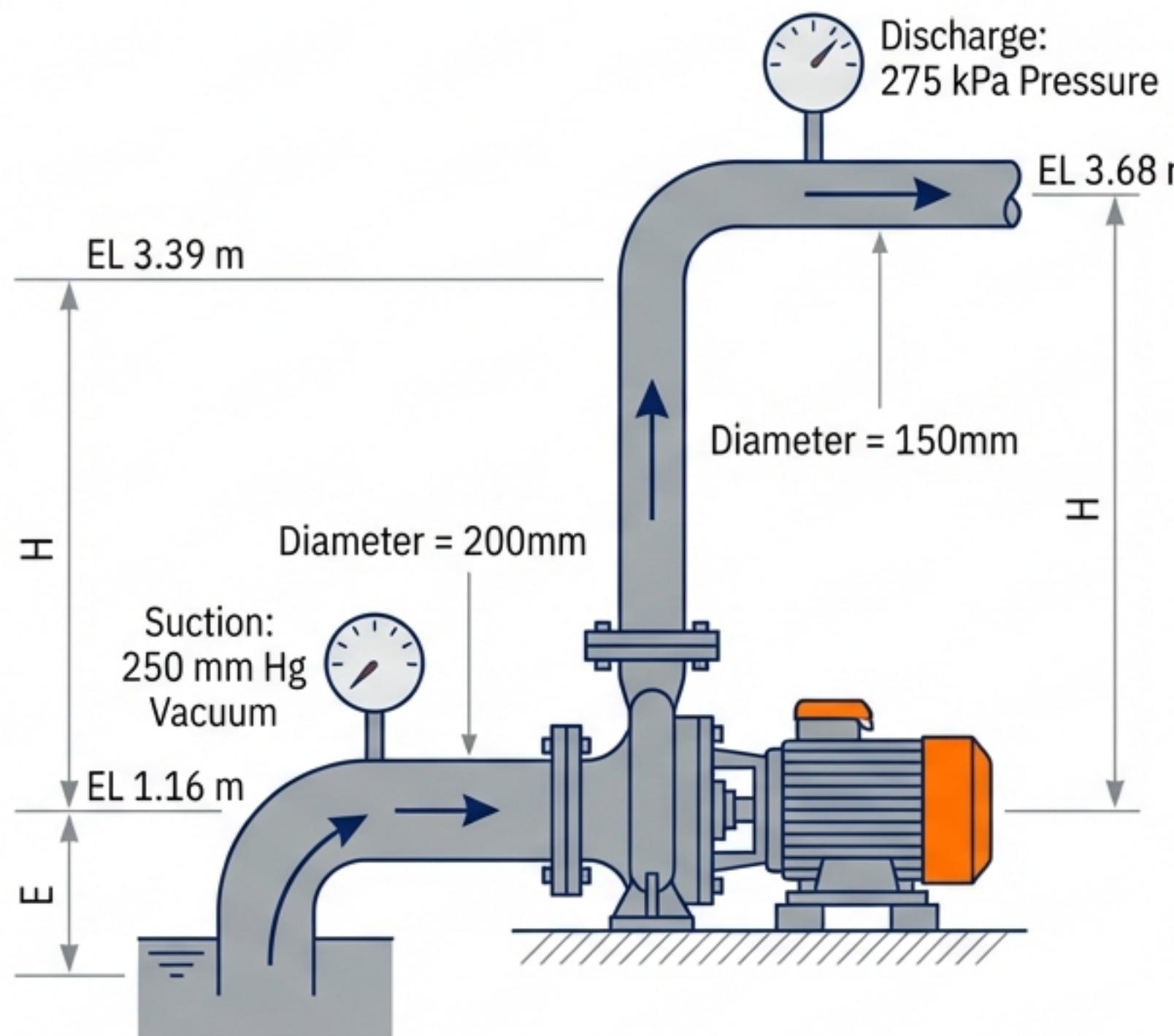
IBM Plex Mono Specific Weight of Water (9,810 N/m³)

IBM Plex Mono Flow Rate (m³/s)

IBM Plex Mono Efficiency (0.0 to 1.0)



Case Study: Calculating Pump Requirements



Problem Statement

Goal: Calculate the power (kW) required to deliver $0.15 \text{ m}^3/\text{s}$ of water.

Unit Conversion

1. Convert Gauge Pressures to Head (meters of water):

$$\text{Discharge Head} = \frac{275,000 \text{ Pa}}{9,800 \text{ N/m}^3} = 28.1 \text{ m}$$

$$\begin{aligned}\text{Suction Head} &= -250 \text{ mm Hg} \times 13.57 \text{ (s.g.)} \\ &= -3.4 \text{ m}\end{aligned}$$

Case Study: Solution & Results

Step 1: Calculate Velocities (Continuity Eq)

$$V_{\text{suction}} = 4.77 \text{ m/s}$$

$$V_{\text{discharge}} = 8.48 \text{ m/s} \text{ (Velocity increases as pipe narrows)}$$

Step 2: Apply Energy Equation IBM Plex Sans

$$\text{Total Head Increase } (E_p) = 37.0 \text{ Joules/Newton}$$

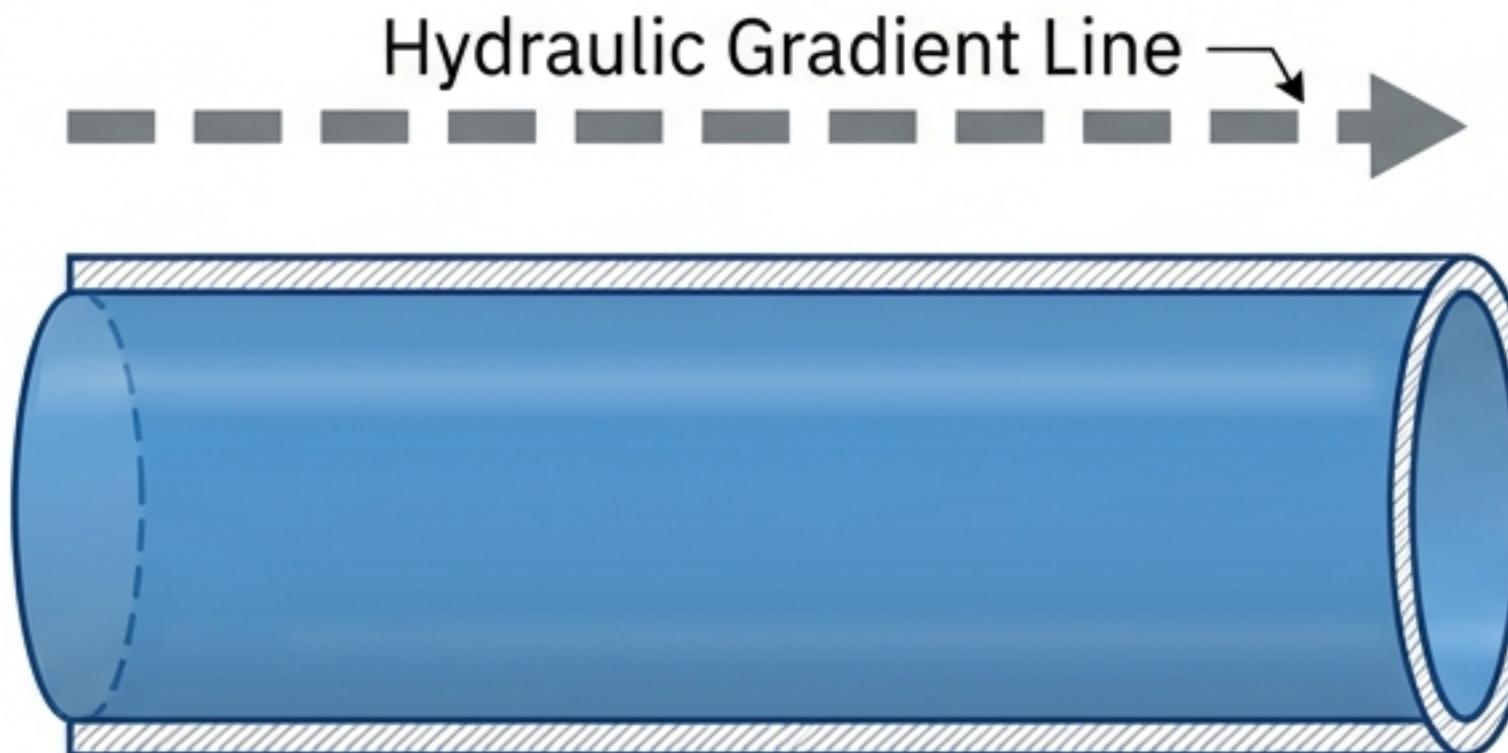
Step 3: Calculate Power IBM Plex Sans

$$P = (0.15 \times 9800 \times 37.0) / 1000 = 54.4 \text{ kW}$$

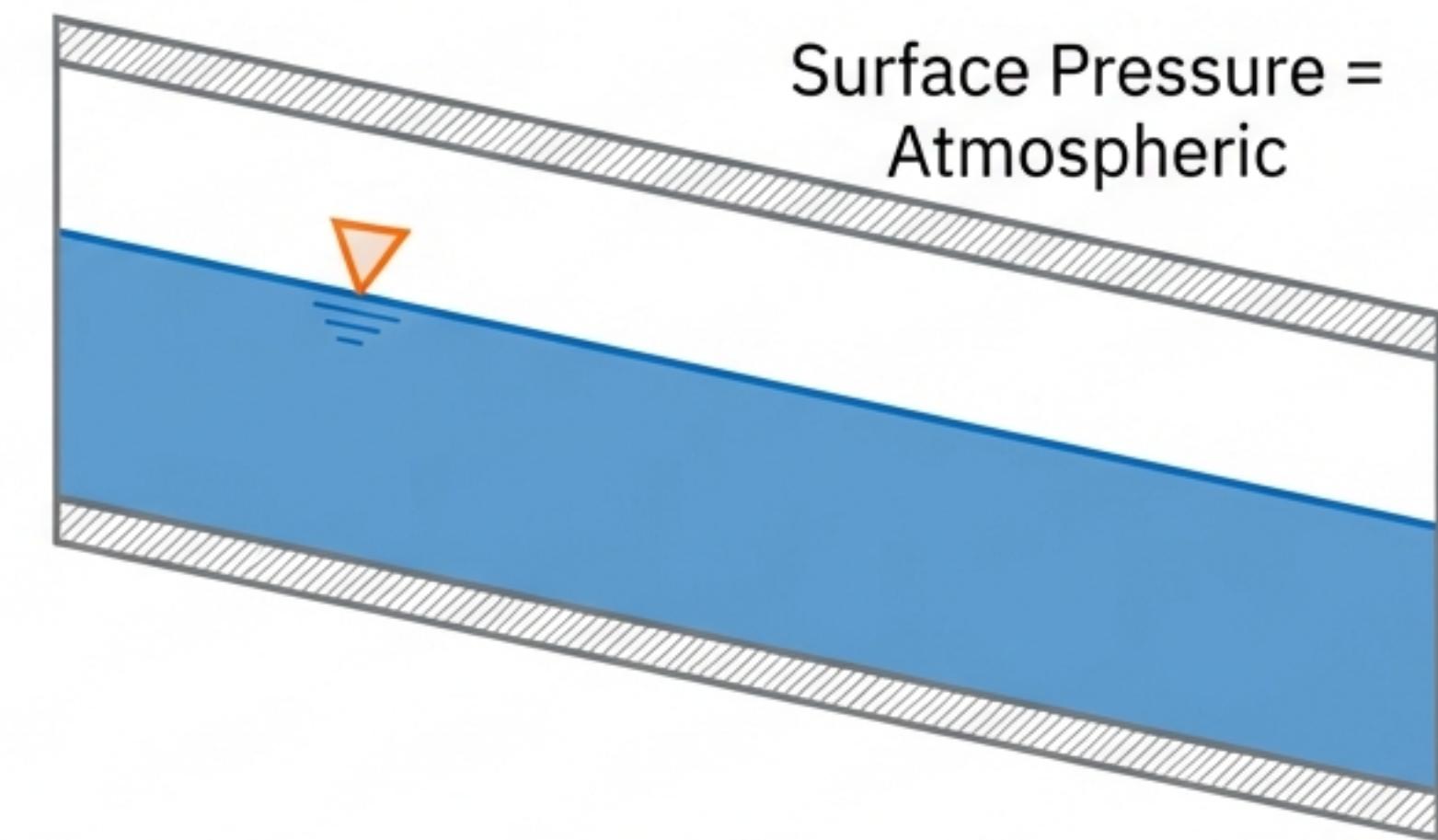
Phase 2: Open Channel Flow

The Critical Difference: The Free Surface

Pressurized Flow

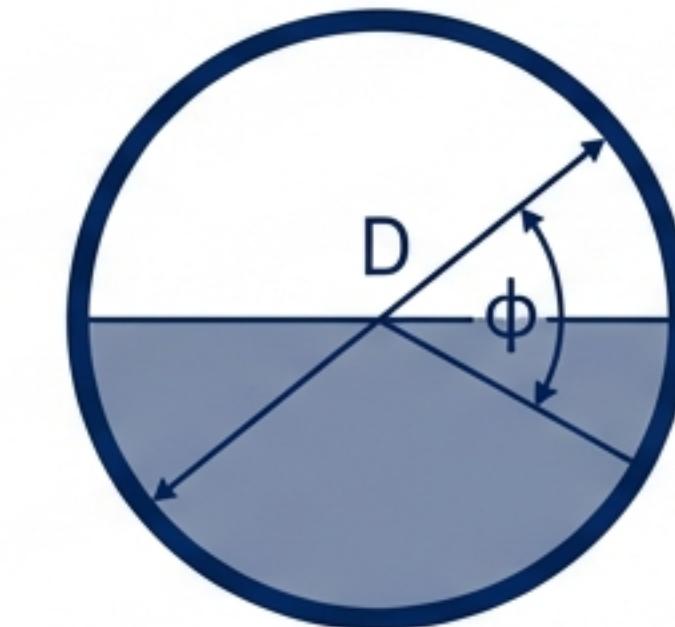
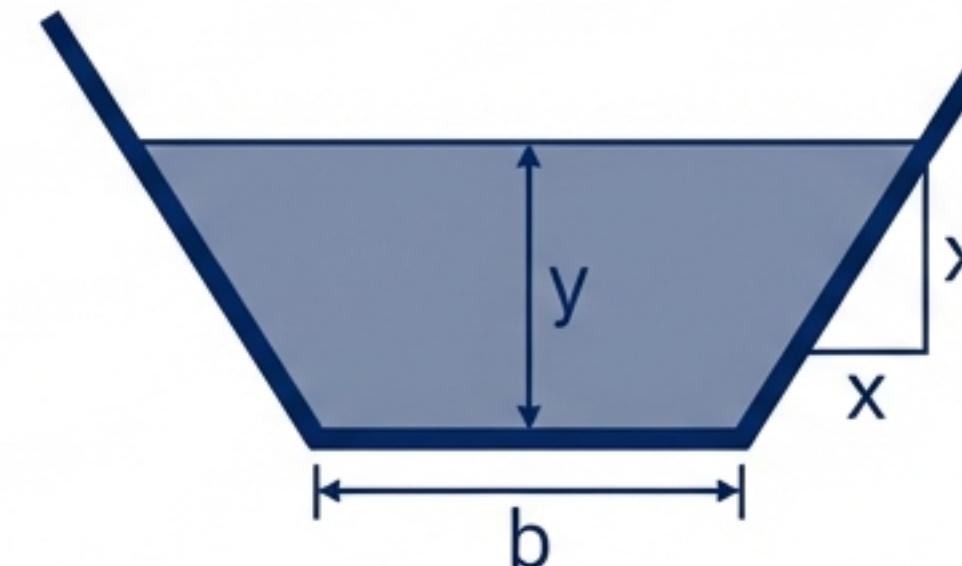
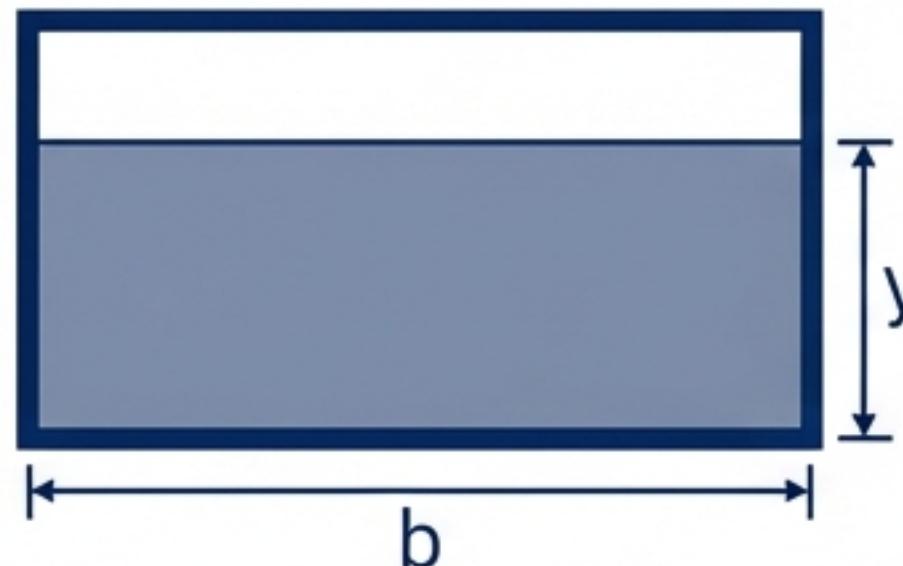


Gravity Flow



In open channels, water is driven by slope and gravity, not mechanical pressure.

Channel Geometry & Hydraulic Radius



$$\text{Area (A)} = b \times y$$

$$\text{Wetted Perimeter (P)} = b + 2y$$

$$\text{Area (A)} = (b + xy)y$$

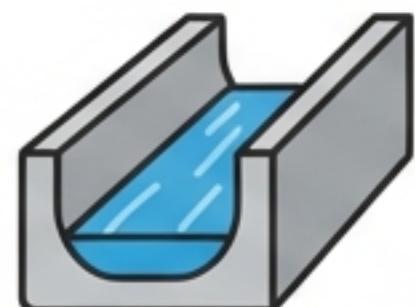
$$\begin{aligned}\text{Wetted Perimeter (P)} &= \\ &= b + 2y\sqrt{(1+x^2)}\end{aligned}$$

$$\text{Hydraulic Radius (R)} = \frac{A}{P}$$

Hydraulic Radius (R) is the ratio of the cross-sectional area to the length of the surface in contact with the water.

The Manning Equation

$$V = (1/n) \times R^{2/3} \times S^{1/2}$$



Finished Concrete
 $n = 0.013$
(Smooth, Fast)



Clean Stream
 $n = 0.030$

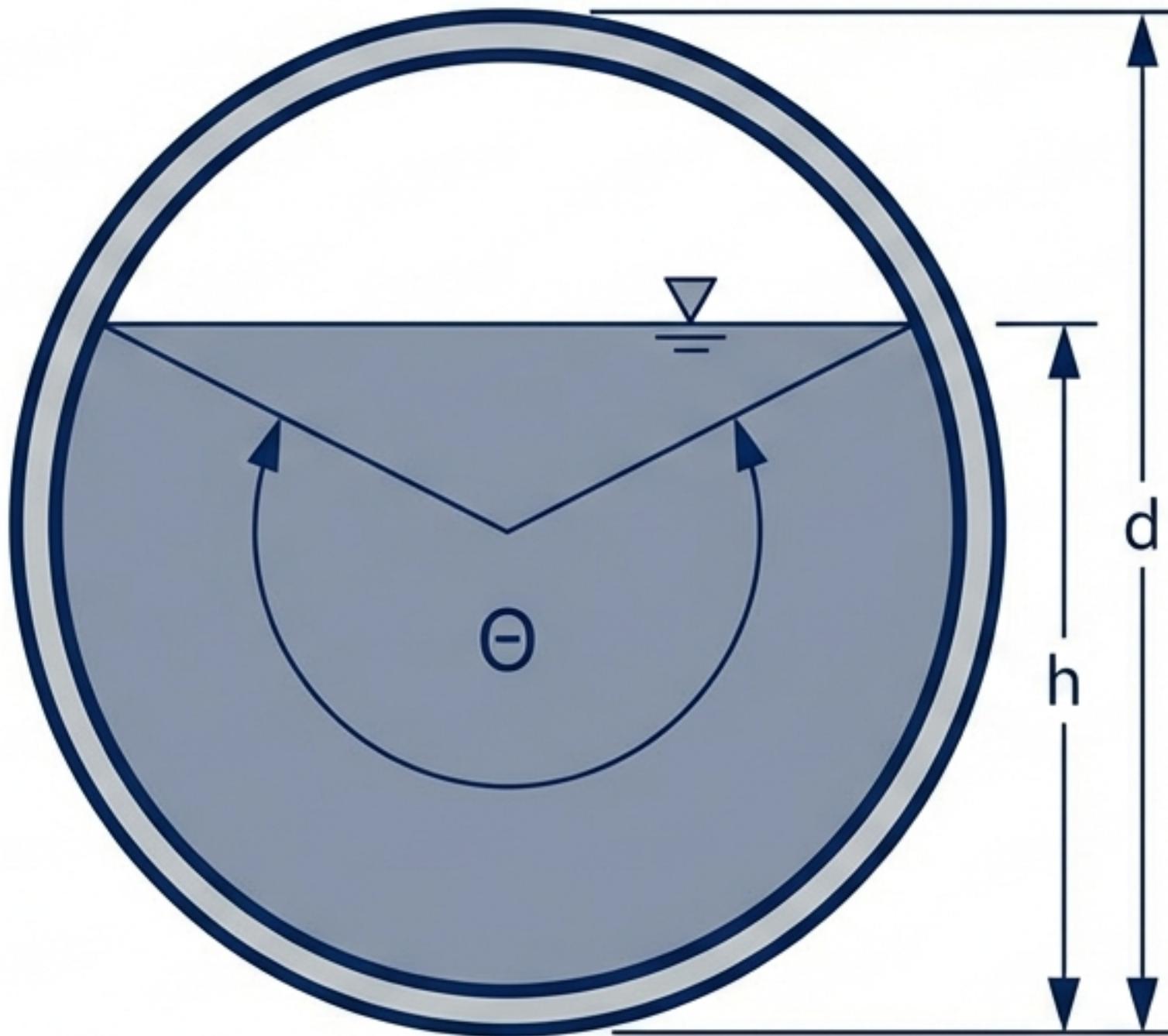


Dense Brush/Weeds
 $n = 0.080$
(Rough, Slow)



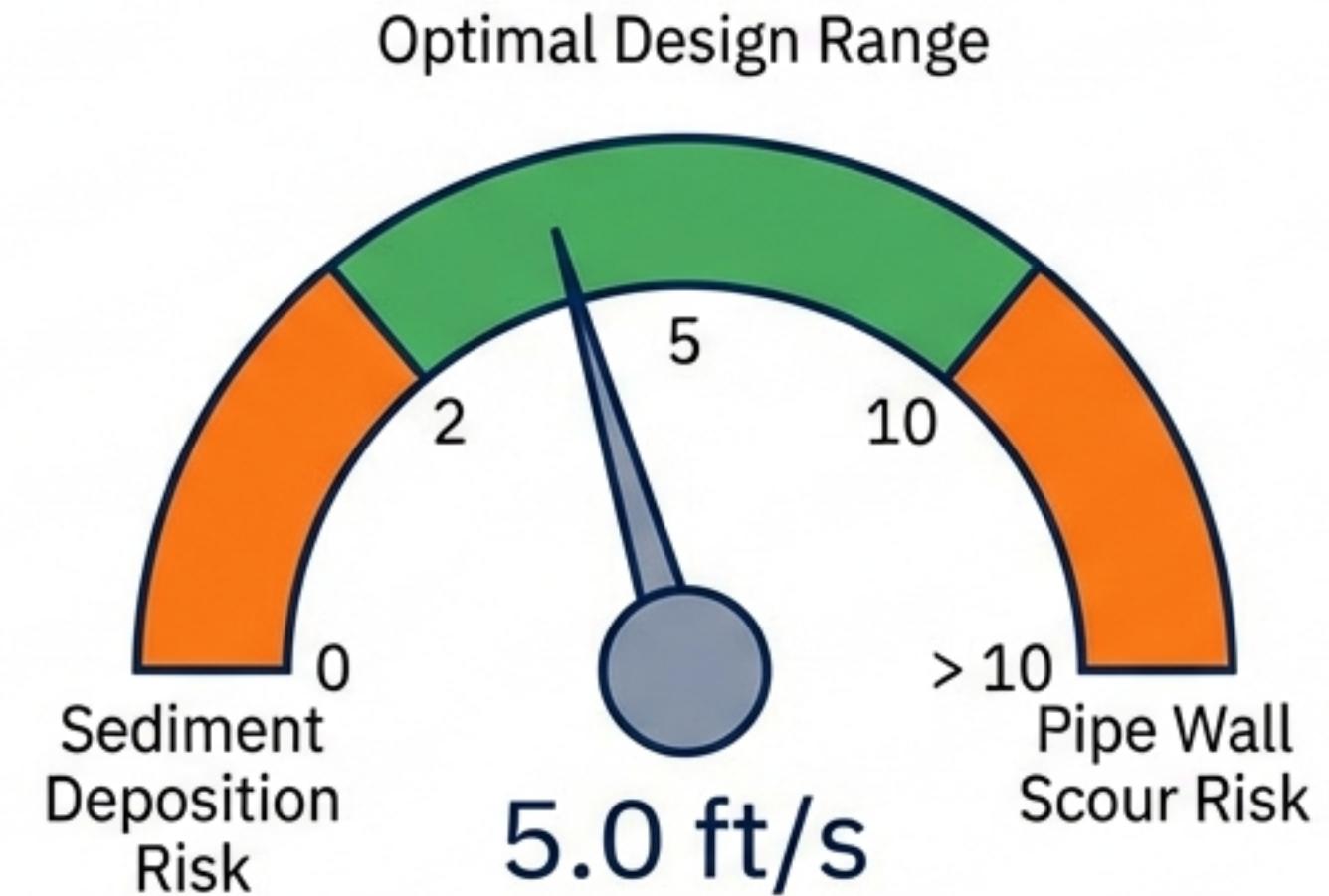
Velocity (V) decreases significantly as vegetation and roughness (n) increase.

Application: Storm Sewer Design



Cross-Section: Circular Storm Sewer

Velocity Design Considerations



Storm sewers act as gravity channels.
Design must balance self-cleaning
velocity against structural damage.

Worked Example: Sizing a Storm Sewer

Calculation Card



Given Data:

- Required Flow (Q): $15 \text{ ft}^3/\text{s}$
- Slope (S): 0.5%
- Material: Concrete ($n = 0.013$)

Calculation:

1. Assume full pipe flow.
2. Solve Manning's for Diameter (D).

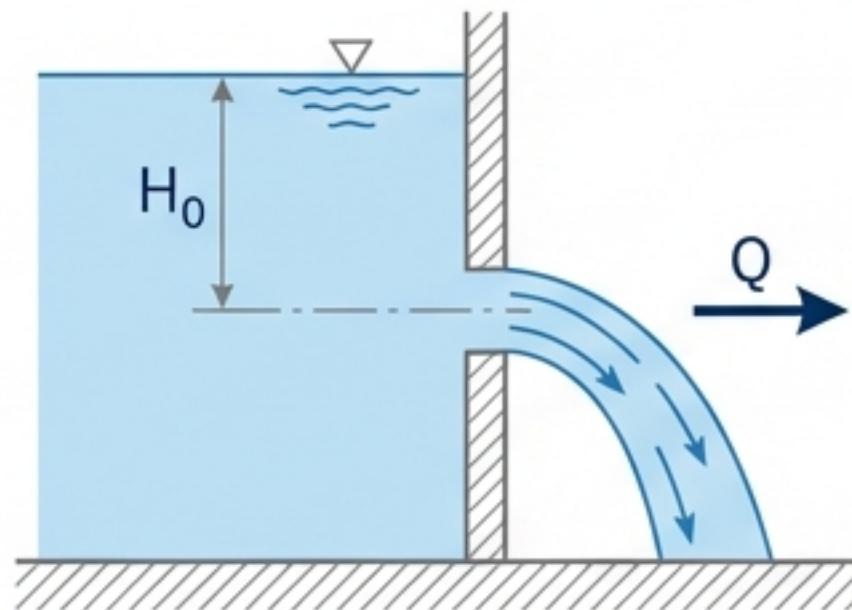
Result:

Required Diameter = **2.0 ft**

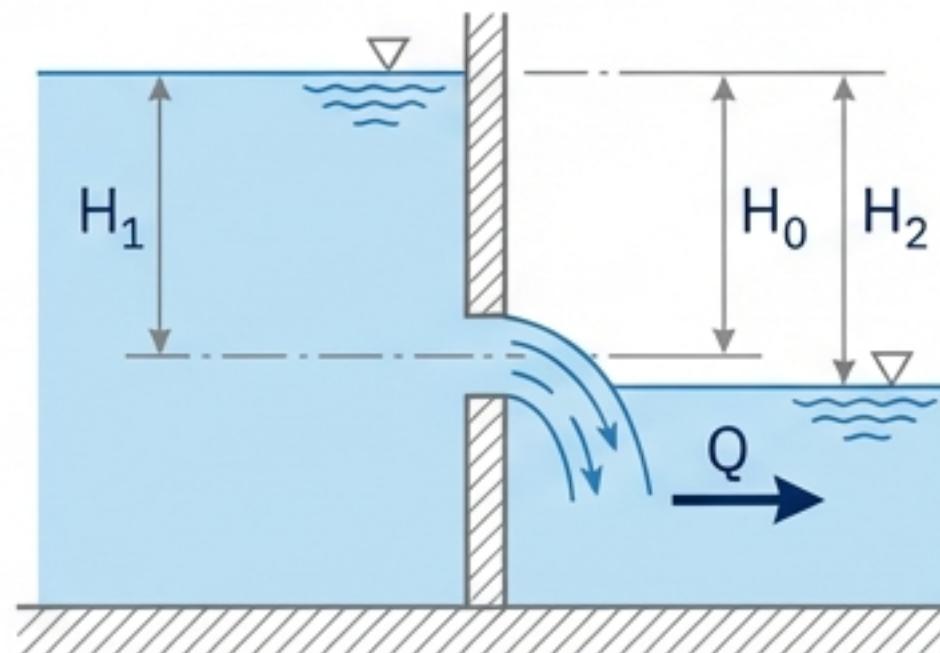
Verification Velocity = **4.78 ft/s**
(Acceptable)

Phase 3: Outlet Control — Orifices

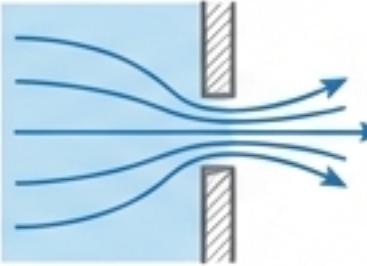
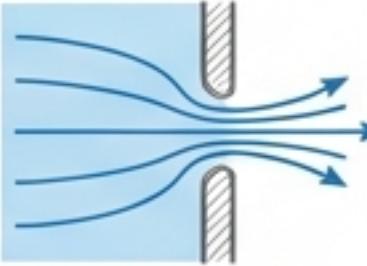
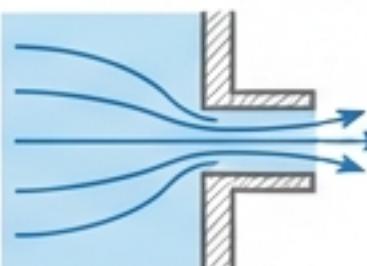
UNSUBMERGED ORIFICE



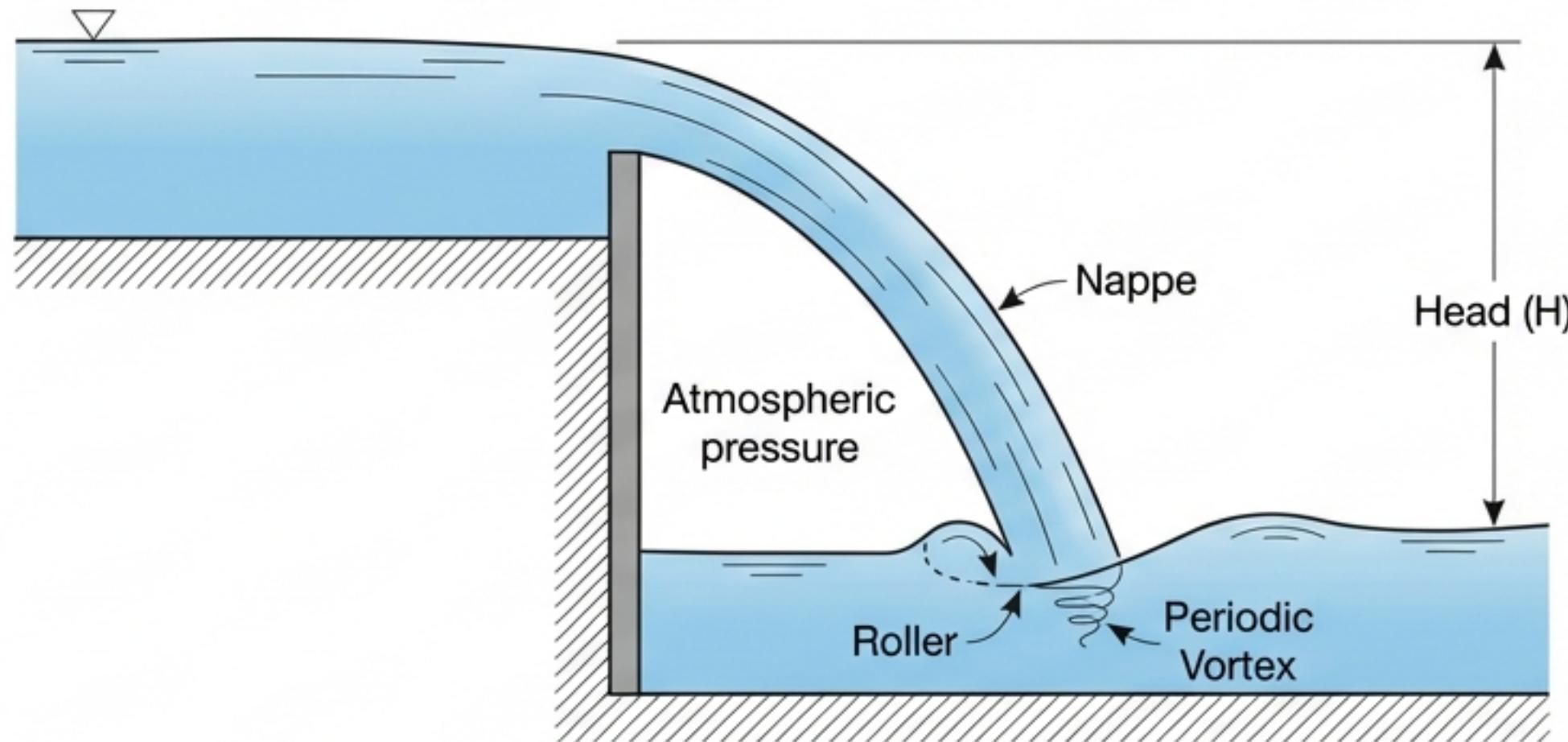
SUBMERGED ORIFICE



$$\text{Discharge Equation: } Q = C_d \times A \times \sqrt{2gh}$$

Discharge Coefficients (C_d)		
Illustration	Type	Coefficient (C)
	Sharp Edge	$C = 0.61$
	Rounded Edge	$C = 0.98$ (Most Efficient)
	Short Tube	$C = 0.80$

Outlet Control – Weirs



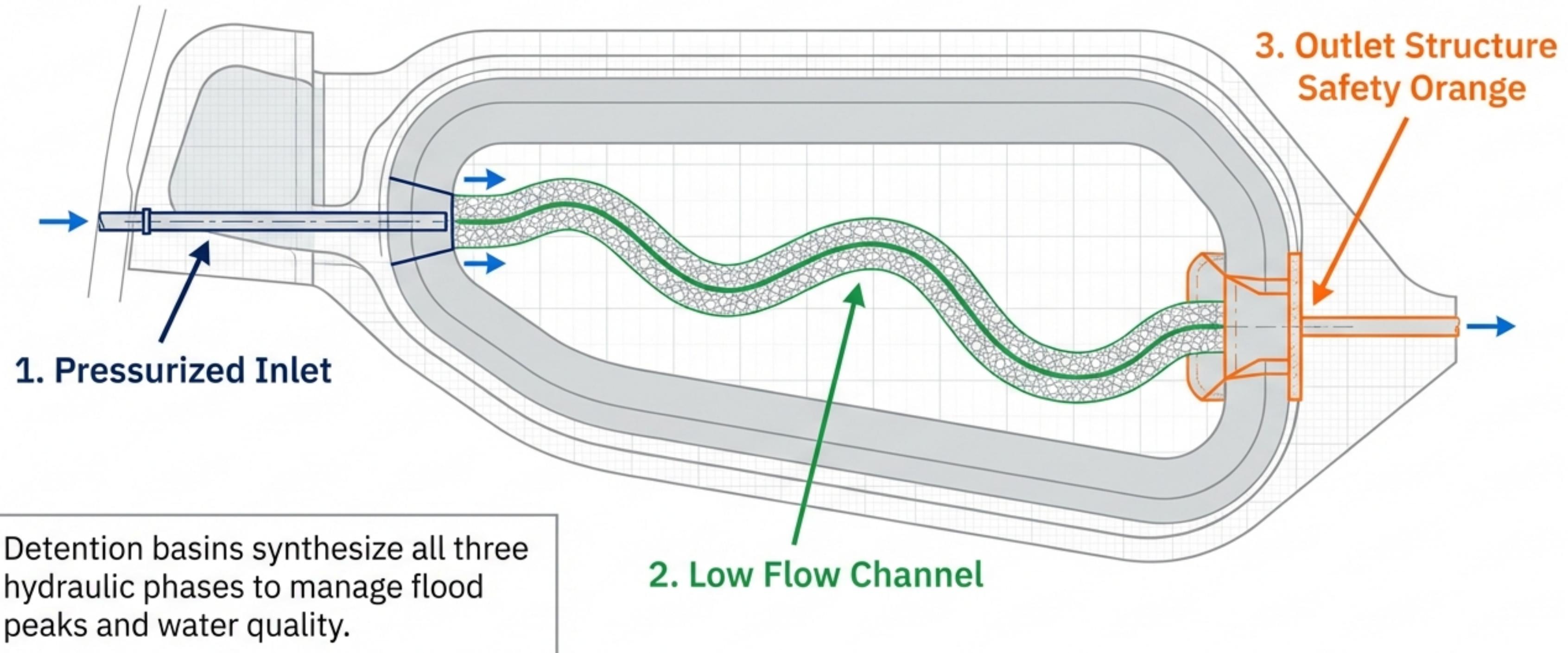
The Rehbock Formula

The weir coefficient (C_w) is not constant. It depends on the ratio of Head (H) to Weir Height (P).

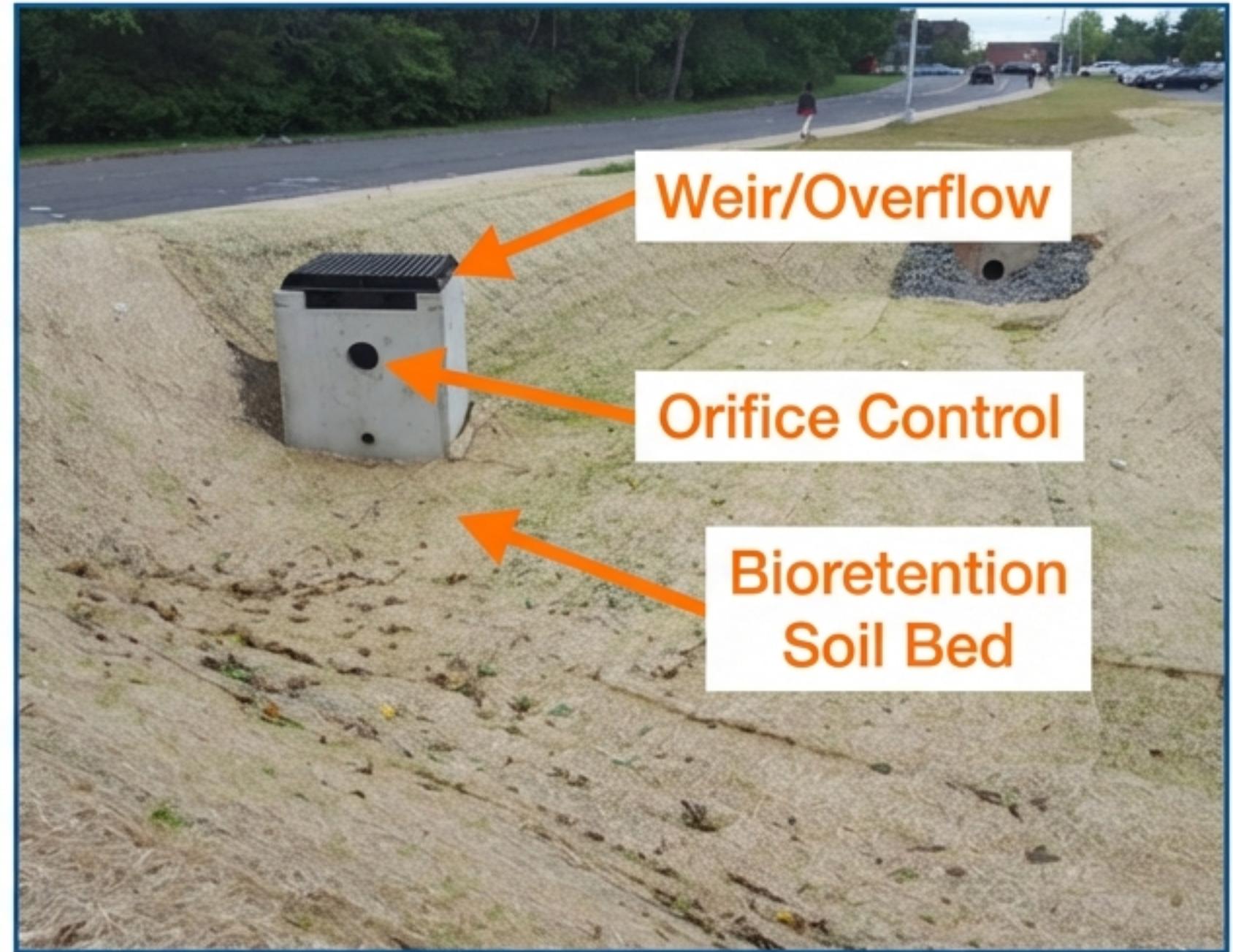
Rectangular Weir Equation:

$$Q = C_w \times \frac{2}{3} \times \sqrt{2g} \times B \times H^{3/2}$$

Integrated Systems: The Detention Basin



Real-World Application: Rutgers Busch Campus



Bioretention system at Brett & Bartholomew Roads, constructed 2016.

REV.
03

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