

Understanding the Concept of Bubble Deck

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By

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ABSTRACT:

As the infrastructure is developing there is need for some changes in the construction field, as one cannot rely on the same method for a long time as it can have different consequences. The main consequence is the shortage of material and manpower. Also, money matters a lot in construction department along with it the machines, equipment and technology in some region is not at a level, which we want. Hence in order to satisfy these results Bubble deck slab is one of the most effective slab techniques to replace conventional slab in terms of money and materials. Also, it requires less time to construct as compared to conventional slab.

1.0 INTRODUCTION:

Bubble Deck is a revolutionary method of virtually eliminating concrete from the middle of a floor slab not performing any structural function, thereby dramatically reducing structural dead weight. Bubble Deck is based on a new patented technique- the direct way of linking air and steel. Void formers in the middle of a flat slab eliminates 35% of a slabs self-weight removing constraints of high dead loads and short spans.

Incorporation of recycled plastic bubbles as void formers permits 50% longer spans between columns. Combination of this with a flat slab construction approach spanning in two directions – the slab is connected directly to insitu concrete columns without any beams -produces a wide range of cost and construction benefits including:-

The overall floor area is divided down into a series of planned individual elements, either 3 or 2.4 meters wide dependent upon site access, which are manufactured off-site using MMC techniques. These elements comprise the top and bottom reinforcement mesh, sized to suit the specific project, joined together with vertical lattice girders with the bubble void formers trapped between the top and bottom mesh reinforcement to fix their optimum position. This is termed a 'bubble-reinforcement' sandwich which is then cast into bottom layer of pre-cast concrete, encasing the bottom mesh reinforcement, to provide permanent form work within part of the overall finished slab depth.

On site the individual elements are then 'stitched' together with loose reinforcement simply laid centrally across the joints between elements. Splice bars are inserted loose above the pre-cast concrete layer between the bubbles and purpose made mesh sheets tied across the top reinforcement mesh to join the elements together. After the site finishing concrete is poured and cured this technique provides structural continuity across the whole floor slab –to create a seamless floor slab.

BubbleDeck has proved to be highly successful in Europe since its invention ten years ago. In Denmark and Holland over 1million square meters of floors have been constructed in the last seven years using the BubbleDeck system in all types of multi-storey buildings.

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OBJECTIVES

1. Used of hollow HDPE balls made up of waste plastic materials in concrete slab.
- 2.Reduced concrete usage – 1 kg recycled plastic replaces 100 kg of concrete.
3. Environmentally Green and Sustainable – reduced energy & carbon emissions.
- 4.Reduced Dead Weight– 35% removed allowing smaller foundation sizes, which result in reduced cost.

2.0 LITERATURE REVIEW: –

In the 1990's, Jorgen Breuning invented a way to link the air space and steel within a voided biaxial concrete slab. The Bubbledeck technology uses spheres made of recycled industrial plastic to create air voids while providing strength through arch action. As a result, this allow hollow slab to act as normal monolithic two way spanning concrete slab. These bubbles can decrease the dead weight up to 35% and can increase the capacity the capacity by almost 100% with same thickness. As a result BubbleDeck slabs can be lighter, stronger and thinner than regular reinforced concrete slabs.

2.1. Millennium Tower Rotterdam Design by WZMH Architects and AGS Architects, completed in (1997-2000):

The first high rise building erected with BubbleDeck filigree-elements and the second highest building in Netherlands, 34 stories and 131 meter high. BubbleDeck was chosen, in spite of being a completely new product, because of its advantages in cost,construction time and flexibility and because of environmental issues. Beams could be excluded resulting in two more stories than planned in the beginning for the same building height. Built in 1998-2000.



Figure 1: Millennium Tower Rotterdam

2.2. Car park(Frankfurt, Germany in 2001):

Car park built with BubbleDeck in Frankfurt Germany in 2001. BubbleDeck was chosen to reduce the weight of the decks and to get wider spans.



Figure 2: Car park built with Bubble Deck in Frankfurt, Germany in 2001

2.3. Media City:

This 32,000m² building was constructed with great transparency, revealing a huge open atrium. This atrium is the fulcrum and heart of the building. The spaces are formed in soft, organic shapes that allow light to spill onto every single workplace in the building. To achieve these wide, open, internal spaces a BubbleDeck structure of post – tensioned 450mm deep floor plates, achieving 16 meter spans between columns was selected – dramatically reducing structure dead weight and enabling long spans. The flexibility of BubbleDeck also facilitated construction of the soft flowing, organic shapes forming the floors around the central atrium.



Figure 3: Media City

The BubbleDeck is a revolutionary biaxial concrete floor system developed in Europe. High-density polyethylene hollow spheres replace the ineffective concrete in the center of the slab, thus decreasing the dead weight and increasing the efficiency of the floor. These biaxial slabs have many advantages over a conventional solid concrete slab: lower total cost, reduced material use, enhanced structural efficiency, decreased construction time and is a green technology.

Through tests, module and analysis from variety of institution BubbleDeck was proven to be superior to traditional solid concrete slab the reduced dead load makes the long term response more economical for the building while offsetting the slightly increased deflection of the slab however the shear and punching shear resistance of BubbleDeck floor is significantly less than a solid deck since resistance is directly related to the depth of concrete design reduction factor have been suggested to compensate for these differences in strength this system is certified in the Netherlands, the United kingdom, Denmark and Germany.

Reinforced concrete slabs are components commonly used in floors, ceiling, garages and outdoor wearing surfaces

The BubbleDeck system is based upon the patented integration technique- the direct way of linking air and steel. The BubbleDeck is a two-way hollow deck in which plastic balls serves the purpose of eliminating concrete that has no carrying effect. By adapting the geometry of the ball and the mesh width, an optimized concrete construction is obtained,

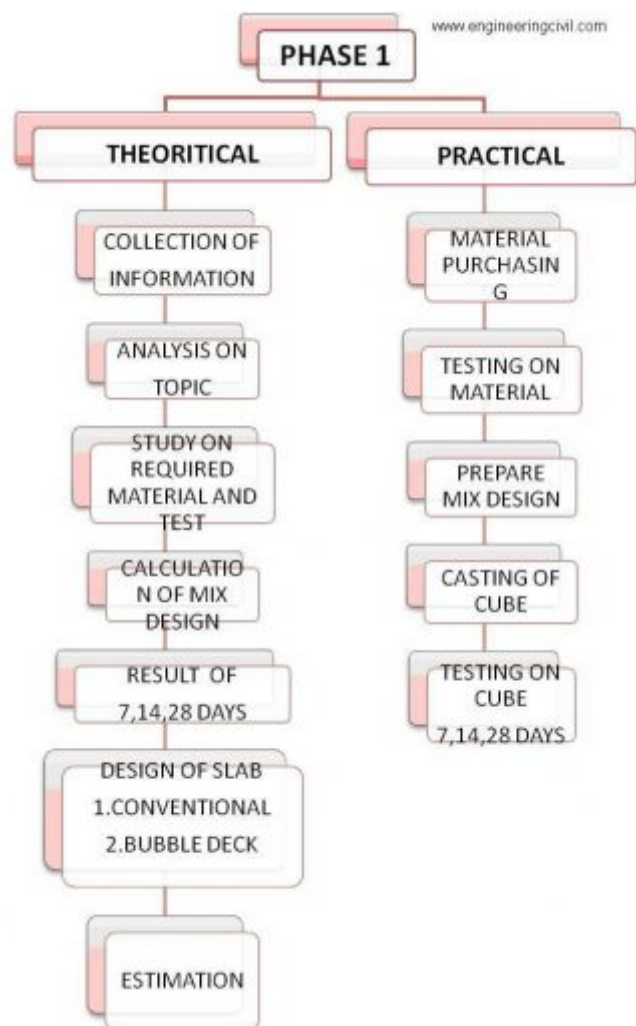
with simultaneous maximum utility of both moment and shear zones. The construction literally creates itself as a result of the geometry of the two well-known components: Welded reinforcing mesh and hollow plastic balls. When the top and bottom reinforcing meshes are connected in the usual way, a geometrical and statically stable BubbleDeck unit evolves.

The reinforcing mesh catches, distributes and locks the balls in exact position, while the balls shape the air volume, control the level of the reinforcing meshes, and at the same time stabilize the spatial lattice. When the steel lattice unit is concreted, a “monolithic” two-way hollow slab is obtained.

3.0 FLOW CHARTS: –

3.1.PHASE I: –

3.2.PHASE II: –





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4. MATERIALS AND TESTS

4.1. MATERIALS: –

4.1.1. POLYETHYLENE BUBBLES

Polyethylene is derived from either modifying natural gas (a methane, ethane, propane mix) or from the catalytic cracking of crude oil into gasoline. In a highly purified form, it is piped directly from the refinery to a separate polymerization plant. Here, under the right conditions of temperature, pressure and catalysis, the double bond of the ethylene monomer opens up and many monomers link up to form long chains. In commercial polyethylene, the number of monomer repeat units ranges from 1000 to 10 000 (molecular weight ranges from 28 000 to 280 000). The bubbles are made of polyethylene they do not react chemically with the concrete and/or the reinforcement steel. The bubbles are non-porous and possess enough strength and stiffness to carry applied loads safely in the phases before and during the pouring of the site concrete.

4.1.2. CEMENT

Ordinary Portland cement can be used it should confirm all standards as specified. Cement remained stored for more than 60 days from the date of receipt from factory shall be rejected.

4.1.3. COARSE AGGREGATE:

It shall be hard broken stone of granite or similar stone free from dust and other organic and non-organic matter. We use 20 mm & mix aggregate.

4.1.4. FINE AGGREGATE

Shall be coarse sand consisting of hard sharp and angular grains and shall pass through screen of 4.75 mm square mesh. Fine aggregate should be free from dust, silt and other organic and non-organic matter.

4.1.5. WATER

The water used should be fresh and free from alkaline and acid matter and it should be potable. Generally drinkable water should be used.

SR. NO.	DESCRIPTION	RESULT
1.	Initial And Final setting time	IST=180min FST=520min
2.	Standard consistency of cement	33.25%
3.	Soundness of cement	2mm
4.	Fineness modulus and grading of fine aggregate and coarse aggregate	fine-2.6 coarse-3.2
5.	Moisture content of fine aggregate	0.403%
6.	Specific gravity of fine aggregate	2.65
7.	Determination of moisture content of coarse aggregate	2.378%
8.	Specific gravity and water absorption test of coarse aggregates	sp.g-2.80 water absorbtion-0.5
9.	Flakiness and Elongation of coarse aggregate	flakiness- 17.83% elongation- 16.47%

Table 1: Result Summary

5.0 MIX DESIGN (M20)

5.1. MIX DESIGN CALCULATION:

Data

Grade of concrete	= M 20
Type of cement	= OPC 53 grade
Maximum size of aggregate	= 40 mm
Minimum cement content	= 300 kg/m ³
Maximum cement content	= 450 kg/m ³

Maximum water cement ratio = 0.55
Degree of workability = 75 mm
Exposure condition = mild
Specific gravity of cement = 3.15

Specific gravity of aggregate-

a) Coarse aggregate = 2.80

b) Fine aggregate = 2.65

Water absorption –

a) Coarse aggregate – 4.02 %

b) Fine aggregate – 1.27 %

Free moisture –

a) Coarse aggregate – 2.378 %

b) Fine aggregate – 0.403 %

Sieve analysis –

a) Coarse aggregate – 3.2

b) Fine aggregate – 2.6

Target mean strength-

$f_{ck} = f_{ck} + 1.65 \times \text{standard deviation}$

$= f_{ck} + 1.65 \times 4$

$= 26.6 \text{ N/mm}^2$

Selection of W/C ratio-

Maximum W/C ratio = 0.55

Water / cement ratio is taken from the experience of the mix designer based on this experience of similar work elsewhere Adopt W/C ratio = 0.48

Selection of water content-

Degree of workability = 75 mm

Maximum water content = 186 kg

Estimated water content for 75 mm slump = $186 \times (3/100) + (186)$

= 191.58 liters

Calculation of cement content

W/C ratio = 0.48

Water used = 191.58 liter

$$\text{Cement} = 191.58 / 0.48$$

$$= 399 \text{ kg/m}^3 > 300 \text{ kg/m}^3 \dots\dots \text{ok}$$

Calculation of coarse and fine aggregate content –

Volume of coarse aggregate for 40 mm maximum size aggregate and fine aggregate 0.05 Falling in the zone-I and w/c ratio 0.48 is found out to be 0.64.

When w/c ratio is 0.5 and fine aggregate zone-I in that case volume of coarse aggregate is 0.60 %.

For every decrease of w/c by 0.05, the coarse aggregate volume may be increase by 1 % to reduce the sand content.

Coarse aggregate for the w/c ratio of 0.48

$$\text{Volume of coarse aggregate} = 0.64$$

$$\text{Volume of fine aggregate} = 1 - 0.64$$

$$= 0.36$$

Mix calculation-

$$\text{Volume of concrete} = 1 \text{ m}^3$$

$$\text{Absolute volume of cement} = 399 / 3.15 \times 1 / 1000$$

$$= 0.126 \text{ kg/m}^3$$

$$\text{Volume of water} = 191.58 / 1000$$

$$= 0.1915 \text{ m}^3$$

$$\text{Total volume of all in aggregate} = 1 - (0.126 + 0.1915)$$

$$= 0.682 \text{ m}^3$$

$$\text{Wt. of coarse aggregate} = 0.682 \times \text{volume of CA} \times \text{specific gravity of CA} \times 1000$$

$$= 0.682 \times 0.64 \times 2.80 \times 1000$$

$$= 1222.14 \text{ kg/m}^3$$

$$\text{Weight of fine aggregate} = 0.682 \times 0.36 \times 2.65 \times 1000$$

$$= 650.62 \text{ kg/m}^3$$

Mix proportioning for trial

$$\text{Cement} = 399 \text{ kg/m}^3$$

Water = 191.58 kg/m³

Fine aggregate = 650.62 kg/m³

Coarse aggregate = 1222.14 kg/m³

Weight density = 399 + 191.58 + 650.62 + 1222.14

= 2463.34 kg/m³

W/c ratio = 0.48

Field correction

Fine aggregate quantity = 650.62 kg/m³

Absorption = 1.27 %

Quantity of absorption = 1.27/100 x 650.62

= 8.26 kg/m³

Surface moisture = 0.403 %

Quantity of surface moisture = 0.403/100 x 650.62

= 2.62 kg/m³

Weight of fine aggregate in field condition = 650.62 + 8.26 – 2.62
= 656.26 kg/m³

Coarse aggregate quantity = 1222.14 kg/m³

Quantity of absorption = 4.02/100 x 1222.14

= 49.13 kg/m³

Quantity of surface moisture = 2.378/100 x 1222.14

= 29.06 kg/m³

Weight of coarse aggregate in field condition

= 1222.14 + 49.13 – 29.06

= 1242.21 kg/m³

Sr. no.	Content	Quantity (kg/m ³)
1	Cement	399
2	Water	191.58
3	Fine aggregate	656.26
4	Coarse aggregate	1242.21

5	Weight density	2489.05
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Table 2: Mix Design Results

Cement: fine aggregate: coarse aggregate

399:656.26: 1242.21

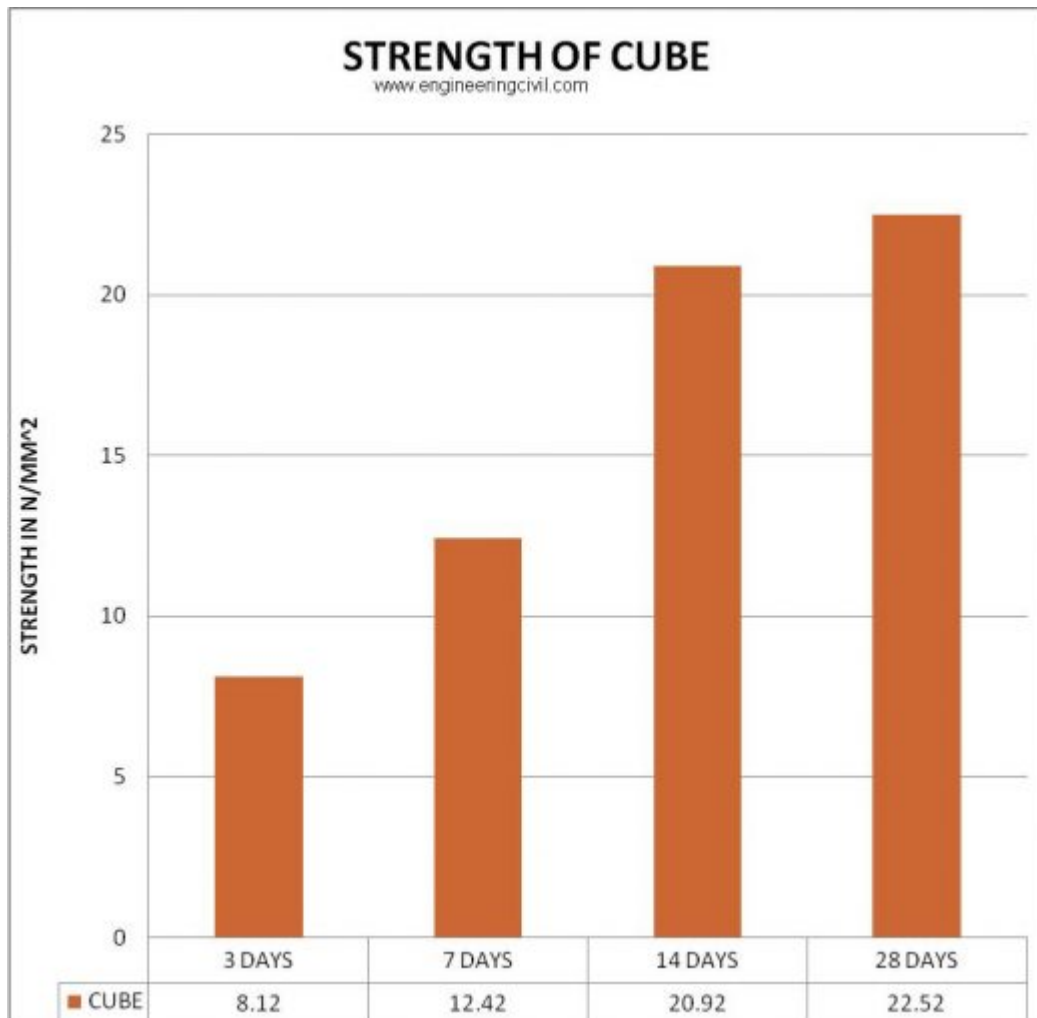
1: 1.64: 3.11

5.2. CUBE CASTING RESULTS: –

	CUBE 1 (N/mm ²)	CUBE 2(N/mm ²)	CUBE 3(N/mm ²)
3 DAYS	7.82	8.12	9.42
7 DAYS	14.22	12.42	12.92
14 DAYS	18.11	20.92	23.76
28 DAYS	25.13	22.52	24.11

Table 3: Cube Casting Results

5.3. BAR CHART: –



6.0 DESIGN OF SLAB: –

6.1. CONVENTIONAL SLAB: –

Design of 3×3 conventional slab

$L_y/L_x = 3/3 = 1 < 2$two way slab 1.

Trial depth

$d = L_x / (20 \text{ MF}) = 3230 / (20 \times 1.4) \dots$
assume 0.35% Pt

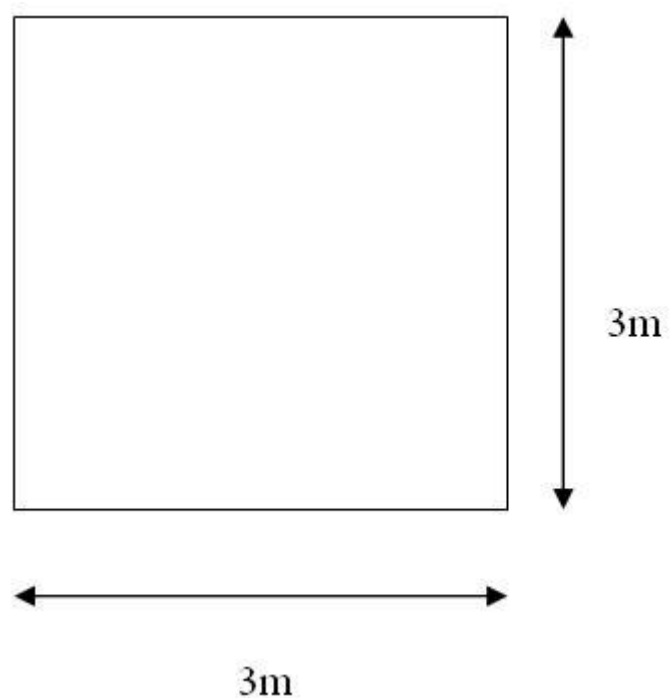
$d = 115.35 \text{ mm}$

$D = 200 \text{ mm}$

$d = D - \text{cover} - \frac{?}{2}$
 $= 200 - 20 - \frac{8}{2}$
 $= 176 \text{ mm}$

$D = 200 \text{ mm}$

$D = 176 \text{ mm}$



2. Effective span

$$L_{eff} = L_x + d = 3000 + 176 = 3176$$

3. Load calculation

$$\text{Self weight} = 25 \times d = 25 \times 176 = 4.4 \text{ KN/m}$$

$$\text{Live load} = 3 \text{ KN/m}$$

$$\text{Floor finish} = 1 \text{ KN/m}$$

$$\text{Total load} = 8.4 \text{ KN/m}$$

$$W_{ud} = \text{factored load} = 8.4 \times 1.5 = 12.6 \text{ KN/m}$$

4. Factored moment $\dot{\alpha}_x = 0.056$

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$$\dot{\alpha}_y = 0.056$$

$$M_{uy} = M_{ux} = \dot{\alpha}_x W_{ud} L_x^2 = 0.056 \times 12.6 \times 3.176^2 = 7.117 \text{ KN.m}$$

5. Check for depth

$$M_{ulim} = 0.138 F_{ck} b d^2$$

$$7.117 \times 10^6 = 0.138 \times 20 \times 1000 \times d^2$$

$$d = 50.78 < 176 \text{ mm} \dots \text{ok}$$

6. R/F in X-direction and Y-direction

$$A_{st \min} = 0.12 \% A_g = 0.12/100 \times b \times D$$

$$= 0.12/100 \times 1000 \times 200 = 240 \text{ mm}^2$$

$$A_{st} = (0.5 f_{ck} / f_y) \times [1 - \sqrt{(1 - 4.6 \times M_u) / (f_{ck} b d^2)}] b d$$

$$= (0.5 \times 20 / 415) \times [1 - \sqrt{(1 - 4.6 \times 7.117 \times 10^6) / (20 \times 1000 \times 176^2)}] \times 1000 \times 176$$

$$= 113.46 \text{ mm}^2 < 240 \text{ mm}^2$$

$$\text{Provide } A_{st} = 240 \text{ mm}^2$$

$$\text{Spacing} = 1000 \times A_{\phi} / A_{st} = (1000 \times \pi / 4 \times 8^2) / 240 = 200 \text{ mm}$$

$$\text{Provide } 8 \text{ mm } \phi @ 200 \text{ mm c/c}$$

7. Check for deflection

$$\%Pt = 100 A_{st} / b d$$

$$= (100 \times 240) / (1000 \times 176)$$

$$= 0.136 \% < 0.35 \%$$

8. Check for development

$$L_d = 47 \phi = 47 \times 8 = 376 \text{ mm}$$

6.2. DESIGN OF BUBBLE DECK SLAB:-

Design of 3×3 bubble deck slab

$L_y / L_x = 3/3 = 1 < 2 \dots \dots$ two way slab 1. Trial depth $D = 200 \text{ mm}$ $d = D - \text{cover} - \phi/2 = 200 - 34 - 8/2 = 162 \text{ mm}$ $D = 200 \text{ mm}$ $d = 162 \text{ mm}$ 2. Effective span

$$L_{eff} = L_x + d = 3000 + 176 = 3176$$

3. Load calculation

$$\text{Self-weight} = 25 \times d = 25 \times 162 = 4.05 \text{ KN/m}$$

Live load = 3 KN/m

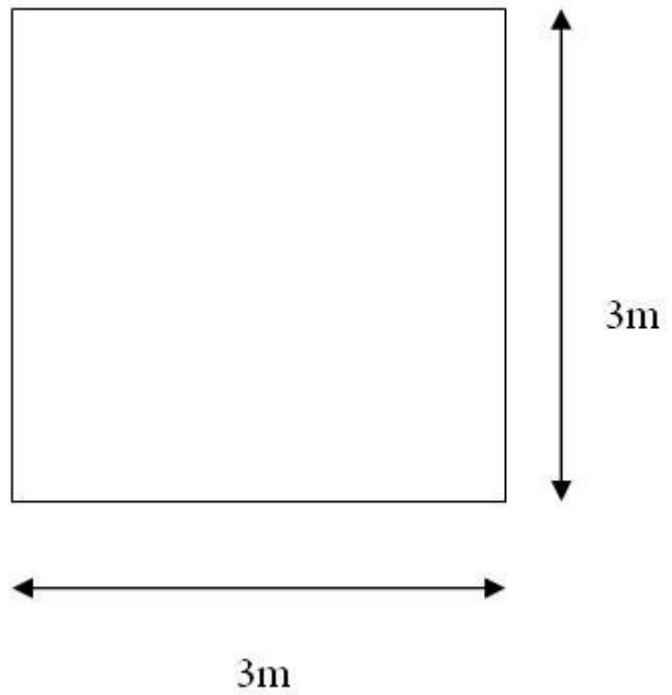
Floor finish = 1 KN/m

Total load = 8.05 KN/m

Reduced 50% =

$50/100 \times 8.05 = 4.025 \text{ KN/m}$

Wud = factored load = $4.025 \times 1.5 = 6.037 \text{ KN/m}$



4. Factored moment

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$$\alpha_x = 0.056$$

$$\alpha_y = 0.056$$

$$M_{ux} = M_{uy} = \alpha_x W_{ud} L_x^2 = 0.056 \times 6.037 \times 3.176^2 = 3.38 \text{ KN.m}$$

5. Check for depth

$$M_{lim} = 0.138 F_{ck} b d^2$$

$$3.38 \times 10^6 = 0.138 \times 20 \times 1000 \times d^2$$

$$d = 35 \text{ mm} < 176 \text{ mm} \dots \text{ok}$$

6. R/F in X-direction and Y-direction

$$A_{st \text{ min}} = 0.12 \% A_g = 0.12/100 \times b \times D$$

$$= 0.12/100 \times 1000 \times 200 = 240 \text{ mm}^2$$

$$A_{st} = (0.5 f_{ck} / f_y) \times [1 - \sqrt{(1 - 4.6 \times M_u) / (f_{ck} b d^2)}] b d$$

$$= (0.5 \times 20 / 415) \times [1 - \sqrt{(1 - 4.6 \times 3.38 \times 10^6) / (20 \times 1000 \times 162^2)}]$$

$$1000 \times 162$$

$$= 58.25 \text{ mm}^2 < 240 \text{ mm}^2$$

$$\text{Provide } A_{st} = 240 \text{ mm}^2$$

$$A_{st \text{ for 1 side}} = 240 / 2 = 120 \text{ mm}^2$$

$$\text{Spacing} = 1000 \times A_{\phi} / A_{st} = (1000 \times \pi / 4 \times 8^2) / 120 = 418.87 \text{ mm} \approx 300 \text{ mm}$$

7. Check for deflection

$$\%Pt = 100 A_{st} / b d$$

$$= (100 \times 240) / (1000 \times 162)$$

$$= 0.148 \% < 0.35 \%$$

8. Check for development

$$L_d = 47 \phi = 47 \times 8 = 376 \text{ mm}$$

7.0 ESTIMATION OF SLABS: –

7.1. CONVENTIONAL SLAB:-

ESTIMATION OF CONVENTIONAL SLAB

1. STEEL

Length of bent up bar

= $L - 2 \times \text{end cover} + 2 \times \text{hooks} + \text{one depth}$

= $3230 - 2 \times 16 \times 8 + 160$

= 3.606 m

Weight of bar per m = $\phi^2/162 = 8^2/162 = 0.395$ kg

Number of bars @ x-direction = 15 NOS.

Number of bars @ Y-direction = 15 NOS.

Total weight of steel in 3×3 slab = weight of bars in x-direction + weight of bars in y-direction

= $(0.395 \times 15 \times 3.606) + (0.395 \times 15 \times 3.606) = 21.36 + 21.36 = 42.74$ kg

Cost of steel (TATA STEEL)

cost per kg = Rs. 44/-

= 42.74×44

= Rs. 1879.92/-

2. Mix proportion M20 (1:1.78:2.15)

Volume of concrete = $3 \times 3 \times 0.2$

= 1.8 m³

3. Cement

Volume of cement = $[(1.52 \times 1.8) / (1 + 1.78 + 2.15)] \times 1$

= 0.554 m³

Number of bags = $0.554 \times 30 = 16$ NOS.

Cost of cement = $300 \times 16 = \text{Rs. } 4800/-$

4. Sand

Volume of sand = $0.554 \times 1.78 = 0.986$ m³

i.e. $0.986 / 2.83 = 0.348$ brass

Cost of sand

(0.5 brass = Rs. 3200/-)

For 0.348 brass = Rs. 2227.2/-

5. Aggregate

Volume of aggregate = $0.554 \times 2.15 = 1.191$ m³

i.e. $1.191 / 2.83 = 0.42$ brass

Cost of aggregate

(1 brass = Rs. 2300/-)

For 0.42 brass = Rs. 966/-

6. Other cost

Head mistry 1 = Rs. 1000/-

Mistry 1 = 750/-

Mixer operator 1 = 400/-

Vibrator operator 1 = 400/-

Labour 6= 1800/-
Water charges =1000/-
Tools and plants =1300/-
Total=6650+850(extra)
=7000/-

7.summary

Steel =1880/-

Cement =4800/-

Sand =2228/-

Aggregate =966/-

Other = 7000/-

total =16877+300=17177/-

Contractor profit=10%x17177=1717.7

Total cost for 3x3x0.2 m slab is 17177+1717.7=18894 =approx 19000/-

7.2.BUBBLE DECK SLAB: –

ESTIMATION OF BUBBLE DECK SLAB

1.STEEL

Length of bent up bar

= L -2xend cover + 2hooks+ one depth
=3230-2x16x8+160
=3.606 m

Weight of bar per m= $\phi^2/162 = 8^2/162=0.395$ kg

Bottom: –

Number of bars @ x-direction=10 NOS.

Number of bars @ Y-direction=10 NOS.

Top:-

Number of bars @ x-direction=10 NOS.

Number of bars @ Y-direction=10 NOS.

Wt. Of bar per meter = 0.395 kg.

Top= $3.395 \times 10 \times 3.606 = 14.243$ kg.....x-direction

= $3.395 \times 10 \times 3.606 = 14.243$ kg.....y-direction

Bottom= $3.395 \times 10 \times 3.606 = 14.243$x-direction

= $3.395 \times 10 \times 3.606 = 14.243$ kg.....y-direction

Total steel=56.974 kg

Cost per kg Rs 44 /-
= 56.974×44
=Rs 2506.89/-

2.Mix proportion M20 (1:1.78:2.15)

Volume of concrete = $3 \times 3 \times 0.2 - (5.235 \times 10^{-4} \times 841)$
= $1.8 - 0.440$
= 1.359 m^3

3.Cement

Volume of cement = $[(1.52 \times 1.359) / (1 + 1.78 + 2.15)] \times 1$
= 0.419 m^3

Number of bags = $0.419 \times 30 = 12.57$ = approx 13NOS.

Cost of cement = $300 \times 13 = \text{Rs.}3900/-$

4. Sand

Volume of sand = $0.419 \times 1.78 = 0.265 \text{ m}^3$

i.e. $0.265 / 2.83 = 0.093$ brass

Cost of sand

(0.5 brass = Rs.3200/-)

For 0.093 brass = Rs.595/-

5. Aggregate

Volume of aggregate = $0.419 \times 2.15 = 0.90 \text{ m}^3$

i.e. $0.90 / 2.83 = 0.318$ brass

Cost of aggregate

(1 brass = Rs.2300/-)

For 0.318 brass = Rs.731/-

6. Other cost

Mistry 1 = 750/-

Mixer operator 1 = 400/-

Vibrator operator 1 = 400/-

Labour 4 = 1200/-

Water charges = 1000/-

Tools and plants = 1300/-

Crane charges = 500/-

Total = 5550/-

= 5550/-

7.Summary

Steel = 2506/-

Cement = 3900/-

Sand = 595/-

Aggregate = 731/-

Other = 5550/-

HDPE balls=1800/-(900 balls).....RS.2/- each
total =13281 + 300+1800 =15381 = approx 15400/-
Contractor profit=10%x15400=1540/-

Total cost for 3x3x0.2 m slab is 15400+1540=16940/-

7.3. COST COMPARISON:-

(TABLE NO. 4)

Sr. no	Description	Cost in Rs.	
		Conventional slab	Bubble Deck slab
1.	Steel	1880/-	2506/-
2.	cement	4800/-	3900/-
3.	sand	2228/-	595/-
4.	Aggregate	966/-	731/-
5.	HDPE Balls	–	1800/-
6.	Other	7000	5550/-
	Total	17177/-	15400/-
	Contractor profit 10%	1717.7/-	1540/-
	Grand Total	19000/-	16940/-
	Net Profit	(19000-16940)/19000×100=10.84%	

Table 4: Cost comparison between Conventional slab and Bubble Deck slab

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9.0 CONCLUSION: –

As per our test we conclude that all results satisfy the requirement of IS 10262-2009. Our Mix design satisfies the IS requirement. Hence we can proceed further for casting slabs. From the results, we can conclude that the cost of Bubble Deck is less as compared to conventional slabs. Hence, it is suitable for construction in terms of cost, quality and maintenance.

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