

# Some additional methods

- What if there are more wells?
- What if a well is close to a boundary?
- What if the system is more complex?

- Method of superposition
- Method of images
- Flow net

# Method of superposition

## Principle of the method

For a linear system (linear partial differential equation):

$$\frac{\partial^2 s}{\partial x^2} + \frac{\partial^2 s}{\partial y^2} = 0$$

If  $s_1$  and  $s_2$  are solutions, the linear combination:

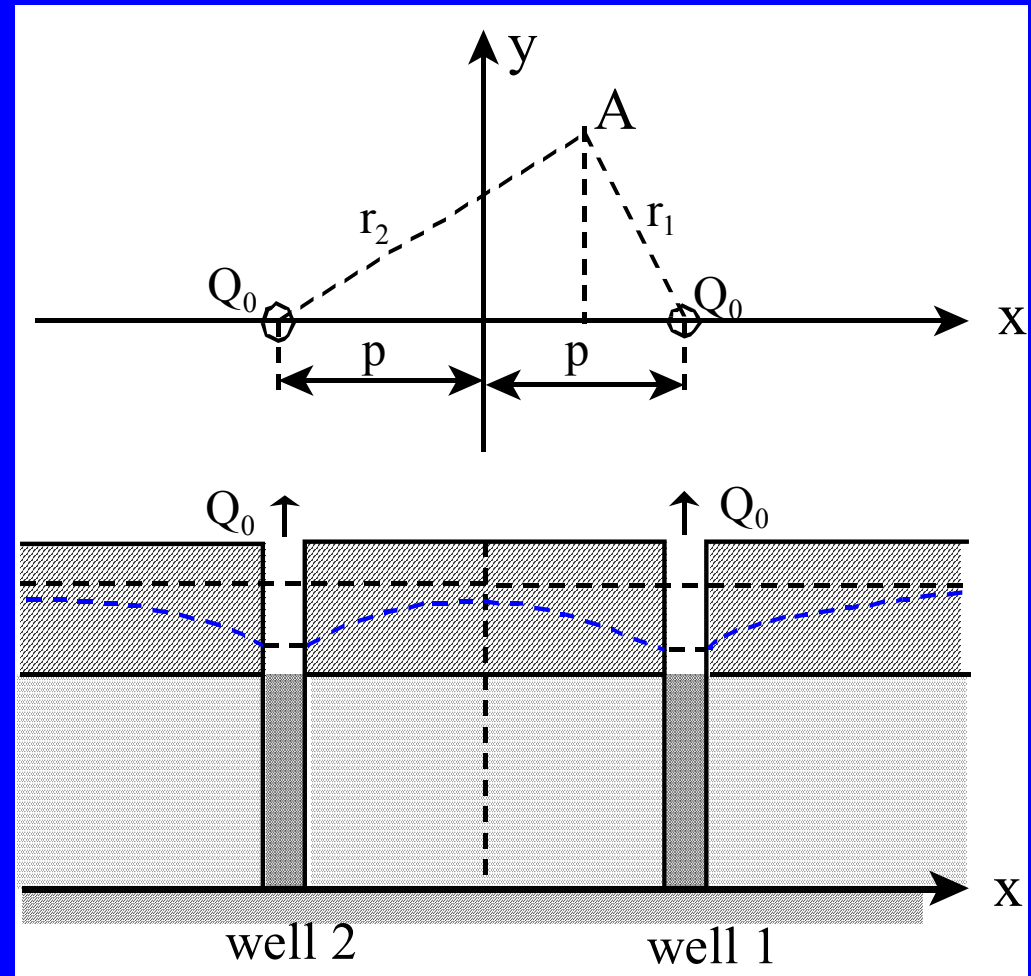
$$s = \alpha s_1 + \beta s_2$$

is also the solution.

# Method of superposition

## Case 1: Two pumping wells in a confined aquifer

- Conceptual hydrogeological model
  - The confined aquifer is bounded with a circular constant head boundary;
  - The aquifer is homogeneous and isotropic;
  - Two pumping wells locate at a distance of  $2p$  with the same pumping rate.



# Method of superposition

## Case 1: Two pumping wells in a confined aquifer

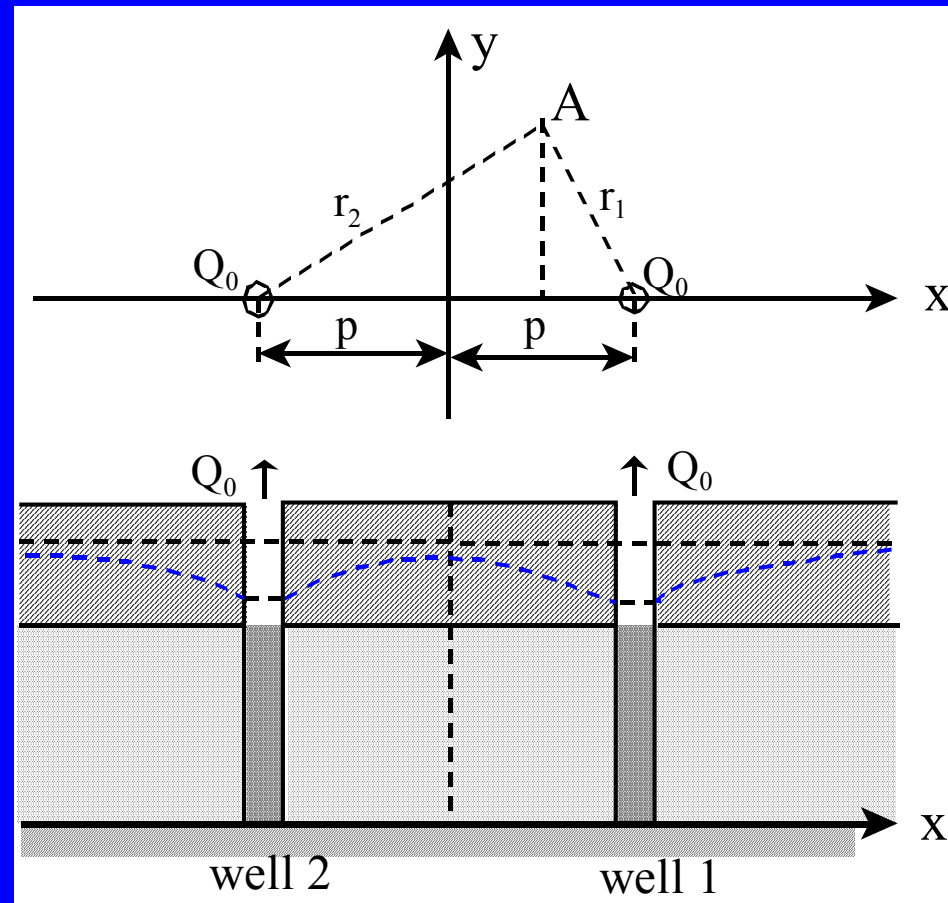
### Drawdown distribution

$$s = \frac{Q_0}{2\pi T} \ln\left(\frac{R}{r}\right)$$

$$r = \sqrt{x^2 + y^2}$$

$$s_1 = \frac{Q_0}{2\pi T} \ln\left(\frac{R}{r_1}\right) \quad r_1 = \sqrt{(x - p)^2 + y^2}$$

$$s_2 = \frac{Q_0}{2\pi T} \ln\left(\frac{R}{r_2}\right) \quad r_2 = \sqrt{(x + p)^2 + y^2}$$



# Method of superposition

Case 1: Two pumping wells in a confined aquifer

Total drawdown caused by two pumping wells

$$s = s_1 + s_2 = \frac{Q_0}{2\pi T} \ln\left(\frac{R^2}{r_1 r_2}\right)$$

$$s = \frac{Q_0}{2\pi T} \ln \left( \frac{R^2}{\sqrt{[(x - p)^2 + y^2][(x + p)^2 + y^2]}} \right)$$

# Method of superposition

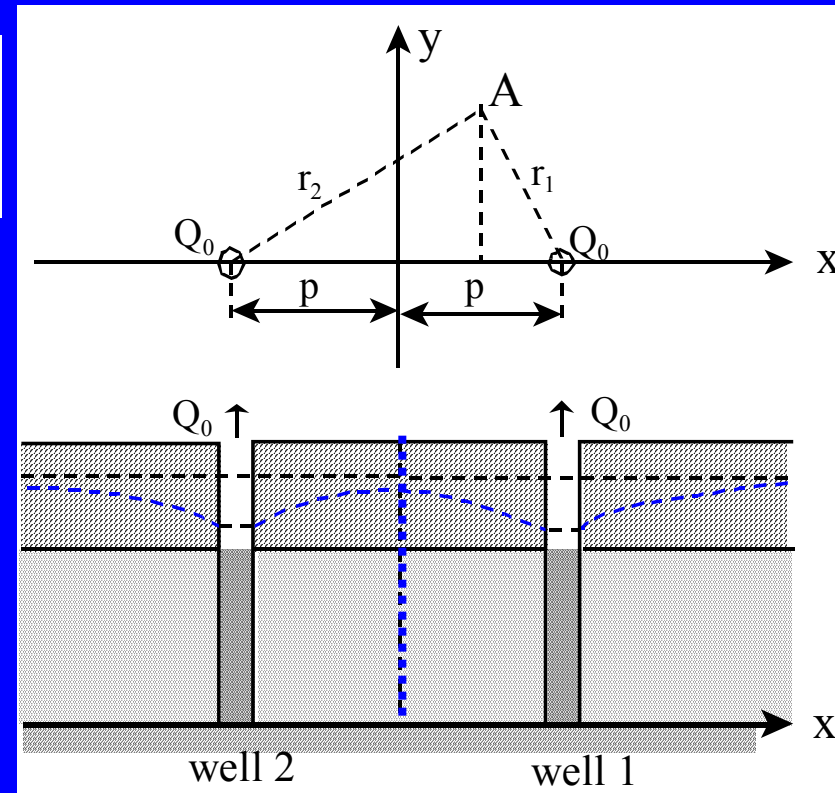
## Case 1: Two pumping wells in a confined aquifer

### Groundwater divide

$$\frac{\partial s}{\partial x} = -\frac{Q_0}{2\pi T} \left[ \frac{x-p}{(x-p)^2 + y^2} + \frac{x+p}{(x+p)^2 + y^2} \right]$$

$$\frac{\partial s}{\partial x} = 0 \text{ when } x=0$$

A groundwater divide exists at the middle of two pumping wells along with the y axis



# Method of superposition

## Case 1: Two pumping wells in a confined aquifer

### Drawdown distribution

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

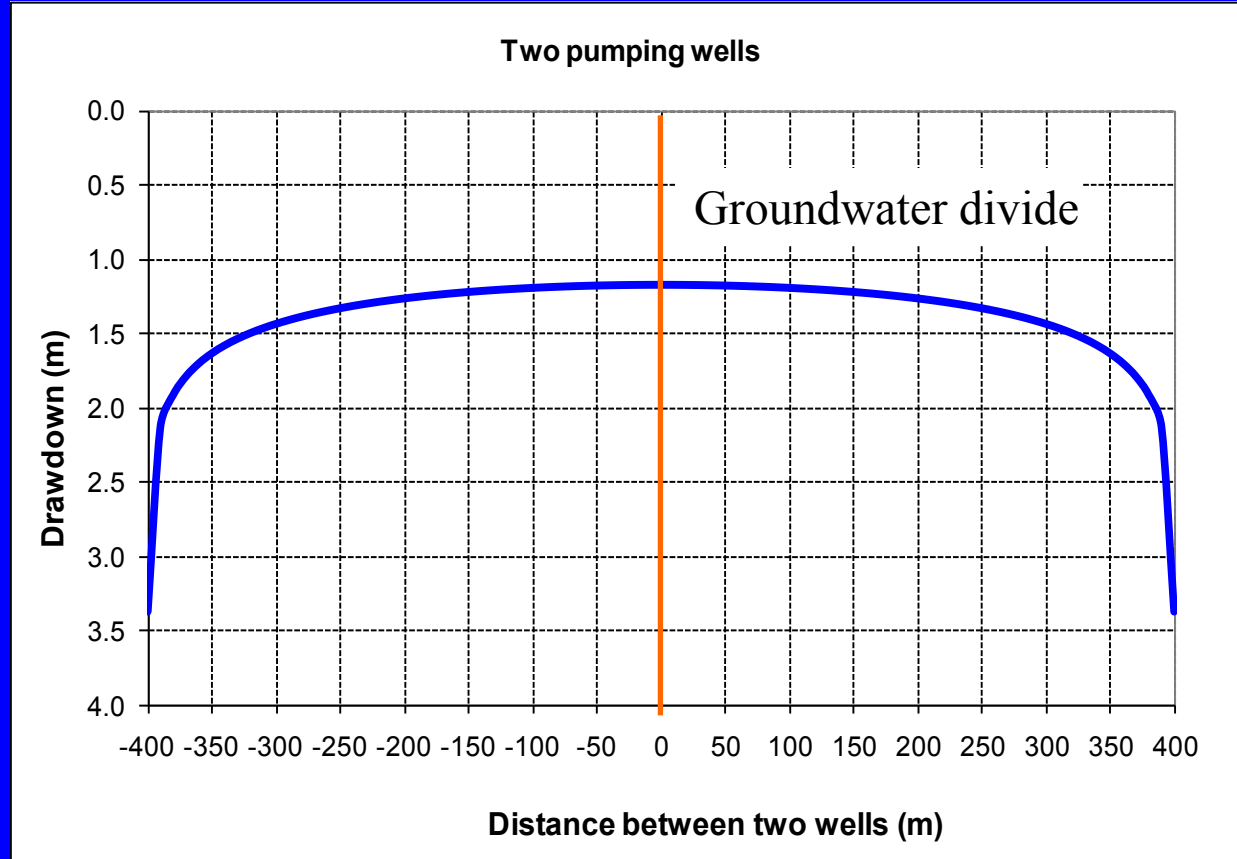
$$K = 50 \text{ m/d}$$

$$H = 50 \text{ m}$$

$$p = 400 \text{ m}$$

$$n = 0.3$$

$$s_{\max} = 3.37 \text{ m}$$



# Method of superposition

## Case 1: Two pumping wells in a confined aquifer

Drawdown  
distribution simulated  
by a numerical model

Example:

$Q_0=5000 \text{ m}^3/\text{d}$

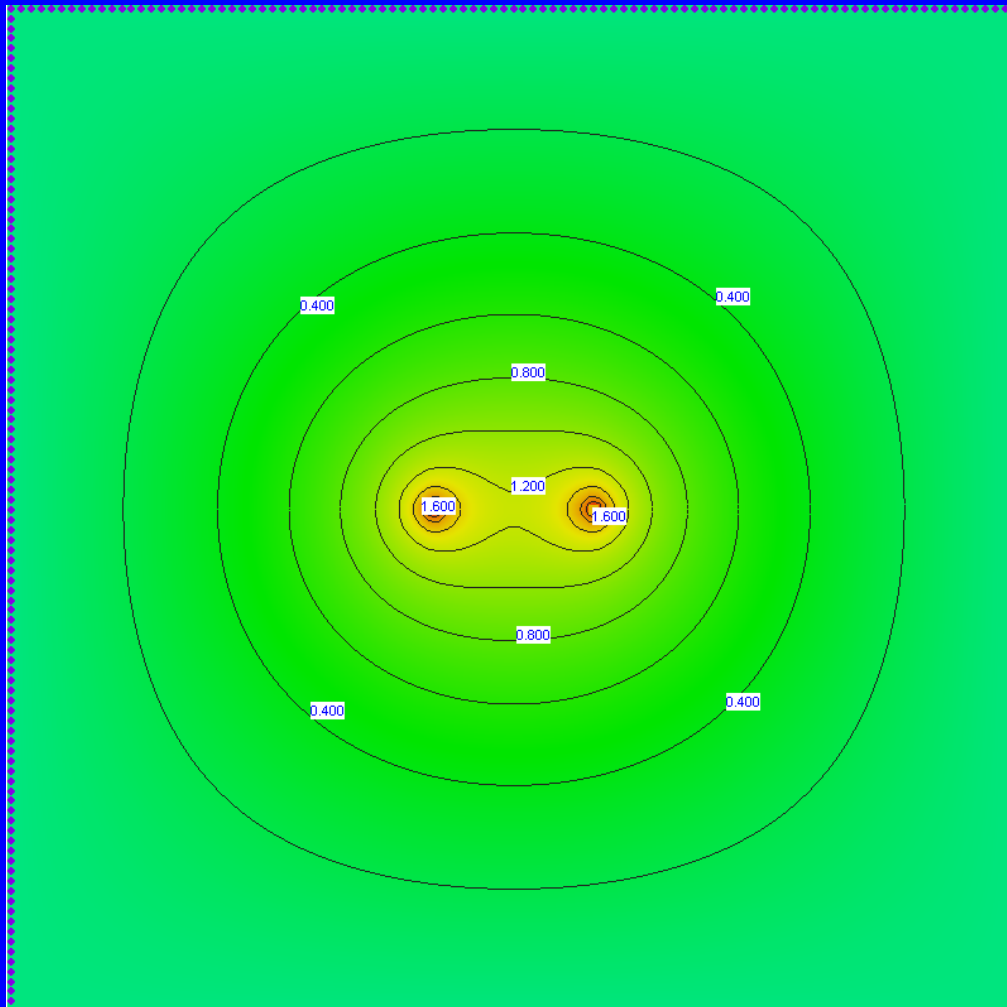
$K=50 \text{ m/d}$

$H=50 \text{ m}$

$p=400 \text{ m}$

$n=0.3$

Side=5050 m





# Method of superposition

## Case 1: Two pumping wells in a confined aquifer

Velocity distribution

$$v_x = \frac{K}{n_e} \frac{\partial s}{\partial x} = - \frac{Q_0}{\pi H n_e} \frac{x}{x^2 - p^2}$$

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

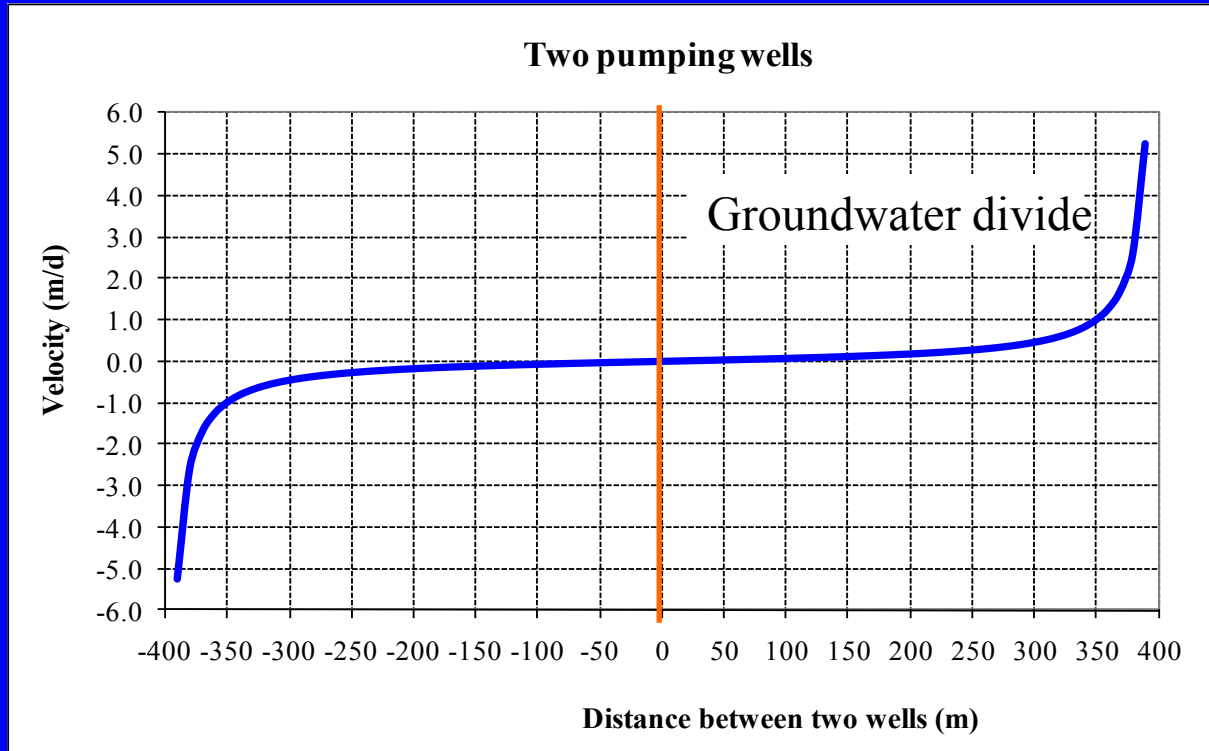
$$K = 50 \text{ m/d}$$

$$H = 50 \text{ m}$$

$$p = 400 \text{ m}$$

$$n = 0.3$$

$$v_{\max} = 265 \text{ m/d}$$



# Method of superposition

Case 1: Two pumping wells in a confined aquifer

➔ on the line that connects 2 wells together

Shortest travel time from the boundary to the pumping well:

$$t = \int_R^p \frac{dx}{v_x} = \frac{\pi H n_e}{Q_0} \int_p^R \frac{x^2 - p^2}{x} dx = \frac{\pi H n_e}{Q_0} \left[ \frac{1}{2} (R^2 - p^2) - p^2 \ln\left(\frac{R}{p}\right) \right]$$

Example:

$Q_0 = 5000 \text{ m}^3/\text{d}$ ,  $K = 50 \text{ m/d}$ ,  $H = 50 \text{ m/d}$ ,  $n = 0.3$

$R = 2525 \text{ m}$ ,  $p = 400 \text{ m}$

$t_{\text{mim}} = 72 \text{ years}$

# Method of superposition

## Case 1: Two pumping wells in a confined aquifer

### Simulated flow field

Example:

$Q_0 = 5000 \text{ m}^3/\text{d}$

$K = 50 \text{ m/d}$

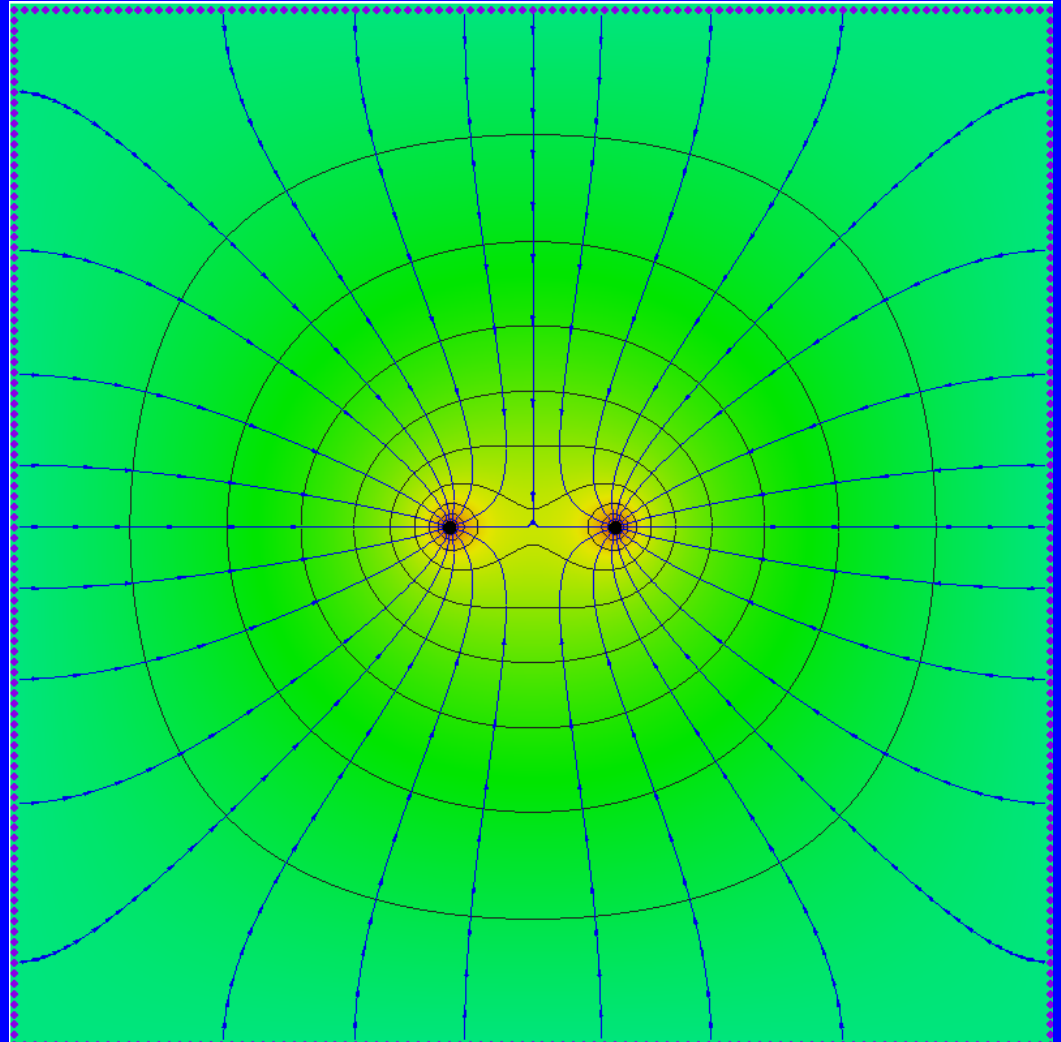
$H = 50 \text{ m}$

Side = 5050 m

$p = 400 \text{ m}$

$n = 0.3$

Travel time between  
two marks is 10 years



# Method of superposition

## Case 1: Two pumping wells in a confined aquifer

### Drawdown near a well

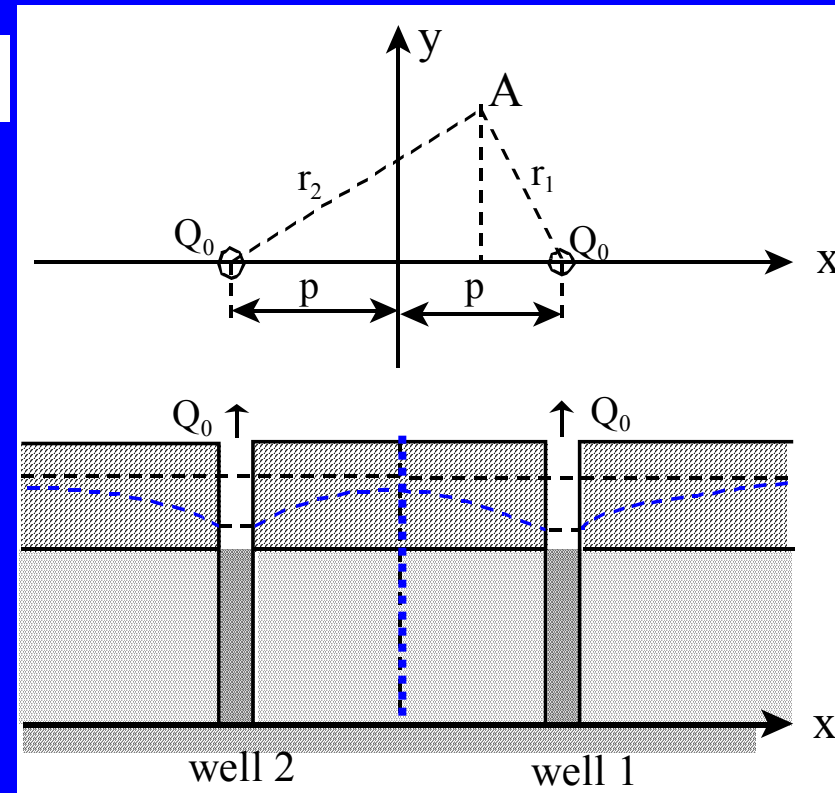
$$x = p + r \cos \theta, \quad y = r \sin \theta, \quad r \ll p$$

$$s = \frac{Q_0}{2\pi T} \ln\left(\frac{R^2/2p}{r}\right)$$

$$s = \frac{Q_0}{2\pi T} \ln\left(\frac{R_{eq}}{r}\right)$$

$$R_{eq} = \frac{R^2}{2p}$$

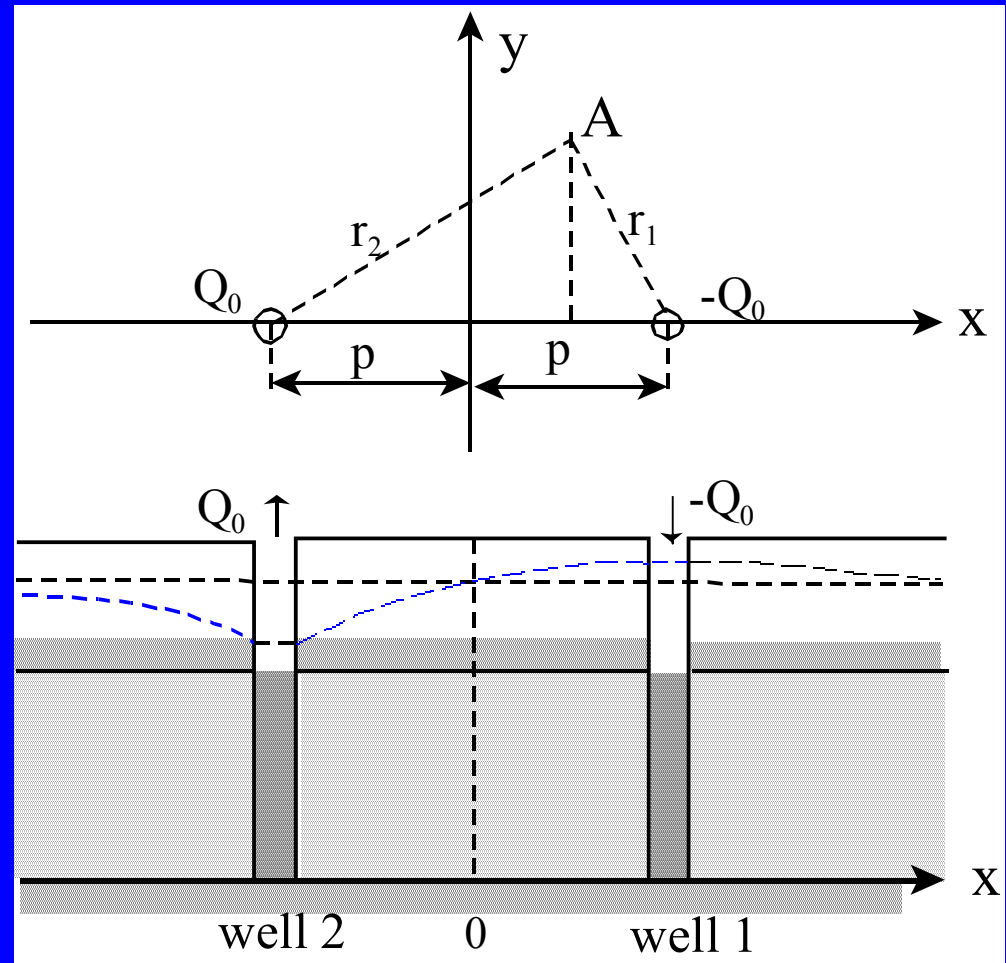
$R_{eq}$  is the equivalent radius of influence



# Method of superposition

## Case 2: One pumping and one injection wells in a confined aquifer

- Conceptual hydrogeological model
  - The confined aquifer is bounded with a circular constant head boundary;
  - The aquifer is homogeneous and isotropic;
  - One pumping well and one injection well operate at a distance of  $2p$  with the same rate.



# Method of superposition

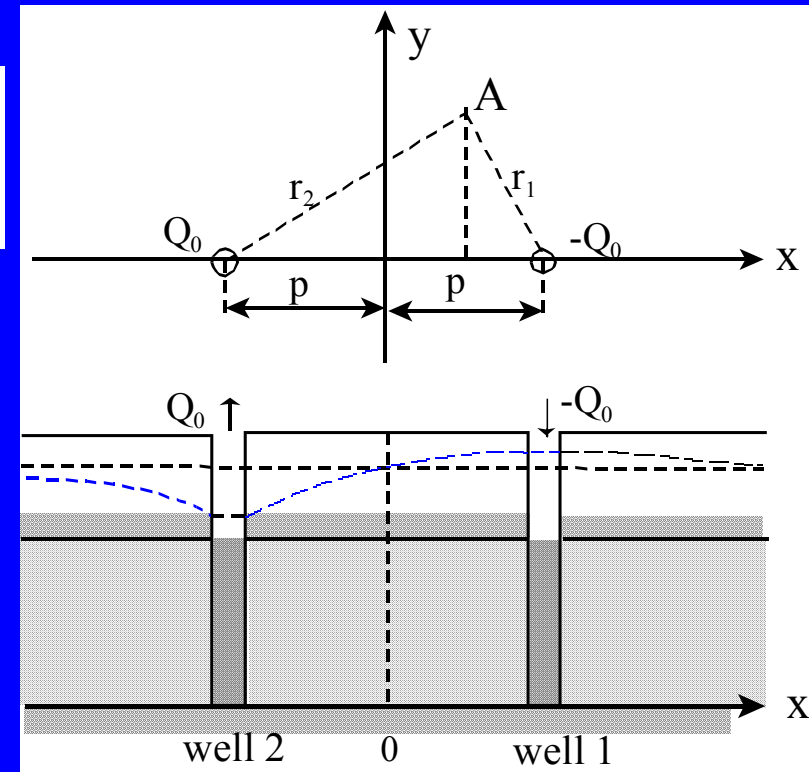
Case 2: One pumping and one injection wells in a confined aquifer

## Drawdown distribution

$$s_1 = -\frac{Q_0}{2\pi T} \ln\left(\frac{R}{r_1}\right) \quad r_1 = \sqrt{(x-p)^2 + y^2}$$

$$s_2 = \frac{Q_0}{2\pi T} \ln\left(\frac{R}{r_2}\right) \quad r_2 = \sqrt{(x+p)^2 + y^2}$$

$$s = s_1 + s_2 = \frac{Q_0}{2\pi T} \ln\left(\sqrt{\frac{(x-p)^2 + y^2}{(x+p)^2 + y^2}}\right)$$



# Method of superposition

Case 2: One pumping and one injection wells in a confined aquifer

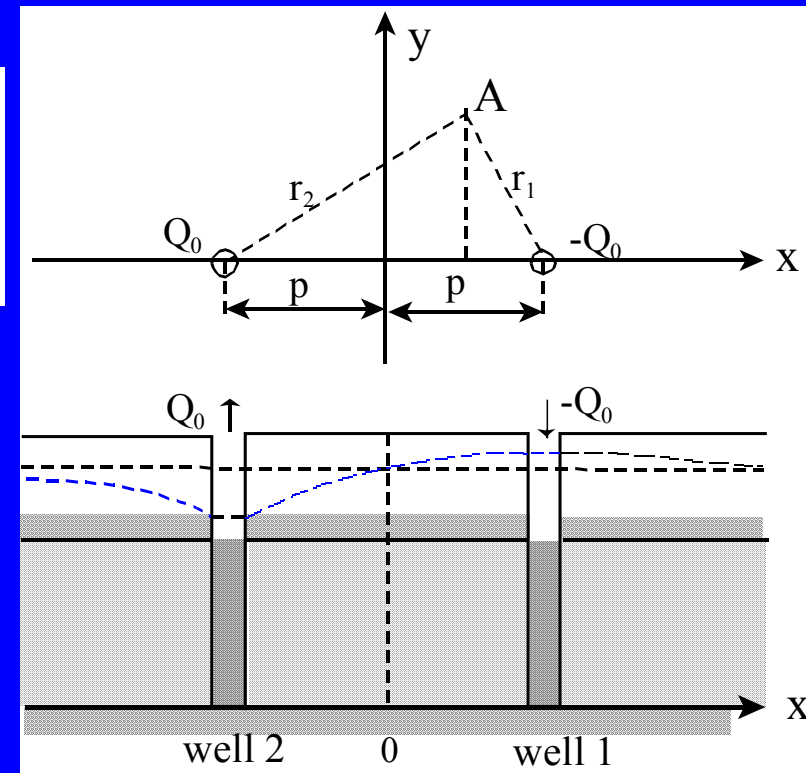
## Drawdown distribution

$$s = s_1 + s_2 = \frac{Q_0}{2\pi T} \ln\left(\sqrt{\frac{(x-p)^2 + y^2}{(x+p)^2 + y^2}}\right)$$

- 1) The drawdown  $s$  is independent of  $R$ ;
- 2) At  $x = 0$ ,  $s = 0$  ;
- 3) Drawdown near the pumping well:

$$s = \frac{Q_0}{2\pi T} \ln\left(\frac{R_{eq}}{r}\right)$$

$$R_{eq} = 2p$$



# Method of superposition

Case 2: One pumping and one injection wells in a confined aquifer

## Drawdown distribution

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

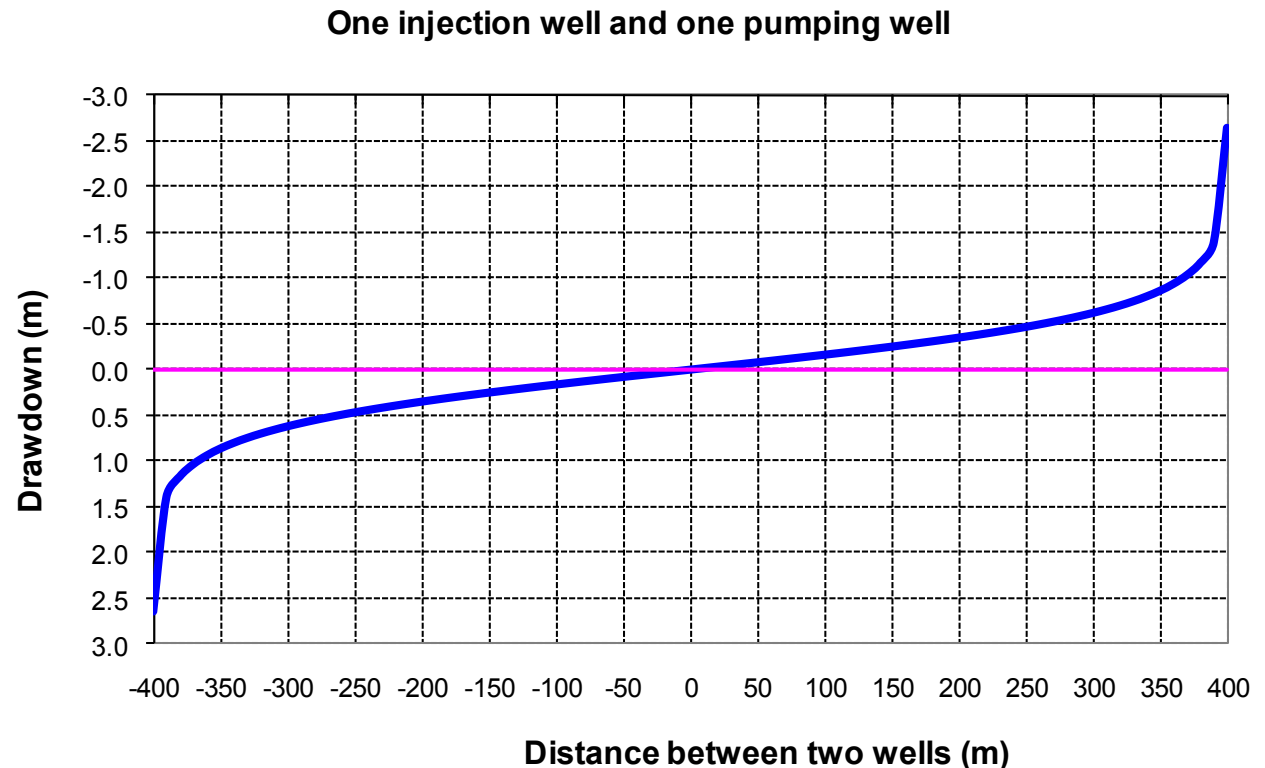
$$K = 50 \text{ m/d}$$

$$H = 50 \text{ m}$$

$$p = 400 \text{ m}$$

$$n = 0.3$$

$$s_{\max} = 2.64 \text{ m}$$





# Method of superposition

Case 2: One pumping and one injection wells in a confined aquifer

Drawdown  
distribution simulated  
by a numerical model

Example:

$Q_0 = 5000 \text{ m}^3/\text{d}$

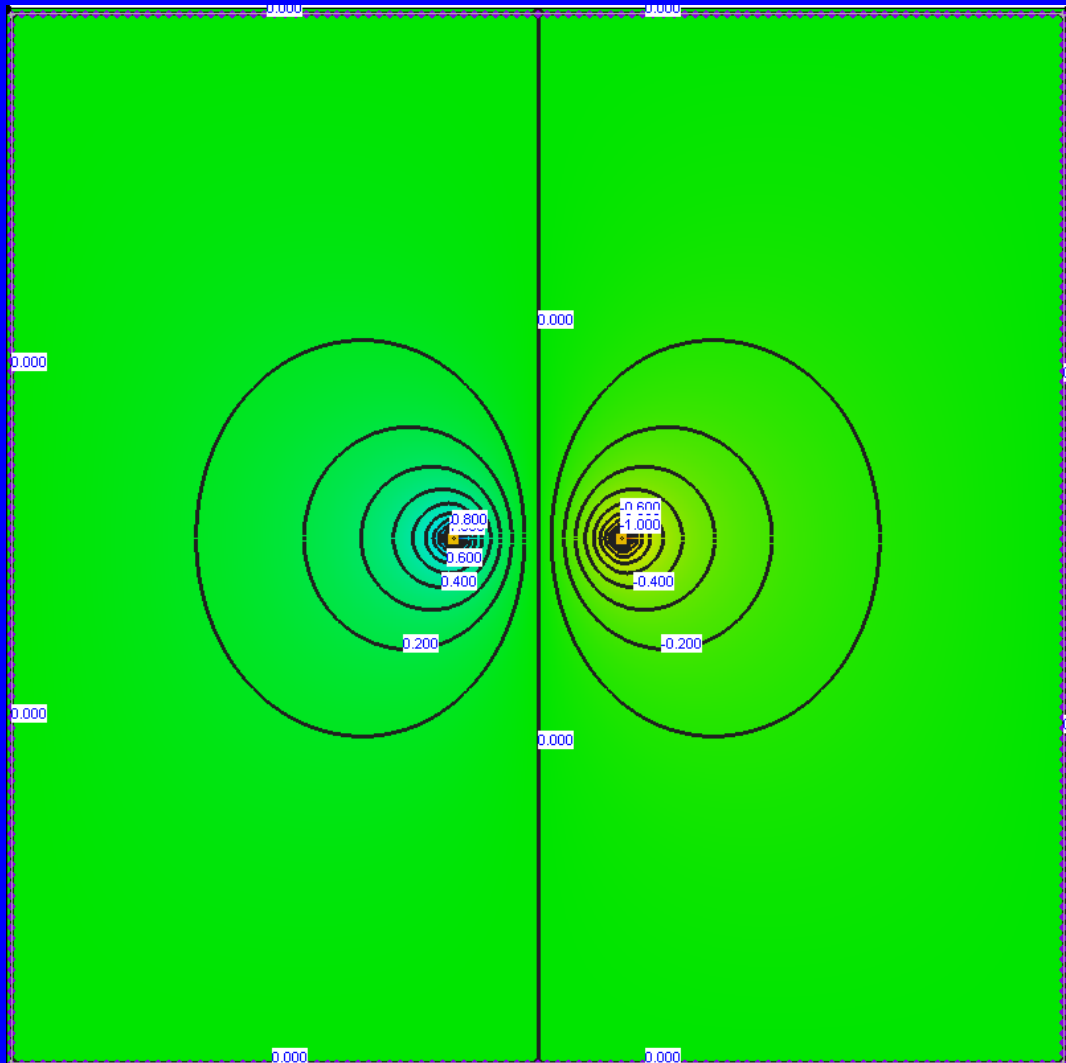
$K = 50 \text{ m/d}$

$H = 50 \text{ m}$

$p = 400 \text{ m}$

$n = 0.3$

Side = 5050 m



# Method of superposition

Case 2: One pumping and one injection wells in a confined aquifer

Flow distribution between two wells

Since  $y = 0$  along the x-axis

$$s = \frac{Q_0}{2\pi T} \ln\left(\frac{p-x}{p+x}\right) \quad |x| < p$$

$$q_x = -K \frac{\partial \phi}{\partial x} = K \frac{\partial s}{\partial x} = -\frac{Q_0}{2\pi H} \frac{2p}{p^2 - x^2}$$

$$v_x = -\frac{Q_0}{2\pi n_e H} \frac{2p}{p^2 - x^2}$$

# Method of superposition

Case 2: One pumping and one injection wells in a confined aquifer

Flow distribution between two wells

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

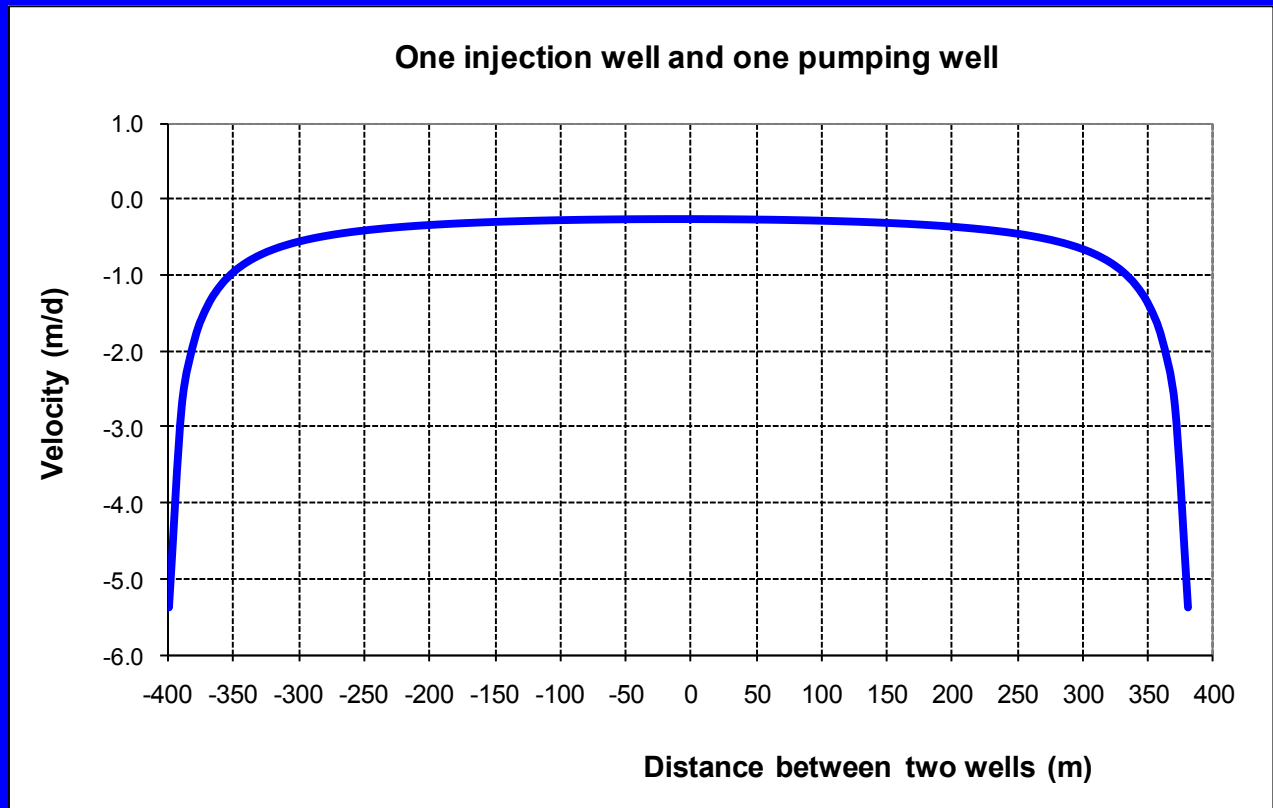
$$K = 50 \text{ m/d}$$

$$H = 50 \text{ m}$$

$$p = 400 \text{ m}$$

$$n = 0.3$$

$$v_{max} = 265 \text{ m/d}$$



# Method of superposition

Case 2: One pumping and one injection wells in a confined aquifer

Minimum travel time from injection well to pumping well

$$t = \int_{-p}^p \frac{dx}{V_x} = \frac{\pi n_e H}{p Q_0} \int_{-p}^p (p^2 - x^2) dx$$

$$t = \frac{4\pi n_e H p^2}{3 Q_0}$$

straight line between 2 wells  
60 days for T is good

Example:

$Q_0 = 5000 \text{ m}^3/\text{d}$

$K = 50 \text{ m/d}$

$H = 50 \text{ m}$

$p = 400 \text{ m}$

$n = 0.3$

**$t = 2010 \text{ days}$**

**$5.5 \text{ years}$**

# Method of superposition

Case 2: One pumping and one injection wells in a confined aquifer

Flow field simulated  
by a numerical model

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

$$K = 50 \text{ m/d}$$

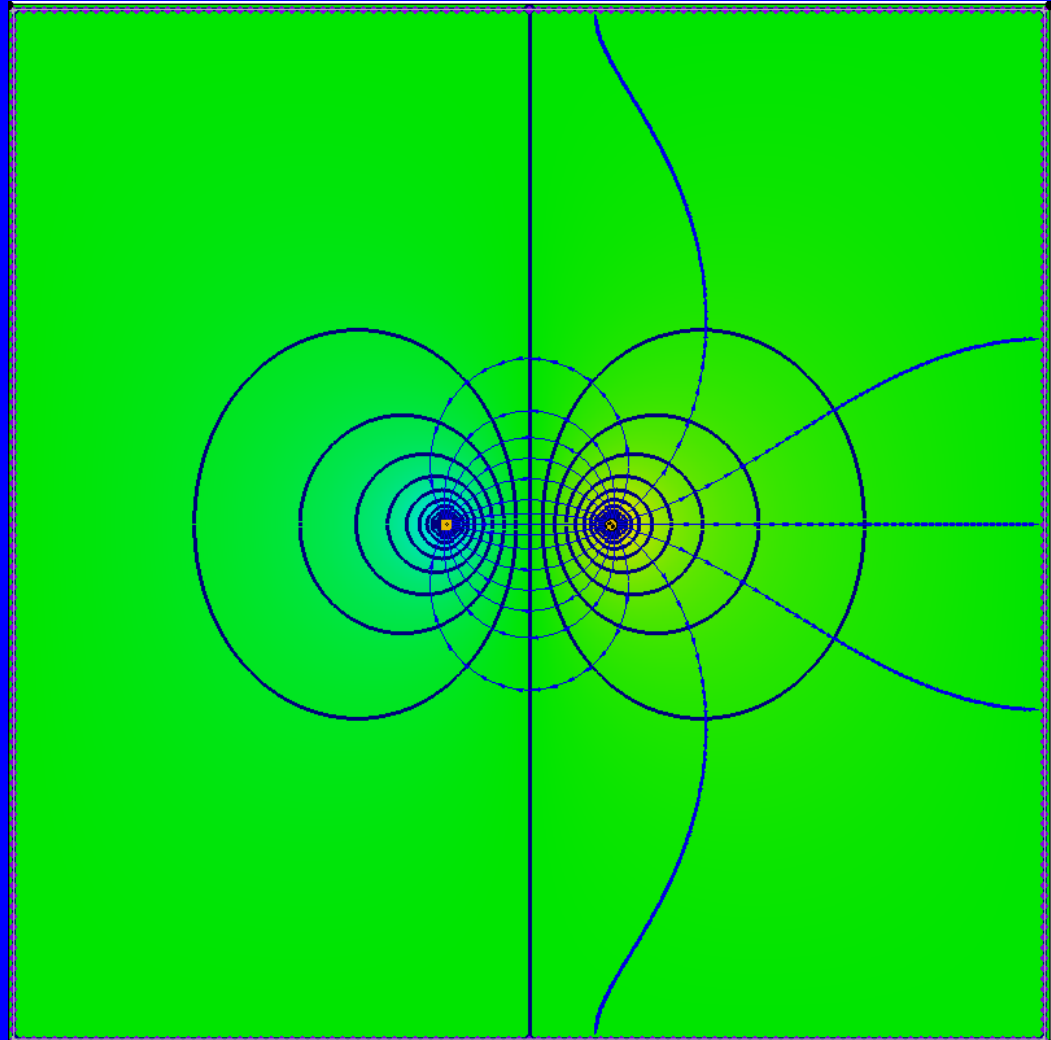
$$H = 50 \text{ m}$$

$$\text{Side} = 5050 \text{ m}$$

$$p = 400 \text{ m}$$

$$n = 0.3$$

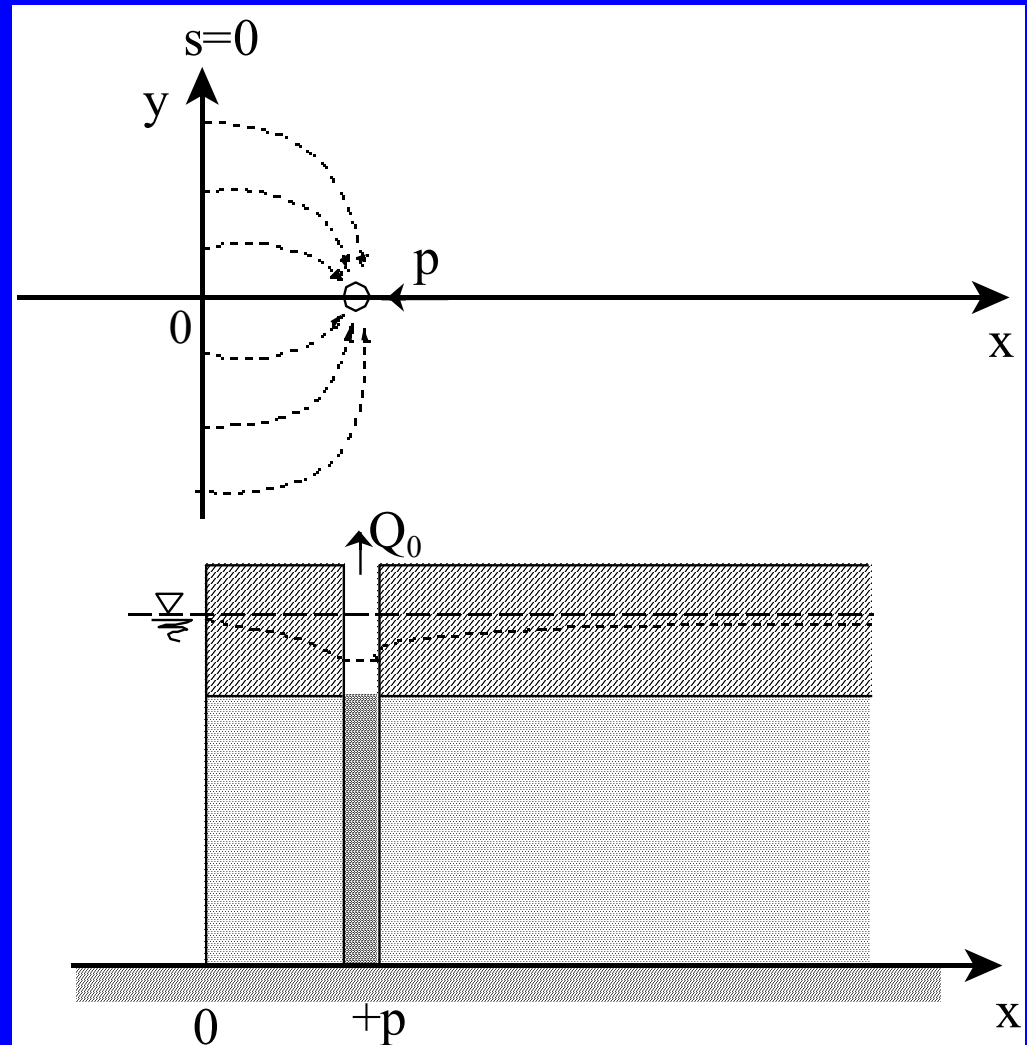
The travel time  
between two  
marks is 5 years



# Method of images

## Case 1: A pumping well near a constant head boundary

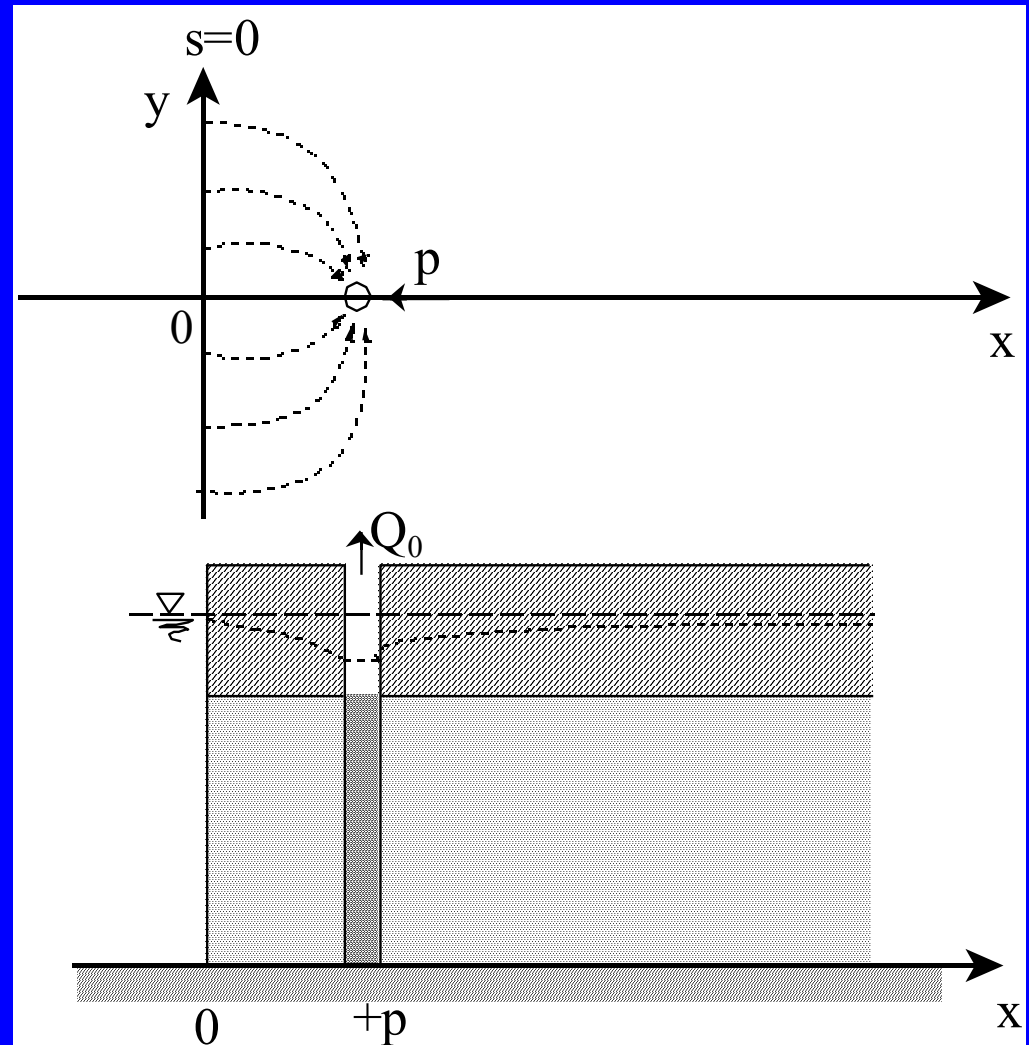
- Conceptual hydrogeological model
  - The confined aquifer is homogeneous and isotropic;
  - The aquifer is bounded by a straight constant head boundary on the left;
  - The aquifer extends to infinity in the right half;
  - A pumping well is located near the boundary with the constant pumping rate.



# Method of images

## Case 1: A pumping well near a constant head boundary

- Conditions for the solution:
  - 1) It must satisfy the Laplace's equation for  $x > 0$ , except in the point  $(x=p, y=0)$ ;
  - 2)  $s = 0$  when  $x$  approaches the infinity;
  - 3)  $s = 0$  when  $x = 0$  along the  $y$ -axis;
  - 4) the amount of water leaves the aquifer at point  $(x=p, y=0)$  must equal pumping rate.



# Method of images

## Case 1: A pumping well near a constant head boundary

- Method of the image:
  - 1) replacing the semi-infinite aquifer with an fictitious infinite aquifer with the same hydrogeological parameters;
  - 2) considering the constant head boundary as a mirror;
  - 3) putting an imaginary injecting well at the image location of the pumping well, i.e. at point  $(x=-p, y=0)$ ;
  - 4) giving the injecting rate equal to the pumping rate;
  - 5) using the principle of superposition to find the solution.



# Method of images

Case 1: A pumping well near a constant head boundary

Drawdown caused by the pumping well:

$$s_1 = \frac{Q_0}{2\pi T} \ln\left(\frac{R}{\sqrt{(x-p)^2 + y^2}}\right)$$

Drawdown caused by the injection well:

$$s_2 = -\frac{Q_0}{2\pi T} \ln\left(\frac{R}{\sqrt{(x+p)^2 + y^2}}\right)$$

Total drawdown:

$$s = s_1 + s_2 = \frac{Q_0}{2\pi T} \ln\left(\sqrt{\frac{(x+p)^2 + y^2}{(x-p)^2 + y^2}}\right)$$

Please verify that  $s$  satisfies the 4 conditions!

# Method of images

Case 1: A pumping well near a constant head boundary

Drawdown along the x axis:

$$s = \frac{Q_0}{2\pi T} \ln \left( \frac{x+p}{p-x} \right)$$

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

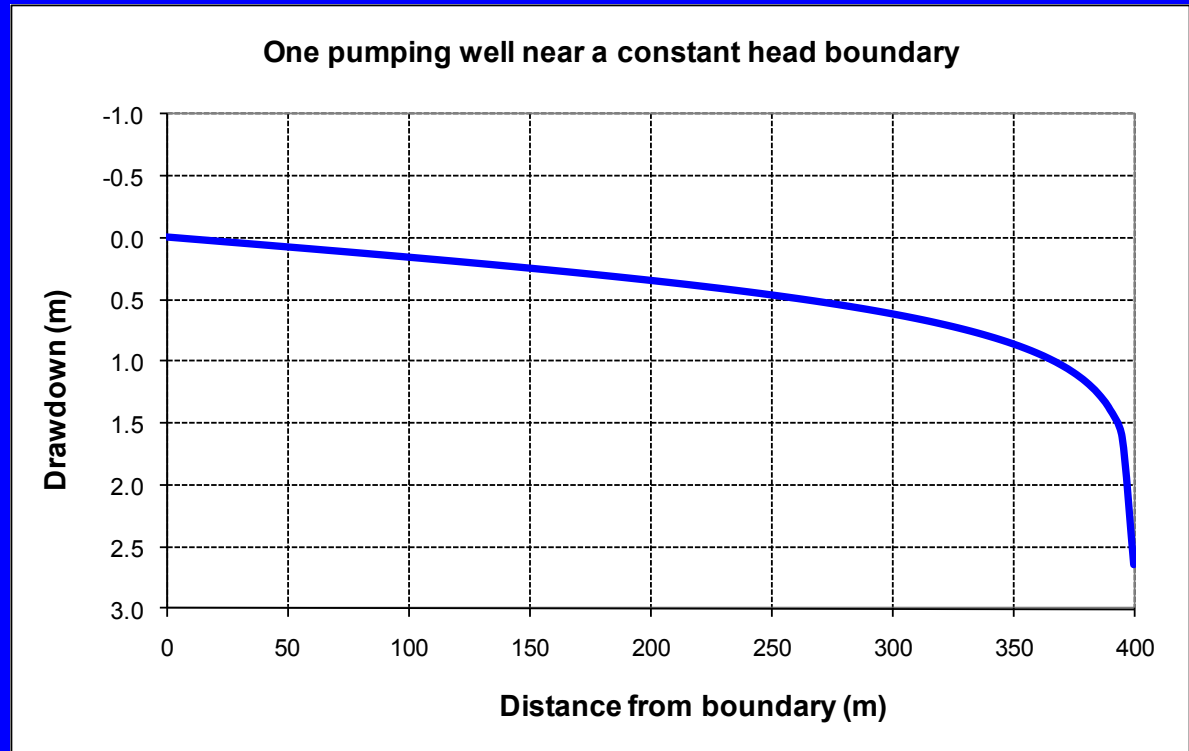
$$K = 50 \text{ m/d}$$

$$H = 50 \text{ m}$$

$$p = 400 \text{ m}$$

$$n = 0.3$$

$$s_{\max} = 2.64 \text{ m}$$



# Method of images

Case 1: A pumping well near a constant head boundary

Travel time along the x axis:

$$\frac{\partial s}{\partial x} = \frac{Q_0}{2\pi T} \left[ \frac{1}{x+p} + \frac{1}{p-x} \right] = \frac{Q_0}{2\pi t} \frac{2p}{p^2 - x^2}$$

$$q_x = K \frac{\partial s}{\partial x} = \frac{Q_0}{2\pi H} \frac{2p}{p^2 - x^2}$$

$$t = \frac{2\pi H n_e}{2pQ_0} \int_0^p (p^2 - x^2) dx = \frac{2\pi H n_e}{2pQ_0} \left[ p^3 - \frac{1}{3} p^3 \right] = \frac{2}{3} \frac{\pi n_e H p^2}{Q_0}$$

# Method of images

Case 1: A pumping well near a constant head boundary

Velocity distribution along the x axis:

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

$$K = 50 \text{ m/d}$$

$$H = 50 \text{ m}$$

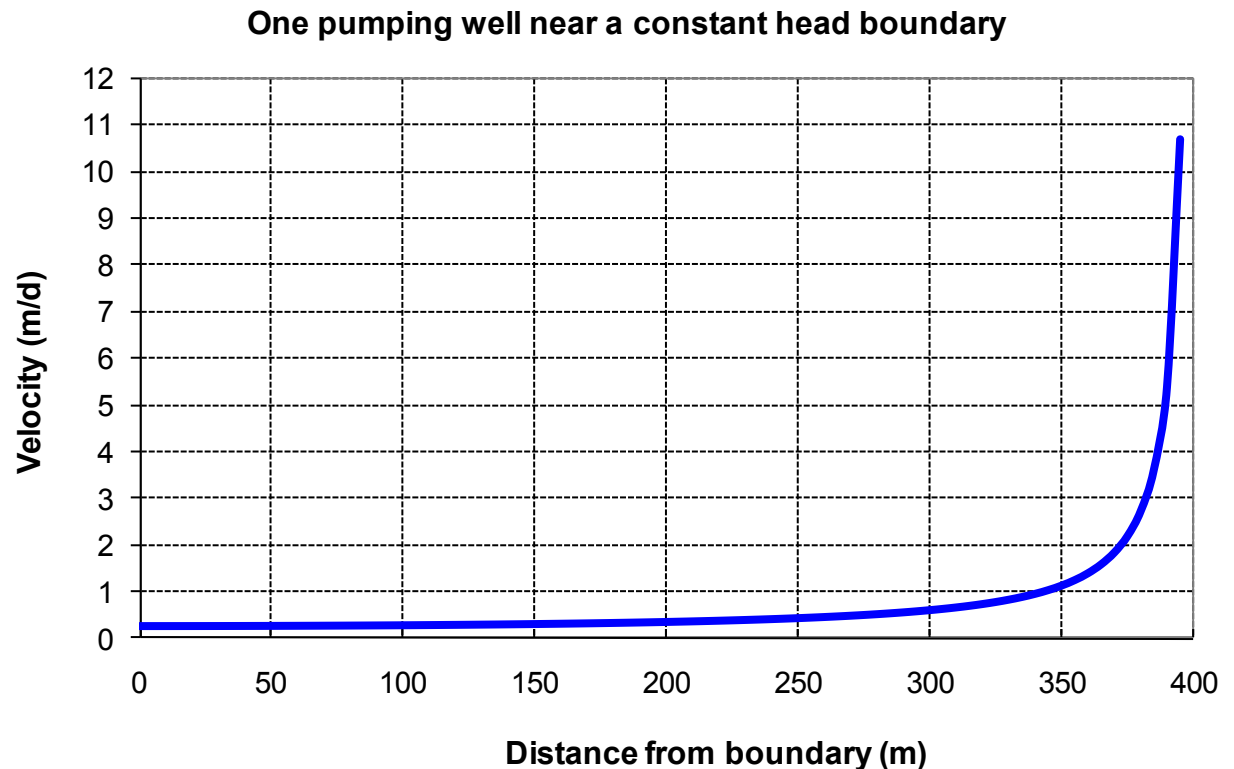
$$p = 400 \text{ m}$$

$$n = 0.3$$

$$v_{\max} = 265 \text{ m/d}$$

$$t = 1005 \text{ days}$$

$$2.7.5 \text{ years}$$



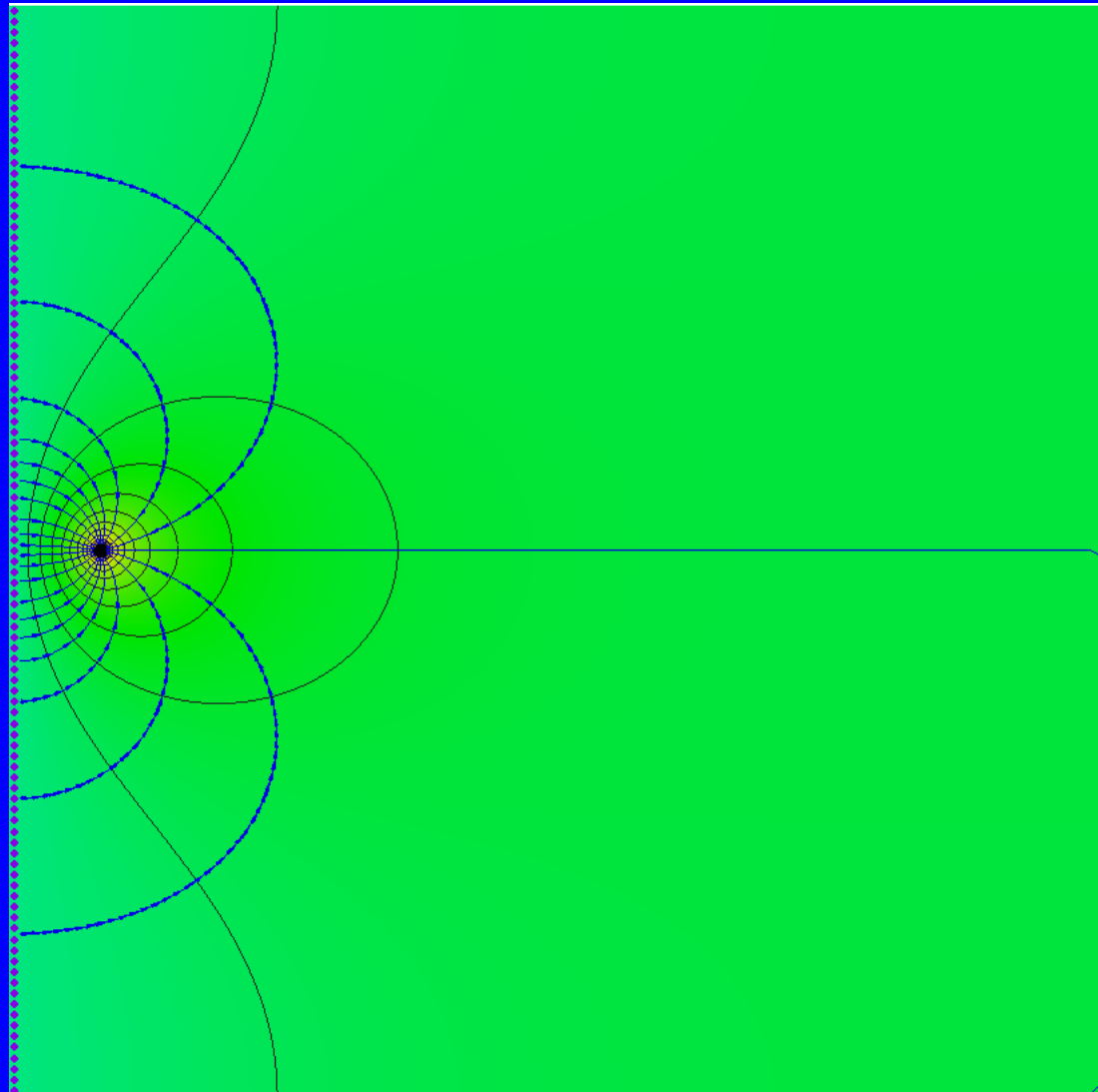
# Method of images

Case 1: A pumping well near a constant head boundary

Model  
simulated  
flow field

Pumped water  
comes from  
constant head  
boundary

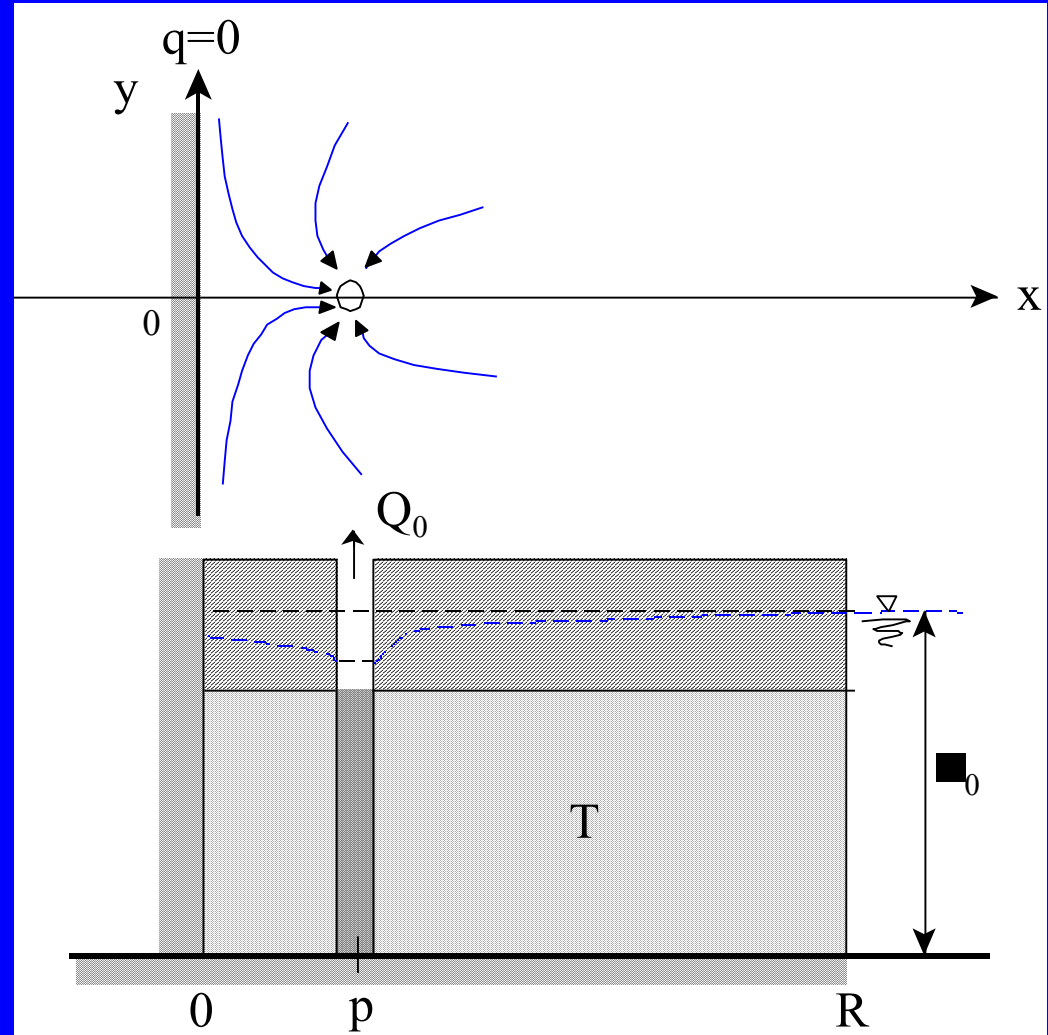
Travel time  
between two  
marks is 2 years



# Method of images

## Case 2: A pumping well near an impermeable boundary

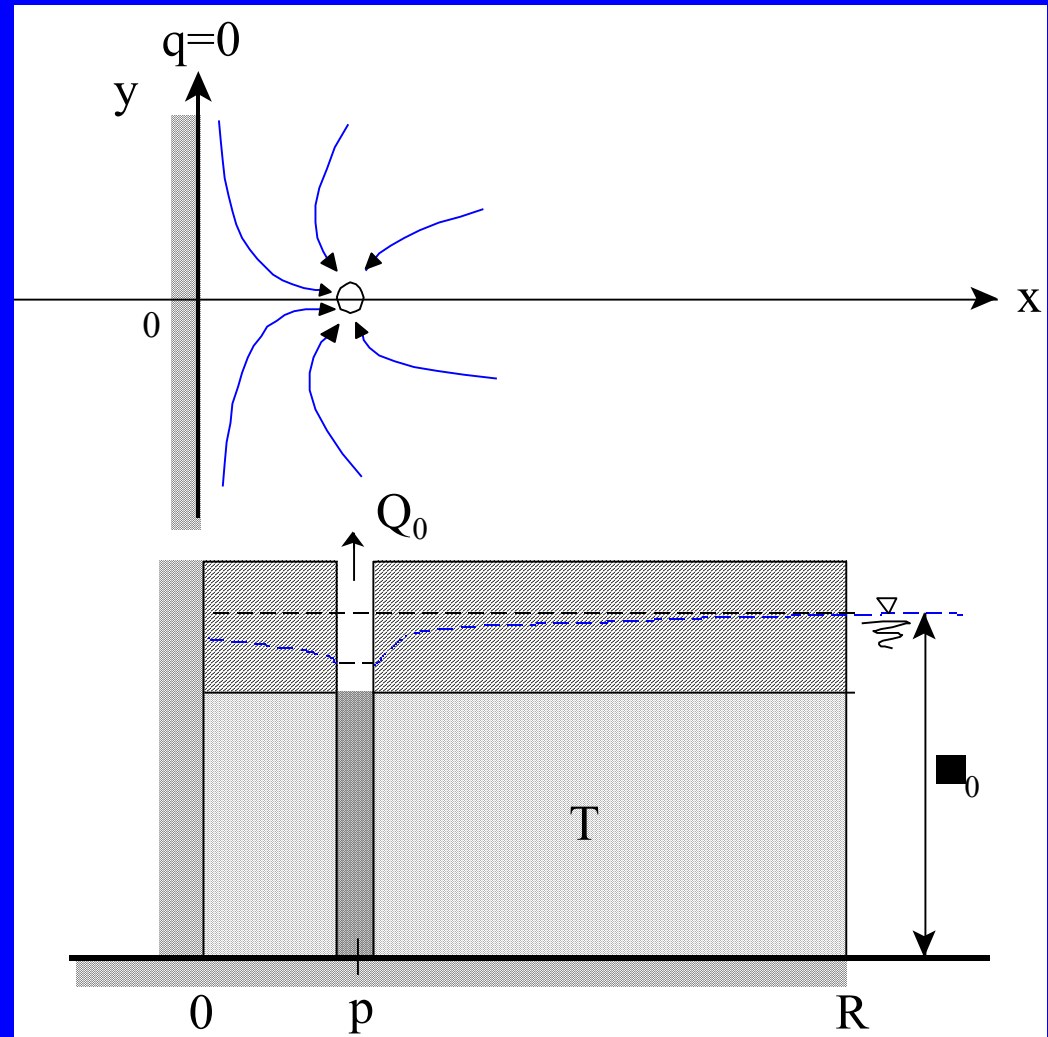
- Conceptual hydrogeological model
  - The confined aquifer is homogeneous and isotropic;
  - The half circular aquifer is bounded by a straight impermeable boundary on the left;
  - A pumping well is located near the boundary with the constant pumping rate.



# Method of images

## Case 2: A pumping well near an impermeable boundary

- Conditions for the solution:
  - 1) It must satisfy the Laplace's equation for  $x > 0$ , except in the point  $(x=p, y=0)$ ;
  - 2)  $s = 0$  when  $r = R$  at the circular head boundary;
  - 3)  $q_x$  or  $ds/dx = 0$  at  $x = 0$  along the  $y$ -axis;
  - 4) The amount of water leaves the aquifer at point  $(x=p, y=0)$  must equal pumping rate.



# Method of images

## Case 2: A pumping well near an impermeable boundary

- Method of the image:
  - 1) replacing the half-circular aquifer with an fictitious circular aquifer with the same hydrogeological parameters;
  - 2) considering the impermeable boundary as a mirror;
  - 3) putting an imaginary pumping well at the image location of the real pumping well, i.e. at point  $(x=-p, y=0)$ ;
  - 4) giving the imaginary pumping rate equal to the real pumping rate;
  - 5) using the principle of superposition to find the solution.



# Method of images

Case 2: A pumping well near an impermeable boundary

Drawdown caused by the real pumping well:

$$s_1 = \frac{Q_0}{2\pi T} \ln\left(\frac{R}{\sqrt{(x - p)^2 + y^2}}\right)$$

Drawdown caused by the imaginary pumping well:

$$s_2 = \frac{Q_0}{2\pi T} \ln\left(\frac{R}{\sqrt{(x + p)^2 + y^2}}\right)$$

Total drawdown:

$$s = s_1 + s_2 = \frac{Q_0}{2\pi T} \ln\left(\frac{R^2}{\sqrt{[(x + p)^2 + y^2][(x - p)^2 + y^2]}}\right)$$

Please verify that  $s$  satisfies the 4 conditions!

# Method of images

Case 2: A pumping well near an impermeable boundary

Drawdown along x-axis:

$$s = \frac{Q_0}{2\pi T} \ln\left(\frac{R^2}{p^2 - x^2}\right)$$

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

$$K = 50 \text{ m/d}$$

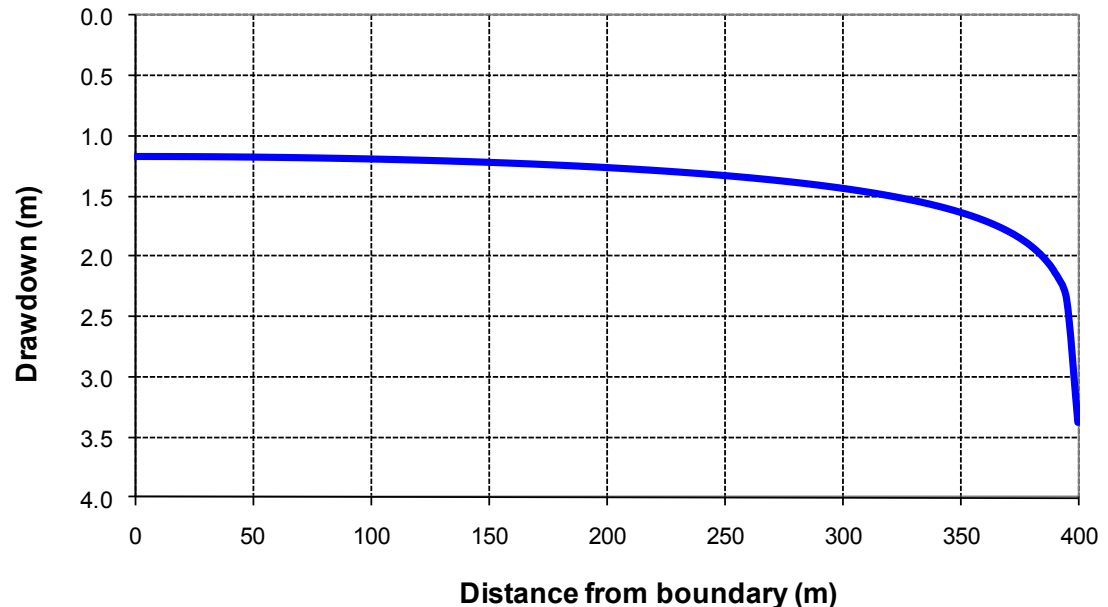
$$H = 50 \text{ m}$$

$$p = 400 \text{ m}$$

$$n = 0.3$$

$$s_{\max} = 3.37 \text{ m}$$

One pumping well near a no-flow boundary



# Method of images

## Case 2: A pumping well near an impermeable boundary

Velocity along x-axis:

$$v_x = \frac{Q_0}{2\pi H n_e} \frac{2x}{p^2 - x^2}$$

Example:

$$Q_0 = 5000 \text{ m}^3/\text{d}$$

$$K = 50 \text{ m/d}$$

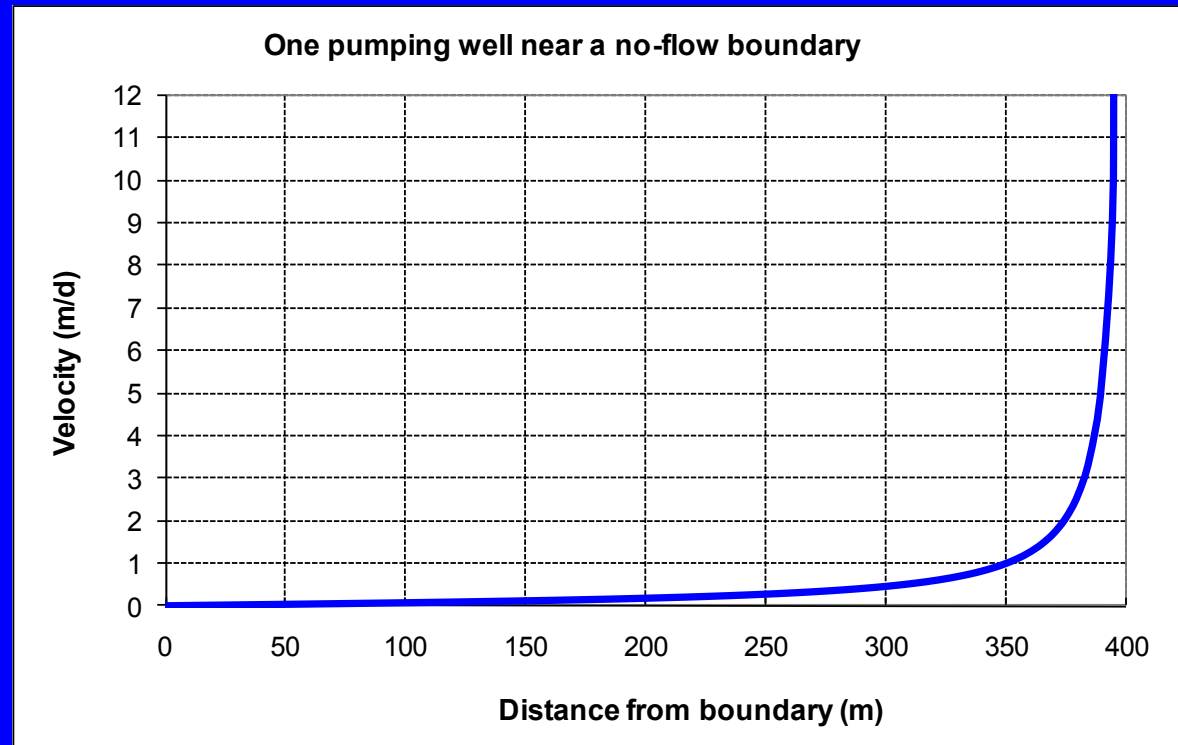
$$H = 50 \text{ m}$$

$$p = 400 \text{ m}$$

$$n = 0.3$$

$$v_{\max} = 265 \text{ m/d}$$

When  $x=0$ , the velocity is zero, no flow at the impermeable boundary



# Method of images

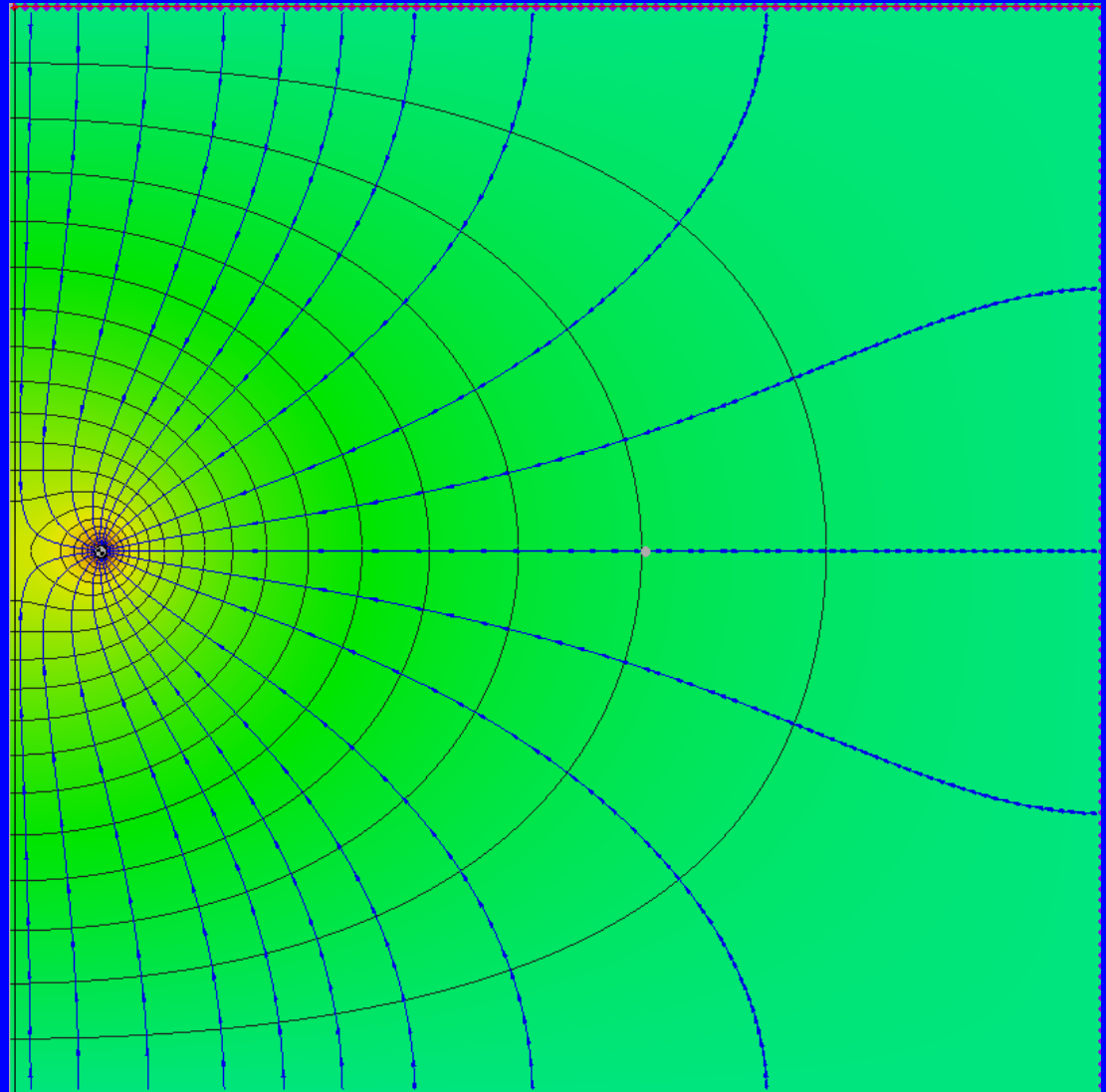
Case 2: A pumping well near an impermeable boundary

Model  
simulated flow  
field

Pumped water  
comes from  
inflow boundary

Flowline is  
parallel to the  
impermeable  
boundary

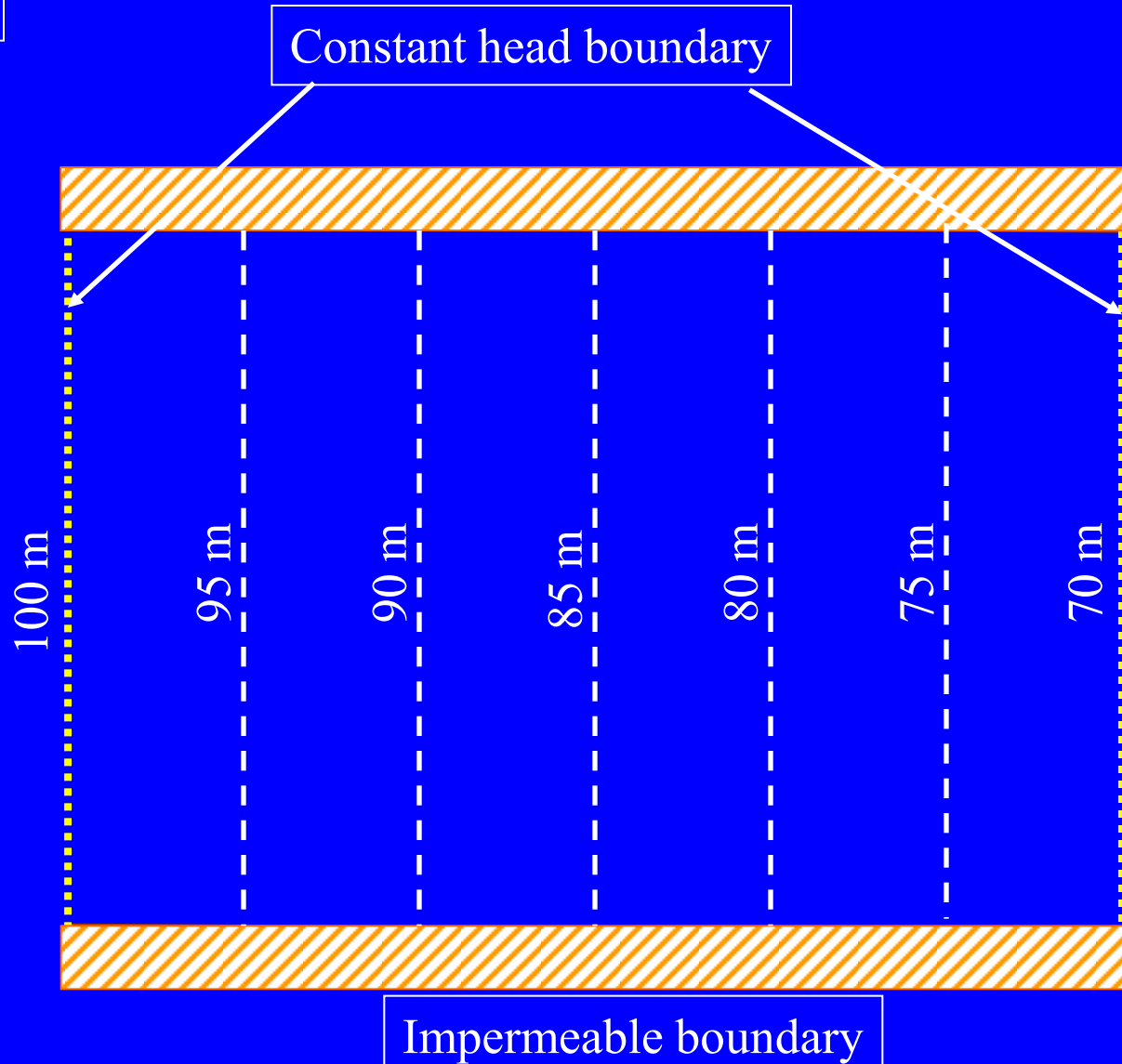
Travel time  
between marks is  
10 years



# Flow net

- Equipotential lines

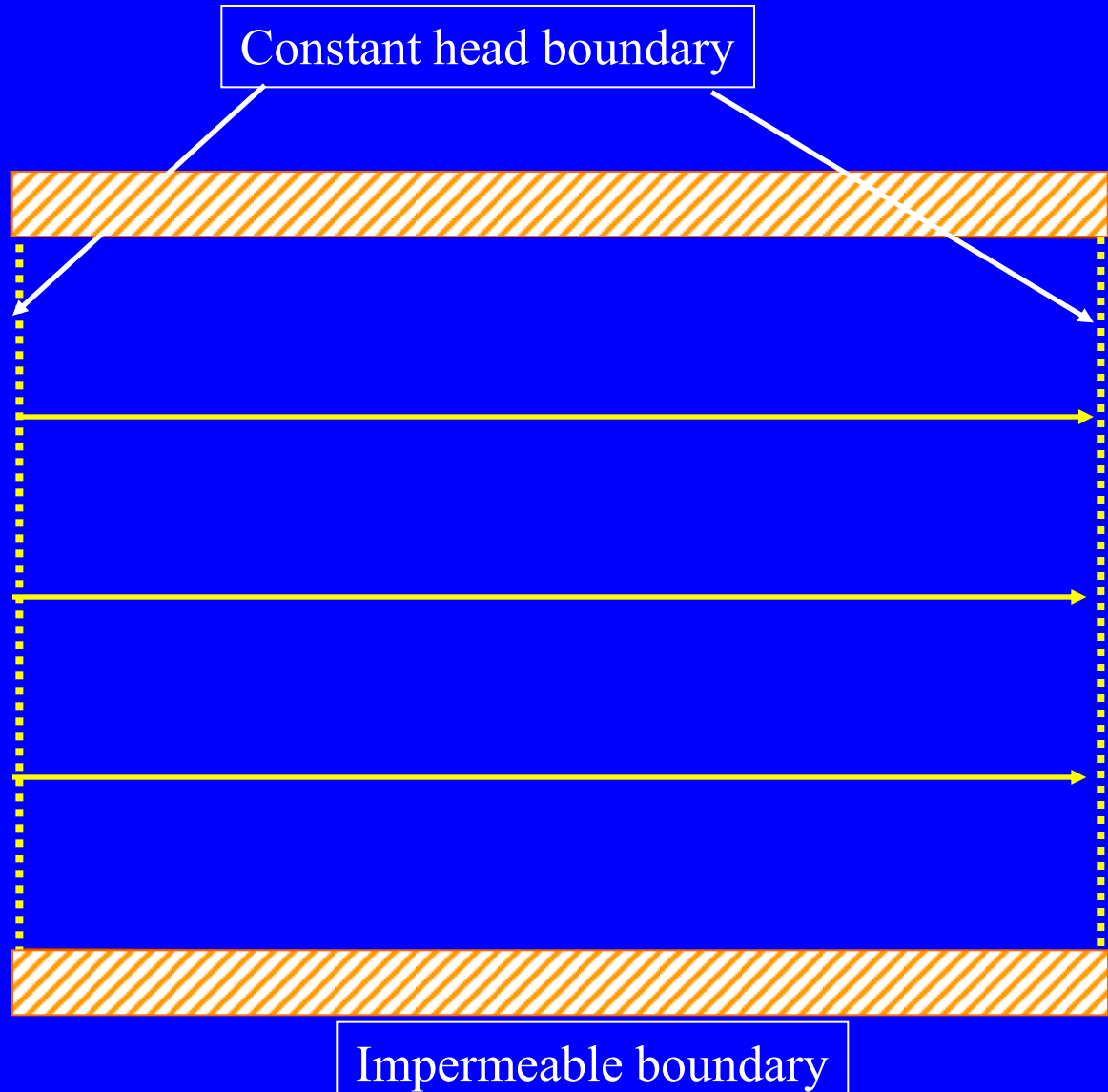
A lines of constant groundwater head is called equipotential lines. If the interval between equipotential lines is constant, equipotential lines form a contour map of groundwater head.



# Flow net

- Flow lines

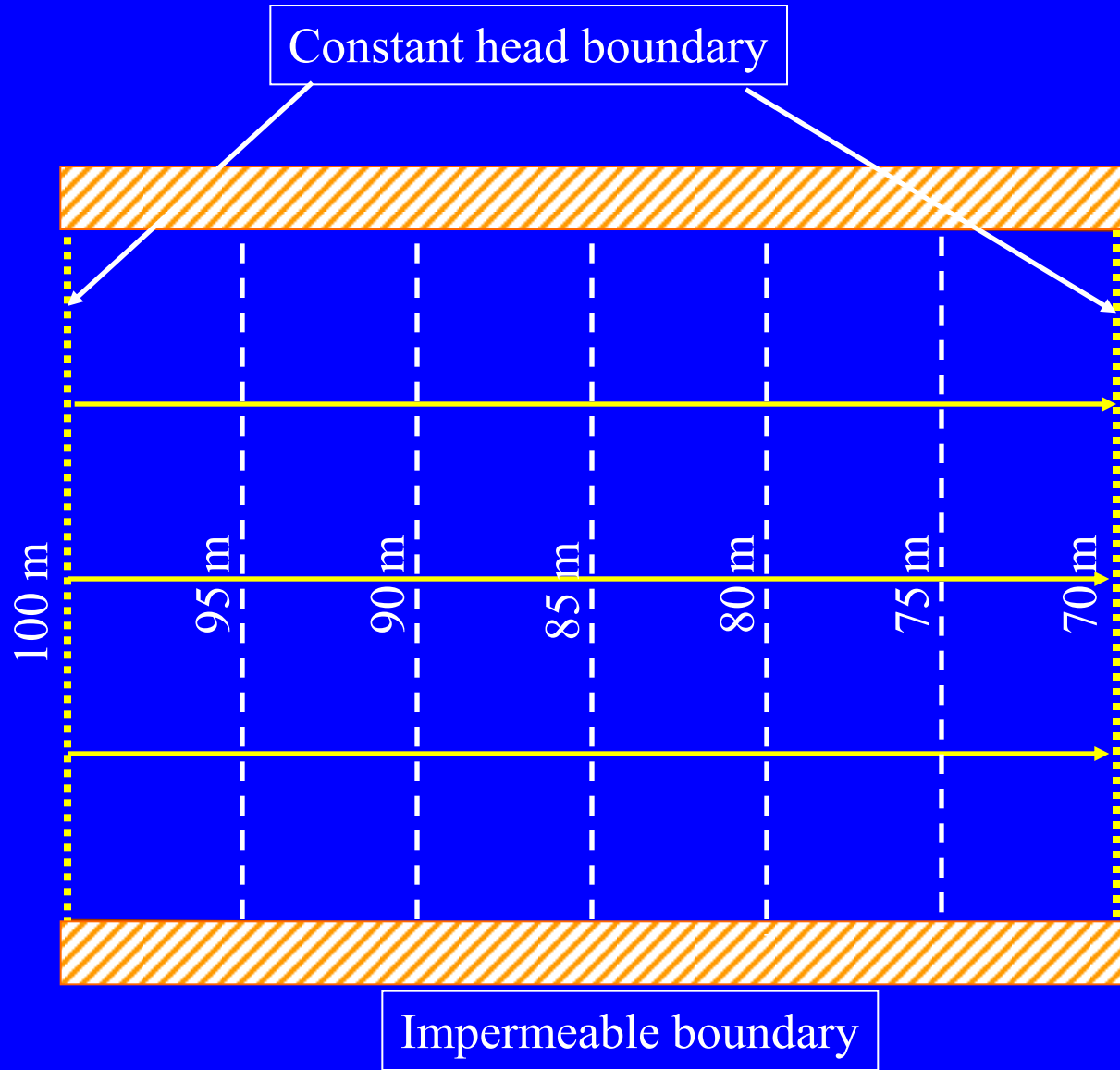
A flow line is an imaginary line that traces the path of a water particle flowing through an aquifer. In an isotropic aquifer, flow lines are parallel to impermeable boundary.



# Flow net

- Flow net

In a two-dimensional flow field, a network of equipotential lines and flow lines constitutes a flow net. In an isotropic aquifer, equipotential line and flow lines will form a network of elementary squares.



# Flow net

- Calculation of discharge

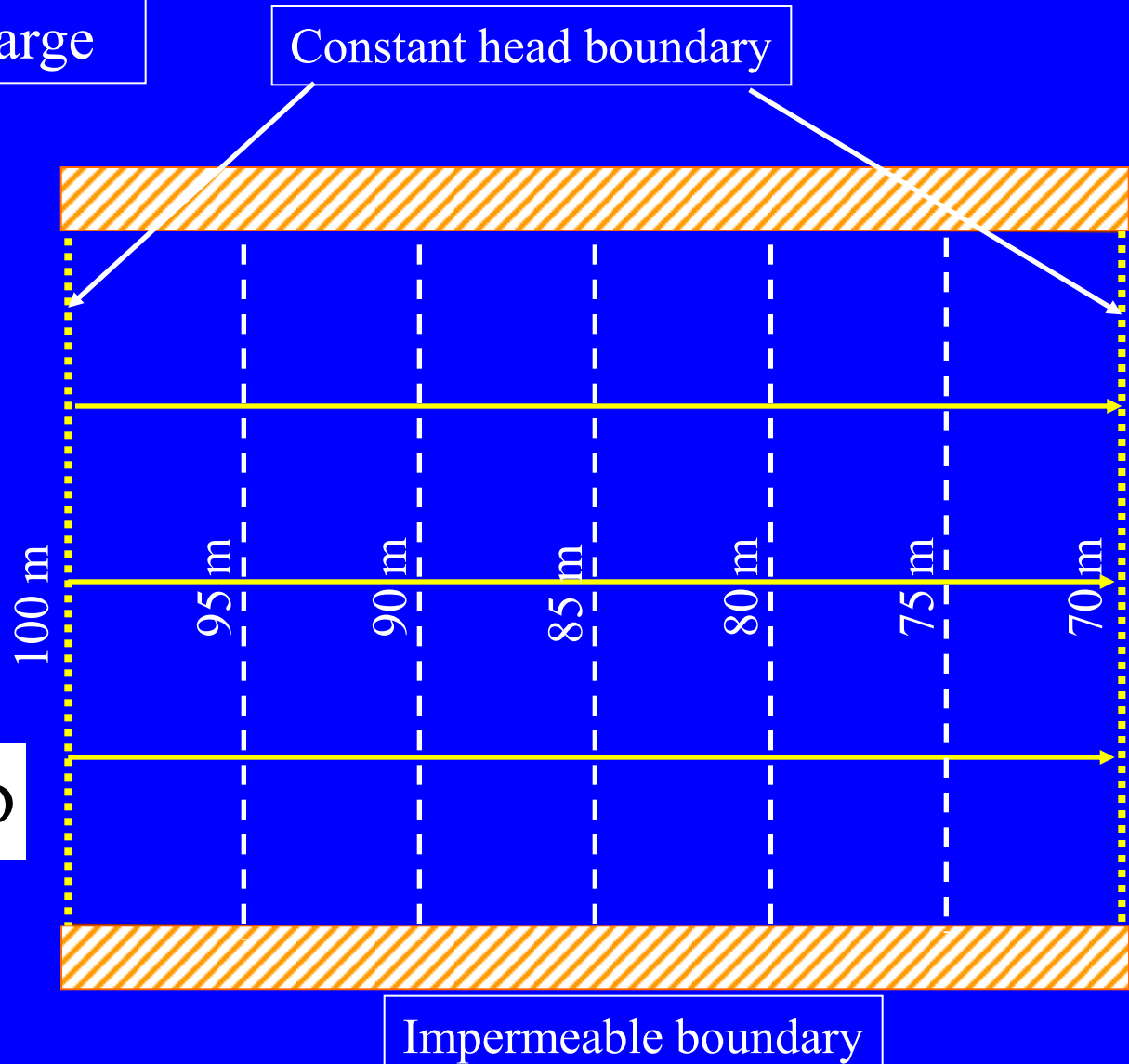
$$q = K \frac{\Delta\phi}{\Delta s} \cdot \Delta n = K\Delta\phi$$

$\Delta s$ : distance between two potential lines;

$\Delta n$ : distance between two flow lines.

$$Q = mq = mK\Delta\phi$$

$m$ : number of flow channels.





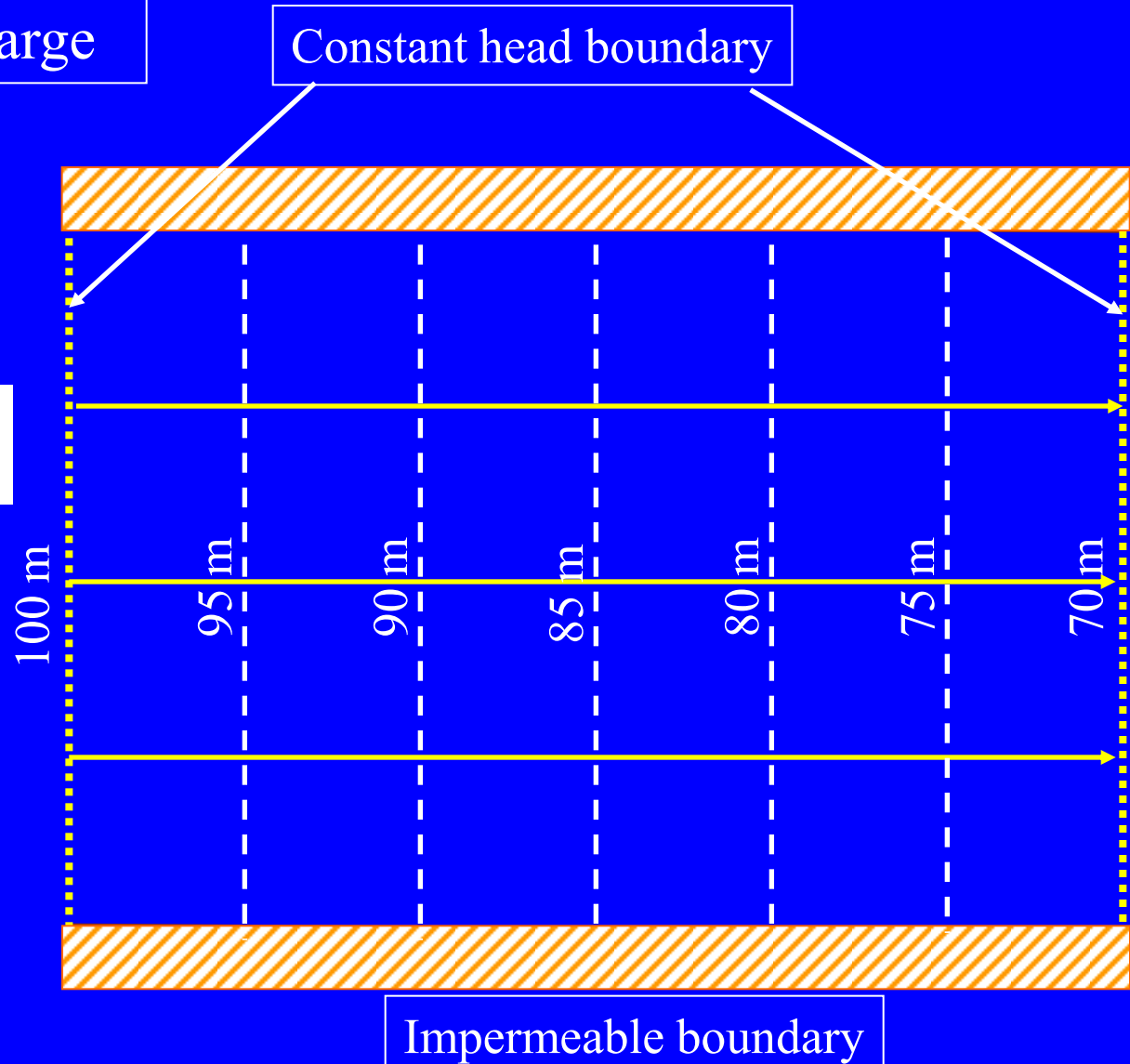
# Flow net

- Calculation of discharge

$$Q = 4q$$

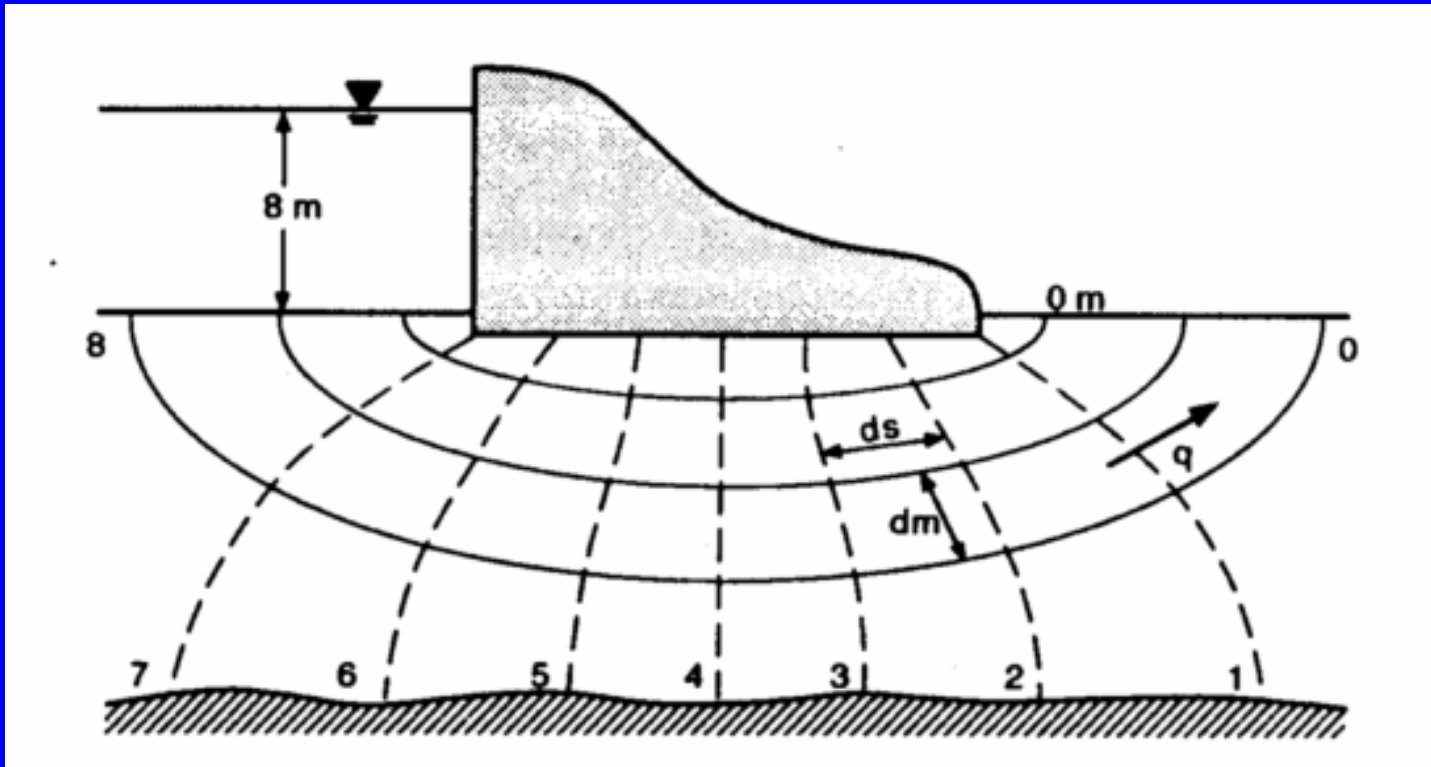
$$Q = 4 * K * 5 = 20K$$

$$Q_{\text{total}} = 20T$$



# Flow net

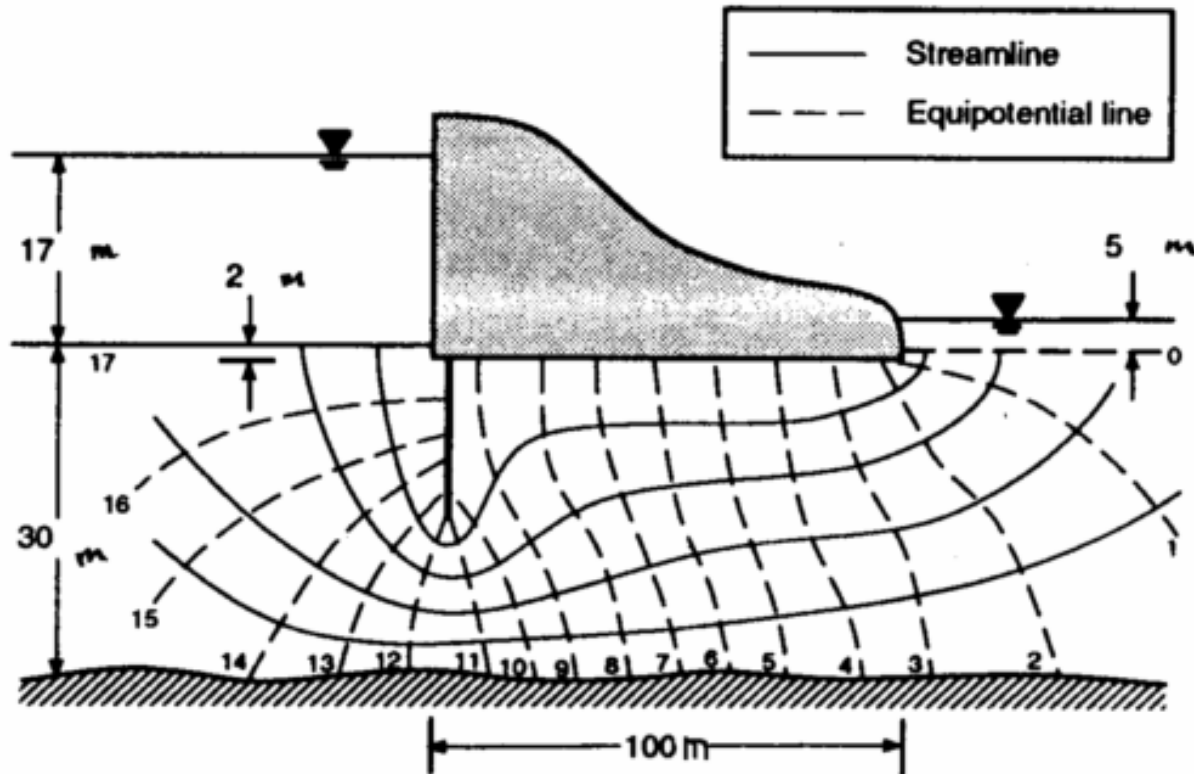
- Seepage underneath the dam



$$Q = 4q = 4K$$

# Flow net

- Seepage underneath the dam

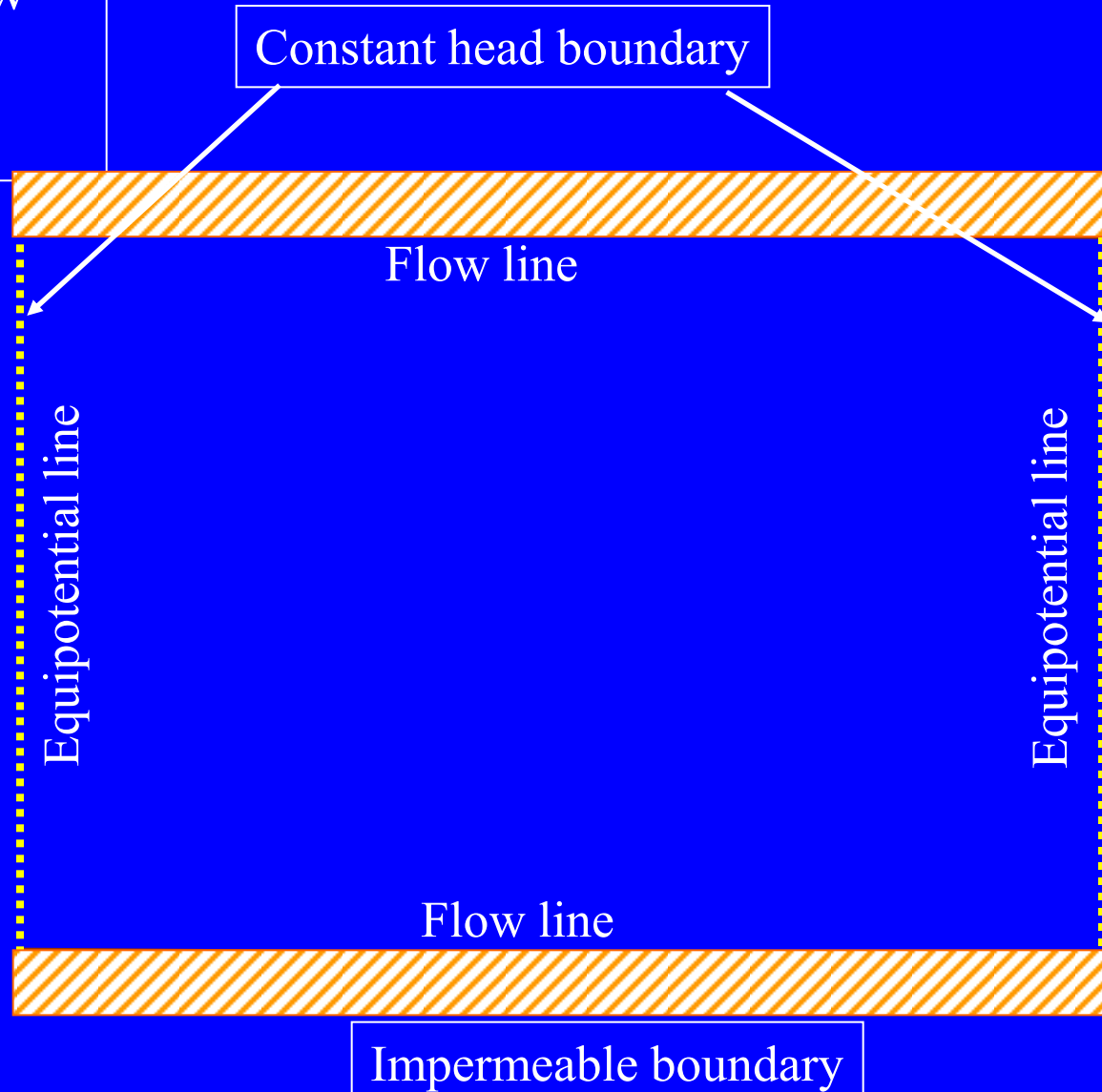


$$Q = 5q = 5K$$

# Flow net

- Construction of the flow net in the isotropic and homogeneous aquifer

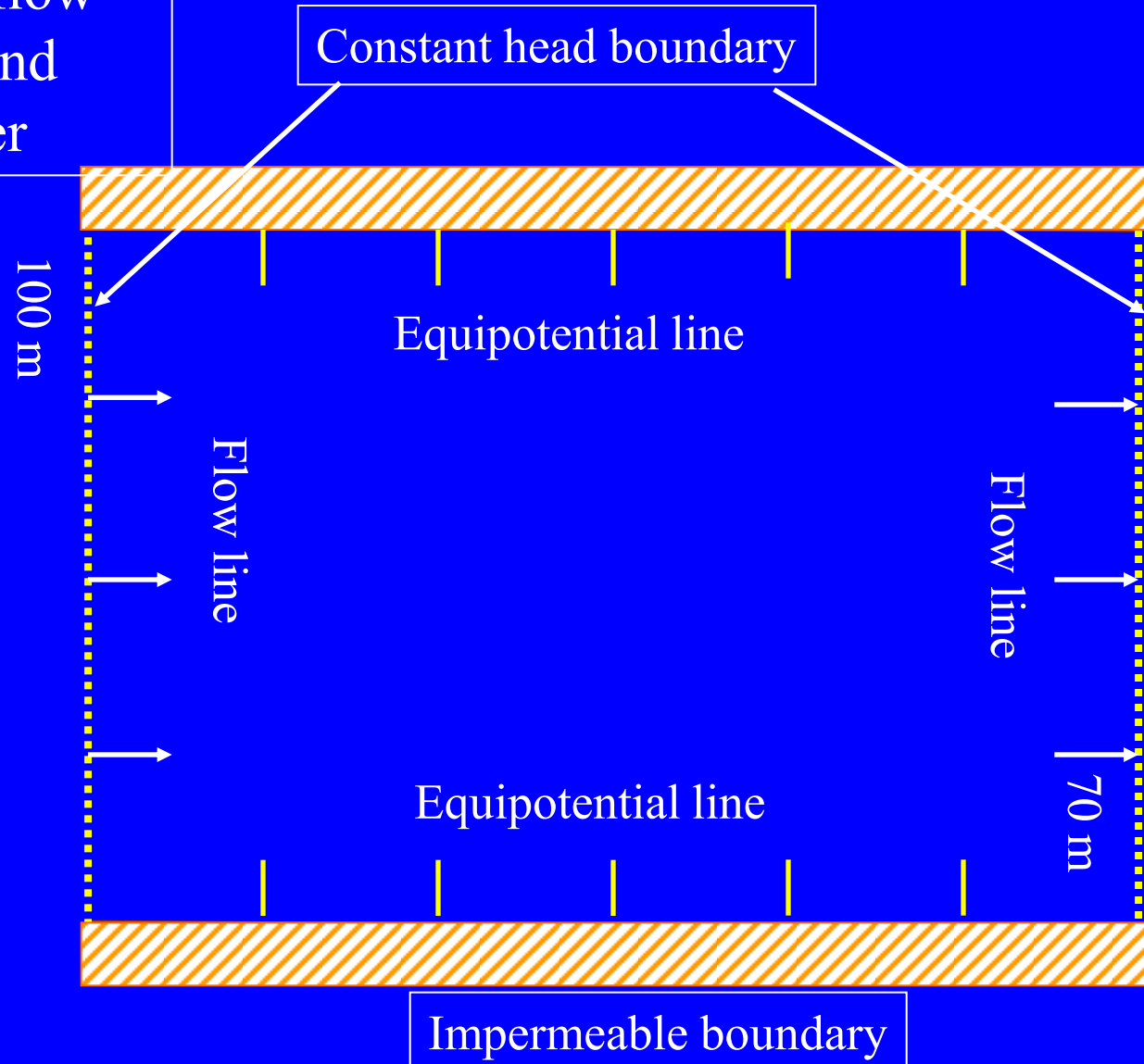
Step 1: Sketch the flow system and identify the specified equipotential lines and flow lines.



# Flow net

- Construction of the flow net in the isotropic and homogeneous aquifer

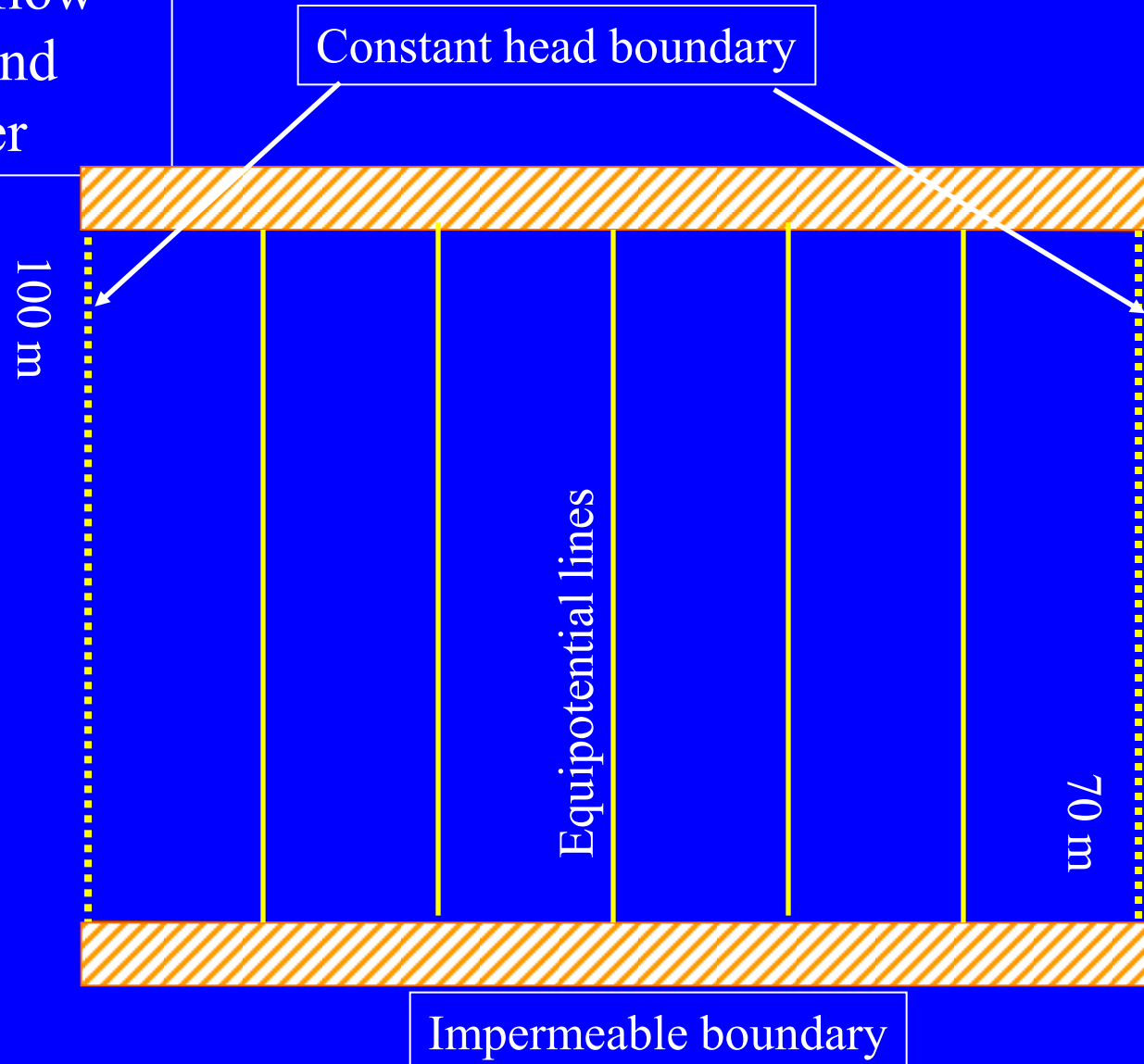
Step 2: Mark the positions of equipotential lines and flow lines.



# Flow net

- Construction of the flow net in the isotropic and homogeneous aquifer

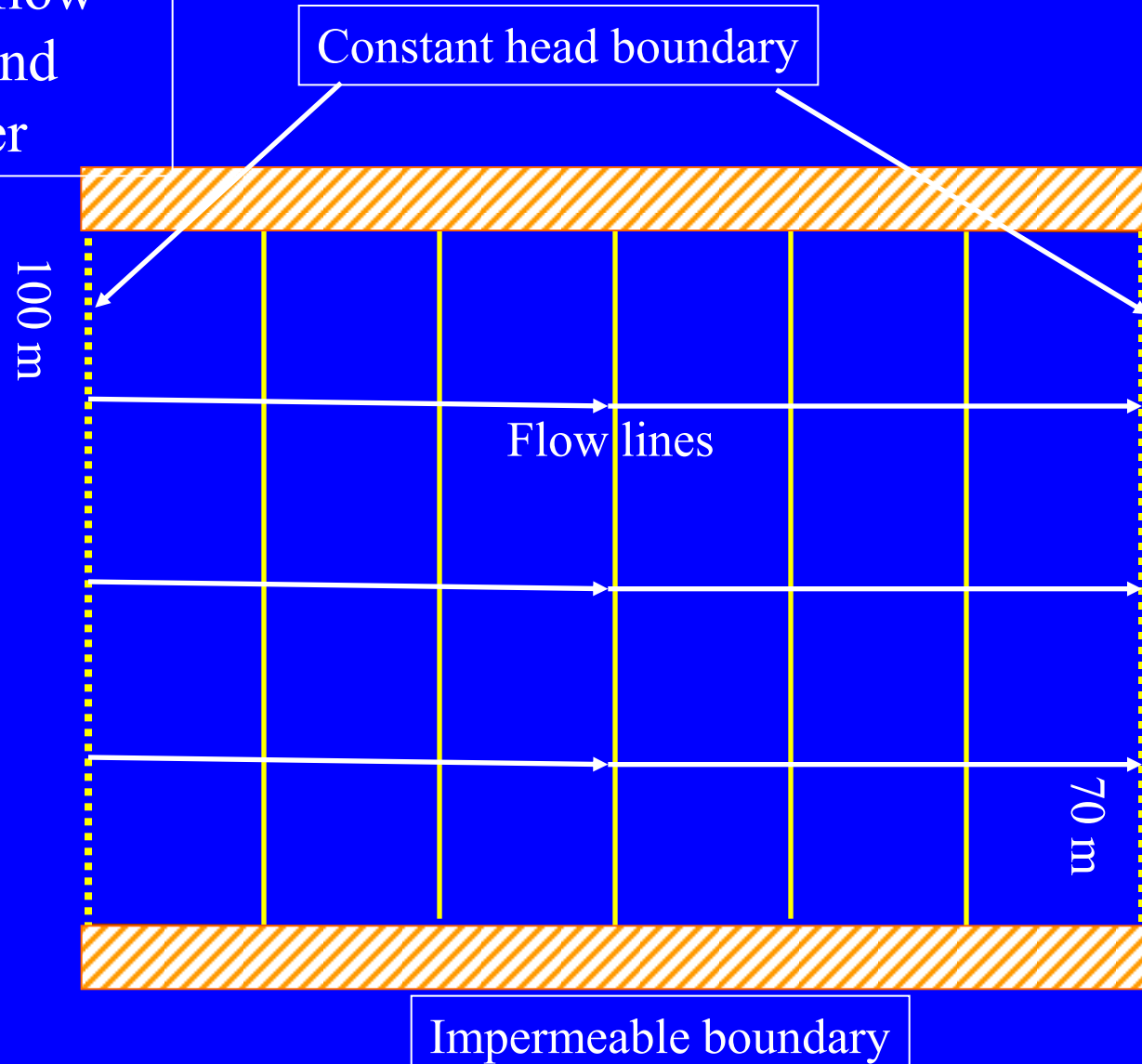
Step 3: Draw trial set of equipotential lines.



# Flow net

- Construction of the flow net in the isotropic and homogeneous aquifer

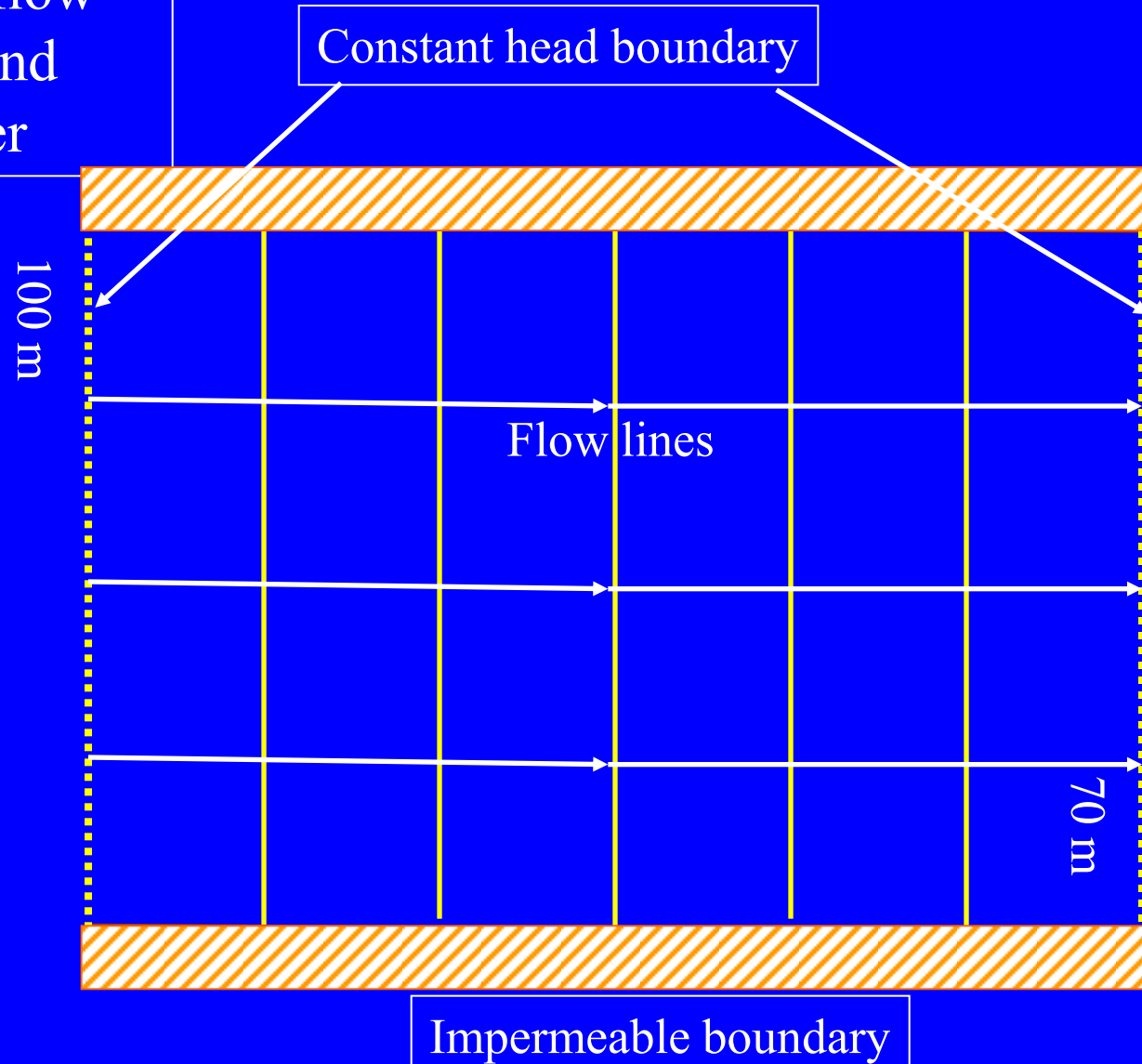
Step 4: Draw trial set of flow lines perpendicular to the equipotential lines.



# Flow net

- Construction of the flow net in the isotropic and homogeneous aquifer

Step 5: Erase and redraw the trial equipotential and flow lines until the orthogonal flow net is obtained.





# Flow net

- Refraction of flow lines in the heterogeneous aquifer

$$Q_1 = aK_1 \frac{dh_1}{dl_1} \quad Q_2 = cK_2 \frac{dh_2}{dl_2}$$

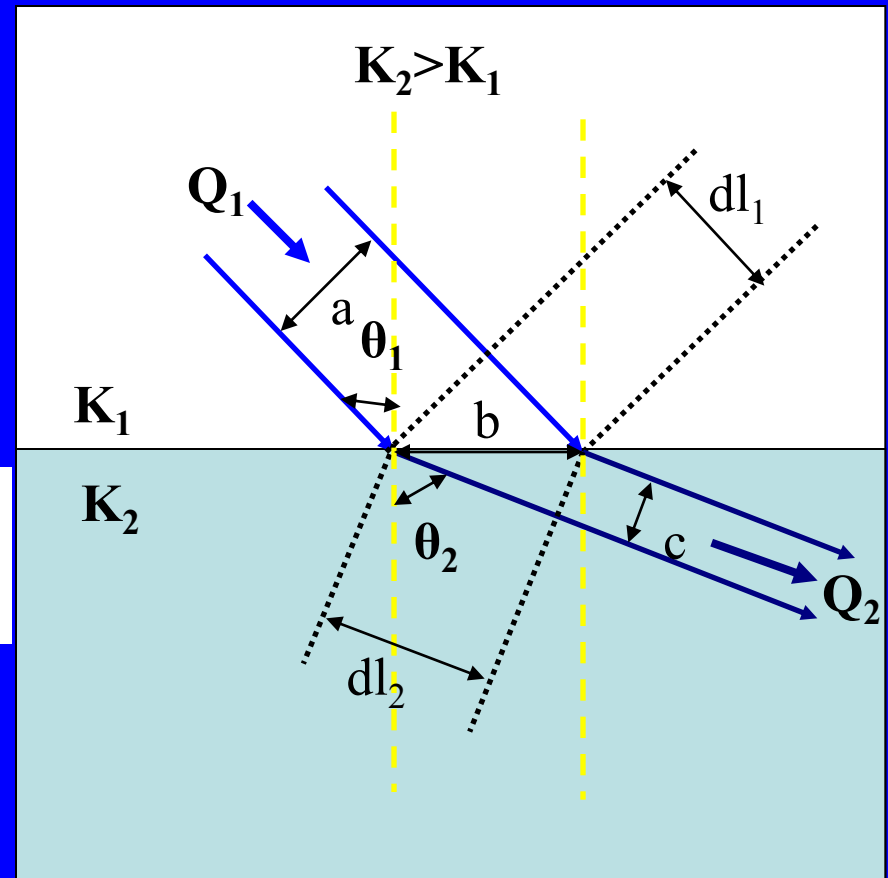
$$Q_1 = Q_2 \quad dh_1 = dh_2$$

$$a = b \cos(\theta_1) \quad c = b \cos(\theta_2)$$

$$\frac{dl_1}{b} = \sin(\theta_1) \quad \frac{dl_2}{b} = \sin(\theta_2)$$

$$\frac{K_1}{K_2} = \frac{\tan(\theta_1)}{\tan(\theta_2)}$$

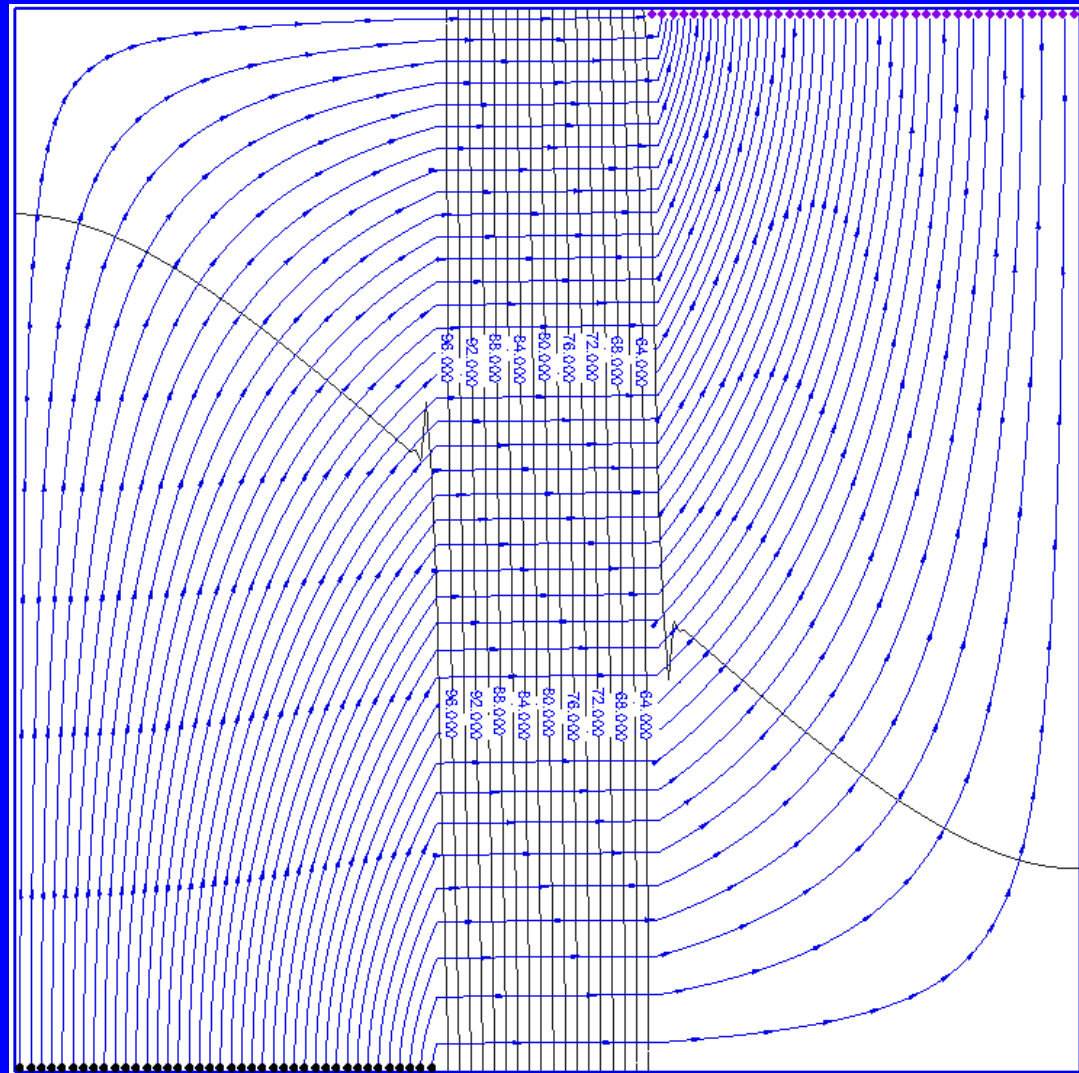
$$\theta_2 > \theta_1$$



# Flow net

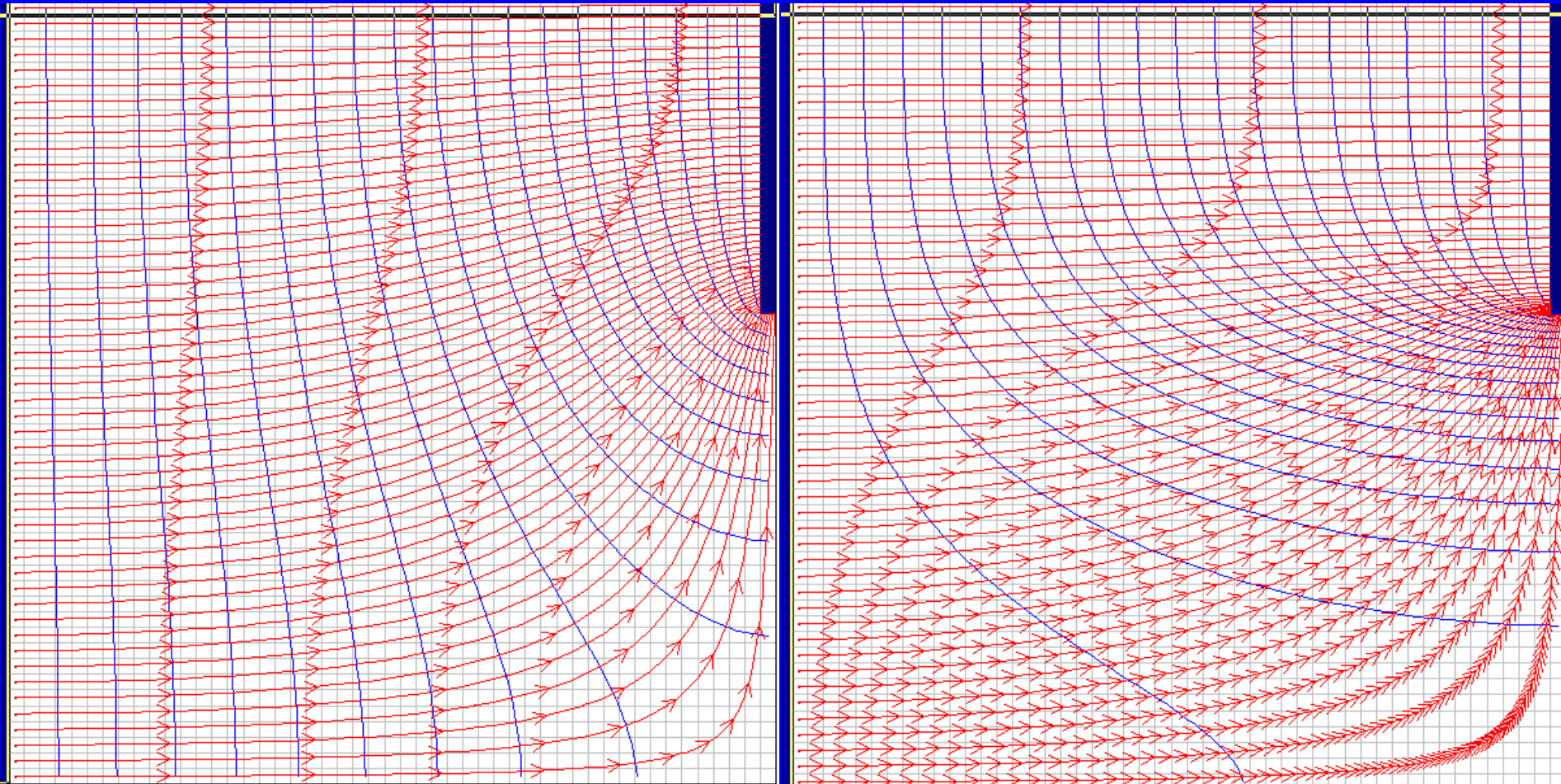
- Refraction of flow lines in the heterogeneous aquifer

Example:  
Confined aquifer  
Thickness = 50m  
5050x5050m  
 $H_1=100\text{m}$   
 $H_2=60\text{m}$   
 $K_1=100\text{m/d}$   
 $K_2=1\text{m/d}$   
Travel time  
between two  
marks is 10 years



# Flow net

- Flow net in the homogeneous and anisotropic aquifer

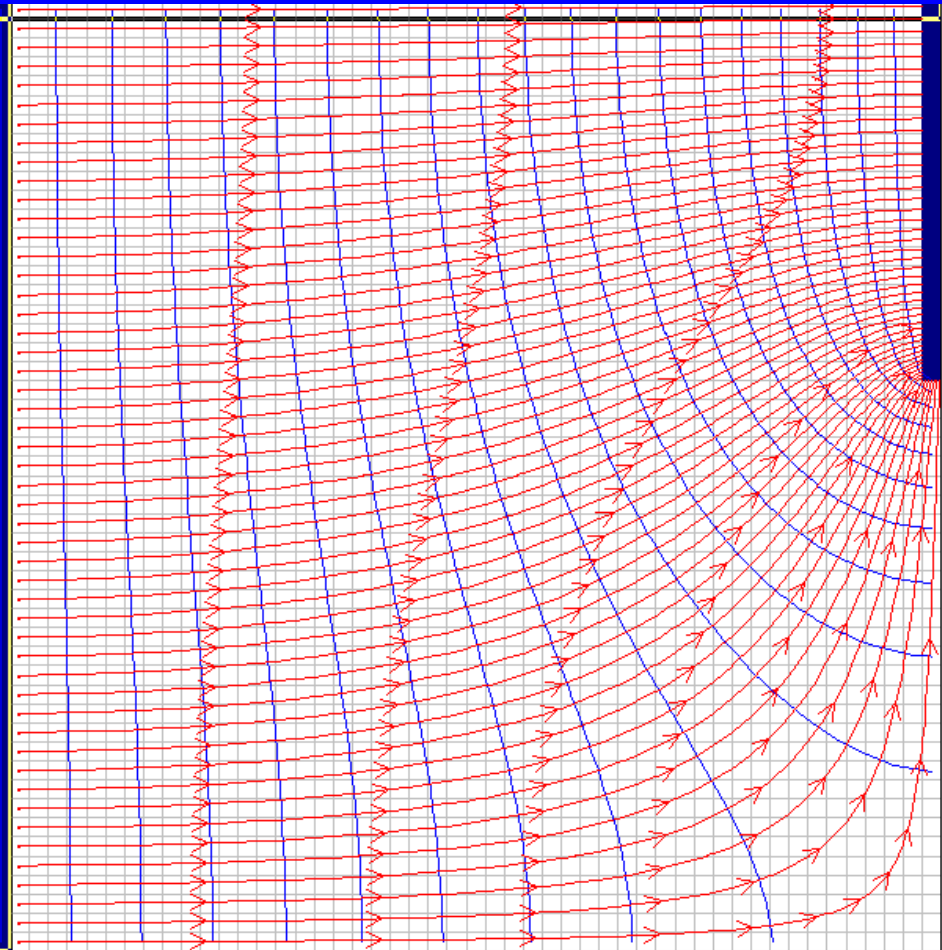


$$K_x = K_y = 10 \text{ m/d}$$

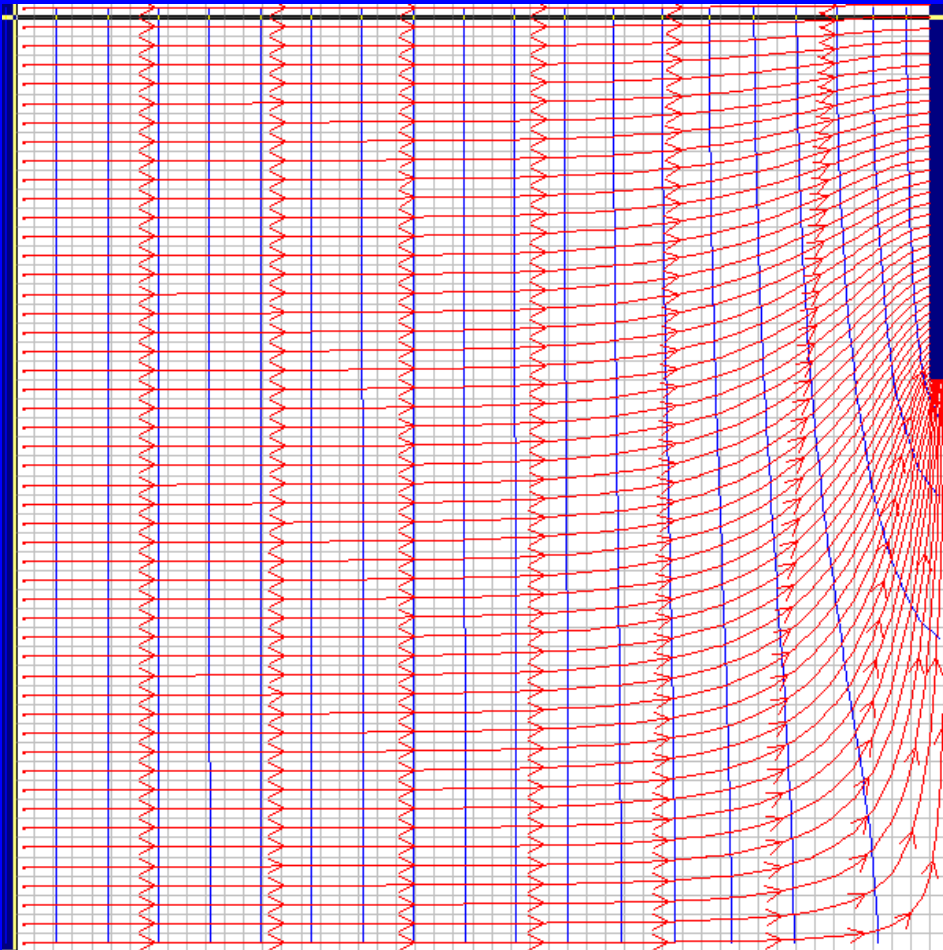
$$K_y = 0.1 K_x$$

# Flow net

- Flow net in the homogeneous and anisotropic aquifer



$$K_x = K_y = 10 \text{ m/d}$$



$$K_y = 10 K_x$$