

# 弹性力学

## 一、平面问题

### 1. 基本假设

#### - 平面应变

$$\varepsilon_x, \varepsilon_y, \gamma_{xy} \sim (x, y)$$

$$\varepsilon_z = \gamma_{xz} = \gamma_{yz} = 0 \Rightarrow \sigma_x = \tau_{yz} = 0$$

#### - 平面应力

$$\sigma_x, \sigma_y, \tau_{xy} \sim (x, y)$$

$$\sigma_z = \tau_{xz} = \tau_{yz} = 0 \Rightarrow \gamma_{xz} = \gamma_{yz} = 0$$

### 2. 本构关系

#### - 平面应变

$\tilde{\sigma}(\tilde{\varepsilon})$  易得

$$\varepsilon_x = \frac{1-\nu^2}{E} (\sigma_x - \frac{\nu}{1-\nu} \sigma_y)$$

$$\varepsilon_y = \frac{1-\nu^2}{E} (\sigma_y - \frac{\nu}{1-\nu} \sigma_x)$$

$$\gamma_{xy} = \frac{1}{G} \tau_{xy}$$

$$\begin{aligned} \sigma_z &= \lambda(\varepsilon_x + \varepsilon_y) \\ &= \nu(\sigma_x + \sigma_y) \end{aligned}$$

#### - 平面应力

$\tilde{\varepsilon}(\tilde{\sigma})$  易得

$$\sigma_x = \frac{2G}{1-\nu} (\varepsilon_x + \nu \varepsilon_y)$$

$$\sigma_y = \frac{2G}{1-\nu} (\varepsilon_y + \nu \varepsilon_x)$$

$$\tau_{xy} = G \gamma_{xy}$$

$$\begin{aligned} \varepsilon_z &= -\frac{\nu}{1-\nu} (\varepsilon_x + \varepsilon_y) \\ &= -\frac{\nu}{E} (\sigma_x + \sigma_y) \end{aligned}$$

### 3. 平衡方程

$$\sigma_{\alpha\beta}, \beta + f_\alpha = 0$$

$$\left( \begin{array}{l} \frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau_{xy}}{\partial y} + f_x = 0 \\ \frac{\partial \tau_{xy}}{\partial x} + \frac{\partial \sigma_y}{\partial y} + f_y = 0 \end{array} \right)$$

$$f_z = 0$$

### 4. 协调方程

#### - 平面应变

$$\varepsilon_{11,22} + \varepsilon_{22,11} - 2\varepsilon_{12,12} = 0$$

#### - 平面应力

$$\left\{ \begin{array}{l} \varepsilon_{11,22} + \varepsilon_{22,11} - 2\varepsilon_{12,12} = 0 \\ \varepsilon_{33,11} = \varepsilon_{33,22} = \varepsilon_{33,12} = 0 \end{array} \right.$$

$$\text{解得 } \varepsilon_z = Ax + By + C$$

$$\Rightarrow \text{④} = \sigma_x + \sigma_y = ax + by + c$$

\* 定义平面应力： $z \ll r$  时， $\sigma_z$  相对面内应力分量可略，近似可按平面问题处理

### 5. 几何方程

$$\varepsilon_x = \frac{\partial u}{\partial x}, \quad \varepsilon_y = \frac{\partial v}{\partial y}, \quad \gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}$$

$$\text{平面应变: } \varepsilon_z = \frac{\partial w}{\partial z} = 0 \Rightarrow w = 0$$

$$\text{平面应力: } \varepsilon_z = \frac{\partial w}{\partial z} = -\frac{\nu}{1-\nu} \left( \frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} \right)$$

$$\Rightarrow w = \varepsilon_z(x, y) z$$

$$\text{由4. } \varepsilon_z = Ax + By + C$$

### 6. 边界条件

$$\begin{cases} \sigma_x \cos(\gamma, x) + \tau_{xy} \cos(\gamma, y) = \bar{x} \\ \tau_{xy} \cos(\gamma, x) + \sigma_y \cos(\gamma, y) = \bar{y} \end{cases} \quad \left. \begin{array}{l} \text{侧面} \\ \text{端面} \end{array} \right\} \quad 0 = \bar{z}$$

$$\text{平面应变: } \bar{x} = 0, \bar{y} = 0, \bar{z} = 0 \quad \left. \begin{array}{l} \text{端面} \end{array} \right\}$$

$$\text{平面应力: } \bar{x} = \bar{y} = \bar{z} = 0$$

### 7. 求解 - 位移解法 (以应力为例)

代入本构、平衡方程，得：

$$G\nu^2 u_{xx} + G \frac{1+\nu}{1-\nu} u_{yy,xx} + f_x = 0$$

边界条件亦可用位移表示

### 8. 求解 - 应力解法 I

#### 应力函数

若体力  $f = -\nabla V$ ，则平衡方程化为：

$$\begin{cases} \frac{\partial(\sigma_x - V)}{\partial x} + \frac{\partial \tau_{xy}}{\partial y} = 0 \\ \frac{\partial \tau_{xy}}{\partial x} + \frac{\partial(\sigma_y - V)}{\partial y} = 0 \end{cases}$$

引入 A, B, s.t.

$$\begin{cases} \frac{\partial A}{\partial y} = \sigma_x - V & \frac{\partial A}{\partial x} = -\tau_{xy} \\ \frac{\partial B}{\partial y} = -\tau_{xy} & \frac{\partial B}{\partial x} = \sigma_y - V \end{cases}$$

$$\frac{\partial A}{\partial x} = \frac{\partial B}{\partial y} \text{, 再引入 } \phi \text{ s.t.}$$

$$\frac{\partial \phi}{\partial y} = A \quad \frac{\partial \phi}{\partial x} = B.$$

即为平面问题的艾里应力函数  
由上得：

$$\begin{cases} \sigma_x = \frac{\partial^2 \phi}{\partial y^2} + V \\ \sigma_y = \frac{\partial^2 \phi}{\partial x^2} + V \\ I_{xy} = -\frac{\partial^2 \phi}{\partial x \partial y} \end{cases}$$

代回协调方程即得应力函数法的基本方程：

$$\nabla^2 \nabla^2 \phi = \begin{cases} -(1-\nu) \nabla^2 V & \text{平面应力} \\ -\frac{1-2\nu}{1-\nu} \nabla^2 V & \text{平面应变} \end{cases}$$

若无体力/常体力，进一步可得：

$$\nabla^2 \nabla^2 \phi = 0$$

### 9. 求解-应力函数法 II

I.  $\phi = \phi_1 + ax + by + c$  也是解

II. 边界条件以 A 为参考点，列：

$$\frac{\partial \phi}{\partial y}|_B = \int_A^B \bar{x} ds = R_x \quad \text{另: } R_n = \frac{\partial \phi}{\partial s} \\ R_s = -\frac{\partial \phi}{\partial n}$$

$$\frac{\partial \phi}{\partial x}|_B = -\int_A^B \bar{y} ds = -R_y$$

$$\phi|_B = \int_A^B [-(x_B - x_A)\bar{y} + (y_B - y_A)\bar{x}] ds = M_p \quad \begin{matrix} \text{相对 B 点} \\ \text{或相对 A 点的相反数} \end{matrix}$$

若  $V \neq 0$ ，则将  $\bar{x}, \bar{y}$  换为  $\bar{x} - LV, \bar{y} - MV$

IV. 单值条件：作用在闭合边界上的载荷

构成平自平衡力系

### ~~求解-应力函数法 III~~

1. 计算边界上的  $\phi$

2. 选择  $\phi(x, y)$  的形式

3. 代入协调方程

4. 由边界条件确定积分常数

5. 求  $\hat{\sigma}, \hat{\epsilon}, u, v, w$ .

\* 直角坐标

\* 极坐标

10. 极坐标，解与轴对称

$$\epsilon_r = \frac{\partial u_r}{\partial r} \quad \epsilon_\theta = \frac{1}{r} \frac{\partial u_\theta}{\partial \theta} + \frac{u_r}{r}$$

$$v_{r\theta} = \frac{1}{r} \frac{\partial u_r}{\partial \theta} + \frac{\partial v_\theta}{\partial r} - \frac{v_\theta}{r}$$

平衡、协调、

$$\text{平面应力: } \sigma_r = \frac{2G}{1-\nu} (\epsilon_r + \nu \epsilon_\theta) \quad \epsilon_r = \frac{1}{E} (\sigma_r - \nu \sigma_\theta)$$

$$\sigma_\theta = \frac{2G}{1-\nu} (\epsilon_\theta + \nu \epsilon_r) \quad \epsilon_\theta = \frac{1}{E} (\sigma_\theta - \nu \sigma_r) \\ t_{r\theta} = G v_{r\theta}$$

$$\text{平面应变: } G_r^* = G_r, \nu^* = \frac{\nu}{1-\nu}, E^* = \frac{E}{1-\nu^2}$$

$$\nabla^2 \nabla^2 \phi = \left( \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2} \right)^2 \phi$$

$$\sigma_r = \frac{\partial^2 \phi}{\partial r^2} + \frac{\partial^2 \phi}{\partial \theta^2}$$

$$\sigma_\theta = \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} + \frac{1}{r} \frac{\partial \phi}{\partial r}$$

$$v_{r\theta} = \frac{\partial \phi}{\partial r} \quad t_{r\theta} = -\frac{\partial}{\partial r} \left( \frac{1}{r} \frac{\partial \phi}{\partial \theta} \right)$$

轴对称(严格),  $\phi = \phi(r)$

$$\text{此时: } \nabla^2 \nabla^2 \phi = \frac{d^4 \phi}{dr^4} + \frac{2}{r} \frac{d^3 \phi}{dr^3} - \frac{1}{r^2} \frac{d^2 \phi}{dr^2} + \frac{1}{r^3} \frac{d \phi}{dr} = 0$$

$$\epsilon_r = \frac{du_r}{dr} \quad \epsilon_\theta = \frac{u_r}{r} \quad v_{r\theta} = \frac{dv_\theta}{dr} - \frac{v_\theta}{r}$$

$$\sigma_r = \frac{1}{r} \frac{d\phi}{dr} \quad \sigma_\theta = \frac{d^2 \phi}{dr^2} \quad t_{r\theta} = 0$$

设解为  $r^k$ , 代入解, 而后可得:

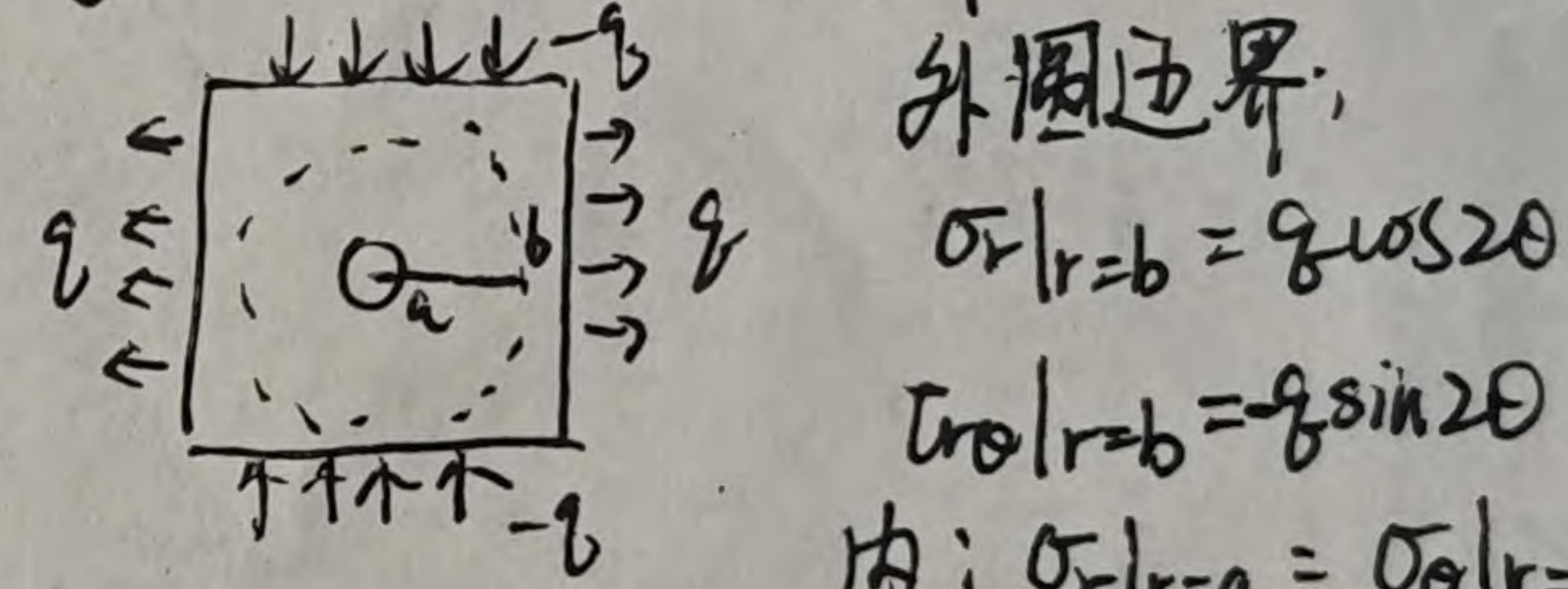
$$\phi = A \ln r + B r^2 \ln r + C r^2 + D$$

(也可用位移解法)

### 11. 非对称

$$\text{一般: } \phi = \phi_0(r) + \sum_{n=1}^{\infty} [f_n(r) \cos n\theta + g_n(r) \sin n\theta]$$

e.g. 1. 小圆孔应力集中



$$\text{内: } \sigma_r|_{r=a} = \sigma_\theta|_{r=a} = 0$$

应设  $\phi = f(r) \cos 2\theta$ , 代入得:

$$f(r) = Ar^4 + Br^2 + C + Dr^{-2}$$

由此算出  $\sigma_r, \sigma_\theta, t_{r\theta}$ , 得到 A, B, C, D

再取  $b \rightarrow \infty$ , 则:

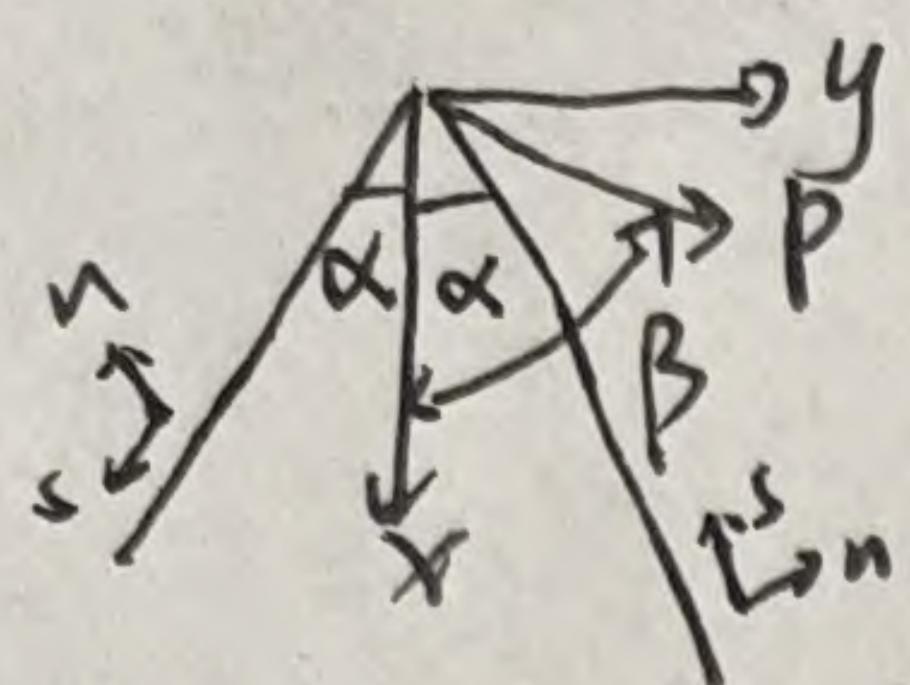
$$f(r) = -\frac{q}{2} r^2 + q a^2 - \frac{q a^4}{2 r^2}$$

$$\text{附: } \sigma_r = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + t_{xy} \sin 2\theta$$

$$\sigma_\theta = \frac{\sigma_x + \sigma_y}{2} - \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - t_{xy} \sin 2\theta$$

$$t_{r\theta} = -\frac{\sigma_x - \sigma_y}{2} \sin 2\theta + t_{xy} \cos 2\theta$$

eg<sub>2</sub>: 檔板



$$\theta = \frac{\pi}{2}; \phi = 0$$

$$\frac{\partial \phi}{\partial s} = -\frac{\partial \phi}{\partial r} = 0$$

$$\frac{\partial \phi}{\partial n} = \frac{1}{r} \frac{\partial \phi}{\partial \theta} = 0$$

$$\theta = -\frac{\pi}{2}; \phi = -Pr \sin(\frac{\pi}{2} + \beta)$$

$$\frac{\partial \phi}{\partial s} = \frac{\partial \phi}{\partial r} = -P \sin(\frac{\pi}{2} + \beta)$$

$$-\frac{\partial \phi}{\partial n} = \frac{1}{r} \frac{\partial \phi}{\partial \theta} = P \cos(\frac{\pi}{2} + \beta)$$

$$\phi = r f(\theta), \text{代入得:}$$

$$\frac{1}{r^3} \left[ \frac{d^4 f}{d\theta^4} + 2 \frac{d^2 f}{d\theta^2} + 1 \right] = 0$$

$$f = A \cos \theta + B \sin \theta + C (\cos \theta + D \sin \theta)$$

$$\sigma_r = \frac{2}{r} (D \cos \theta - C \sin \theta)$$

$$\sigma_\theta = \tau_{r\theta} = 0$$

$$\text{边界条件: } \int_{-\pi}^{\pi} \sigma_r \cos \theta r d\theta + P \cos \beta = 0$$

$$\int_{-\pi}^{\pi} \sigma_r \sin \theta r d\theta + P \sin \beta = 0$$

$$\Rightarrow A, B, C$$

$$\text{特别地, } \rho x = \pi R \text{ 时, } \sigma_r = -\frac{2P}{\pi R} \cos \theta$$

$$\text{平衡方程: } \frac{\partial \sigma_r}{\partial r} = 0 \quad \& \quad \frac{\partial \tau_{rz}}{\partial z} = 0$$

$$\phi = \frac{1}{2} Cr^2 \quad \frac{\partial \phi}{\partial \theta} = Cr^2$$

## 二、柱形杆问题

### 1. 位移解法

$$\begin{cases} u_r = 0 \\ v_\theta = \alpha z r \end{cases} \Rightarrow \begin{cases} u = -\alpha z y \\ v = \alpha z x \\ w = \alpha \psi(x, y) \end{cases}$$

$$\Rightarrow \varepsilon_x = \varepsilon_y = \varepsilon_z = \gamma_{xy} = 0$$

$$\gamma_{xz} = \alpha \left( \frac{\partial \psi}{\partial x} - y \right)$$

$$\gamma_{yz} = \alpha \left( \frac{\partial \psi}{\partial y} + x \right)$$

$$\sigma_x = \sigma_y = \sigma_z = \tau_{xy} = 0$$

$$\tau_{xz} = G \alpha \left( \frac{\partial \psi}{\partial x} - y \right)$$

$$\tau_{yz} = G \alpha \left( \frac{\partial \psi}{\partial y} + x \right)$$

$$\omega_x = \omega_y = \omega_z = \frac{1}{2} \alpha \left( \frac{\partial \psi}{\partial y} - x \right)$$

$$\omega_y = \omega_{zx} = -\frac{1}{2} \alpha \left( \frac{\partial \psi}{\partial x} + y \right)$$

$$\omega_z = \omega_{xy} = \alpha z$$

$$\text{平衡方程: } \nabla^2 \psi = 0$$

侧面边界, 端面边界(剪力, 扭矩)

$$\text{扭转刚度: } \frac{D_t}{L} = \frac{M}{\psi} = G \int_A (x^2 + y^2) dA + G_2 \int_A \left( \frac{\partial \psi}{\partial y} x - \frac{\partial \psi}{\partial x} y \right) dA$$

### 2. 应力函数解法

$$\text{平衡方程: } \frac{\partial \tau_{zx}}{\partial x} + \frac{\partial \tau_{zy}}{\partial y} = 0$$

$$\text{引入 } \phi \text{ s.t. } \frac{\partial \phi}{\partial y} = \tau_{zx} \quad \frac{\partial \phi}{\partial x} = -\tau_{zy}$$

$$\text{协调方程: } \nabla^2 \tau_{zx} = \nabla^2 \tau_{zy} = 0$$

$$\Rightarrow \nabla^2 \phi = C \quad \nabla \phi = -2G\alpha$$

$$\text{侧边界: } \phi|_P = C \quad \text{对单连通域, 可取为0}$$

$$\text{扭矩: } M = 2 \int \phi dx dy \quad (\text{前提: } \phi|_P = 0)$$

求法: 1. 由边界条件, 泊松方程求  $\phi$

2. 求  $\tau_{zx}, \tau_{zy}, \alpha$ , 进而得  $M, D_t$ .

3. 对位移, 宜用  $c_{ij}$  得  $\psi$ .

### 3. 解例.

假设圆由  $N$  段曲线  $f_n(x, y) = 0$  组成, 则

$$\phi = m f_1(x, y) \cdots f_n(x, y)$$

#### 4. 薄膜比拟

周边固定，内部有拉张力 $S$ 的薄膜。

受横向均匀压力 $\bar{\tau}$ 。

$$\left\{ \begin{array}{l} \nabla^2 z = \frac{\partial^2 z}{\partial x^2} + \frac{\partial^2 z}{\partial y^2} = -\frac{1}{S} \bar{\tau} \\ z|_P = 0 \end{array} \right.$$

$$\text{比拟关系: } \phi \sim \frac{2G\alpha S}{\bar{\tau}} z$$

补充普朗特应力函数的两个性质:

$$1^{\circ} \text{. 沿截面内任意点: } \bar{\tau} = -\frac{\partial \phi}{\partial y}$$

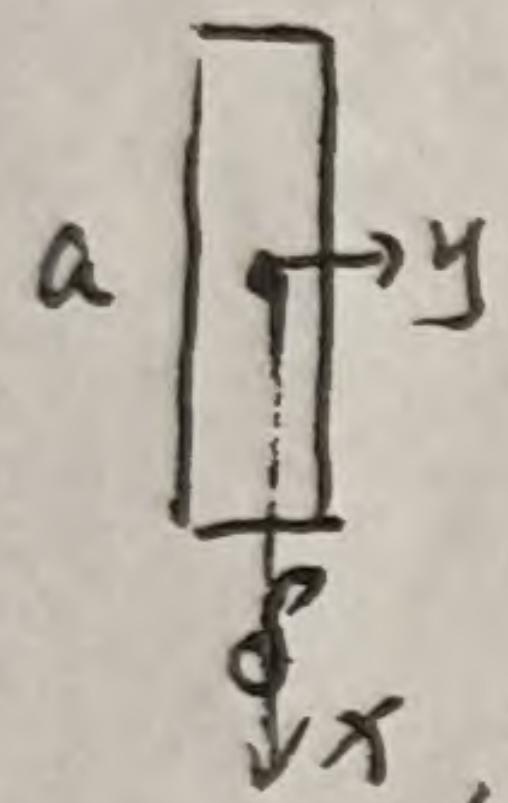
$\bar{\tau}$ 指向中等值线法向

$\bar{\tau}$ 方向同等值线切向

$$2^{\circ} \text{. 在中等值线上: } \oint \bar{\tau} ds = 2G\alpha A_L$$

下面给出几个解例:

##### ① 狹长矩形杆.



参考薄膜实验，设  $\phi = \phi(y)$ 。  
则由边界条件与对称性:

$$\frac{\partial \phi}{\partial y}|_{y=0} = 0 \quad \phi|_{y=\pm h} = 0$$

$$\text{解得 } \phi = G\alpha \left( \frac{h^2}{4} - y^2 \right)$$

$$\Rightarrow D_t = \frac{1}{3} G \alpha \delta^3$$

$$\bar{\tau} = -T_{2x} = -\frac{\partial \phi}{\partial y} = 2G\alpha y$$

剪应力产生的扭矩为  $\frac{1}{2} G \alpha \delta^3 \alpha$ , 其他  $M_t$   
的另一半由  $T_{2y}$  提供。

##### ② 开口薄壁杆.

可将其看作若干狭长矩形杆拼接而成

$$D_t = \frac{1}{3} G \sum_i a_i \delta_i^3$$

$$\alpha = \frac{M_t}{D_t} = \frac{3M_2}{G \sum_i a_i \delta_i^3}$$

$$\text{第 } i \text{ 部分: } T_{imax} = \frac{3M_t \delta_i}{\sum_i a_i \delta_i^3}$$

##### ③ 闭口薄壁杆管



薄膜实验得  $\bar{\tau} = -\frac{\partial \phi}{\partial v} \approx \frac{\Phi_P}{\delta(S)}$

$$\bar{\tau}_1 \cdot 2A = M_t$$

$$\Rightarrow T = \frac{M_t}{2A\delta(S)}$$

$$\alpha = \frac{M_t}{4GA^2} \oint \frac{ds}{\delta(S)}$$

$$D_t = \frac{4GA^2}{\oint \frac{ds}{\delta(S)}}$$

#### ④ 多闭室薄壁管

K连通域、K个未知量:  $\alpha, \bar{\tau}_1, \dots, \bar{\tau}_{k-1}$

$$\text{eg: } \begin{array}{c} \text{图示一个闭合薄壁管} \\ \text{有 } k \text{ 个室, 宽度 } \delta_1, \delta_2, \dots, \delta_k \end{array} \quad \bar{\tau}_1 = \frac{\bar{\tau}_1}{\delta_1} \quad \bar{\tau}_2 = \frac{\bar{\tau}_2}{\delta_2} \\ \bar{\tau}_3 = \frac{\bar{\tau}_1 - \bar{\tau}_2}{\delta_3}$$

$$M_t = 2A_1 \bar{\tau}_1 + 2A_2 \bar{\tau}_2$$

$$\int_{A_1} \bar{\tau} ds = 2G\alpha A_1$$

$$\int_{A_2} \bar{\tau} ds = 2G\alpha A_2$$

由上可解得  $\alpha, \bar{\tau}_i$ . 进而得其它量

### 三、能量法补充—梁弯曲理论

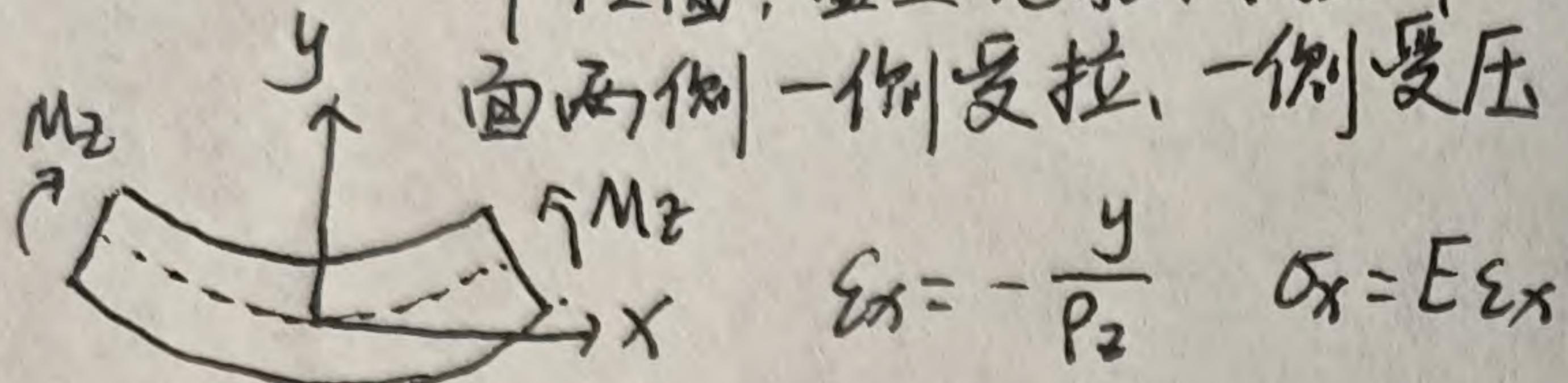
#### 1. 基本假设.

平截面：截面变形前垂直于轴线，变形后保持平面

直线段：直线段变形前垂直于轴线，变形后保持直线，垂直于变形后轴

共曲率：平直截面上的弯曲曲率相同  
(由上而下假设导出)

中性面：纯弯曲中，沿梁的厚度方向存在中性面，面上无轴向拉伸/压缩，面两侧一侧受拉、一侧受压



$$\varepsilon_x = -\frac{y}{P_z} \quad \sigma_x = E\varepsilon_x$$

#### 2. 几何、应力关系.



$$\text{转角 } \theta = \frac{dw}{dx}$$

$$\text{曲率 } K = \frac{1}{\rho} = \frac{d^2w}{dx^2}$$

$$\text{由 } \varepsilon_x = -\frac{y}{P_z}, \quad \sigma_x = E\varepsilon_x$$

$$\Rightarrow M_z = - \int_A y \sigma_x dA$$

$$= -\frac{E}{P_z} \underbrace{\int_A y^2 dA}_{(\text{即为 } I_z)} = \frac{EI_z}{P_z}$$

$$\Rightarrow \sigma_x = -\frac{M_z y}{I_z} \quad \varepsilon_x = -\frac{M_z y}{EI_z}$$

$$\text{另: } \int_A \sigma_x dA = 0$$

$$\int_A (\sigma_x dA) z = 0$$

$$\text{附: 静矩 } S_y = \int_A z dA \quad S_z = \int_A y dA$$

$$\text{惯性矩 } I_y = \int_A z^2 dA \quad I_z = \int_A y^2 dA$$

$$I_{yz} = \int_A yz dA$$

若截面上弯矩还有  $M_y$ ，则：

$$\sigma_x = -\frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

$$\text{仅考虑 } M_z \text{ 时: } \frac{\sigma_{x_{max}}}{W_z} = \pm \frac{M_z}{W_z} \quad W_z =$$

$W_z = \frac{I_z}{y_{max}}$  : 横截面对 z 轴的弯曲截面系数

$$\text{或 } M_y \text{ 时: } \sigma_{x_{max}} = \pm \frac{M_y}{W_y}$$

$$W_y = \frac{I_y}{z_{max}}$$

若载荷轴向偏心，则：

$$F \quad \sigma_x = \pm \frac{F}{A} - \frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

#### 3. 功能关系.

$\vec{F}$	$\vec{u}$	$\vec{F} \cdot \vec{u}$
$\vec{M}_T$	$\vec{\gamma}$	$\int \vec{G} \cdot \vec{\gamma} dV$
$\vec{M}_z$	$\vec{x}$	$\int \vec{M}_T \cdot \vec{x} dl$
	$\vec{R}_{\text{外}}$	$\int \vec{M}_z \cdot \vec{R}_{\text{外}} dl$

## 四、能量原理

### 1. 基本概念

\* 基本关系  $\left\{ \begin{array}{l} \text{变形状态 } (\vec{u}, \vec{\epsilon}) \\ \text{静力关系 } (\vec{\sigma}, \vec{f}) \\ \text{本构关系 } (\text{力学} \leftrightarrow \text{几何}) \end{array} \right.$

\* 真实状态, ~ 变形可能状态

$$\begin{array}{ll} u_i^{(k)} & \epsilon_{ij}^{(k)} \\ \delta u_i^{(k)} & \delta \epsilon_{ij}^{(k)} \end{array}$$

~ 静力可能状态

$$\begin{array}{lll} \sigma_{ij}^{(s)} & f_i^{(s)} & p_i^{(s)} \\ \delta \sigma_{ij}^{(s)} & \delta f_i^{(s)} & \delta p_i^{(s)} \end{array}$$

\* 变形功: 载荷在其本身所引起的物体准静态弹性变形上所做的功  
线弹性: 变形功 =  $\frac{1}{2} F_i u_i$

可能功: 约束允许的范围内载荷在(虚功)任何变形可能位移(虚位移)上做的功

$$A = F_i u_i \quad A = F_i \delta u_i$$

\* 总势能 = 应变能 + 载荷外力势

$$\Pi = U + V$$

$$V = - \int_V f_i u_i dV - \int_{S_0} \bar{p}_i u_i ds$$

总余能 = 应变余能 + 支承系统余势

$$\Pi_c = U_c + V_c$$

$$V_c = - \int_{S_0} \bar{p}_i \bar{u}_i ds$$

### 2.(1) 可能功原理

可能外力在可能位移上做的功等于可能应力在相应可能应变上做的功.

$$\underbrace{\int_V f_i^{(s)} u_i^{(k)} dV}_{A_e = Af} + \underbrace{\int_S p_i^{(s)} u_i^{(k)} ds}_{Ap} = \underbrace{\int_V \sigma_{ij}^{(s)} \epsilon_{ij}^{(k)} dV}_{-A_i \text{ 内力功}}$$

或: 外力功 + 内力功 = 0

### 2.(2) 功的互等定理

上原理用于线性弹性体, 得:

$$(\text{内功互等}) \quad \sigma_{ij}^{(1)} \epsilon_{ij}^{(2)} = \sigma_{ij}^{(2)} \epsilon_{ij}^{(1)}$$

$$(\text{外功互等}) \quad \int_V f_i^{(1)} u_i^{(2)} dV + \int_{S_0} p_i^{(1)} u_i^{(2)} ds$$

$$\text{Betti} = \int_V f_i^{(2)} u_i^{(1)} dV + \int_S p_i^{(2)} u_i^{(1)} ds$$

### 3.(1) 虚功原理

$$\int_V f_i \delta u_i dV + \int_S \bar{p}_i \delta u_i ds = \int_V \sigma_{ij} \delta \epsilon_{ij} dV$$

正定理:  $\uparrow$

逆定理: 满足上式, 则满足平衡方程、力边界条件,  $\sigma_{ij}$  是当外载平衡的静力可能应力场

### 3.(2) 余虚功原理

$$\int_S \bar{u}_i \delta p_i ds = \int_V \epsilon_{ij} \delta \sigma_{ij} dV$$

正定理:  $\uparrow$

逆定理: 满足上式, 则是协调的变形可能状态, 满足协调方程、位移边界条件.

### 4. 对弹性保守系统, 可能功原理有推论

#### (1) 最小势能原理

$$\delta \Pi = 0$$

$$\text{or: } \delta U = \int_V f_i \delta u_i dV + \int_{S_0} \bar{p}_i \delta u_i ds$$

#### (2) 最小余能原理

$$\delta \Pi_c = 0$$

$$\text{or: } \delta U_c = \int_{S_0} \bar{u}_i \delta p_i ds$$

两个原理的一些关系

$$\Pi + \Pi_c = 0, \text{ 故: }$$

$$\Pi_c^{(s)} \geq \Pi_c = -\Pi \geq -\Pi^{(k)}$$

$$\text{且 } \Pi_c = -\Pi = A_e - U \quad (\text{外力功 - 应变能})$$

$$\Rightarrow A_e = U + \Pi_c = U + U_c + V_c$$

对整个弹性系统, 与  $U$  互余的是  $\Pi_c$ .

仅当支承均为固定边界时,  $V_c = 0$ ,  $U$  与  $U_c$  互余.

### 5. 变分原理之应用 - 拉格朗日法

$$\delta \Pi = 0 \sim \text{平衡方程}$$

$$\delta \Pi_c = 0 \sim \text{协调方程}$$

### 6. 变分原理之应用 - 直接法

A. 利用  $\delta \Pi = 0$

#### I. 里茨法 (Ritz)

##### (1) 选择可能的试验函数

$$u_i^{(k)} = u_i^{(1)} + a_i \sin \varphi_i \quad (i \text{ 为自由指针})$$

满足非齐次  
次边界条件  
满足齐次  
边界条件

(2) 代入, 得  $\Pi(a_{in})$ , 求解:

$$\frac{\partial \Pi}{\partial a_{in}} = 0$$

## II. 协辽金法 (Galerkin)

由最小势能原理:

$$\delta \Pi = - \int_V (\sigma_{ij} j + f_i) \delta u_i dV + \int_{S_0} (\sigma_{ij} v_j - \bar{p}_i) \delta u_i ds \\ = 0$$

取试验函数:

在  $S_u$  上满足位移边界条件

在  $S_0$  上满足力边界条件,

在域内只需满足

$$\int_V (\sigma_{ij} j + f_i) \delta u_i dV = 0$$

加权残量法的另一种

B. 利用  $\delta \Pi_c = 0$

## I. 里茨法

$$\sigma_{ij} = \sigma_{ij}^0 + a_{in} \sigma_{ijn} \quad \text{or} \quad \phi_i = \phi_i^0 + a_{in} \phi_{in}$$

$$\text{代入, 解 } \frac{\partial \Pi_c}{\partial a_{in}} = 0$$

## 7. 可变边界条件、卡氏定理

(1) 载荷可变,

此时由可能功原理:

$$\int_V \varepsilon_{ij}^{(k)} \delta \sigma_{ij}^{(s)} = \int_V u_i^{(k)} \delta f_i dV + \int_{S_0} u_i^{(k)} \delta \bar{p}_i ds \\ + \int_{S_u} \bar{u}_i^{(k)} \delta p_i ds$$

此即卡斯提良诺方程/应力变分方程.

取上式中  $(k)$  为真实状态,  $(s)$  为载荷虚变化相对应的静力状态, 则:

$$\delta \Pi_c = \int_V \varepsilon_{ij} \delta \sigma_{ij} dV - \int_{S_u} \bar{u}_i \delta p_i ds \\ = \int_V u_i \delta f_i dV + \int_{S_0} u_i \delta \bar{p}_i ds \quad \begin{matrix} \text{卡氏定理} \\ \text{之变分形式} \end{matrix}$$

外载荷常表示为广义力形式  $P_i$ , 设  $\bar{u}_i$  之对应的广义位移为  $\Delta_i$ , 则可得:

$$\Delta_i = \frac{\partial \Pi_c}{\partial P_i} \quad \begin{matrix} \text{卡氏定理} \\ \text{之微分形式} \end{matrix}$$

当位移边界固定或均为力边界时,  $V_c = 0$

$$\text{故: } \Delta_i = \frac{\partial V_c}{\partial P_i} \quad \text{Crotti-Engesser}$$

若再使用线弹性材料, 则:

$$\Delta_i = \frac{\partial U}{\partial P_i} \quad \text{卡氏第二定理}$$

(2) 位移可变,

由可能功原理:

$$\int_V \sigma_{ij}^{(s)} \delta \varepsilon_{ij}^{(k)} dV = \int_V f_i \delta u_i^{(k)} dV + \int_{S_0} \bar{p}_i \delta u_i^{(k)} ds \\ + \int_{S_u} p_i^{(s)} \delta \bar{u}_i ds$$

此即拉格朗日变分方程/位移变分方程  
取上式中  $(s)$  为真实状态,  $(k)$  为位移虚变化  
对应的真实状态, 则:

$$\delta \Pi = \int_V \sigma_{ij} \delta \varepsilon_{ij} dV - \int_V f_i \delta u_i dV - \int_{S_0} \bar{p}_i \delta u_i ds \\ = \int_{S_u} p_i \delta \bar{u}_i ds$$

引入广义力  $P_i$ , 广义位移  $\Delta_i$ , 则同样有:

$$P_i = \frac{\partial \Pi}{\partial \Delta_i}$$

外力势  $V=0$  时, 进一步得:

$$P_i = \frac{\partial U}{\partial \Delta_i} \quad \text{卡氏第一定理}$$