# **Unsaturated Flow**

Geohydraulics | CE60113

Lecture:18

# **Learning Objective(s)**

• To estimate infiltration

#### **Unsaturated Flow**

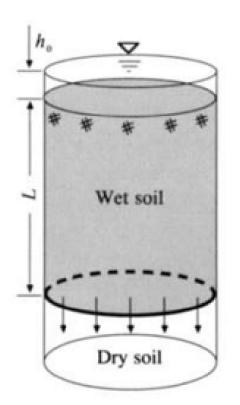
Continuity

$$F(t) = L(\eta - \theta_i)$$
$$= L\Delta\theta$$

where 
$$\Delta \theta = \eta - \theta_i$$
.

Momentum

$$q = -K \frac{\partial h}{\partial z}$$
$$f = K \left[ \frac{h_1 - h_2}{z_1 - z_2} \right]$$



$$f = K \left[ \frac{h_0 - (-\psi - L)}{L} \right]$$

$$\approx K \left[ \frac{\psi + L}{L} \right]$$

$$f = K \left[ \frac{\psi \Delta \theta + F}{F} \right]$$

$$f = K \left[ \frac{\psi \Delta \theta + F}{F} \right] \qquad \frac{dF}{dt} = K \left[ \frac{\psi \Delta \theta + F}{F} \right]$$

$$\left[\frac{F}{F + \psi \Delta \theta}\right] dF = K dt$$

$$\left[ \left( \frac{F + \psi \Delta \theta}{F + \psi \Delta \theta} \right) - \left( \frac{\psi \Delta \theta}{F + \psi \Delta \theta} \right) \right] dF = K dt$$

$$\int_0^{F(t)} \left( 1 - \frac{\psi \Delta \theta}{F + \psi \Delta \theta} \right) dF = \int_0^t K dt$$

$$F(t) - \psi \Delta \theta \bigg\{ \ln \big[ F(t) + \psi \Delta \theta \big] - \ln \big( \psi \Delta \theta \big) \bigg\} = Kt$$

or

$$F(t) - \psi \Delta \theta \ln \left(1 + \frac{F(t)}{\psi \Delta \theta}\right) = Kt$$

Infiltration rate

$$f(t) = K \left( \frac{\psi \Delta \theta}{F(t)} + 1 \right)$$

Cumulative Infiltration

$$F(t) = Kt + \psi \Delta \theta \ln \left( 1 + \frac{F(t)}{\psi \Delta \theta} \right)$$

$$s_e = \frac{\theta - \theta_r}{\eta - \theta_r}$$

$$\Delta\theta = \eta - \theta_i = \eta - (s_e\theta_e + \theta_r)$$

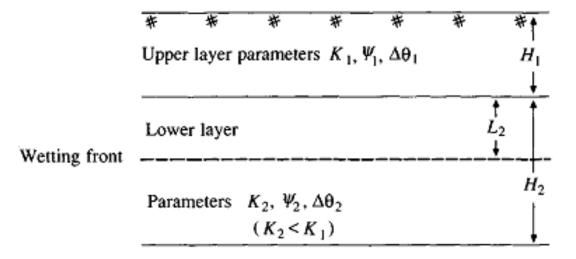
$$\Delta\theta = (1 - s_e)\theta_e$$

TABLE 4.3.1 Green-Ampt infiltration parameters for various soil classes

Soil class	Porosity $\eta$	Effective porosity $ heta_e$	Wetting front soil suction head $\psi$ (cm)	Hydraulic conductivity <i>K</i> (cm/h)
(0.374-0.500)	(0.354 - 0.480)	(0.97-25.36)		
Loamy sand	0.437	0.401	6.13	2.99
	(0.363-0.506)	(0.329 - 0.473)	(1.35-27.94)	
Sandy loam	0.453	0.412	11.01	1.09
	(0.351-0.555)	(0.283 - 0.541)	(2.67-45.47)	
Loam	0.463	0.434	8.89	0.34
	(0.375 - 0.551)	(0.334 - 0.534)	(1.33-59.38)	
Silt loam	0.501	0.486	16.68	0.65
	(0.420 - 0.582)	(0.394-0.578)	(2.92-95.39)	
Sandy clay loam	0.398	0.330	21.85	0.15
	(0.332 - 0.464)	(0.235 - 0.425)	(4.42-108.0)	
Clay loam	0.464	0.309	20.88	0.10
	(0.409 - 0.519)	(0.279 - 0.501)	(4.79 - 91.10)	
Silty clay loam	0.471	0.432	27.30	0.10
	(0.418 - 0.524)	(0.347-0.517)	(5.67-131.50)	
Sandy clay	0.430	0.321	23.90	0.06
	(0.370 - 0.490)	(0.207-0.435)	(4.08-140.2)	
Silty clay	0.479	0.423	29.22	0.05
	(0.425 - 0.533)	(0.334-0.512)	(6.13-139.4)	
Clay	0.475	0.385	31.63	0.03
	(0.427-0.523)	(0.269-0.501)	(6.39–156.5)	

The numbers in parentheses below each parameter are one standard deviation around the parameter value given. Source: Rawls, Brakensiek, and Miller, 1983.

• Two-layer Green-Ampt Model



**FIGURE 4.3.4** 

Parameters in a two-layer Green-Ampt model.

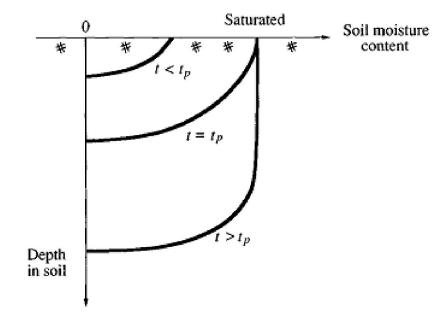
$$f = \frac{K_1 K_2}{H_1 K_2 + L_2 K_1} (\psi_2 + H_1 + L_2)$$

cumulative infiltration is given by

$$F = H_1 \Delta \theta_1 + L_2 \Delta \theta_2$$

$$L_2 \frac{\Delta \theta_2}{K_2} + \frac{1}{K_1 K_2} [\Delta \theta_2 H_1 K_2 - \Delta \theta_2 K_1 (\psi_2 + H_1)] \ln \left[ 1 + \frac{L_2}{\psi_2 + H_1} \right] = t$$

#### PONDING TIME



$$f = K \left( \frac{\psi \Delta \theta}{F} + 1 \right)$$

#### **FIGURE 4.4.1**

Soil moisture profiles before, during, and after ponding occurs.

the cumulative infiltration at the ponding time  $t_p$  is given by

$$F_p = i t_p$$

the infiltration rate by f = i

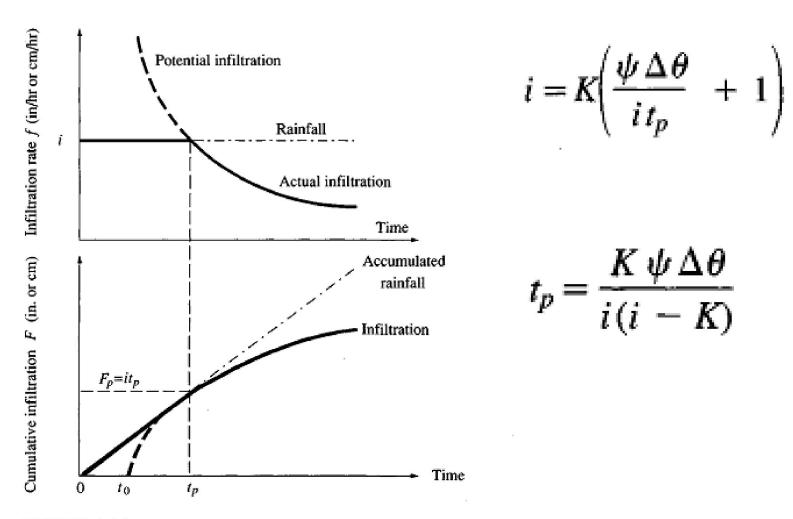


FIGURE 4.4.2 Infiltration rate and cumulative infiltration for ponding under constant intensity rainfall.

$$F_p - \psi \Delta \theta \ln \left( 1 + \frac{F_p}{\psi \Delta \theta} \right) = K(t_p - t_0) \qquad f = F_p$$

$$t = t_p - t_0$$

For  $t > t_p$ ,

$$F - \psi \Delta \theta \ln \left( 1 + \frac{F}{\psi \Delta \theta} \right) = K(t - t_0)$$

$$F - F_p - \psi \Delta \theta \left[ \ln \left( \frac{\psi \Delta \theta + F}{\psi \Delta \theta} \right) - \ln \left( \frac{\psi \Delta \theta + F_p}{\psi \Delta \theta} \right) \right] = K(t - t_p)$$

$$F - F_p - \psi \Delta \theta \ln \left[ \frac{\psi \Delta \theta + F}{\psi \Delta \theta + F_p} \right] = K(t - t_p)$$

- Total Head
- Darcy's Law
- Relative Permeability

 $k_r(\theta) = \frac{K(\theta)}{K_S}$ 

 $h = z + \psi$ 

 $\mathbf{q} = -K(\theta)\nabla h$ 

- $0 \le k_r(\theta) \le 1$
- Expanded form of Darcy's Law

$$\mathbf{q} = -K_{S}k_{r}(\theta)\nabla(z + \psi)$$
$$= -K_{S}k_{r}(\theta)\mathbf{e}_{z} - K_{S}k_{r}(\theta)\nabla\psi$$

Continuity Equation

$$\frac{\partial(\rho\theta)}{\partial t} + \nabla \cdot (\rho\mathbf{q}) = \rho^* q$$

• Temporal Term:

$$\frac{\partial(\rho\theta)}{\partial t} = \frac{\partial(\rho\eta S_w)}{\partial t} = \eta S_w \frac{\partial\rho}{\partial t} + \rho S_w \frac{\partial\eta}{\partial t} + \eta \rho \frac{\partial S_w}{\partial t}$$

First Term

$$\frac{\partial \rho}{\partial t} = \frac{\partial \rho}{\partial p} \frac{\partial p}{\partial t} + \frac{\partial \rho}{\partial C} \frac{\partial C}{\partial t} = \rho \beta \frac{\partial p}{\partial t} + \frac{\partial \rho}{\partial C} \frac{\partial C}{\partial t}$$

Second Term

$$\frac{\partial \eta}{\partial t} = \frac{\partial \eta}{\partial p} \frac{\partial p}{\partial t} = (1 - \eta)\alpha \frac{\partial p}{\partial t}$$

• Discharge Vector

$$\mathbf{q} = -\mathbf{K} \cdot \nabla h = -\mathbf{K} \cdot \nabla (\psi + z) = -\frac{\mathbf{k}}{\mu} \cdot (\nabla p + \rho g \nabla z)$$

• Continuity equation can be written as

$$\rho \left[ \theta \beta + \frac{\theta}{\eta} (1 - \eta) \alpha \right] \frac{\partial p}{\partial t} + \theta \frac{\partial \rho}{\partial C} \frac{\partial C}{\partial t} + \rho \eta \frac{\partial S_w}{\partial t} = \nabla \cdot \left[ \frac{\rho \mathbf{k}}{\mu} \cdot (\nabla p + \rho g \nabla z) \right] + \rho^* q$$

Relationship between pressure and reference pressure head

$$dp = \rho g d\psi = \rho g \frac{d\psi}{d\theta} d\theta = \rho g \frac{d\psi}{d\theta} (\eta \ dS_w + S_w \ d\eta) \approx \rho g \frac{d\psi}{d\theta} (\eta \ dS_w) \left[ \because d\eta \ll dS_w \right]$$

Rearranging the terms

$$\frac{1}{g}\frac{d\theta}{d\psi}\frac{\partial p}{\partial t} = \rho\eta \frac{\partial S_w}{\partial t}$$

Substituting values

$$\rho \left[ \theta \beta + \frac{\theta}{\eta} (1 - \eta) \alpha \right] \frac{\partial p}{\partial t} + \theta \frac{\partial \rho}{\partial C} \frac{\partial C}{\partial t} + \frac{1}{g} \frac{d\theta}{d\psi} \frac{\partial p}{\partial t} = \nabla \cdot \left[ \frac{\rho \mathbf{k}}{\mu} \cdot (\nabla p + \rho g \nabla z) \right] + \rho^* q$$

Reference pressure head

$$\hbar = \frac{p}{\rho_0 g}$$

Relationship between pressure head and reference pressure head

$$\rho_0 d\hbar = \rho d\psi$$

Continuity equation can be written as

• Defining modified compressibilities of media and water as

$$\alpha' = (1 - \eta)\alpha\rho_0 g$$
$$\beta' = \beta\rho_0 g$$

• Final form can be written as

$$\frac{\rho}{\rho_0} \left[ \theta \beta' + \frac{\theta}{\eta} \alpha' + \frac{d\theta}{d\hbar} \right] \frac{\partial \hbar}{\partial t} + \frac{\theta}{\rho_0} \frac{\partial \rho}{\partial C} \frac{\partial C}{\partial t} = \nabla \cdot \left[ \frac{\rho g \mathbf{k}}{\mu} \cdot \left( \nabla \hbar + \frac{\rho}{\rho_0} \nabla z \right) \right] + \frac{\rho^*}{\rho_0} q$$

In compact form

$$\frac{\rho}{\rho_0} F \frac{\partial \hbar}{\partial t} + \frac{\theta}{\rho_0} \frac{\partial \rho}{\partial C} \frac{\partial C}{\partial t} = \nabla \cdot \left[ \mathbf{K} \cdot \left( \nabla \hbar + \frac{\rho}{\rho_0} \nabla z \right) \right] + \frac{\rho^*}{\rho_0} q$$

where

$$\frac{\rho}{\rho_0} = a_0 + a_1C + a_2C^2 + a_3C^3$$

$$\frac{\mu}{\mu_0} = b_0 + b_1C + b_2C^2 + b_3C^3$$

• In case of seawater intrusion

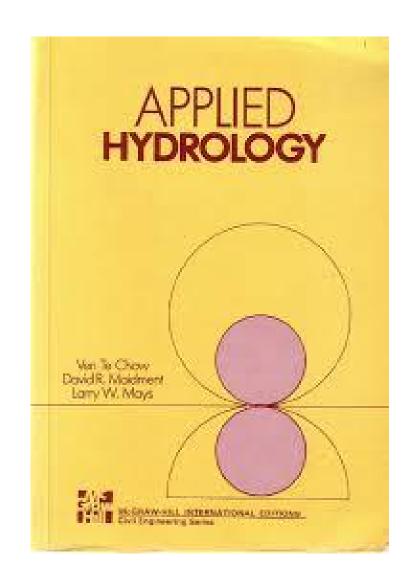
$$\frac{\rho}{\rho_0} = 1 + \epsilon C$$

where

$$\epsilon = \frac{\rho_{max}}{\rho_0} - 1$$

## **Learning Strategy**

Chapter 4: Subsurface Water



# Thank you