

Discrete Mechanical Vibrations SM32

Sino-European Institute of Aviation Engineering

December 2015

Alain Berlioz 2015-16

Professor: Alain BERLIOZ
alain.berlioz@univ-tlse3.fr

1

Contents:

✓ **Single Degree of Freedom Systems**

✓ **Two Degree of Freedom Systems**

✓ **N Degree of Freedom Systems**

Discretization of continuous Systems

Alain Berlioz 2015-16

December 2015

Discretization of systems having an infinite number of degree of freedom.

Contents:

Flexural Motion of a beam

Derivation of equation of motion

Frequencies and Mode Shapes

Application

Beam in bending

Orthogonality relations

Modal Masses and modal stiffness matrices

Approximate Method

Beam in bending

December 2015

Alain Berlioz 2015-16

Flexural Motion of a beam

Hypotheses are based on theory of strength of materials

The motion is defined by:

V	lateral deflection	m
ψ	slope of neutral axis	rd
T	lateral shear force	N
M	flexural moment	mN

and mechanical or geometrical properties

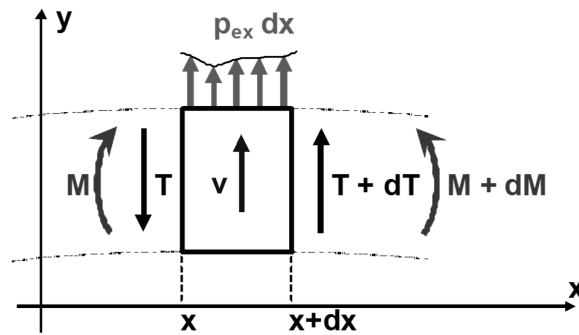
E	Young's modulus	N/m ²
I	area moment of inertia of beam cross-section about the neutral axis	m ⁴
ρ	density	kg/m ³
S	area	m ²
L	length	m

and external force

p_{ex}	lateral external force per unit length	N/m
----------------------------	--	-----

Alain Berlioz 2015-16

Derivation of equation of motion



The application of Newton's laws to the element depicted above in the lateral y-direction and about the z-direction gives

About y axis:

$$\left\{ \begin{aligned} \rho S(x) dx \frac{\partial^2 v(x,t)}{\partial t^2} &= -\cancel{T(x,t)} + \cancel{T(x,t)} + \frac{\partial T(x,t)}{\partial x} dx + p_{ex} dx \\ \rho S(x) \frac{\partial^2 v(x,t)}{\partial t^2} &= \frac{\partial T(x,t)}{\partial x} + p_{ex} \end{aligned} \right.$$

5

Around z axis:

$$\left\{ \begin{aligned} 0 &= -\cancel{M(x,t)} + \cancel{M(x,t)} + \frac{\partial M(x,t)}{\partial x} dx + T(x,t) dx + p_{ex} \frac{dx^2}{2} \end{aligned} \right.$$

From the theory of strength of materials, the relations among **T**, **M** are

$$0 = \frac{\partial M(x,t)}{\partial x} + T(x,t) \qquad \frac{\partial M(x,t)}{\partial x} = -T(x,t)$$

and

$$M(x,t) = EI(x) \frac{\partial^2 v(x,t)}{\partial x^2}$$

which simplify to:

$$\boxed{\rho S(x) \frac{\partial^2 v(x,t)}{\partial t^2} + \frac{\partial^2}{\partial x^2} \left(EI(x) \frac{\partial^2 v(x,t)}{\partial x^2} \right) = p_{ex}}$$

$$\boxed{\rho S \frac{\partial^2 v}{\partial t^2} + EI \frac{\partial^4 v}{\partial x^4} = 0}$$

This approach, in which some second order effects are ignored is called the Bernoulli-Euler approach. So, the classical obtained partial differential equation of a beam in motion of bending is called the **Bernoulli-Euler beam equation**.

6

Frequencies and Mode Shapes

Homogeneous previous equations are considered. The free-vibration solutions to these equations will be obtained by the method of separation of variables.

$$v(x,t) = \phi(x) f(t)$$

and substitute into EOM

$$EI \frac{d^4 \phi(x)}{dx^4} f(t) + \rho S \phi(x) \frac{d^2 f(t)}{dt^2} = 0$$

After some manipulations, this gives

$$\frac{EI}{\rho S} \frac{1}{\phi(x)} \frac{d^4 \phi(x)}{dx^4} = - \frac{1}{f(t)} \frac{d^2 f(t)}{dt^2} = C^{te}$$

which can be separated into two ordinary differential equations of motion, one in space and one in time:

$$\frac{d^2 f(t)}{dt^2} + C^{te} f(t) = 0 \quad \text{time equation}$$

$$\frac{d^4 \phi(x)}{dx^4} - C^{te} \frac{\rho S}{EI} \phi(x) = 0 \quad \text{space equation}$$

7

Alain Berlioz 2015-16

Frequencies and Mode Shapes

Time function

$$\frac{d^2 f(t)}{dt^2} + C^{te} f(t) = 0$$

The separation constant has been set equal to $+\omega^2$ so that the solutions will be bounded in time. It follows that

$$\frac{d^2 f(t)}{dt^2} + \omega^2 f(t) = 0$$

Time solution is:

$$\| f(t) = A \sin \omega t + B \cos \omega t$$

8

Alain Berlioz 2015-16

Frequencies and Mode Shapes

Space function

$$\frac{d^4 \phi(x)}{dx^4} - \omega^2 \frac{\rho S}{EI} \phi(x) = 0$$

Solutions are sought in the form e^{rx} resulting in the characteristic equations:

$$r^4 - \omega^2 \frac{\rho S}{EI} = 0$$

Roots are

$$r = \beta, -\beta, j\beta, -j\beta$$

with

$$\beta = \sqrt[4]{\frac{\rho S \omega^2}{EI}}$$

then, a typical solution is:

$$\phi(x) = \dots e^{+\beta x} + \dots e^{-\beta x} + \dots e^{+j\beta x} + \dots e^{-j\beta x}$$

9

Alain Berlioz 2015-16

Frequencies and Mode Shapes

Trigonometric form is more suitable, so for each value of β , one has a solution of the form:

$$\phi(x) = C \sin \beta x + D \cos \beta x + E \operatorname{sh} \beta x + F \operatorname{ch} \beta x$$

The frequencies ω associated with each β are determined by application of the boundary conditions (*geometric* or *essential*). The most frequent boundary conditions for beams are:

Free (F):	$M = 0,$	$T = 0$
Clamped (C):	$v = 0,$	$\theta = 0$
Simply-supported (S):	$v = 0,$	$M = 0$

Then for each value of ω one has a solution of the form

$$\begin{aligned} v(x, t) &= \sum_{n=1}^{\infty} \phi_n(x) f_n(t) \\ &= \sum_{n=1}^{\infty} (A_n \sin \omega_n t + B_n \cos \omega_n t) \\ &\quad (C_n \sin \beta_n x + D_n \cos \beta_n x + E_n \operatorname{sh} \beta_n x + F_n \operatorname{ch} \beta_n x) \end{aligned}$$

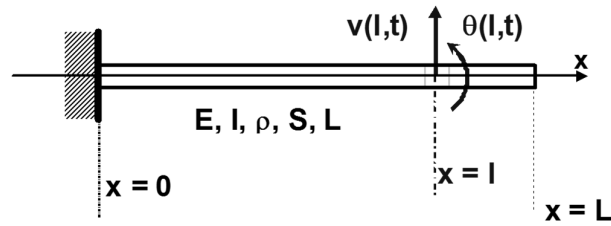
The constants C_n , D_n , E_n et F_n will be determined with boundary conditions. This classical procedure will lead to the frequencies of the beam. A_n and B_n are determined by initial conditions.

10

Alain Berlioz 2015-16

Application:

Beam in bending



$$\phi(x) = C \sin \beta x + D \cos \beta x + E \operatorname{sh} \beta x + F \operatorname{ch} \beta x$$

In that case, (Clamped-Free) boundary conditions are:

x = 0

Lateral displacement $V(0,t) = 0 \Rightarrow 0 + D + 0 + F = 0$

rotation of section (dv/dx) $\theta(0,t) = 0 \Rightarrow C + 0 + E + 0 = 0$

x = L

lateral shear force $\rightarrow (d^3v/dx^3) T(L,t) = 0$

$$\Rightarrow -C \cos \beta L + D \sin \beta L + E \operatorname{ch} \beta L + F \operatorname{sh} \beta L = 0$$

flexural moment $\rightarrow (d^2v/dx^2) M(L,t) = 0$

$$\Rightarrow -C \sin \beta L - D \cos \beta L + E \operatorname{sh} \beta L + F \operatorname{ch} \beta L = 0$$

$$\begin{matrix} \beta \times \\ \beta^2 \times \\ \beta^3 \times \end{matrix} \begin{bmatrix} 0 & 1 & 0 & 1 \\ 1 & 0 & 1 & 0 \\ -\sin \beta L & -\cos \beta L & \operatorname{sh} \beta L & \operatorname{ch} \beta L \\ -\cos \beta L & \sin \beta L & \operatorname{ch} \beta L & \operatorname{sh} \beta L \end{bmatrix} \begin{bmatrix} C \\ D \\ E \\ F \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \\ 0 \end{bmatrix} \begin{matrix} \leftarrow \text{Lateral displacement} \\ \leftarrow \text{Slope of neutral axis} \\ \leftarrow \text{Lateral shear force} \\ \leftarrow \text{Flexural moment} \end{matrix}$$

If C, D, E and F are all zeroes, this is a non acceptable trivial solution. So, the determinant associated with the matrix must be zero. After some manipulations,


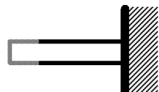

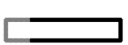
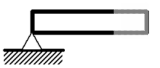
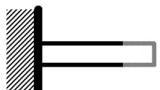
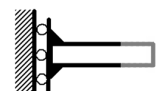
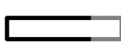
$$1 + \cos \beta L \operatorname{ch} \beta L = 0$$

Numerical solutions can be easily obtained:

βL	1.875	4.692	7.854	10.99	14.14
	3.516	22.03	61.69	120.9	199.8

Note: $\beta = \sqrt[4]{\frac{\rho S \omega^2}{EI}} \Rightarrow \omega = \frac{\beta^2}{L^2} \sqrt{\frac{EI}{\rho S}}$

Using symmetries for boundary conditions, it remains 16 different cases. They are presented in the following table.







				
	$\sin \beta L = 0$	$\text{th } \beta L - \text{tg } \beta L = 0$	$\cos \beta L = 0$	$\text{th } \beta L - \text{tg } \beta L = 0$
		$1 - \cos \beta L \text{ ch } \beta L = 0$	$\text{th } \beta L + \text{tg } \beta L = 0$	$1 + \cos \beta L \text{ ch } \beta L = 0$
			$\sin \beta L = 0$	$\text{th } \beta L + \text{tg } \beta L = 0$
				$1 - \cos \beta L \text{ ch } \beta L = 0$

Note that only 6 equations are obtained for all the cases.

13

The lowest values of X_n^2 are given in the following table

$$\omega_n = \frac{X_n^2}{L^2} \sqrt{\frac{EI}{\rho S}}$$

B.C.		X_1^2	X_2^2	X_3^2	X_4^2	X_5^2
	$\cos \beta L = 0$	2.467	22.21	61.68	120.9	199.9
	$1 + \cos \beta L \text{ ch } \beta L = 0$	3.516	22.03	61.69	120.9	199.8
	$\text{th } \beta L + \text{tg } \beta L = 0$	5.593 0	30.22 5.593	74.63 30.22	138.8 74.63	222.7 138.8
	$\sin \beta L = 0$	9.869 0	39.47 9.869	88.82 39.47	157.9 88.82	246.7 157.9
	$\text{th } \beta L - \text{tg } \beta L = 0$	0 15.41	15.41 49.96	49.96 104.2	104.2 178.2	178.2 272.0
	$1 - \cos \beta L \text{ ch } \beta L = 0$	22.37 0	61.67 22.37	120.9 61.67	199.8 120.9	298.5 199.8

Note: Rigid body modes occurred for GF, GG and FF

14

Orthogonality relations

With the method of separation of variables, the EOM:

$$\rho S \frac{\partial^2 v}{\partial t^2} + \frac{\partial^2}{\partial x^2} (EI \frac{\partial^2 v}{\partial x^2}) = 0$$

becomes:

$$\frac{d^2}{dx^2} (EI \frac{d^2 \phi}{dx^2}) = \rho S \omega^2 \phi$$

This is true for each of the solution pairs: ω_i, ϕ_i and ω_j, ϕ_j (frequencies and mode shapes)

$$\frac{d^2}{dx^2} (EI \frac{d^2 \phi_i}{dx^2}) = \rho S \omega_i^2 \phi_i$$

$$\frac{d^2}{dx^2} (EI \frac{d^2 \phi_j}{dx^2}) = \rho S \omega_j^2 \phi_j$$

15

Multiplying the first equation by ϕ_j

$$\phi_j \frac{d^2}{dx^2} (EI \frac{d^2 \phi_i}{dx^2}) = \phi_j \rho S \omega_i^2 \phi_i$$

and the second by ϕ_i it becomes:

$$\phi_i \frac{d^2}{dx^2} (EI \frac{d^2 \phi_j}{dx^2}) = \phi_i \rho S \omega_j^2 \phi_j$$

That must be verified for the whole beam:

$$\int_0^L \phi_j \frac{d^2}{dx^2} \left(EI \frac{d^2 \phi_i}{dx^2} \right) dx = \int_0^L \phi_j \rho S \omega_i^2 \phi_i dx$$

$$\int_0^L \phi_i \frac{d^2}{dx^2} \left(EI \frac{d^2 \phi_j}{dx^2} \right) dx = \int_0^L \phi_i \rho S \omega_j^2 \phi_j dx$$

16

1st integration by parts:

Putting:

with $u = \phi_j$ and $v = \frac{d}{dx} \left(EI \frac{d^2 \phi_i}{dx^2} \right)$

$u' = \frac{d\phi_j}{dx}$ and $v' = \frac{d^2}{dx^2} \left(EI \frac{d^2 \phi_i}{dx^2} \right)$

$$\int_0^L (u v)' dx = \int_0^L u' v dx + \int_0^L u v' dx$$

integrating by parts leads:

$$\int_0^L (u v)' dx \Leftrightarrow \left[\phi_j \frac{d}{dx} \left(EI \frac{d^2 \phi_i}{dx^2} \right) \right]_0^L$$

So, it becomes:

$$\left[\phi_j \frac{d}{dx} \left(EI \frac{d^2 \phi_i}{dx^2} \right) \right]_0^L = \int_0^L \frac{d\phi_j}{dx} \frac{d}{dx} \left(EI \frac{d^2 \phi_i}{dx^2} \right) dx + \int_0^L \phi_j \frac{d^2}{dx^2} \left(EI \frac{d^2 \phi_i}{dx^2} \right) dx$$

17

So, using the following boundaries at $x = 0$ and $x = L$.

	$\phi(x)f(t)$	$\frac{d^2 \phi(x)}{dx^2} f(t)$
Free	?	0
Supported	0	0
Clamped	0	?

hence

$$\left[\phi_j \frac{d}{dx} \left(EI \frac{d^2 \phi_i}{dx^2} \right) \right]_0^L = 0$$

Finally:

$$0 = \int_0^L \frac{d\phi_j}{dx} \frac{d}{dx} \left(EI \frac{d^2 \phi_i}{dx^2} \right) dx + \int_0^L \phi_j \frac{d^2}{dx^2} \left(EI \frac{d^2 \phi_i}{dx^2} \right) dx$$

or:

$$\int_0^L \phi_j \frac{d^2}{dx^2} \left(EI \frac{d^2 \phi_i}{dx^2} \right) dx = - \int_0^L \frac{d\phi_j}{dx} \frac{d}{dx} \left(EI \frac{d^2 \phi_i}{dx^2} \right) dx$$

18

2nd integration by parts:

Putting:

$$u = \frac{d\phi_j}{dx} \quad \text{and} \quad u' = \frac{d^2\phi_j}{dx^2}$$

$$v = EI \frac{d^2\phi_i}{dx^2} \quad \text{and} \quad v' = \frac{d}{dx} \left(EI \frac{d^2\phi_i}{dx^2} \right)$$

hence

$$\int_0^L (u v)' dx \Leftrightarrow \left[\frac{d\phi_j}{dx} \left(EI \frac{d^2\phi_i}{dx^2} \right) \right]_0^L$$

So, using the following boundaries at $x = 0$ and $x = L$.

	$\frac{d\phi(x)}{dx} f(t)$	$\frac{d^2\phi(x)}{dx^2} f(t)$
Free	?	0
Supported	?	0
Clamped	0	?

19

Substituting, it becomes:

$$- \left[\frac{d\phi_j}{dx} \left(EI \frac{d^2\phi_i}{dx^2} \right) \right]_0^L = 0$$

and

$$0 = \int_0^L \frac{d^2\phi_j}{dx^2} \left(EI \frac{d^2\phi_i}{dx^2} \right) dx + \int_0^L \frac{d\phi_j}{dx} \frac{d}{dx} \left(EI \frac{d^2\phi_i}{dx^2} \right) dx$$

or:

$$\int_0^L \frac{d^2\phi_j}{dx^2} \left(EI \frac{d^2\phi_i}{dx^2} \right) dx = - \int_0^L \frac{d\phi_j}{dx} \frac{d}{dx} \left(EI \frac{d^2\phi_i}{dx^2} \right) dx$$

Finally, combining that result with first equation leads to:

$$\int_0^L \frac{d^2\phi_j}{dx^2} \left(EI \frac{d^2\phi_i}{dx^2} \right) dx = \omega_i^2 \int_0^L \rho S \phi_i \phi_j dx$$

$$\int_0^L \frac{d^2\phi_i}{dx^2} \left(EI \frac{d^2\phi_j}{dx^2} \right) dx = \omega_j^2 \int_0^L \rho S \phi_i \phi_j dx$$

20

Subtracting:

$$0 = (\omega_i^2 - \omega_j^2) \int_0^L \rho S \phi_i \phi_j dx$$

As $\omega_i \neq \omega_j$

$$\int_0^L \rho S \phi_i \phi_j dx = 0$$

and

$$\int_0^L EI \frac{d^2 \phi_i}{dx^2} \frac{d^2 \phi_j}{dx^2} dx = 0$$

Previous equations are the **orthogonality conditions** for a continuous system deforming of a beam in flexion.

21

Modal Masses and modal stiffness matrices

As previously shown:

$$\omega_i^2 = \frac{\int_0^L EI \left(\frac{d^2 \phi_i}{dx^2} \right)^2 dx}{\int_0^L \rho S \phi_i^2 dx} = \frac{k_i}{m_i}$$

with:

$$m_i = \int_0^L \rho S \phi_i^2 dx$$

and

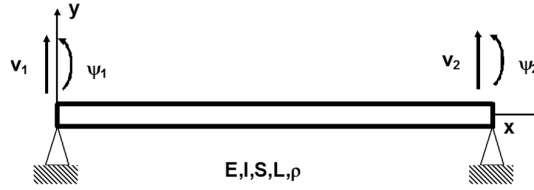
$$k_i = \int_0^L EI \left(\frac{d^2 \phi_i}{dx^2} \right)^2 dx$$

where k_i and m_i are the i^{th} **modal stiffness** and **modal mass** of this continuous system (beam in flexion).

22

Application :

Poutre de section constante sur deux appuis simple en flexion :



On connaît :

$$\phi_i(x) = C_i \sin \frac{i\pi x}{L} \quad \text{et} \quad \omega_i = \frac{(i\pi)^2}{L^2} \sqrt{\frac{EI}{\rho S}} \quad \text{avec } i \neq 0 \text{ positif}$$

On vérifie bien que :

$$\left[\phi_j EI \frac{d}{dx} \frac{d^2 \phi_i}{dx^2} \right]_0^L = - \left[EIC_j C_i \left(\frac{i\pi}{L} \right)^3 \sin \frac{j\pi x}{L} \cos \frac{i\pi x}{L} \right]_0^L = 0$$

$$\left[EI \frac{d\phi_j}{dx} \frac{d^2 \phi_i}{dx^2} \right]_0^L = - \left[EIC_j C_i \left(\frac{\pi}{L} \right)^3 i^2 j \cos \frac{j\pi x}{L} \sin \frac{i\pi x}{L} \right]_0^L = 0$$

23

Raideurs modales ($i = j \neq 0$)

$$\begin{aligned} k_i &= EI \int_0^L \left(\frac{d^2 \phi_i}{dx^2} \right)^2 dx = EIC_i^2 \left(\frac{i\pi}{L} \right)^4 \int_0^L \sin^2 \frac{i\pi x}{L} dx \\ &= C_i^2 EI \left(\frac{i\pi}{L} \right)^4 \frac{L}{2} \end{aligned}$$

soit

$$k_i = C_i^2 \frac{EI(i\pi)^4}{2L^3}$$

Masses modales ($i = j \neq 0$)

$$\begin{aligned} m_i &= \int_0^L \rho S \phi_i^2 dx \\ &= \rho S C_i^2 \int_0^L \sin^2 \frac{i\pi x}{L} dx \end{aligned}$$

soit

$$m_i = C_i^2 \rho S \frac{L}{2}$$

24

Pulsation propre :

$$\begin{aligned}\omega_i^2 &= \frac{\int_0^L EI \left(\frac{d^2 \phi_i}{dx^2} \right)^2 dx}{\int_0^L \rho S \phi_i^2 dx} = \frac{k_i}{m_i} \\ &= \frac{C_i^2 \frac{EI(i\pi)^4}{2L^3}}{C_i^2 \frac{\rho S L}{2}} \\ &= \frac{i^4 \pi^4}{L^4} \frac{EI}{\rho S}\end{aligned}$$

Vérification

$$\omega_i = \sqrt{\frac{k_i}{m_i}} = \frac{i^2 \pi^2}{L^2} \sqrt{\frac{EI}{\rho S}}$$

25

La $i^{\text{ème}}$ équation (découplée) s'écrit :

$$m_i \ddot{q}_i + k_i q_i = 0$$

avec

$$m_i = C_i^2 \rho S \frac{L}{2} \quad \text{et} \quad k_i = C_i^2 \frac{EI(i\pi)^4}{2L^3}$$

Il est possible d'écrire (avec un choix judicieux des constantes) :

pour la 1^{ère} équation,

$$\rho S \frac{L}{2} \ddot{q}_1 + \frac{EI(\pi)^4}{2L^3} q_1 = 0$$

pour la 2^{ème} équation,

$$\rho S \frac{L}{2} \ddot{q}_2 + \frac{EI(2\pi)^4}{2L^3} q_2 = 0$$

et sous forme matricielle pour n équations

$$\rho S \frac{L}{2} \begin{bmatrix} 1 & & 0 \\ & 1 & \\ & & 1 \\ 0 & & \ddots \end{bmatrix} \begin{Bmatrix} \ddot{q}_1 \\ \ddot{q}_2 \\ \ddot{q}_3 \\ \vdots \end{Bmatrix} + \frac{EI(\pi)^4}{2L^3} \begin{bmatrix} 1 & & 0 \\ & 16 & \\ & & 81 \\ 0 & & \ddots \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \\ q_3 \\ \vdots \end{Bmatrix} = 0$$

Rappel :

$$\omega_i = \frac{(i\pi)^2}{L^2} \sqrt{\frac{EI}{\rho S}} \quad \text{pour } i = 1, 2, \dots$$

26