

**Indian Institute of Technology-Guwahati**

**Department of Civil Engineering**



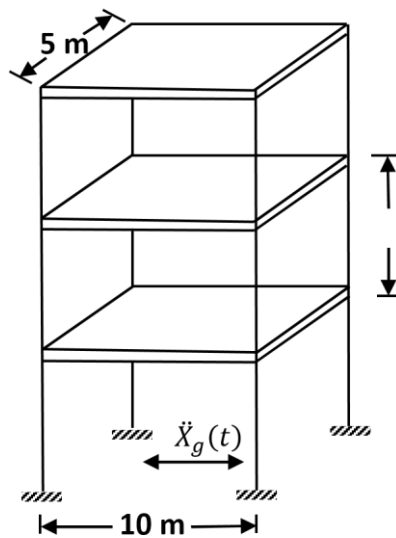
**Course No. CE-607**

**Random Vibrations Assignment-1**

ROHAN DHARMADHIKARI

154104002

## QUESTION 1



### Inter-Storey Drift:

The given system was analysed in the frequency domain. The standard deviation for the displacements at each floor level and the inter-storey drift were found. Taking the maximum of the two inter-storey s.d., the probability of failure was found.

The Moment of inertia of the columns was adjusted in iterations till the probability of failure was just below 0.001. That M.I. was considered as design M.I. So a column with M.I. greater than or equal to the design M.I. can be used.

Design M.I. =  $0.001148 \text{ m}^4$

The standard deviation of the deflection at each floor level: (Mean = 0)

$\sigma_1 = 1.4837 \text{ mm}$ ;  $\sigma_2 = 2.6638 \text{ mm}$ ;  $\sigma_3 = 3.3162 \text{ mm}$ ;

The standard deviation for the inter-storey drift: (Mean = 0)

$\sigma_{12} = 3.0491 \text{ mm}$ ;  $\sigma_{23} = 4.2536 \text{ mm}$

The probability of failure:

$pf = 0.00099705$

Now, let's provide a column of dimensions 350mm x 350mm,

M.I. <sub>provided</sub> =  $0.00125$

The standard deviation of the deflection at each floor level: (Mean = 0)

$\sigma_1 = 1.2388 \text{ mm}$ ;  $\sigma_2 = 2.2219 \text{ mm}$ ;  $\sigma_3 = 3.3162 \text{ mm}$ ;

The standard deviation for the inter-storey drift: (Mean = 0)

$$\sigma_{12} = 2.5439 \text{ mm}; \sigma_{23} = 3.5469 \text{ mm}$$

The probability of failure:

$$pf = 7.908 \times 10^{-5} \quad \dots \text{ safe.}$$

### Base Shear:

Input ground acceleration is a zero-mean gaussian with Kanai-Tajimi spectrum. The forces at each level can be determined by the expressions:

$$F = M \cdot I \cdot x_{ag}$$

Therefore, the PSDF of the forces are:

$$S_{ff} = M \cdot I \cdot x_{ag} \cdot x_{ag} \cdot I \cdot M$$

The base shear force is equal to sum of all forces at all levels. The standard deviation of base shear,  $\sigma_F = 92.8914 \text{ N}$ .

Allowable Shear Force at the base,  $F_{all} = 46.4 \text{ kN}$ .

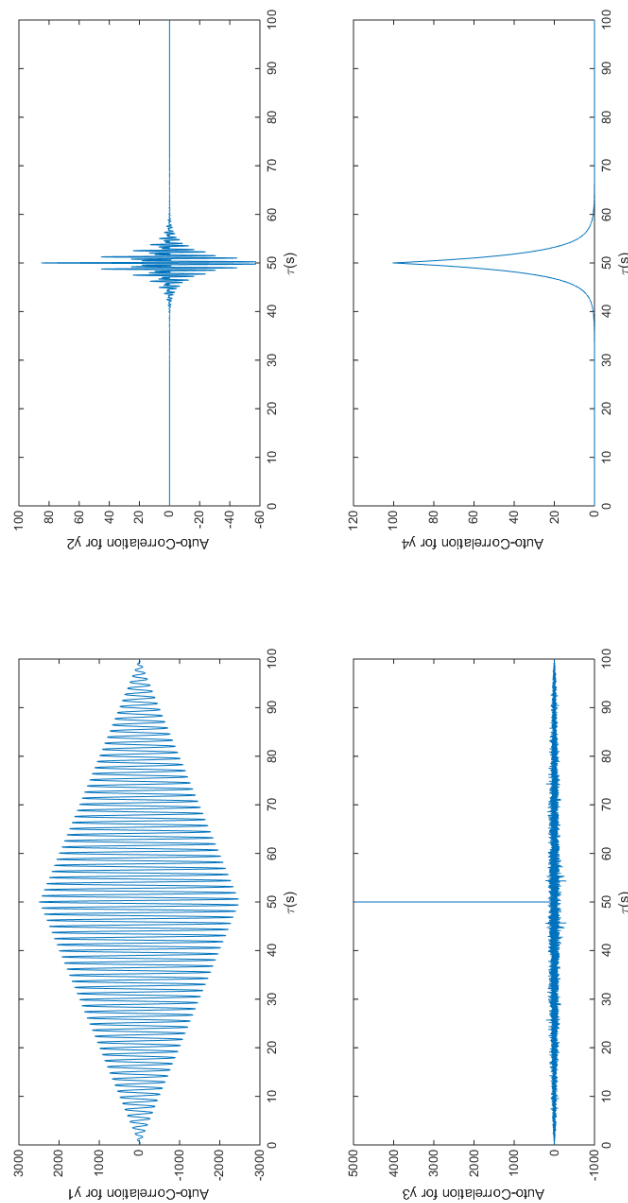
Hence, probability of failure,  $pf = 0$ .

## QUESTION 2

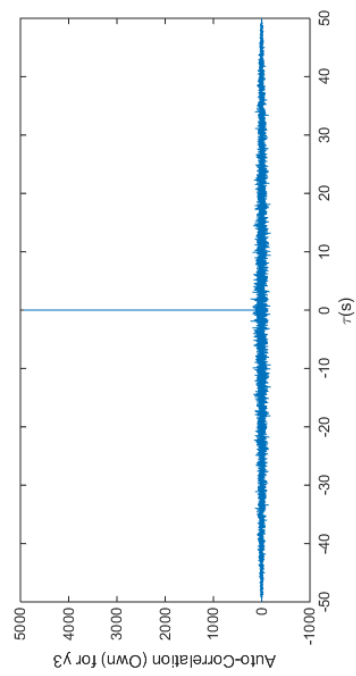
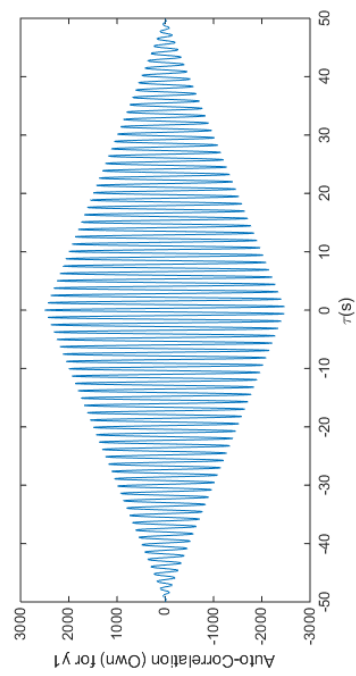
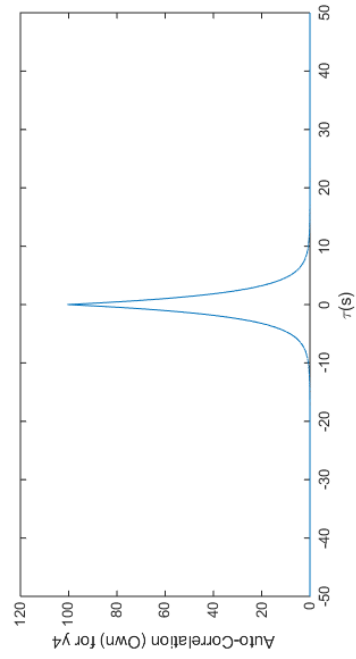
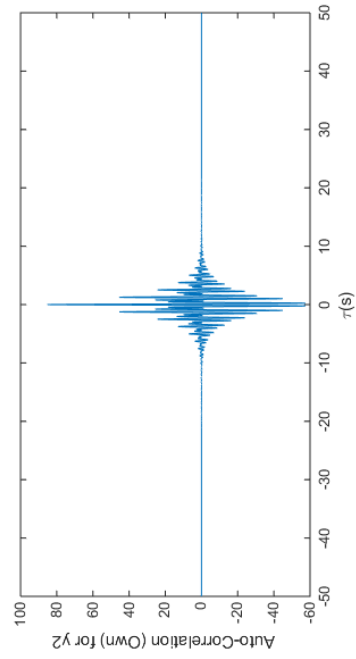
For the given functions, the plots of auto-correlation:

1.  $y(t) = \cos(5t)$
2.  $y(t) = e^{-0.5t}(\sin(10t) + \cos(15t))$
3.  $y(t) = \text{Band Limited White Noise}$
4.  $y(t) = e^{-0.5t}$

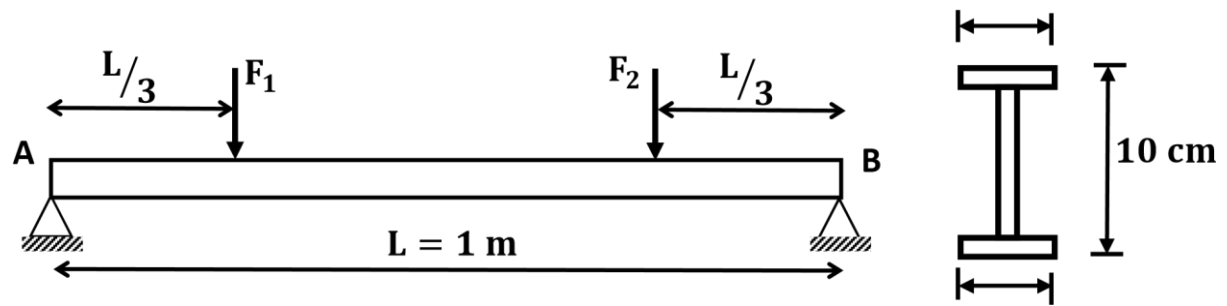
Autocorrelation using in-built function:



Autocorrelation using own function:



### QUESTION 3



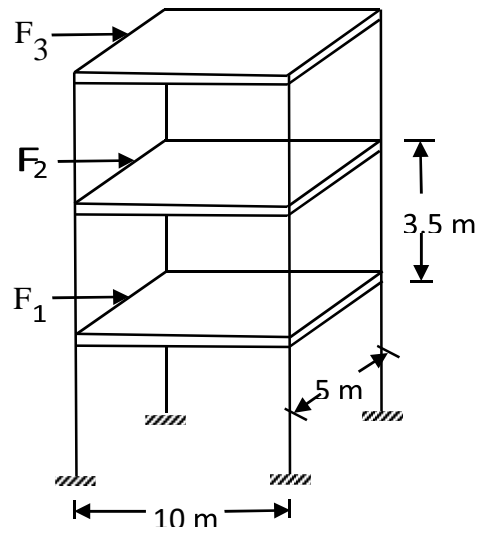
Using Finite-Element Method, the beam was divided in 3 parts. Then, the Mass and Stiffness matrices were generated for the whole beam. Using these matrices, the natural frequencies and the mode shapes were determined.

Using the influence factor as 1 at node 3 in direction of  $F_1$  and 0.75 at node 5 in same direction. and taken the intensity of PSDF of both forces as 1, we get the PSDF of the displacement. The value of  $S_{xx}$  is actually the coefficient of  $I_0$  if the intensity was  $I_0$ .

Thus, using the pf given, we find the  $\sigma_x$  required and dividing that by the  $S_{xx}$  calculated, we get the value of intensity as:

$$I_0 = 0.0158$$

#### QUESTION 4



The system is modelled in SAP2000. The slabs are assigned dead weight and not the columns. Using Modal Analysis, the Mass and Stiffness matrices are obtained. Then using the same technique as in question 1, the  $S_{xx}$  is obtained. And the technique used in question 3 is used to find the required intensity.

$$I_0 = 15.2644 \text{ N}$$