



UNIVERSITY OF LEEDS

MATH3001: Project in Mathematics.

Analysing the November 2012 Flood of the River Avon in
Stratford-upon-Avon, Warwickshire, and assessing examples of
flood mitigation strategy; using flood-excess volume.

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1 Introduction

Major flooding events have had a huge detrimental effect on the UKs' economy, its business and the community at large; *"In England and Wales alone, over 4 million people and properties valued at over 200 billion are at risk"* [1] while the cost of the flooding across the UK on Boxing Day 2015 was estimated to have topped £5 billion nationwide [2]. Thus, this project aims to use the analysis of flood data from the floods of a range of rivers and subsequently assess, by means of flood-excess volume, the cost efficacy of various proposed and hypothetical flood-mitigation policy with the aim of assisting policy makers, businesses and the general public in reducing the negative affects of floods. One way of achieving this, besides the creation of this report, is to produce an automated code that can produce a value for the flood-excess volume of any flood, thereby allowing policy makers and the general public to analyse floods using flood-excess volume which may be pertinent their interests or to investigate methods of mitigating, or even preventing, future flooding in their area.

To mitigate a flood is to reduce the amount of flooding, and thereby the damage as a result of such flooding, caused when a river breaks its banks. Flood-mitigation has been attempted since the very origins of non-nomadic humanity as the negative financial and social repercussions of a large flood (such as loss of property, crops, infrastructure and even life) are severe; early examples of attempts to mitigate floods include the construction of stilt-houses by the Yue people in Ancient China over 7000 years ago [3]. In the modern world there are many different methods of flood-mitigation that can be used including, but not limited to: Sustainable Urban Drainage Systems (SuDS), Natural Flood Management (NFM), high flood walls or barriers, river bed widening and the drawing down of drinking water reservoirs before predicted heavy rainfall. Each of these stated methods will be further explored as part of this report.

Floods can be categorised into six broadly different types: coastal, surface water (generally caused by rainfall in an area with poor permeability), river, sewer, groundwater and the flooding that results from the failure of a dam [4]. The focus of this report will be on river flooding, also known as fluvial flooding, and the methods designed to mitigate this specific type of flooding.

Flood-excess volume can be simplistically defined as the difference between the total volume of water discharged at a certain spatial location on a river during the time it is in flood and the maximum volume of water that can be discharged by the river at the same location over the same time period without causing flooding [5]. This makes the flood-excess volume the volume of flood water that is actively causing flooding at that location on the river and therefore, by quantifying the volume of water a certain flood-mitigation policy reduces a flood by as well as the cost of implementing it, it is possible to evaluate the percentage of flood-excess volume mitigated by said policy. The cost per percent of flood-excess volume mitigated of each policy can then be calculated and it is this method of analyzation that will be employed in this report.

As the integral work forming the basis of some sections in this report was performed as part of a team (comprised of A. Feilden, S. Kennet, M. Saunders, J. Willis and A. Chapman), an outline of the report is included below, highlighting which sections were produced individually and which as part of the wider team.

– A review on the concept of flood-excess volume, introduced above, is performed in §2.

This has been interpreted individually by the author based on the work performed by O. Bokhove, T. Kent and M. Kelmanson in [5] and [6].

- Analysis of the 2015 Boxing Day flooding of the Rivers Aire in Leeds within the county of West Yorkshire is included in §3. This analysis includes calculation of flood-excess volume and the main reason for including this example analysis is to verify that the techniques used to calculate the flood-excess volume in the rest of the report are accurate. The original analysis was performed by the whole team, with the assistance of O. Bokhove and T. Kent.
- The rest of the report includes analysis and assessment individually performed by the author. §4 includes the calculation of the flood-excess volume for the November 2012 floods along the River Avon, looking at two locations along the river; Stratford-upon-Avon and Warwick. Also included is the flood-excess volume calculated by A. Fielden as a verification of the authors results.
- Having analysed flooding of the River Avon in §4, §5 contains a cost efficacy assessment of both proposed and hypothetical flood-mitigation scenarios, using the flood-excess volumes calculated in §4.
- A summary of the report is in §6 and conclusions from the report are drawn there. Acknowledgements and suggested further reading, including a web page created by the team giving instructions on how the general public can calculate the flood-excess volume for themselves, is also included here.

2 Definition of flood-excess volume

The flood-excess volume (henceforth abbreviated to FEV) is defined to be the volume of flood water that causes flooding at a specified location along a flooded river. FEV is therefore dependant on the threshold river level h_T . The definition of h_T is the maximum level a river can reach without flooding; i.e. the river floods if the in situ river level \bar{h} (the estimated river level across the entire width of the river at that location at that time) rises above h_T . The flow rate Q is given from a pre determined rating curve which gives Q as a function of \bar{h} . All the floods analysed as part of this report have the same equation relating Q and \bar{h}

$$Q(\bar{h}) = c_k(\bar{h} - a_k)^{b_k}, \quad k = 1, \dots, m, \quad (2.1)$$

where the rating curve coefficients a_k , b_k and c_k have been provided by the Environment Agency via email. The dimension of a_k must be the same as the river level \bar{h} it is being subtracted from, i.e. m^3 . As b_k is a power, it must be dimensionless which then forces c_k to have dimension $\text{m}^{3-b_k}/\text{s}$ in order for the dimension of the right side of the equation to match the dimension of the left, $[Q(\bar{h})] = \text{m}^3/\text{s}$. Each value of k corresponds to a range of \bar{h} values, i.e. $h_{k-1} < \bar{h} < h_k$. During flooding events the maximum river level achieved is often higher than the upper bound of the last interval at which point the upper bound can be extrapolated to the maximum value of \bar{h} . As $\bar{h} = \bar{h}(t)$, the rating curve $Q = Q(\bar{h})$ means that the flow rate Q can be found implicitly to be a function of time; $Q(\bar{h}(t)) = Q(t)$.

2.1 Approximations of the flood-excess volume

FEV can be approximated using three different methods, each of varying accuracy. It should be noted that as the FEV is a volume it is in units of m^3 . The total volume V discharged by a river with flow rate $Q(\bar{h})$ over a given time interval can be approximated as

$$V \approx \sum_{i=1}^n (Q(\bar{h}_i)) \Delta t, \quad (2.1a)$$

where n is the number of values for $Q(\bar{h})$ within the time interval and t is the time elapsed between each value of $Q(\bar{h})$. As the FEV (V_e) is defined as the excess volume discharged by the river once it has reach a level above h_T until it is back below h_T , equation 2.1a can easily be used to provide the best approximation for FEV

$$V_e \approx V_{e1} = \sum_{i=1}^n (Q(\bar{h}_i) - Q_T) \Delta t, \quad (2.1b)$$

where $Q_T = Q(h_T)$ is constant as h_T is a fixed value [6]. In this case, n is the number of $Q(\bar{h})$ values from when the river level \bar{h} goes above h_T until it is back below h_T . The time that this takes is therefore the equivalent of the duration of the flood T_f . The time interval between subsequent $Q(\bar{h})$ values is thus $\Delta t = T_f/n$ and so as $n \rightarrow \infty$, $\Delta t \rightarrow 0$ this approximation becomes the exact area between the curve $Q(\bar{h})$ and the constant value Q_T , seen to be the shaded area in Figure 1 b) [6].

A second, less accurate, approximation of V_e is given to be the area of the emboldened rectangle in Figure 1 b)

$$V_e \approx V_{e2} = T_f(Q_m - Q_T), \quad (2.1c)$$

where $Q(h_m) = Q_m$ is the mean flow rate above Q_T . Q_m can be found relatively accurately for each of the floods analysed by first finding the FEV using equation 2.1b and then rearranging equation 2.1c so that

$$Q_m = \frac{V_{e1}}{T_f} + Q_T, \quad (2.1d)$$

from which h_m is then found by rearranging equation 2.1 [5].

The least accurate third approximation of V_e is

$$V_e \approx V_{e3} = (h_m - h_T) T_f \frac{Q_{max}}{h_{max}}, \quad (2.1e)$$

which is found because $Q_m \approx (Q_{max}/h_{max})h_m$ and similarly $Q_T \approx (Q_{max}/h_{max})h_T$, this approximation is mostly used when there is no rating curve that explicitly gives $Q(\bar{h})$ [6]. h_{max} and Q_{max} stand for the maximum river level and flow rate value achieved during the period being analysed respectively.

All FEVs calculated for the five floods analysed in this report have been calculated using the most accurate approximation, equation 2.1a, where Δt is 15 minutes.

3 Analysis: the Boxing Day 2015 floods of the River Aire in Leeds

The Boxing Day flooding in the year 2015 of the River Aire was devastating to the population of the city of Leeds. “The direct cost to the city was an estimated £38.3m with the cost to the wider

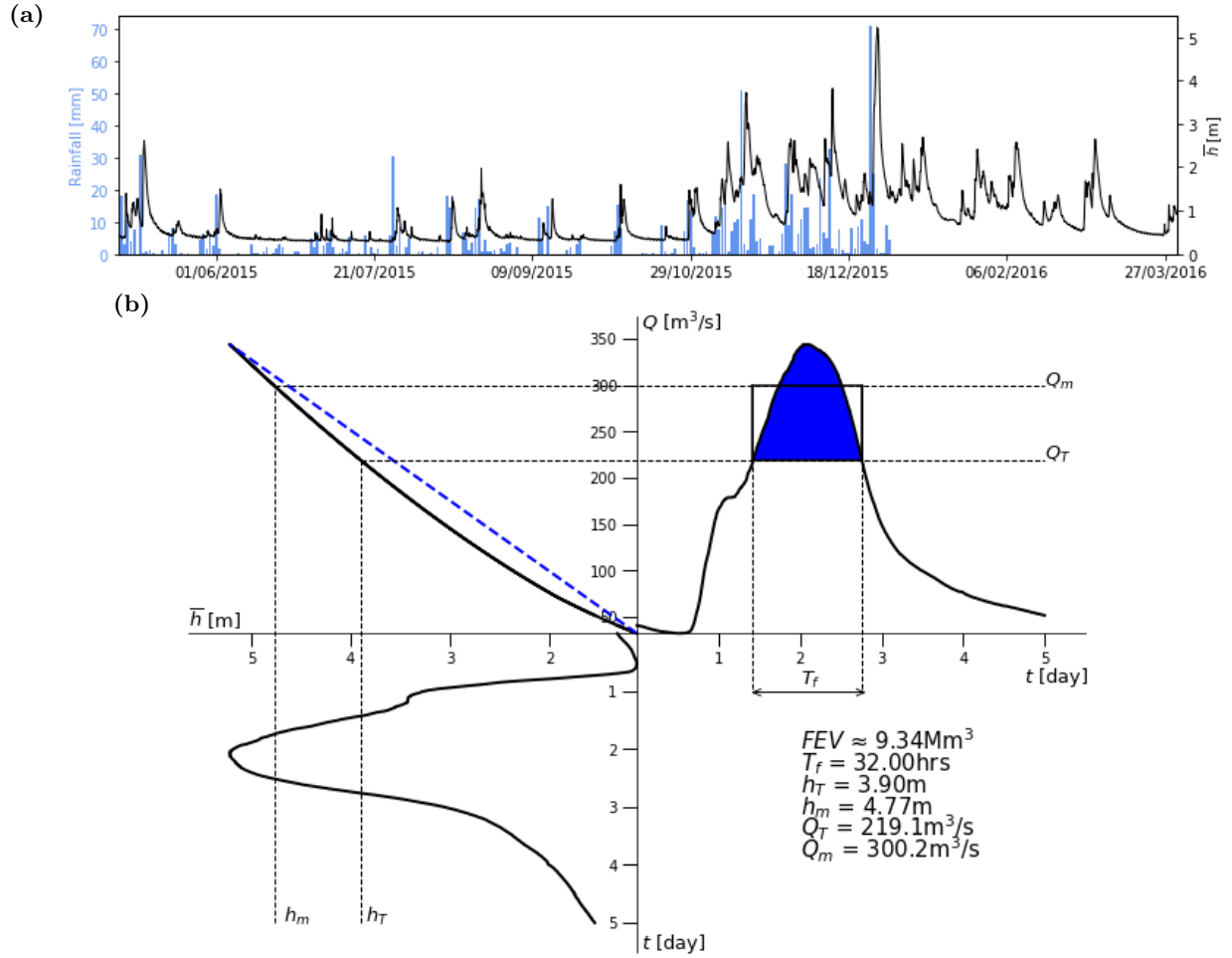


Figure 1: Flow rate and river height data for the River Aire at the Armely gauging station in Leeds: a) river height (\bar{h} [m]) [5] and catchment rainfall [mm] data [8] for a year starting from May 2015 (note, catchment rainfall data was not available for 2016) and b) quadrant plot for the 2015 Boxing Day floods of the River Aire. The top-left quadrant displays the rating curve, the rating curve parameters used to plot \bar{h} against Q may be found in Table 1, alongside its linear approximation (the blue dashed line); whilst the top-right and bottom-left quadrants display the river height (\bar{h} [m]) and flow rate (Q [m³/s]), respectively, during the floods. FEV is then the blue area below the flow rate curve (a function of \bar{h} , $Q(\bar{h})$, obtained from the rating curve) and above the flow rate corresponding to a given threshold height, $Q_T = Q(h_T)$. $\bar{h} = \bar{h}(t)$, where t is the number of days after 00:00 on the 25th of December 2015, meaning $Q(\bar{h}) = Q(t)$. Equation 2.1a is applied to provide $V_e \approx V_{e1} = 9.34\text{Mm}^3$, the blue shaded area. The emboldened rectangle approximates the FEV using equation 2.1c so that $V_{e2} = (Q_m - Q_T) \cdot T_f \approx V_{e1} = 9.34\text{Mm}^3$, thereby providing an approximation of Q_m given fixed $Q_T=219.1\text{m}^3/\text{s}$ and calculated $T_f=32$ hours. Rearranging the rating curve equation 2.1 will provide an evaluation of $h_m=4.77\text{m}$ from Q_m .

k	h_{k-1} [m]	h_k [m]	c_k [m ^{3-b_k} /s]	b_k [-]	a_k [m]
1	0.2	0.685	30.69	1.115	0.156
2	0.685	1.917	27.884	1.462	0.028
3	1.917	4.17	30.127	1.502	0.153

Table 1: The coefficients a_k , b_k and c_k and their corresponding upper (h_k) and lower (h_{k-1}) bounds on \bar{h} [5] applied in the calculation of the rating curve, using equation 2.1, at the Armley gauging station on the River Aire.

city region being more than £500m” [7] due to the flooding of an estimated 3355 properties in Leeds. In contrast, the most recent major flooding of the River Aire in Leeds before the Boxing Day 2015 floods occurred in 2000 when only around 100 properties were flooded [7]. Due in part to the severity of the floods in December 2015, the Leeds City Council alongside the Environment Agency have developed the Leeds Flood-Alleviation Scheme II aimed at preventing any future major flooding from occurring within the city limits [5].

3.1 Verification: the FEV of the flood

A verification that the Python code the author has written to calculate the FEV, T_f , h_m , Q_T and Q_m from specified rating curve parameters, a value for h_T and river level data is necessary before any further calculations of the FEV, and therefore any analysis based on these calculations, are attempted. Hence, the values for each that the code provides given river level data taking from the Armley gauging station on River Aire for the time encompassing the Boxing Day 2015 floods given a h_T of 3.9m and the rating curve parameters specified in Table 1 have been compared to the values found by O. Bokhove, M.A. Kelmanson and T. Kent in [5]. The values obtained by the author may be seen in Figure 1 b) and are identical to the values found in [5] when taking the values to the same degree of accuracy, two decimal places. As such, any subsequent calculations of the FEV in this report can be assumed to be reasonably accurate. Further verification was performed by comparing the values obtained by the author for the Boxing Day 2015 floods of the River Calder (Figure 12 b) in Appendix A.1) and the June 2007 floods of the River Don (Figure 13 b) in Appendix A.2) with the values discovered by O. Bokhove, M. Kelmanson, T. Kent, G. Piton and J. Tacnet in [6]. These values were again accurate to two decimal places.

The chosen threshold height $h_T = 3.9\text{m}$, estimated in [5], therefore yields:

$$V_{e1}(h_T = 3.9\text{m}) \approx 9.34\text{Mm}^3, \quad (2.1e)$$

where $V_{e1}(h_t = 3.9\text{m})$ is found from the application of equation 2.1b whilst, by converting the length of time in hours that flooding occurs for, $T_f = 32\text{hr}$, into seconds by the multiplication of 60^2 , the least accurate third approximation of the FEV provided by equation 2.1e gives $V_e \approx V_{e3}(h_T = 3.9\text{m}) = 6.2\text{Mm}^3$ where $Q_{max} = 350\text{m}^3/\text{s}$ and $h_{max} = 5.21\text{m}$. It can therefore be seen that the least accurate approximation V_{e3} of the FEV V_e is roughly 66% of the most accurate approximation, V_{e1} .

4 Analysis: the November 2012 flood of the River Avon in Warwickshire

Warwickshire has a long history of flooding as the large amount of agricultural land, which is relatively impervious, within the county makes it particularly vulnerable to surface water flooding whilst the large number of rivers, the two biggest being the River’s Avon and Leam, make fluvial flooding a threat. Numerous flooding events in recent history confirm the risk that Warwickshire faces from floods. These include: January 1992, Easter 1998, August 1999, June 2005, June/July 2007, December 2008, November 2012, July 2014, December 2015 and August 2016 [9]. In fact, “around one in seven commercial properties and one in ten residential properties are at risk from flooding from rivers or surface water” [10] in Warwickshire and that statistically “the fear of flooding is greater than the fear of crime in large areas of the county” [10].



Figure 2: Flooding in Stratford-upon-Avon and Warwick: the top left photo is of a plaque in Stratford-upon-Avon displaying historic flood level highs; it was erected before the 2012 flooding event [12]. The top right is a photo of Bancroft in Stratford-upon-Avon during a flood [13]. The bottom two photos are of the famous Warwick Castle, the left [14] unflooded and the right [15] during a flood. The gauging stations in both Warwick and Stratford-upon-Avon are very slightly upstream from the Castle and Bancroft respectively meaning it is these areas of flooding that will be analysed.

4.1 The flood in Stratford-upon-Avon

The local knowledge of the author about the plaque in Figure 2 raised Stratford-upon-Avon, situated on the banks of the River Avon (an eastern tributary of the longest river in Britain, the River Severn [11]), as a location of interest within Warwickshire that has a history of flooding. Although there have been many instances of flooding in Stratford-upon-Avon in recent years, River levels UK [16] states that the most recent high achieved by the River Avon at the Stratford-upon-Avon gauging station was during the November 2012 floods, reaching a river level of 1.89m at 15:30 on Monday the 26th of November. The government run flood information service [17] states that property flooding is possible in Stratford-upon-Avon once the River Avon has reached a height of 1.7m, and so this is the threshold height h_T that will be used to calculate the FEV of the November 2012 floods. It should be noted that personal correspondence with the Environment Agency [18] suggested a threshold height of 1.2m but as the flood information surface states that low lying land may be flooded once the River Avon has reached 1.2m, the threshold height corresponding to a more significant flood of property is taken. The chosen threshold height of $h_T = 1.7\text{m}$ yields

$$V_{e1}(h_T = 1.7\text{m}) \approx 1.83\text{Mm}^3, \quad (5.1a)$$

where $V_{e1}(h_T = 1.7\text{m})$ is found by applying equation 2.1b. Thus, the less accurate approximation to V_e , $V_{e3}(h_T = 1.7\text{m}) \approx 1.45\text{Mm}^3$ given by equation 2.1e where the $Q_{max} = 131.03\text{m}^3/\text{s}$ and $h_{max} = 1.89\text{m}$, is roughly 79% of V_{e1} . This is actually a relatively accurate approximation; the same approximation using equation 2.1e for the FEV of the River Aire flood in 2015 performed in §3.1 is just 66% of the best approximation V_{e1} . The river levels for the River Avon in Stratford-upon-Avon for November and December 2012 shown in Figure 3 a) suggest another spike in the

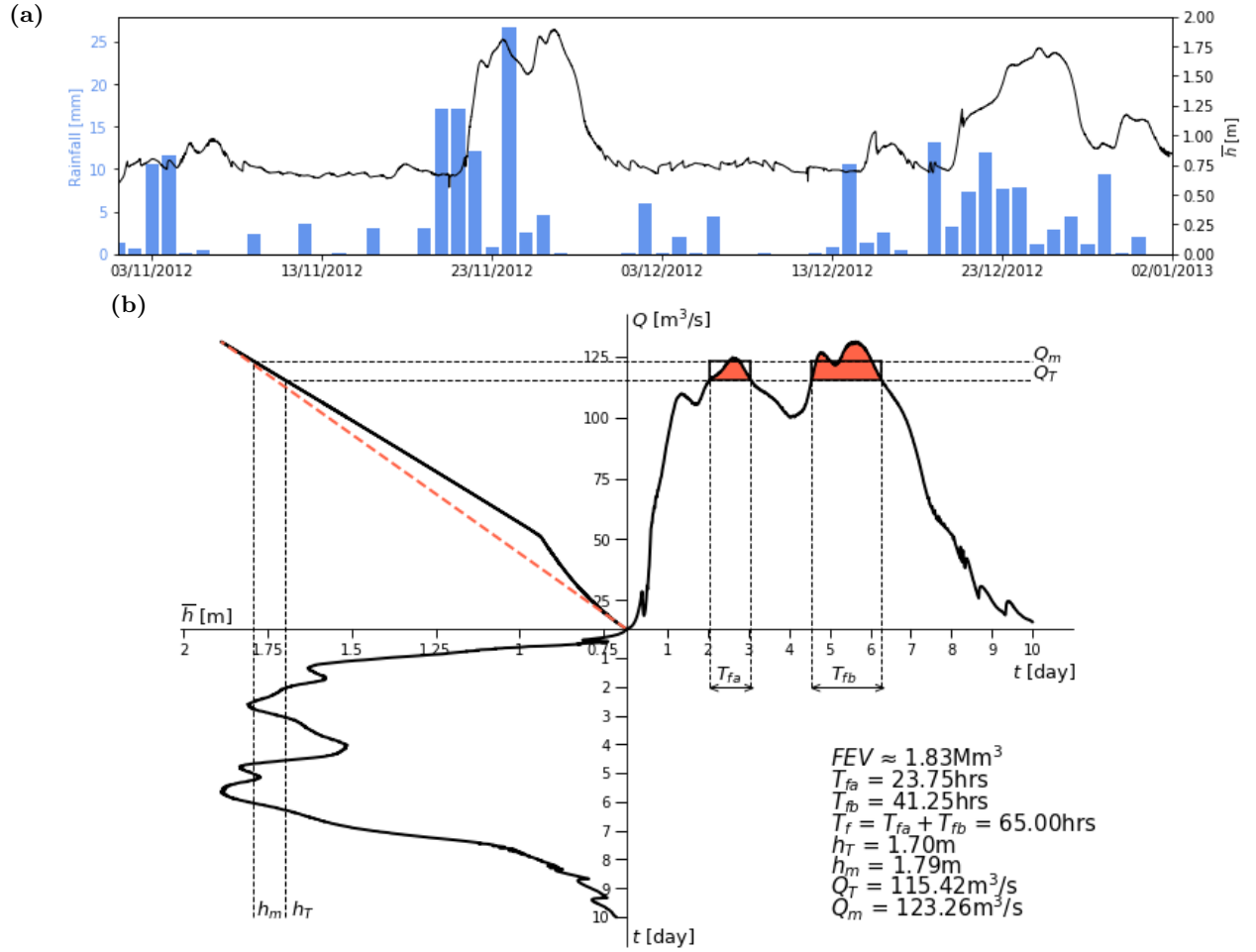


Figure 3: Flow rate and river height for the River Avon at the Cox's Yard gauging station in Stratford-upon-Avon: a) river height (\bar{h} [m]) [18] and catchment rainfall [mm] data [8] for the months of November and December 2012 (note, catchment rainfall data was not available for this gauging station so data was instead obtained for the next nearest geographical station, Alscot Park on the River Stour) and b) quadrant plot for the November 2012 floods of the River Avon at Stratford-upon-Avon (*cf.* Figure 1 b)), where the time values in this case are the number of days from 00:00 on the 21st of November 2012. The rating curve parameters used to plot \bar{h} against Q may be found in Table 2

height of the river towards the end of December. The FEV for this flood has been calculated and found to be minimal in comparison to the FEV of the flood event in November, see Figure 14 in Appendix A.3.

k	h_{k-1} [m]	h_k [m]	c_k [m ^{3-b_k} /s]	b_k [-]	a_k [m]
1	0.136	0.938	158.04	2.85438	0.262919
2	0.938	1.427	87.0362	0.962129	0.358741

Table 2: The coefficients a_k , b_k and c_k and their corresponding upper (h_k) and lower (h_{k-1}) bounds on \bar{h} [18] applied in the calculation of the rating curve, using equation 2.1, at the Cox's Yard gauging station on the River Avon. This rating curve has then been used in the calculation of the FEV in Figure 3

The FEV of $1.83Mm^3$ can be handily represented by the capacity of a square lake of depth 2m with side-length 956.56m, as seen in Figure 4 b). The speculative nature of estimating h_T means that the FEV of a flood can vary wildly depending on the chosen h_T . The relationship

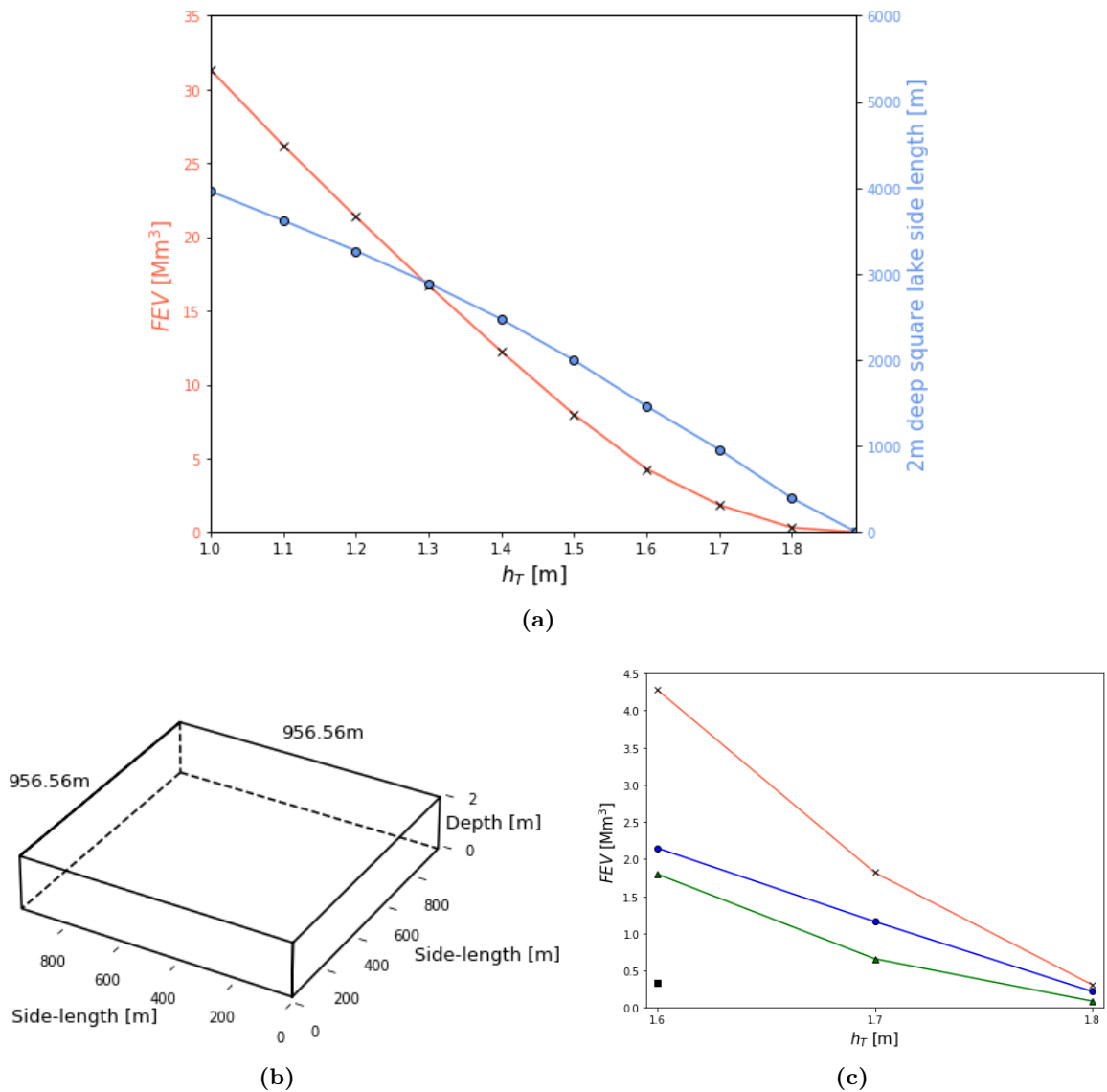


Figure 4: Stratford-upon-Avon square lake graphs for the November 2012 flood: a) is a visual aid demonstrating the relationship between a chosen h_T and both its corresponding FEV (the red line with crosses) and square lake side length (the light blue line with circles) where b) demonstrates what is meant by square lake side length, the side length of our calculated FEV's (1.83Mm^3) 2m deep square lake being 956.56m. c) shows how the calculated FEV (*cf.* Figure 1 b)) for the seperated peaks (the middle peak corresponds to the green line with triangles, the right peak to the blue line with circles and the leftmost peak, which then disapears above $h_T = 1.6\text{m}$, to the black square) of $Q(t)$ above Q_T (as seen in Figure 4 b)) is then added together to approximate the total FEV (the red line with crosses).

between possible threshold heights and the estimate of the FEV that they result in alongside their equivalent square lake of depth 2m side-length can be seen in Figure 4. a). The splitting of the flow rate into two peaks above the threshold flow rate Q_T means the River actually floods twice in a short space of time in November 2012. The FEV of each peak must then be added to provide a value for the total FEV discharged during the floods of November 2012; Figure 4. c) gives the FEV of each peak at a certain threshold height alongside the total FEV summing them provides. Although the mean flow rate of the River Avon once it has begun flooding at the Cox's Yard gauging station in Stratford-upon-Avon, $Q_m = 123.26\text{m}^3/\text{s}$, is much smaller than

the mean flow rate of the River Calder once it began flooding in December 2015, $197.50\text{m}^3/\text{s}$ *cf.* Figure 12 b) in Appendix A.1, the fact that the Stratford-upon-Avon flood lasted just under eight times as long makes the FEV produced by both of the floods comparable.

4.1.1 Verification: Stratford-upon-Avon FEV from the November 2012 floods

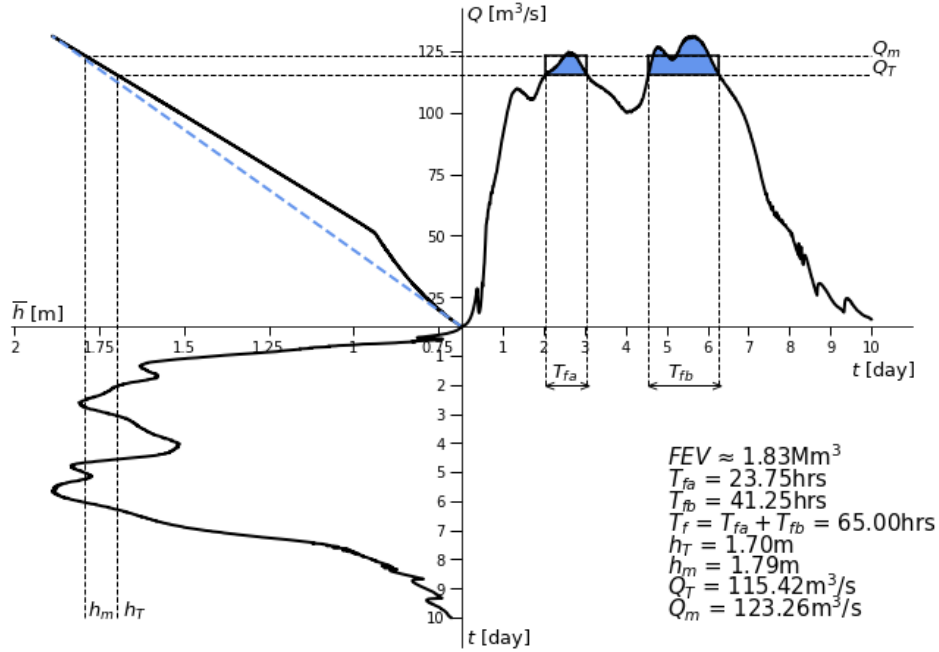


Figure 5: Graph produced by A. Feilden to verify the FEV and other values calculated for the November 2012 floods in Stratford-upon-Avon, *cf.* Figure 3 b).

A. Feilden has kindly plotted their own graph using the same river level data as the author in order to verify the results for FEV, T_f , h_m , Q_T and Q_m for the November 2012 floods in Stratford-upon-Avon, as given in Figure 3 b). Their graph may be found in Figure 5 and provides reassurance that the values calculated in Figure 3 b), in particular the calculated FEV value which is then subsequently used in analysis throughout the report, are correct.

4.2 The flood in Warwick

k	h_{k-1} [m]	h_k [m]	c_k [$\text{m}^{3-b_k}/\text{s}$]	b_k [-]	a_k [m]
1	0.960	3.000	40.6178	1.44854	0.917837

Table 3: The coefficients a_k , b_k and c_k and their corresponding upper (h_k) and lower (h_{k-1}) bounds on \bar{h} [18] applied in the calculation of the rating curve, using equation 2.1, at the Warwick gauging station on the River Avon.

Having analysed the November 2012 flood in Stratford-upon-Avon, it would be useful to next investigate the same flood at the next gauging station upstream from Stratford-upon-Avon in order to determine where the best placement is for flood mitigation structures along the River Avon. That gauging station is the Warwick gauging station. The locations of the gauging stations in Stratford-upon-Avon and Warwick are given in Figure 8 alongside an overview of the route of the River Avon between them. The flood information does not provide a river

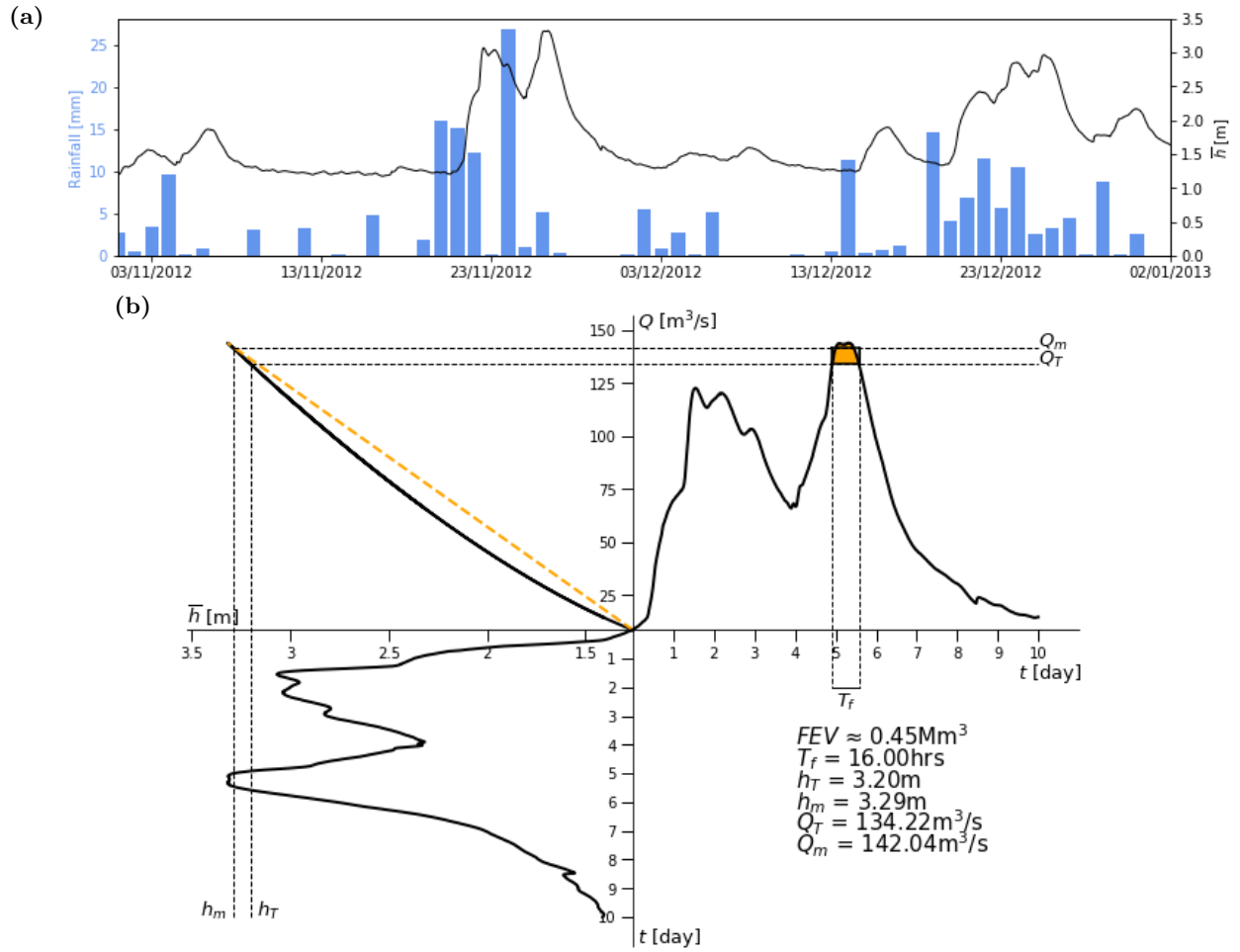


Figure 6: Flow rate and river height for the River Avon at the Warwick gauging station: a) river height (\bar{h} [m]) [18] and catchment rainfall [mm] data [8] for the months of November and December 2012. b) quadrant plot for the November 2012 floods of the River Avon at Warwick (*cf.* Figure 1 b)), where the time values in this case are the number of days from 00:00 on the 21st of November 2012. The rating curve parameters used to plot \bar{h} against Q may be found in Table 3

level at which property may be flooded in Warwick, the method of approximating h_T used for Stratford-upon-Avon, but the Environment Agency estimates that the flooding of low lying land may occur above a level of 2.3m [18]. Thus, a rough estimate for the threshold height can be found by noticing that the level at which property may flood is roughly 40% higher than the level at which low lying may flood in Stratford-upon-Avon. Thus $h_T = 3.2\text{m}$, 40% higher than 2.3m, is the very rough threshold height used in the calculation of the FEV produced in Warwick during the November 2012 floods.

This threshold height yields an FEV of

$$V_{e1}(h_T = 3.2\text{m}) \approx 0.45 \text{ Mm}^3, \quad (5.2a)$$

using equation 2.1b. The crudest approximation to V_e , $V_{e3}(h_T = 3.2\text{m}) \approx 0.23 \text{ Mm}^3$ given by equation 2.1e where the $Q_{max} = 144.38 \text{ m}^3/\text{s}$ and $h_{max} = 3.32\text{m}$, is roughly 51% of V_{e1} making V_{e3} a very poor approximation of V_e . An FEV of 0.45 Mm^3 is the equivalent volume of a square lake of depth 2m with side-lengths 608.28m, as seen in Figure 7 b). The difference in the FEV's for Warwick and Straford-upon-Avon is expected, the River Avon should collect more surface

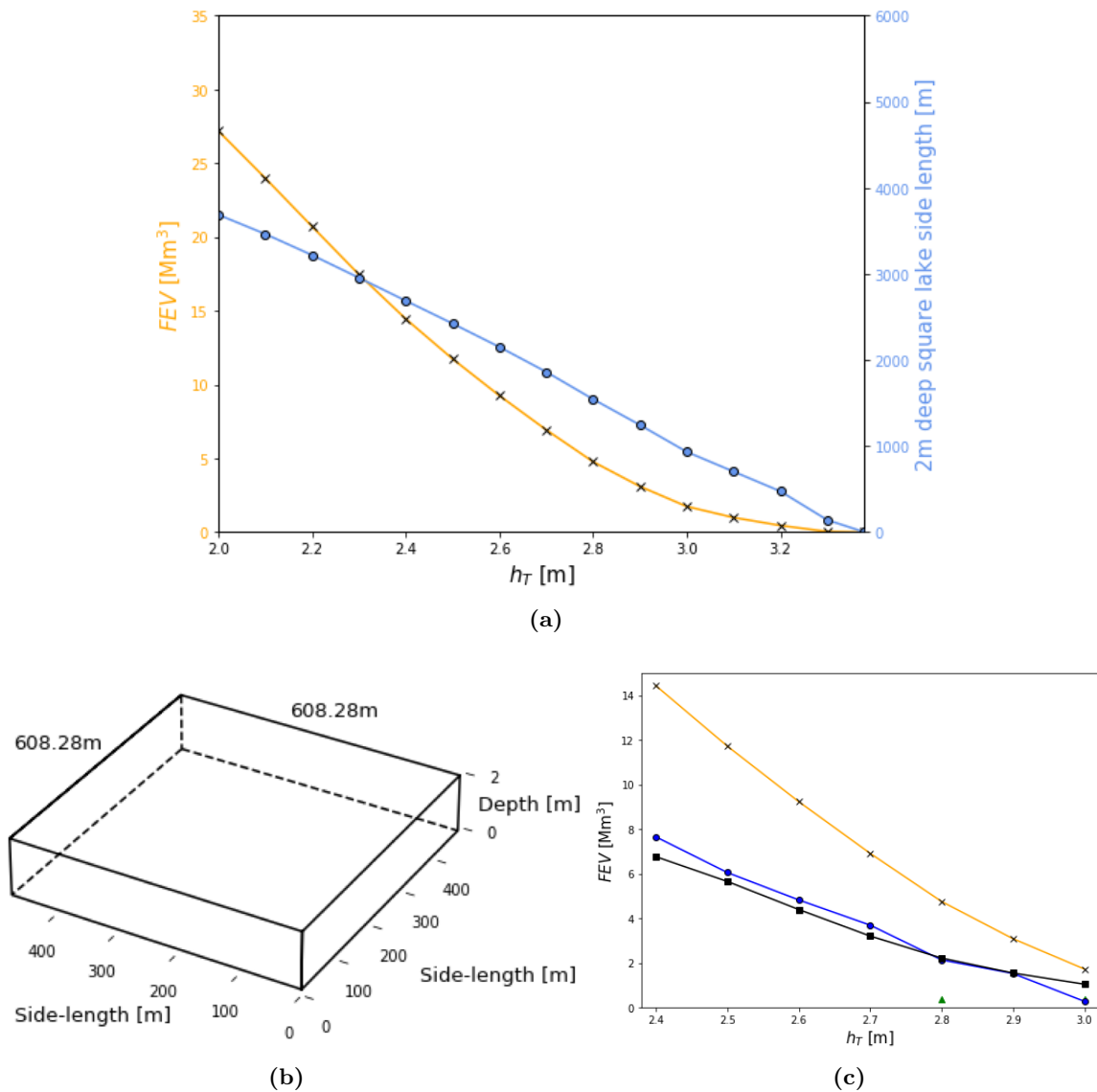


Figure 7: Warwick square lake graphs for the November 2012 flood: a) is a visual aid demonstrating the relationship between a chosen h_T and both its corresponding FEV (the orange line with crosses) and square lake side length (the light blue line with circles) where b) demonstrates what is meant by square lake side length, the side length of our calculated FEV's (0.74Mm^3) 2m deep square lake being 908.28m. c) gives the individual FEV of separated peaks when they occur ;the leftmost peak corresponds to the black line with squares, the right peak to the blue line with circles and the middle peak, when it does exist, to the green triangles (*cf.* Figure 4 c)). The total FEV is given by the orange line with crosses.

233 water run-off, rain water and/or ground water as it flows from Warwick to Stratford-upon-Avon.
 234 However, the very large discrepancy in FEV may suggest that the approximate threshold height
 235 of 3.2m is too large. Figure 7 a) shows the FEV of the Warwick flood given different values of
 236 h_T and may imply that h_T should be chosen to be in the range of 3m to 3.1m, resulting in an
 237 FEV of between 1.76Mm^3 , and 0.99Mm^3 .

5 Main results: flood-mitigation cost efficacy assessment using FEV

5.1 Past and planned mitigation projects upstream of Stratford-upon-Avon

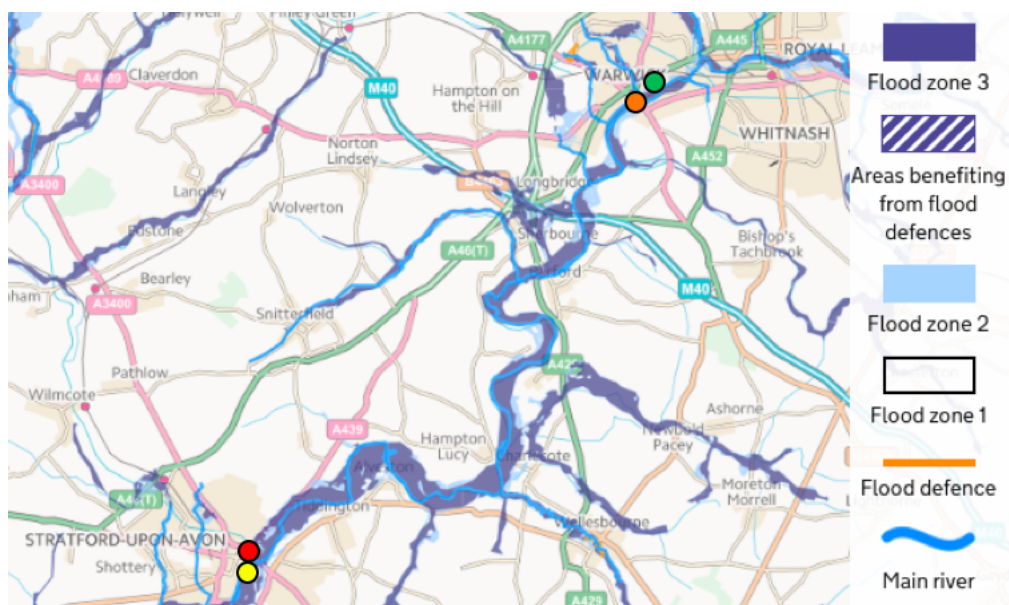


Figure 8: Flood-risk map for the stretch of the River Avon in between Warwick and Stratford-upon-Avon [19]. The location of the gauging stations in Warwick and Stratford-upon-Avon are given by the orange and red markers respectively. St. John's brook is denoted by the green marker and the Racecourse brook by the yellow marker; each of which could provide extra flood water storage.

Warwickshire County Council in association with the Environment Agency and Severn Trent Water have produced a local flood risk management strategy [10] that approaches the task of managing flooding from two angles, non-structural (aimed at mitigating the affects of a flood once it has begun as well as the general public's preparedness for a flood) and structural flood mitigation (aimed at preventing, or reducing the volume of, a flood). One example of non-structural flood mitigation being explored by the council is the introduction of a comprehensive sandbag protocol so that the public has quick and plentiful access to sandbags in the event of a flood [10].

However, in order to perform a cost effectiveness assessment using FEV of different methods of flood mitigation, the volume the relevant mitigation policy reduces the FEV by must be quantifiable and as such the assessment in this report will be focussed solely on structural flood mitigation. There are two structural flood mitigation projects mentioned in the preparatory pack sent out to the English Severn and Wye regional flood coastal committee ahead of their meeting on tuesday the 8th of January 2019 [20] which are the planned building of two brooks, the St John's brook in Warwick and the Racecourse brook in Stratford-upon-Avon. Other structural flood mitigation projects that have already been undertaken was the building of portable flood barriers owned by the Environment Agency that can be deployed ahead of a predicted flood, the installation of three pumps at the Stanford-on-Avon reservoir to enable the drawing down of the reservoir before predicted heavy rainfall as well as improvements to the storage capacity of the spillway of the reservoir.

St John's brook in Warwick (The green marker in Figure 8) is expected to be built less than

50 metres upstream of the river level gauging station in Warwick (the orange marker in Figure 8) and is expected to provide 32000m^3 of flood water storage potential at a cost of £1.45M whilst the Racecourse brook in Stratford-upon-Avon (represented by the yellow marker in Figure 8) will give around 16000m^3 of storage for £720000 [20]. Both brooks therefore provide 48000m^3 (equivalent to 2.62% of the November 2012 flood in Stratford-upon-Avon) of extra flood water storage. Taking the annual maintenance costs to be £600 [21] for both brooks, the price for installation and maintenance of the brooks over 50 years is approximately £2.2M meaning that the price of the two projects per percentage of FEV mitigated is £0.84M.

The third structural flood mitigation project is the manufacturing of 40km of temporary portable flood barriers by the Environment Agency for use in numerous locations including Stratford-upon-Avon [22]. This project has recently been completed and a flooding scenario simulation involving the barrier has been undertaken in the centre of Stratford-upon-Avon [13]. The cost per metre of constructing the barrier has been estimated to be between £5 and £10, meaning the cost of constructing 40km is in the range of £[0.2, 0.4]M [23]. The cost of the full 40km is included as it needs to be this length so that it can be used in numerous locations during a flood as if Stratford-upon-Avon is flooded then it is likely other locations near to Stratford-upon-Avon will be flooded and thus need the barriers too. The cost per deployment of these barriers is estimated to be in the range of £[1000, 20000] [23], and this along with the knowledge that the Avon has flooded 10 times (as mentioned in §4) since January of 1992, the cumulative costs of deployment over 50 years can be approximated as £[19000, 380000]. Assuming an average annual maintenance cost of £5 per m [24], giving a 50 year maintenance cost of 40km of barrier of £10M, the cost of building, running and maintaining the temporary flood barriers for 50 years is £[10.22, 10.38]M. The temporary nature of the barriers make it difficult to quantify the percentage of FEV mitigated because if the barrier is properly and punctually in place before the flood begins then 100% of the FEV is mitigated. If, however, the barrier is not put in place in time then 0% of the FEV will be mitigated. Thus, the cost per percentage of FEV mitigated could potentially be £[0.102, 0.104]M if the barrier is in place every time the river floods; on the other hand, the project cost of £[10.22, 10.38]M may not mitigate the flood at all if it is never in place when the river floods and is therefore a wasted expense. More detailed assessment on the probability of the barrier being in place is therefore needed before any concrete conclusions may be drawn.

Upstream of Warwick there is a drinking water reservoir in Stanford-on-Avon, named the Stanford reservoir, that has a maximum capacity of 500M gallons, equivalent to 2.27Mm^3 [25]. In 2017, the installation of what the Environment Agency classifies as ‘large’ pumps, capable of pumping 415 litres of water a second out of the reservoir [26], has allowed for the possibility of reducing the volume of the reservoir by 10% ahead of predicted heavy rainfall so as to provide an additional 0.227m^3 of flood water storage, thereby mitigating downstream flooding in such places as Warwick and Stratford-upon-Avon by this volume. The estimated capital cost of the installation of a large pump is £0.41M alongside annual maintenance, annual running and annualised periodic refurbishment costs of £8000, £5000 and £13000 respectively [27]. Therefore the cost over 50 years of three large pumps is £5.13M and as 0.227m^3 constitutes 12.4% of the FEV in Stratford-upon-Avon, this is equivalent to £0.41M per 1% of the FEV mitigated.

As part of the project that involved the installation of the three pumps in the Stanford reservoir, an extension of the spillway of the reservoir was conducted. This involved widening the 6m deep and 1.73km long spillway by 5.4m to a new width of 11.5m [28], providing an extra 56052m^3 of flood water storage. The widening of the spillway itself should not necessitate the need for any more maintenance besides that which the unwidened spillway required anyway. The budget of the project, including the installation of the pumps, is given to be £2.4M [28] meaning the estimated cost of the spillway modifications can be estimated as the overall cost

of the project minus the capital cost of installing three large pumps, as provided above. This estimated cost is therefore £1.17M and as 56052m^3 is 3.06% of the FEV of the November 2012 flood in Stratford-upon-Avon, the estimated cost of widening the spillway per percentage of the FEV mitigated is £0.38M.

Having performed a cost efficacy assessment of structural flood mitigation tactics in and around Warwickshire and how they could affect flooding in Stratford-upon-Avon, various hypothetical scenarios will now be similarly assessed and then compared with the nonspeculative methods discussed above.

5.2 Sustainable urban drainage systems (SuDS)

Sustainable urban drainage systems (or SuDS) encompasses flood mitigation techniques aimed at reducing the detrimental effects urbanisation has on surface water run off rates, which can cause both surface water and fluvial flooding [29]. Types of SuDS include green roofs and permeable paving and the potential effect of installing 50000m^2 , comprising an area sufficiently smaller than the area comprised of the Stratford-upon-Avon town limits, of both of these types of SuDS within Stratford-upon-Avon will be specifically explored. The estimated cost per m^2 of installing green roofs and permeable pavements is £90 and £40 alongside 50 year maintenance costs of £313 and £20 respectively [30]. Hence, the combined cost over 50 years of 50000m^2 of green roofs and 50000m^2 is approximately £22.17M. Models of both green roofs and permeable pavements and their affects on the volume of a flood have shown that about 0.116m^3 and 0.138m^3 of the flood is mitigated per m^2 of green roofing and permeable paving respectively [29]. Therefore, the combined effects of 50000m^3 of each of these methods could mitigate a flood by as much as 12700m^3 , comprising 0.69% of the FEV. The cost per 1% of FEV is then found to be £32.13M, an extremely large value. The apparent exceedingly poor cost effectiveness of SuDS in relation to the purpose of mitigating a fluvial flood may explain why Warwickshire County Council have decided to make it a requirement for any new properties built to have enough SuDS to mitigate their negative effect on surface water flooding [9]; as opposed to simply buying and installing new SuDS themselves.

5.3 Natural flood management

The term natural flood management (NFM) is an umbrella term describing the use of natural processes in reducing the risk and subsequent impact of flooding [31]. NFM can be broadly divided into 4 main subcategories which are the management of: the river and the flood plain itself, the causes of run off, woodland within the catchment area of the river and, finally, the coast [32]. Coastal flooding is not within the remit of this report and as some examples of run-off management were explored in §6.2, the next two flood mitigation ideas will focus on the first two listed subcategories of NFM.

5.3.1 Flow-attenuation features

Flow-attenuation features are designed to reduce the flow rate of a river, thereby reducing the FEV produced by a river over the same time interval, with the aid of natural processes. The flood management scheme in Blackbrook, St Helens, has comprised of the construction of four engineered log dams at a cost of £2000 [32]. These types of leaky dams are designed to allow the passage of water when it is moving at a low flow rate but to then reduce the flow of the river once the flow rate increases and the combination of the four dams in Blackbrook is expected to mitigate the FEV by 2500m^3 [33]. Scaling up this project by a factor of 20 would then involve the building of eighty of these leaky dams spread evenly along the River Avon upstream

of Stratford-upon-Avon and would constitute a reduction in the FEV of 50000m³, assuming uniform rainfall distribution. The natural nature of a leaky dam makes the life span of one limited and so, assuming a life span of 25 years, the base cost of building the eighty dams, £40000, would need to be doubled over a 50 year period. Regular maintenance would also have to be performed throughout the 50 years and so, by assuming the need for one permanently employed maintenance worker with an annual salary of £50000, the total 50 year cost of eighty engineered log dams can be expected to be in the region of £2.58M. As 50000m³ is the equivalent of 2.73% of the FEV, the approximate cost of installing eighty log dams is £0.95M.

5.3.2 Tree planting

The planting of new forestation reduces the impact of flooding by effectively ‘drinking’ rainwater from the ground thereby extending the length of time it takes ground to become water logged at which point any excess rain fall would run into the river, contributing to fluvial flooding. There is a flood mitigation scheme for the River Torne in Doncaster that has involved the creation of 46.5 hectares (equivalent to 0.465km²) of new catchment woodland which provided over 4000m³ of extra flood storage capacity at a cost of £130000 [32]. The catchment area of the River Avon is 2308km² [11] and one hypothetical flood mitigation measure could be the transformation of 3% of this area into woodland. This would be equivalent of the creation of 69.24km² of new forest or reproducing the River Torne scheme 148.9 times over. This scenario would then have a base cost of £19.36M which would provide additional flood storage of 596000m³, or 32.57%. An estimation of the maintenance cost per km² of new woodland is £864 [24], giving the 50 year cost of maintaining 69.24km² to be £2.99M. Thus, the cost per percentage of the FEV mitigated is estimated to be £0.69M.

5.4 Giving room to the river

5.4.1 Theory

The option explored here involves the widening of the river bed along the stretch of the River Avon that flows through Stratford-upon-Avon, a process that is also known as giving-room-to-the-river (GRR). Figure 10 gives an overview of the chosen location as well as the approximate length of the stretch of river that is proposed to be widened, 1.72km. In the 1800’s, an engineer, R. Manning, discovered a formula that could be applied to find the flow rate Q of a stretch of fluid in an open channel

$$Q = \frac{k}{n} AR^{2/3} S^{1/2}, \quad (6.4.1a)$$

given a parameter n , the longitudinal slope of the river bed S , the hydraulic radius R of the channel and the cross sectional area of the fluid in the channel A [34]. As this report deals only with quantities within the international system of units, $k=1$ [34]. The hydraulic radius R of the channel is defined as the cross sectional area A divided by the wetted perimeter P meaning equation 6.4.1a may be re-written as

$$Q = \frac{\sqrt{S}}{n} \frac{A^{5/3}}{P^{2/3}}. \quad (6.4.1b)$$

If a river is proposed have one if its banks widened by length w at a height of h_b above the river bed, the cross sectional area of the river when it is at level \bar{h} above h_b will be increased by $w(\bar{h} - h_b)$. This is equivalent to the area of the dashed parallelogram in Figure 9 a) with width $w = 3.5\text{m}$ and height $\bar{h} - h_b$ where $h_b = 0.5\text{m}$. Similarly, the wetted perimeter of the river will increase by $w + (\bar{h} - h_b)\sqrt{1 + b^2}$, equivalent to the perimeter of the left hand side of the

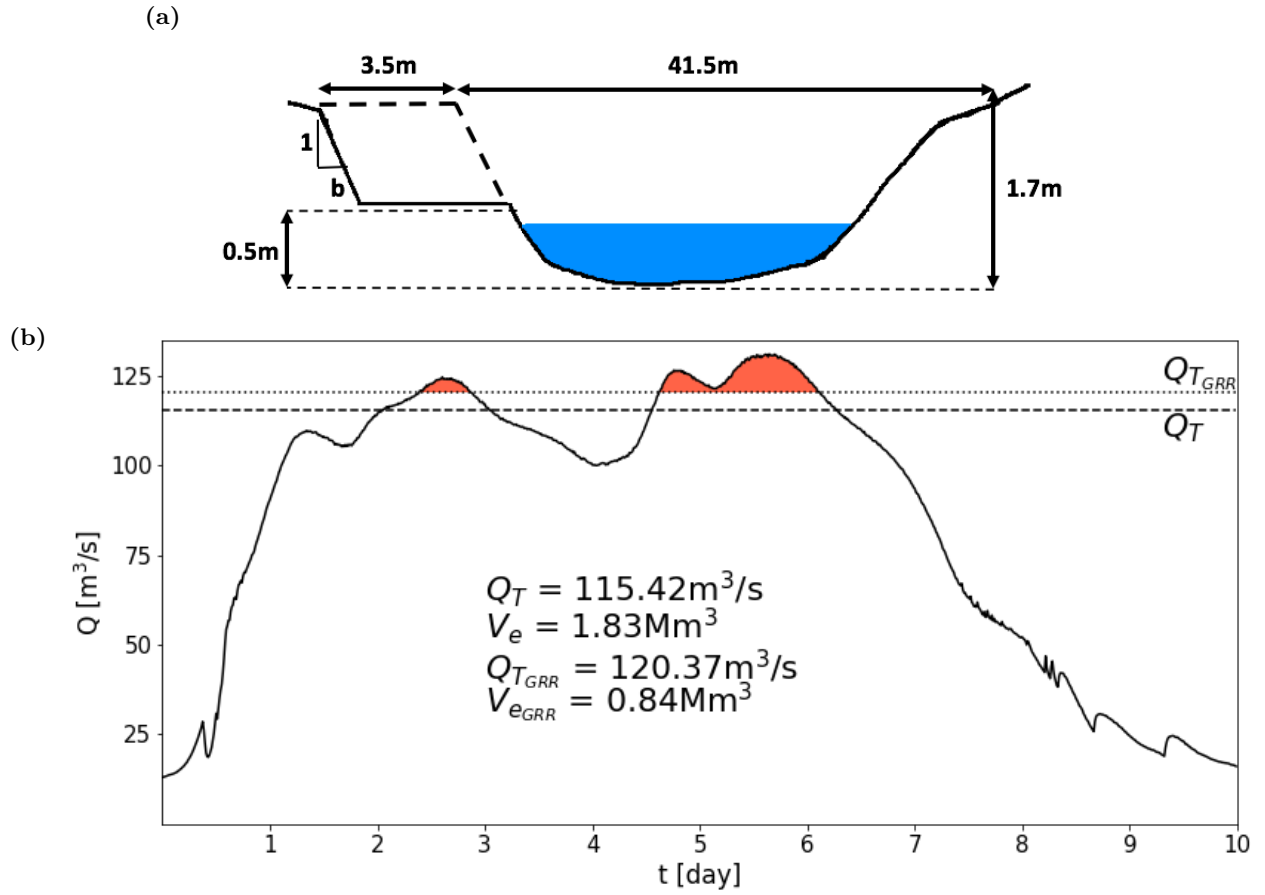


Figure 9: Giving-room-to-the-river: a) is a heavily simplified transverse profile of the River Avon if it were to be widened by 3.5m at a height of 0.5m on one bank where its width before the process is 41.5m and its threshold height remains as it was before, 1.7m [37]. The gradient of the new slope (1/b) is approximately the same as before the process ; b) is a zoom in of the top left quadrant of Figure 3 b). It is a plot of the flow rate $Q(\bar{h})$ against t , cf. Figure 3 b). The threshold level h_T does not change as part of the GRR process but the relationship between \bar{h} and Q , the rating curve, does. The two threshold flow rates on the graph, Q_T and $Q_{T_{GRR}}$, correspond to the river before it is widened and after respectively. Thus, the FEV mitigated by the GRR process can be quantified as the difference of the FEV's given by the two different threshold flow rates, $V_e - V_{e_{GRR}} = 0.99\text{Mm}^3/\text{s}$, which is equivalent to $\approx 54.10\%$ of the FEV before GRR. $V_{e_{GRR}}$ is the equivalent of the shaded red region on the graph and V_e is the FEV calculated in Figure 3 b).

395 parallelogram in Figure 9 a). Thus, the change in flow rate Q for a river widened along one
 396 bank by length w at height h_b above the river bed is

$$\Delta Q = \frac{\sqrt{S}}{n} \cdot \frac{(w(\bar{h} - h_b))^{5/3}}{(w + (\bar{h} - h_b)\sqrt{1 + b^2})^{2/3}}. \quad (6.4.1c)$$

397 Thus, the total flow rate of a river Q_{GRR} once it has undergone the giving-room-to-the-river
 398 procedure, will be given by adding the change in flow rate, ΔQ , to the original flow rate $Q(\bar{h})$.
 399 In the case of the hypothetical widening of the River Avon in Stratford-upon-Avon, both banks

are suggested to be widened not one; in which case ΔQ will double and

$$Q_{GRR} = Q(\bar{h}) + 2 \left[\frac{\sqrt{S}}{n} \cdot \frac{(w(\bar{h} - h_b))^{5/3}}{(w + (\bar{h} - h_b)\sqrt{1 + b^2})^{2/3}} \right]. \quad (6.4.1d)$$

The proposal for this hypothetical situation is that the River Avon has both of its banks widened by $w = 3.5\text{m}$ at a height $h_b = 0.5\text{m}$ above the river bed. The Gauckler-Manning constant n is dependant on numerous factors but most significantly the sinuosity and roughness of the channel the river flows through [34]. The River Avon can thus be expected to have a Gauckler-Manning constant of around 0.045, the typical value of n for a winding channel with some vegetation and stones [35]. The Encyclopedia Britannica [11] states that the length of the River Avon, 154km, corresponds to a fall of 150m. Therefore, the average longitudinal slope of the River Avon S is 0.001. Choosing b to be 1.73 and using the values for w , h_b , n and S gives the threshold flow rate after the GRR process $Q_{T_{GRR}}$ to be

$$Q_{T_{GRR}} = Q_T + 2 \left[\frac{\sqrt{0.001}}{0.045} \cdot \frac{(3.5(h_T - 0.5))^{5/3}}{(3.5 + (h_T - 0.5)\sqrt{1 + 1.3^2})^{2/3}} \right], \quad (6.4.1e)$$

where Q_T and h_T are as given in Figure 3 b). This is because although the channel the River Avon flows through is being widened, the height of the channel does not change meaning that neither will the height at which the River bursts its banks (h_T). Thus the threshold flow rate of the River Avon in Stratford-upon-Avon after the channel has been widened is $Q_{T_{GRR}} = 120.37\text{m}^3/\text{s}$, where $h_T = 1.7\text{m}$, which would then result in an FEV of 0.84Mm^3 (the red shaded area in Figure 9 b)). The FEV has then been reduced by 0.99Mm^3 , or 54.10% of the FEV of the November 2012 floods in Stratford-upon-Avon.

5.4.2 Cost efficacy assessment



Figure 10: Satellite (left) and map (right) view [37] of the proposed stretch of the river to undertake giving-room-to-the-river, i.e. the stretch of river that flows through Stratford-upon-Avon that when flooded puts properties at risk. The right photo gives the approximate length of this stretch of the river as 1.72km.

The cost per metre of excavating a channel of depth 1.5m and width 20m is £1510, assuming that a third of the of the material excavated is disposed of off-site [24]. This constitutes a channel of cross sectional area 30m^2 whilst the River Avon channel in Stratford-upon-Avon is estimated to have average width 41.5m [37] with a depth of 1.7m, equivalent to h_T , resulting in a cross sectional area of 70.55m^2 . The River Avon channel is thus approximately 2.35 times the size of the channel whose excavation costs £1510 per metre meaning this cost can be scaled

up to provide a rough estimate of the price for excavating the channel in Stratford-upon-Avon, £3548.50 per metre. Figure 10 gives that the channel through Stratford-upon-Avon is 1.72km long so the cost of giving-room-to-the-river for the entirety of this channel is £6.10M. The banks of the River Avon are heavily cladded as it passes through Stratford-upon-Avon and the cost of replacing this heavy cladding onto the newly excavated channel would be £1075 per metre per banking or £3.70M per 1.72km of both banks. Similarly to the widening of the Stanford reservoir spill way in §5.1, the cost of maintaining the channel before and after it has undergone GRR should be the same meaning these costs will not be considered as part of the total cost. In §5.4.1, the percentage the FEV is reduced by was found to be 54.10%. The cost of mitigating by this percentage has now been found to be £9.80M which means the price per percentage of the FEV mitigated is £0.18M.

6 Summary and discussion

Flood mitigation measure	Cost per percentage of FEV mitigated [£]
Nonspeculative	
The St. Johns and Racecourse brooks	0.84M
Temporary flood barriers	0.10M
Drawing down of the Stanford reservoir	0.41M
Widening of the Stanford reservoir spillway	0.38M
Hypothetical	
Green roofs and permeable paving	32.13M
Flow-attenuation features	0.95M
Tree planting	0.69M
Giving-room-to-the-River	0.18M

Table 4: Table showing the cost, over 50 years, of mitigating the FEV of the November 2012 floods in Stratford-upon-Avon by 1% by implementing each of the flood mitigation measures assessed in this report. The flood mitigation measures are split into those that are nonspeculative, i.e. they have already been or are planned to be undertaken, and hypothetical scenarios.

When restricted to only the flood mitigation projects that have been planned or already implemented with the aim of preventing flooding in Stratford-upon-Avon, two scenarios can be foreseen, one where the temporary barriers are in place on time to prevent flooding, and one when they are not. The scenarios involving nonspeculative flood mitigation measures without the temporary barriers is seen in square lake form in Figure 11 a). In this scenario, only 18.08% of the FEV is mitigated at a cost of £8.5M, or £0.45M per percentage mitigated. Thus the flood mitigation measures already planned to mitigate flooding in Stratford-upon-Avon, not including the temporary flood barriers, will not be sufficient to mitigate a flood of the same magnitude as the November 2012 floods despite it being a relatively cost-efficient scenario in terms of the cost per percentage of the FEV mitigated. If, however, the temporary flood barriers are taken into account then the FEV should be 100% mitigated at a total cost of £[18.72, 18.88]M, or £0.19M per percentage of the FEV mitigated, and is depicted in square lake form in Figure 11 b). This scenario takes into account that the St. John's brook, the Racecourse brook, the Stanford reservoir pump project and the Stanford reservoir spillway project have either been or are planned to be undertaken regardless of the temporary flood barriers and as such would only mitigate the flooding by 81.92% as opposed to its full mitigating potential of 100%.

Based on the cost efficacy assessment performed for various planned and possible flood mitigation measures aimed at reducing the impact of flooding in Stratford-upon-Avon, Table

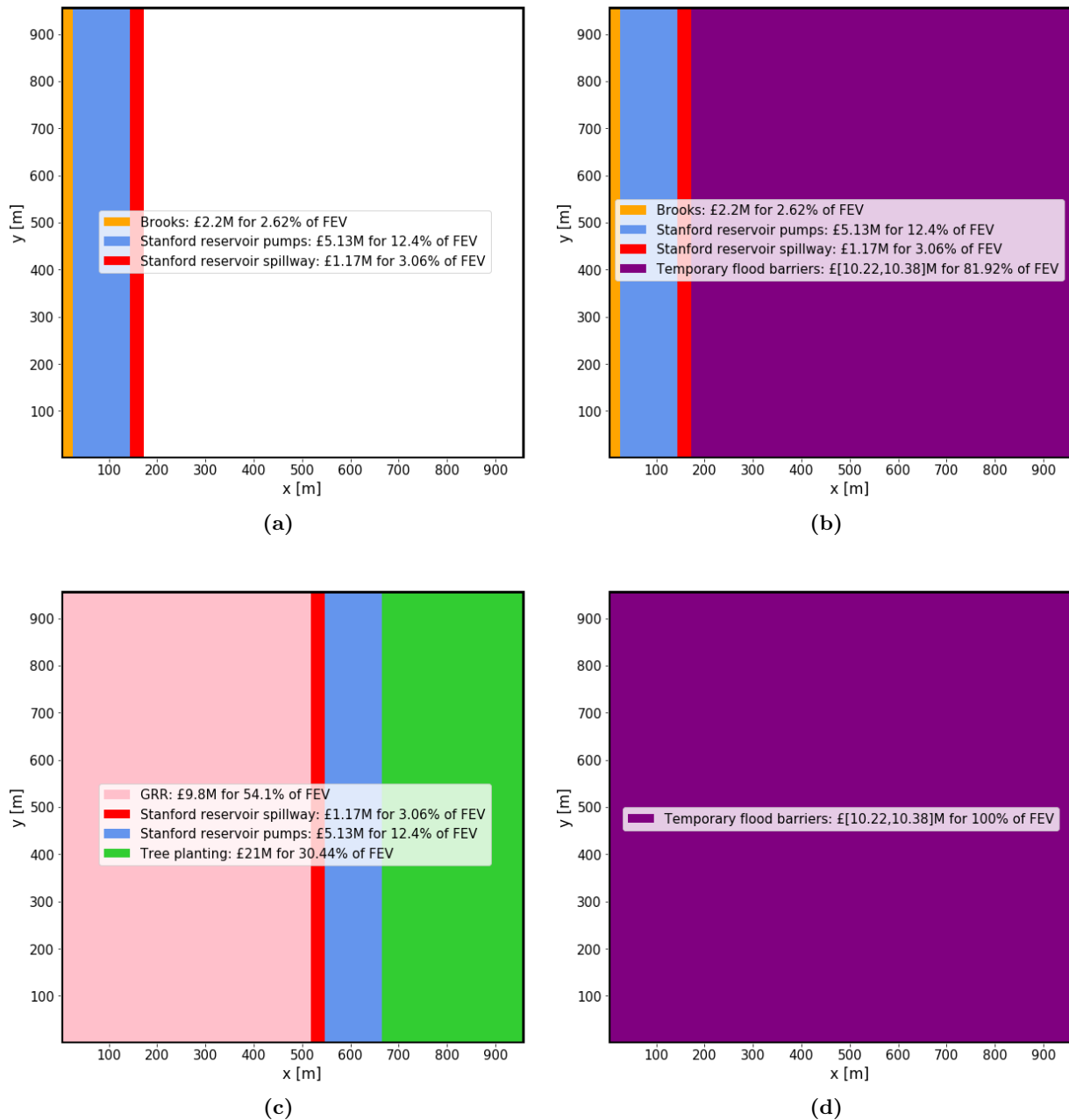


Figure 11: The four flood mitigation scenarios assessed in §5 in square lake form, i.e. each bar corresponds to a different flood mitigation method where the size of the bar depicts the volume of the 2m deep square lake graph from Figure 4 b) it mitigates the FEV by: a) is the scenario involving planned or already implemented flood mitigation strategies when the temporary flood barriers are not included whilst b) is the same scenario as a) but with the temporary flood barriers taken into account. The bottom two graphs show c) the most cost-effective scenario not taking into account the temporary flood barriers and assuming the River Avon cannot be widened by more than 3.5m and the Stanford reservoir projects remain identical to the way they have already been implemented whilst d) shows the most cost effective solution when the temporary barriers are taken into account.

454 4 has been compiled, displaying the cost over 50 years per percentage of FEV mitigated of
 455 these measure were to be implemented. Clearly, the most cost efficient hypothetical way of

mitigating the flood would be to solely use the temporary flood barriers, thereby reducing the FEV by 100% at a cost of £[10.2, 10.38]M (Figure 11 d)). However, as it cannot be assumed that they will be available to mitigate flooding every time the Avon bursts its banks, Figure 11 c) displays in square lake form the most cost efficient way of combining the assessed flood mitigation methods in order to mitigate the FEV by 100%. This scenario involves performing giving-room-to-the-river, i.e. widening the channel the River Avon flows through as it passes through Stratford-upon-Avon by 3.5m at a height of 0.5m above the river bed, which would mitigate the FEV by 54.1% at a cost of £9.8m, both the Stanford reservoir pumps and spillway widening projects (thus the author agrees that these projects were a viable and cost effective solution to mitigating flooding along the River Avon downstream of Stanford-on-Avon), which, when combined, mitigate the FEV by 15.46% at a 50 year cost of £6.3M. The extra 30.44% of the FEV could then be mitigated through the planting of 64.72km² of new catchment woodland, equivalent to 2.80% of the catchment area of the River Avon, at a 50 year cost of £21M. Thus, the most cost effective hypothetical scenario, assuming the River Avon cannot be widened by more than 3.5m, that would completely mitigate the FEV would cost approximately £37.1M. This sum is still vastly more than the £1.1M allocated to the Warwickshire County Council [10] for the purposes of flood alleviation (although this amount does not include the budget the Environment Agency has to tackle flooding in Warwickshire, an as yet undisclosed amount) and may suggest more funding is required in order to completely mitigate flooding in Stratford-upon-Avon.

In regards to the placement of the flood mitigation measures and the flooding in Warwick, the total flood volume capacity of nonspeculative measures implemented upstream of Warwick, St. John's brook and the Stanford reservoir projects, is equivalent to 70.01% of the November 2012 FEV in Warwick meaning that the majority of the flooding in Warwick should be mitigated by projects that have already been or are planned to implemented. If, hypothetically, at least 15.69km² of catchment woodland is planted in the effort to mitigate the flood in Stratford-upon-Avon then, assuming that the extra tree's planted will provide the same extra flood storage capacity in Warwick as it should in Stratford-upon-Avon, the FEV of the Warwick flood in November 2012 should be completely mitigated.

However, it should be noted that there are many other factors that go into the decision to implement a flood mitigation measure besides simply its cost efficacy and the percentage of FEV it mitigates flooding by, although this is certainly a useful tool. For example, there are many environment and social benefits to the planting of new woodland whilst, although it may not mitigate flooding in as cost efficient a manner, constructing natural flow-attenuation features such as leaky dams are largely environmentally friendly as they allow the fauna in the river to travel down the river unhindered [32]. This is in comparison to the giving-room-to-the-river procedure that would disrupt the wildlife in the river and on the banks of the river whilst it is under construction and for a time after. There may also be other reasons why there are no plans to widen the Avon's river bed as it flows through Stratford-upon-Avon, despite it being the most reliable cost-effective flood mitigation measure, that are unknown to us, such as the stability of the ground surrounding the channel or property rights along the edge of the river.

One flaw in the assessment of flood mitigation measures using the FEV of a recent major flooding event is the implicit assumption that mitigating this FEV by 100% is the equivalent to protecting against flooding in any eventuality. This is false for numerous reasons, such as the fact that although the flooding event may have discharged the largest volume of flood water in recent history, it does not then mean any future flooding will have an FEV less than or equal to the one calculated for the historical major flood event. Another reason is that recent models have shown that climate change may have a drastic impact on flooding in the future; by 2050 it is expected that approximately 450 million people at risk from flooding globally will

experience flooding at twice the expected rate [36]. It should be noted however that although 450 million people are expected to see a higher frequency of flooding, 75 million are expected to see a decrease in the frequency of flooding [36]. Also, an increased rate in flooding does not necessarily mean the magnitude, or FEV, of a flood will increase also; i.e. if Stratford-upon-Avon floods with a similar FEV to the November 2012 floods but with doubled frequency, any flood mitigation measures implemented to mitigate the FEV by 100% would still mitigate the more frequent flooding. If this project were to be continued, the impact climate change may have on the FEV of a flood should be investigated.

6.1 Further Reading

The main source of information in regards to the flood-excess volume analysis used in this report was provided from the two articles [5] and [6], co-authored by the supervisors of this project, O. Bokhove and T. Kent. As such it would be beneficial to read these articles if there is a wish gain a broader understanding of the subject. Detailed information on the various methods of NFM is provided by the Environment Agency in [32], their study into working with natural processes. More information on what may happen in the future in regards to flood management nation wide is included [1], produced by the Flood and Coastal Defence project of the Foresight programme.

As part of this project the author, with the assistance of A. Feilden, S. Kennet, M. Saunders and J. Willis, has created two web pages, one giving information on the wider project as well as each individuals chosen rivers¹ and another containing all the instruction necessary to perform the same FEV analysis as in this report². This includes an automated code which only needs the parameters of the rating curve and a chosen h_T to produce a graph and provide an evaluation of the FEV, Q_m , Q_T , h_m and T_f as well as a three dimensional visualisation of the FEV's corresponding square lake of depth 2m. This code was written individually by the author with credit to A. Feilden for suggesting attaching an automated code for producing a representation of the 2m deep square lake. The code and the graphs it produces may be found in Appendix A.4.

There are some limits to the automated nature of this code, such as the fact that if the curve $Q(\bar{h})$ is split into more than one peak above Q_T then, although the FEV calculated will be accurate, T_f and thereby Q_m and h_m will not. The code also will not work if the flood data being analysed does not cover an integer number of days. Therefore, if this project were to be continued, the fixing of these issues should be undertaken. Another web page³ created by the author includes all the data sets and rating curve information integral to the calculations of FEV in this report as well as the basic code for the included graphs.

6.2 Acknowledgements

The author would like to thank and acknowledge the Environment Agency in the West Midlands for contributing river height and flow rate data for the Warwick and Cox's Yard gauging stations, alongside each stations rating curve information, towards this report. This report contains public sector information licensed under the Open Government Licence v3.0.

¹The web page giving an over arching description of the project, and each team members individual work, can be found on GitHub at <https://github.com/Rivers-Project-2018/Group-Project>

²The web page giving instructions on how to FEV analysis can be found on GitHub at <https://github.com/Rivers-Project-2018/How-to-do-FEV-Analysis>

³The web page containing the data sets, rating curve information and code used in this report can be found on GitHub at <https://github.com/Rivers-Project-2018/Abbey-Chapman>

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A Appendices

A.1 Calculation of the FEV for the Boxing Day 2015 floods of the River Calder

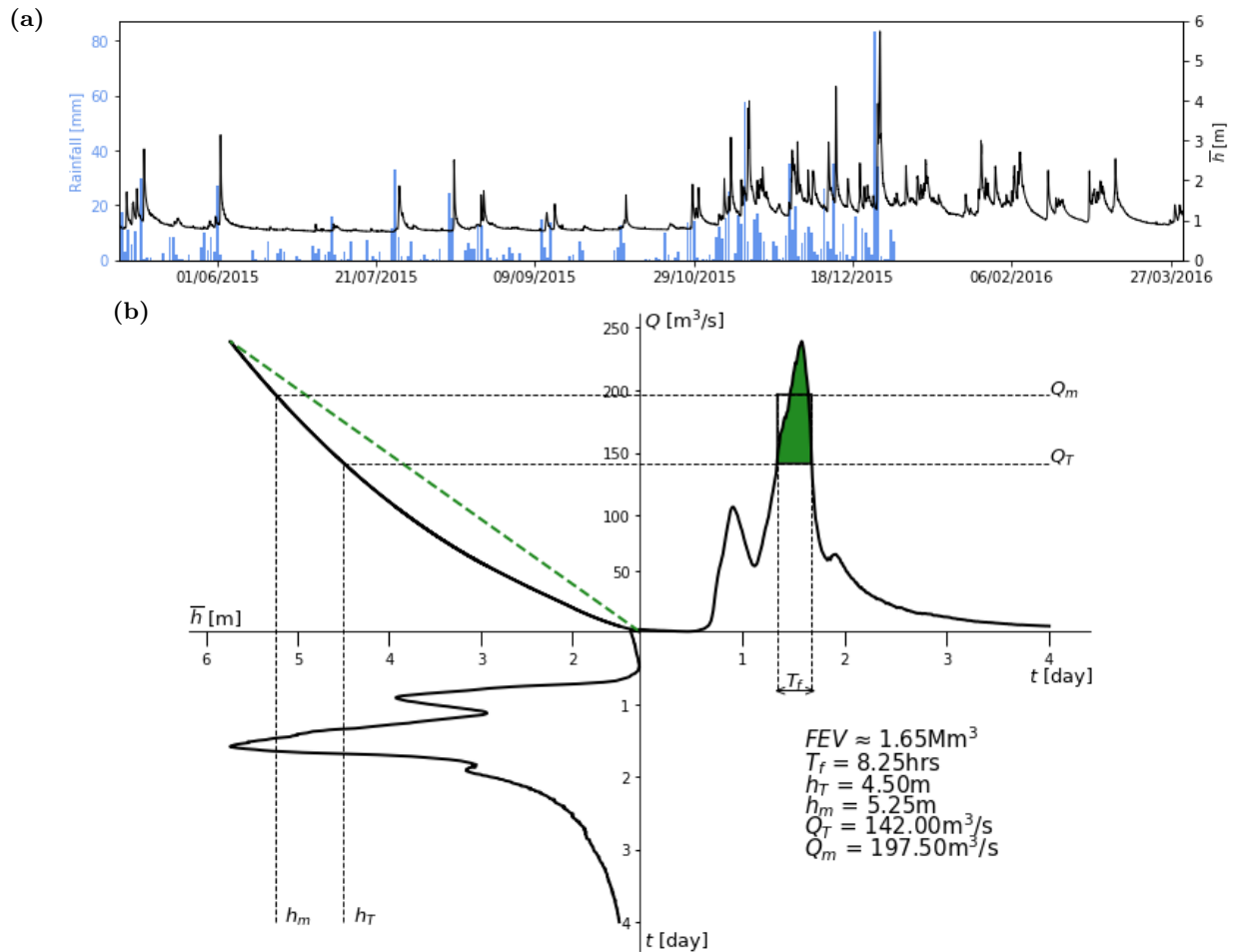


Figure 12: Flow rate and river height data for the River Calder at the Mytholmroyd gauging station: a) river height (\bar{h} [m]) [6] and catchment rainfall [mm] data [8] for a year starting from May 2015 (note, catchment rainfall data was not available for 2016) and b) quadrant plot for the 2015 Boxing Day floods of the River Calder (*cf.* Figure 1 b)). The rating curve parameters used to plot \bar{h} against Q may be found in Table **insert**

k	h_{k-1} [m]	h_k [m]	c_k [m ^{3-b_k} /s]	b_k [-]	a_k [m]
1	0	2.107	8.459	2.239	0.342
2	2.107	3.088	21.5	1.37	0.826
3	3.088	5.8	2.086	2.515	-0.856

Table 5: The coefficients a_k , b_k and c_k and their corresponding upper (h_k) and lower (h_{k-1}) bounds on \bar{h} [6] applied in the calculation of the rating curve, using equation 2.1, at the Mythomroyd gauging station on the River Calder.

632 A.2 Calculation of the FEV for the June 2007 floods of the River Don

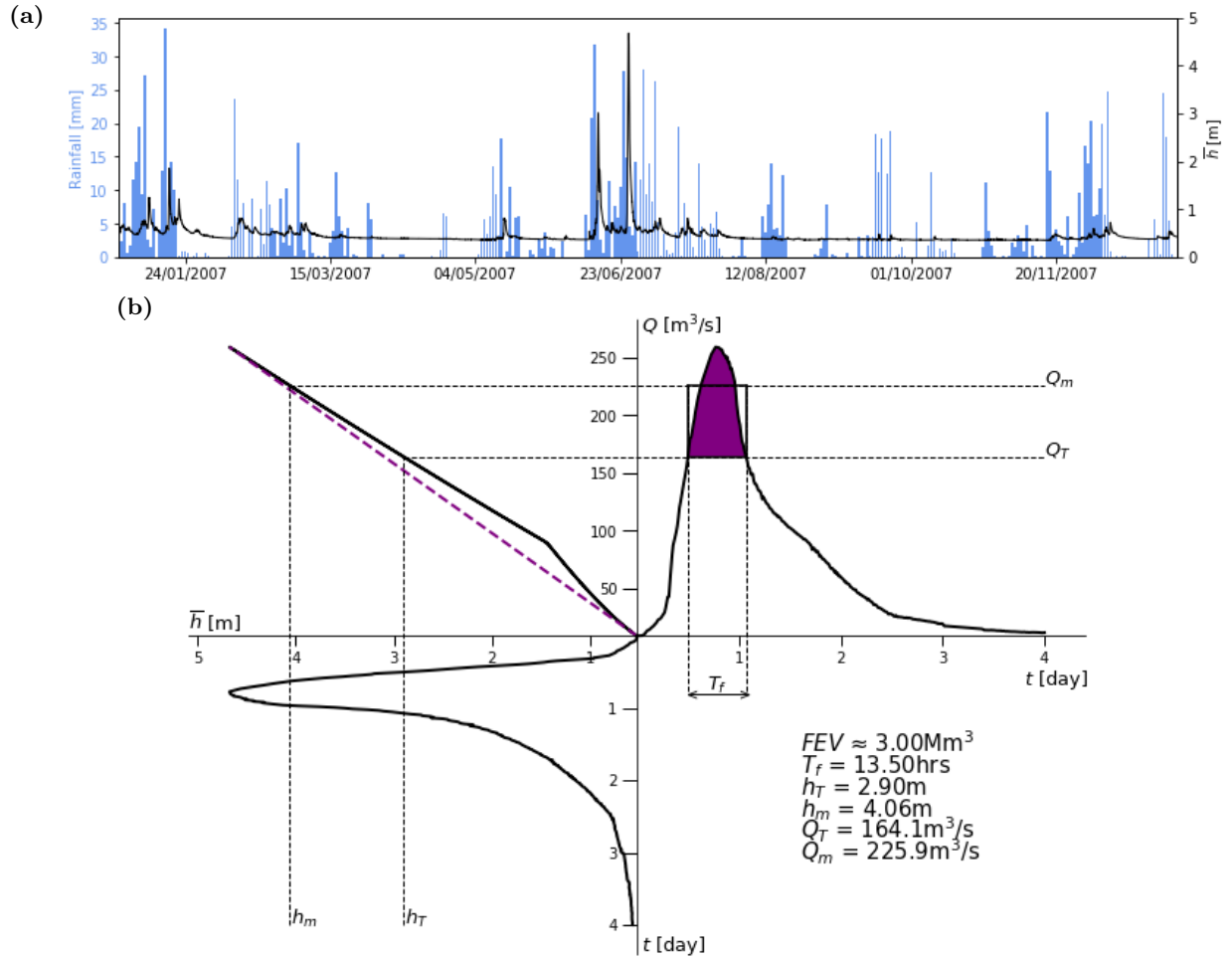


Figure 13: Flow rate and river height data for the River Don at the Sheffield Hadfields gauging station: a) river height (\bar{h} [m]) [6] and catchment rainfall [mm] data [8] for a year starting from January 2007 and b) quadrant plot for the June 2007 floods of the River Don (*cf.* Figure 1 b)), where the time values in this case give the number of days after the 25th of June 2007. The rating curve parameters used to plot \bar{h} against Q may be found in Table **insert**

k	h_{k-1} [m]	h_k [m]	c_k [m ^{3-b_k} /s]	b_k [-]	a_k [m]
1	0	0.52	78.4407	1.7742	0.223
2	0.52	0.931	77.2829	1.3803	0.3077
3	0.931	1.436	79.5956	1.2967	0.34
4	1.436	3.58	41.3367	1.1066	-0.5767

Table 6: The coefficients a_k , b_k and c_k and their corresponding upper (h_k) and lower (h_{k-1}) bounds on \bar{h} [6] applied in the calculation of the rating curve, using equation 2.1, at the Sheffield Hadfields gauging station on the River Don.

A.3 Calculation of the FEV for the December 2012 floods of the River Avon

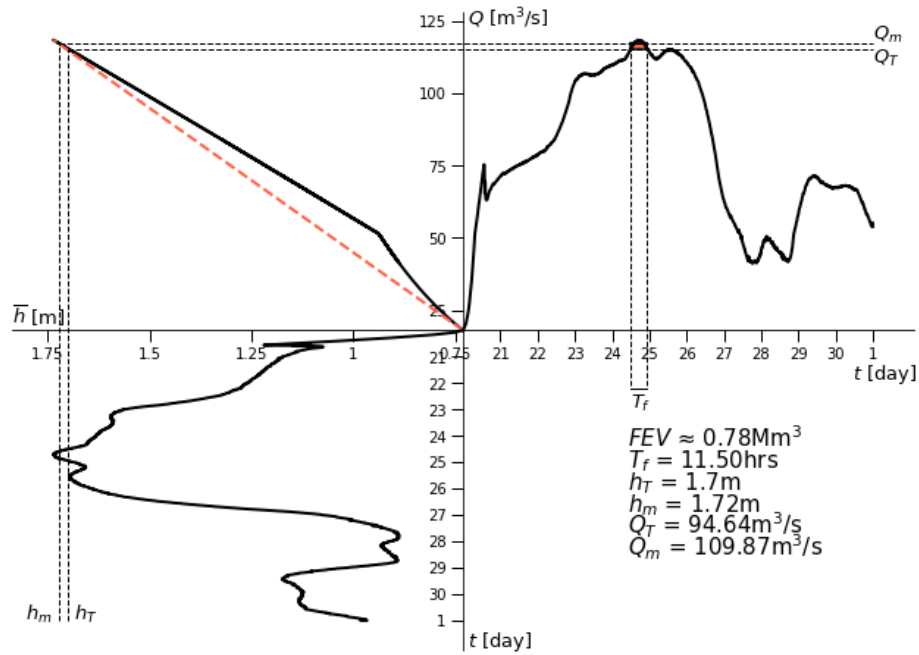


Figure 14: Quadrant plot of the flow rate and river height data for the River Avon at the Cox's Yard gauging station during the December 2012 floods in Stratford-upon-Avon (cf. Figure 1 b)), where the time values in this case are the number of days from the 20th of December 2012. The rating curve parameters used to plot \bar{h} against Q may be found in Table insert

A.4 An automated code for calculating the FEV and the graph it produces

A.4.1 The automated code, written in the Python language, that can be used to calculate the FEV of a given flood provided h_T and the parameters of the rating curve are known. The example data used is that of the Boxing Day 2015 River Aire floods using data from the gauging station Armley in Leeds [5].

```

##Input your own data here:##
#Your chosen threshold height.
ht=3.9

#Your rating curve coefficients, listed starting from those corresponding to
#the lowest range of heights up until the heighest range.
a=[0.156,0.028,0.153]
b=[1.115,1.462,1.502]
c=[30.69,27.884,30.127]

#The upper and lower limits of the ranges of the river heights given for your

```

```

649 #rating curve.
650 lower_limits=[0.2,0.685,1.917]
651 upper_limits=[0.685,1.917,4.17]
652
653 #You do not have to change the following.
654 import matplotlib.pyplot as plt
655 import pandas as pd
656 fig, ax = plt.subplots()
657
658 #Import your data, your river height data must be saved into a csv file
659 #(we are using the file 'Aire Data.csv' but you must change what your csv file
660 #is saved as) within the same folder as this code is saved. The first column must have the
661 #heading 'Time', with time values converted into days (with the digits beyond the
662 #decimal point representing what the hours and seconds elapsed are in terms of a
663 #fraction of a day, more information on how to do this can be found at
664 #https://github.com/Rivers-Project-2018/How-to-do-FEV-Analysis/blob/master/README.md)
665 #and the second column must have the heading 'Height'.
666 Data=pd.read_csv('Aire_Data.csv')
667 time=Data['Time']
668 height=Data['Height']
669
670 #####You do not have to change any of the rest of the code.#####
671 #But if you wish to change elements such as the colour or the x and y axis
672 #there are instructions on how to do so.
673 import bisect
674 import numpy as np
675
676 plt.rcParams["figure.figsize"] = [11,8]
677 plt.rcParams['axes.edgecolor']='white'
678 ax.spines['left'].set_position(('center'))
679 ax.spines['bottom'].set_position(('center'))
680 ax.spines['left'].set_color('black')
681 ax.spines['bottom'].set_color('black')
682
683 time_increment=(time[1]-time[0])*24*3600
684
685 number_of_days=int((len(time)*(time[1]-time[0])))
686
687 def scale(x):
688     return ((x-min(x))/(max(x)-min(x)))
689 scaledtime=scale(time)
690 scaledheight=scale(height)
691
692 w=[]
693 for i in range(len(a)):
694     w.append(i)
695
696 def Q(x):
697     z=0
698     while z<w[-1]:
699         if x>lower_limits[z] and x<=upper_limits[z]:
700             y = (c[z]*((x-a[z])**b[z]))
701             break
702         elif x>upper_limits[z]:
703             z = z+1
704     else:
705         y = (c[w[-1]]*((x-a[w[-1]])**b[w[-1]]))
706     return(y)
707
708 qt = Q(ht)
709

```

```

710 Flow = []
711 for i in height:
712     Flow.append(Q(i))
713
714 scaledFlow = []
715 for i in Flow:
716     scaledFlow.append((i-min(Flow))/(max(Flow)-min(Flow)))
717
718 negheight=-scaledheight
719 negday=-(scaledtime)
720
721 #To change the colour, change 'cornflowerblue' to another colour such as 'pink'.
722 ax.plot(negheight,scaledFlow,'black',linewidth=2)
723 ax.plot([0,-1],[0,1],'cornflowerblue',linestyle='--',marker='',linewidth=2)
724 ax.plot(scaledtime,scaledFlow,'black',linewidth=2)
725 ax.plot(negheight,negday,'black',linewidth=2)
726
727 scaledht = (ht-min(height))/(max(height)-min(height))
728 scaledqt = (qt-min(Flow))/(max(Flow)-min(Flow))
729
730 QT=[]
731 for i in scaledFlow:
732     i = scaledqt
733     QT.append(i)
734
735 SF=np.array(scaledFlow)
736 e=np.array(QT)
737
738 ax.fill_between(scaledtime,SF,e,where=SF>=e,facecolor='cornflowerblue')
739
740 idx = np.argwhere(np.diff(np.sign(SF - e))).flatten()
741
742 f=scaledtime[idx[0]]
743 g=scaledtime[idx[-1]]
744
745 def unscaletime(x):
746     return (((max(time)-min(time))*x)+min(time))
747
748 C=unscaletime(f)
749 d=unscaletime(g)
750
751 Tf=(d-C)*24
752
753 time_increment=(time[1]-time[0])*24*3600
754
755 flow = []
756 for i in Flow:
757     if i>=qt:
758         flow.append((i-qt)*(time_increment))
759
760 FEV=sum(flow)
761
762 Tfs=Tf*(60**2)
763
764 qm=(FEV/Tfs)+qt
765 scaledqm = (qm-min(Flow))/(max(Flow)-min(Flow))
766
767 hm=((qm/c[-1])*(1/b[-1]))+a[-1]
768 scaledhm = (hm-min(height))/(max(height)-min(height))
769
770 ax.plot([-scaledht,-scaledht],[-1,scaledqt],'black',linestyle='--',linewidth=1)

```

```

771 ax.plot([-scaledhm,-scaledhm],[-1,scaledqm], 'black', linestyle='—', linewidth=1)
772 ax.plot([-scaledht,1],[scaledqt,scaledqt], 'black', linestyle='—', linewidth=1)
773 ax.plot([-scaledhm,1],[scaledqm,scaledqm], 'black', linestyle='—', linewidth=1)
774
775 ax.plot([f,f,f],[scaledqt,scaledqm,-1/5], 'black', linestyle='—', linewidth=1)
776 ax.plot([g,g,g],[scaledqt,scaledqm,-1/5], 'black', linestyle='—', linewidth=1)
777 ax.plot([f,f],[scaledqm,scaledqt], 'black', linewidth=1.5)
778 ax.plot([f,g],[scaledqm,scaledqm], 'black', linewidth=1.5)
779 ax.plot([f,g],[scaledqt,scaledqt], 'black', linewidth=1.5)
780 ax.plot([g,g],[scaledqm,scaledqt], 'black', linewidth=1.5)
781 plt.annotate(s='', xy=(f-1/100,-1/5), xytext=(g+1/100,-1/5), arrowprops=dict(arrowstyle='<->'))
782
783 h=[]
784 for i in np.arange(1,number_of_days+1):
785     h.append(i/number_of_days)
786
787 #If you wish to set the flow to be shown on the axis by a certain increment, change all
788 #appearances of 50 in lines 153 and 157 to the desired increment, e.g 25 or 100.
789 #Otherwise leave as is.
790 l=np.arange(0,max(Flow)+50,50)
791 m=bisect.bisect(l,min(Flow))
792
793 n=[]
794 for i in np.arange(l[m],max(Flow)+50,50):
795     n.append(int(i))
796
797 #If you wish to set the height to be shown on the axis by a certain increment, change all
798 #appearances of 1 in lines 163 and 167 to the desired increment, e.g 0.25 or 0.5.
799 #Otherwise leave as is.
800 o=np.arange(0,max(height)+1,1)
801 p=bisect.bisect(o,min(height))
802
803 q=[]
804 for i in np.arange(o[p],max(height)+1,1):
805     q.append(i)
806
807 k=[]
808 for i in q:
809     k.append(-(i-min(height))/(max(height)-min(height)))
810
811 j=[]
812 for i in n:
813     j.append((i-min(Flow))/(max(Flow)-min(Flow)))
814
815 ticks_x=k+h
816
817 r=[]
818 for i in h:
819     r.append(-i)
820
821 ticks_y=r+j
822
823
824 s=[]
825 for i in np.arange(1,number_of_days+1):
826     s.append(i)
827
828 Ticks_x=q+s
829 Ticks_y=s+n
830
831 ax.set_xticks(ticks_x)

```

```

832 ax.set_yticks(ticks_y)
833 ax.set_xticklabels(Ticks_x)
834 ax.set_yticklabels(Ticks_y)
835
836 ax.tick_params(axis='x', colors='black', direction='out', length=9, width=1)
837 ax.tick_params(axis='y', colors='black', direction='out', length=10, width=1)
838
839 plt.text(-scaledht+1/100, -1, '$h_T$', size=13)
840 plt.text(-scaledhm+1/100, -1, '$h_m$', size=13)
841 plt.text(1, scaledqm, '$Q_m$', size=13)
842 plt.text(1, scaledqt, '$Q_T$', size=13)
843 plt.text(((f+g)/2)-1/50, -0.18, '$T_f$', size=13)
844
845 plt.text(0.01, 1.05, '$Q_{m^3/s}$', size=13)
846 plt.text(0.95, -0.17, '$t_{day}$', size=13)
847 plt.text(0.01, -1.09, '$t_{day}$', size=13)
848 plt.text(-1.1, 0.02, '$\overline{h}_{[m]}$', size=13)
849
850 ax.scatter(0,0,color='white')
851
852 A=round(FEV/(10**6),2)
853 B=round(Tf,2)
854 C=round(ht,2)
855 D=round(hm,2)
856 E=round(qt,2)
857 F=round(qm,2)
858
859 plt.text(0.4,-0.4,'$FEV_{m^3}$'+str(A)+'Mm^3$', size=15)
860 plt.text(0.4,-0.475,'$T_f$'+str(B)+'hrs', size=15)
861 plt.text(0.4,-0.55,'$h_T$'+str(C)+'m', size=15)
862 plt.text(0.4,-0.625,'$h_m$'+str(D)+'m', size=15)
863 plt.text(0.4,-0.7,'$Q_T$'+str(E)+'m^3/s', size=15)
864 plt.text(0.4,-0.775,'$Q_m$'+str(F)+'m^3/s', size=15)
865
866 from mpl_toolkits.mplot3d import Axes3D
867
868 fig = plt.figure(figsize=plt.figaspect(1)*0.7)
869 ax = Axes3D(fig)
870 plt.rcParams['axes.edgecolor']='white'
871 plt.rcParams["figure.figsize"] = [10,8]
872
873 ax.grid(False)
874 ax.xaxis.pane.fill = False
875 ax.yaxis.pane.fill = False
876 ax.zaxis.pane.fill = False
877
878 ax.xaxis.pane.set_edgecolor('w')
879 ax.yaxis.pane.set_edgecolor('w')
880 ax.zaxis.pane.set_edgecolor('w')
881
882 sl = (FEV/2)**0.5
883
884 a = [sl, sl]
885 b = [sl, sl]
886 c = [2, 0]
887
888 d = [sl, 0]
889 e = [sl, sl]
890 f = [0, 0]
891
892 g = [sl, sl]

```



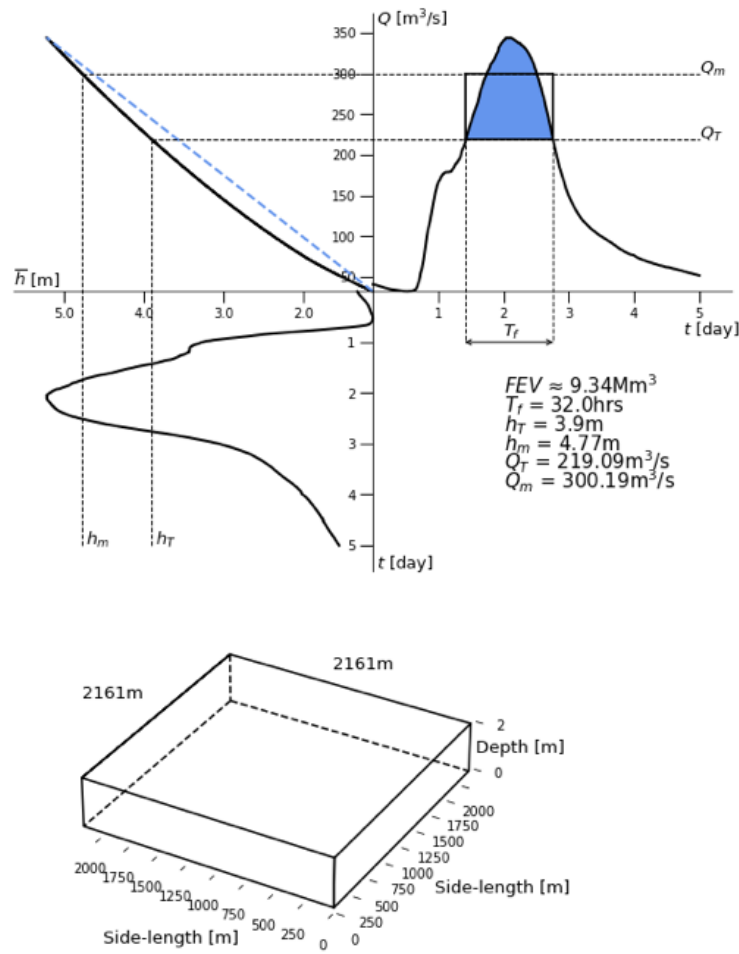
```

893 h = [sl, 0]
894 i = [0, 0]
895
896 ax.plot(a, b, c, '—', color = 'k')
897 ax.plot(d, e, f, '—', color = 'k')
898 ax.plot(g, h, i, '—', color = 'k')
899
900 x = [sl, sl, sl, 0, 0, 0, sl, sl, 0, 0, 0, 0]
901 y = [sl, 0, 0, 0, 0, sl, sl, 0, 0, 0, sl, sl]
902 z = [2, 2, 0, 0, 2, 2, 2, 2, 2, 0, 0, 2]
903
904 ax.plot(x, y, z, color = 'k')
905
906 ax.text(5*(sl/9), -5*(sl/9), 0, 'Side-length[m]', size=13)
907 ax.text(-sl/4, sl/4, 0, 'Side-length[m]', size=13)
908 ax.text(-0.02*sl, 1.01*sl, 0.8, 'Depth[m]', size=13)
909
910 ax.text(7*(sl/10), 5*(sl/4), 1, ''+str(int(round(sl)))+ 'm', size=13)
911 ax.text(14*(sl/10), 6*(sl/10), 1, ''+str(int(round(sl)))+ 'm', size=13)
912
913 ax.set_zticks([0, 2])
914
915 ax.set_xlim(sl,0)
916 ax.set_ylim(0,sl)
917 ax.set_zlim(0,10)

```

918 **A.4.2**

In [215]: `runfile('/Users/abbeyelizabethchapman/Rivers Project/Aire Graphs/Automated-`



In [216]: |

Figure 15: The quadrant plot used to calculate the FEV of a flood, in this case the Boxing Day 2015 floods of the River Aire (*cf.* Figure 1b)), alongside a visualisation of the FEV's corresponding 2m deep square lake, as produced by the automated graph code in Appendix A.4.1.

A.5 The code to produce a river level and rainfall against time graph

The code below, written in the Python language, produces Figure `insert`.

```
import matplotlib
import pandas as pd
import matplotlib.pyplot as plt
from datetime import datetime

plt.rcParams["figure.figsize"] = [13,3]
plt.rcParams['axes.edgecolor']='black'
fig, ax1 = plt.subplots()

#Import the Data
Warwick = pd.read_csv('Aire_Stage.csv')
date = Warwick['Time_stamp']
stage = Warwick['Value[m]']

Warwick2 = pd.read_csv('Aire-Rainfall.csv')
```

```
936 date1 = Warwick2[ 'Time_stamp' ]
937 rainfall = Warwick2[ 'Rainfall_[mm]' ]
938
939 converted_dates = [datetime.strptime(i, '%d/%m/%Y_%H:%M') for i in date]
940 dates = [datetime.strptime(i, '%d/%m/%Y') for i in date1]
941
942 Date = matplotlib.dates.date2num(converted_dates)
943 Dates = matplotlib.dates.date2num(dates)
944
945 hfmt = matplotlib.dates.DateFormatter( '%d/%m/%Y' )
946
947 ax1.set_xlim(Date[0],Date[-1])
948 ax1.xaxis.set_major_formatter(hfmt)
949 ax1.bar(Dates, rainfall, align='center', color='cornflowerblue')
950 ax1.set_ylabel( 'Rainfall_[mm]' , color='cornflowerblue' )
951 ax1.tick_params(axis='y', labelcolor='cornflowerblue')
952
953 ax2 = ax1.twinx()
954
955 ax2.set_ylim(0,5.5)
956 ax2.plot(Date, stage, 'black', linewidth=1, color='k')
957 ax2.set_ylabel( '$\overline{h}$_[m]' )
958 ax2.set_xlabel( 'Date' )
959
960 plt.show()
```