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STORMWATER MANAGEMENT MODEL USER'S MANUAL VERSION III Addendum I EXTRAN

by

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FOREWORD

The U.S. Environmental Protection Agency was created because of increasing public and government concern about the dangers of pollution to the health and welfare of the American people. Noxious air, foul water, and spoiled land are tragic testimonies to the deterioration of our natural environment. The complexity of that environment and the interplay of its components require a concentrated and integrated attack on the problem.

Research and development is that necessary first step in problem solution; it involves defining the problem measuring its impact, and searching for solutions. The Municipal Environmental Research Laboratory develops new and improved technology and systems to prevent, treat, and manage wastewater and solid and hazardous waste pollutant discharges from municipal and community sources, to preserve and treat public drinking water supplies, and to minimize the adverse economic, social, health, and aesthetic effects of pollution. This publication is one of the products of that research and provides a most vital communications link between the researcher and the user community.

Mathematical models are an important tool for use in analysis of quantity and quality problems resulting from urban storm wate. runoff and combined sewer overflows. This report is an updated user's manual and documentation for one of the first of such models, the EPA Storm Water Management Model (SWMM). Detailed instructions on the use of the model are given and its use is illustrated with case studies.

Francis T. Mayo, Director
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Research Laboratory

PREFACE

This document is the user's guide and program documentation for the computer model EXTRAN. EXTRAN is a dynamic flow routing model that routes inflow hydrographs through an open channel and/or closed conduit system, computing the time history of flows and heads throughout the system. While the computer program was developed primarily for use in urban drainage systems -- including combined systems and separate systems -- it can also be used for stream channels if the cross-section can be adequately represented as a trapezoidal channel.

EXTRAN is intended for application in systems where the assumption of steady flow, for purposes of computing backwater profiles, cannot be made. The program solves the full dynamic equation for gradually varied flow (Navier-Stokes equation) using an explicit solution technique to step forward in time. As a result, the solution time-step is governed by the wave celerity in the shorter channels or conduits in the system. Time-steps of 5-seconds to 60-seconds are typically used, which means that computer time is a significant consideration in the use of the model.

The conceptual representation of the drainage system is based on the "link-node" concept which does not constrain the drainage system to a dendritic form. This permits a high degree of flexibility in type of problems that can be examined with EXTRAN. These include parallel pipes, looped systems, lateral diversions such as weirs, orifices, pumps, and partial surcharge within the system.

Because of the versatility of the EXTRAN model, there is a tendency for some users to apply the model to the entire drainage system being analyzed even though flow routing through most of the system could be performed with a simpler model such as RUNOFF or TRANSPORT*. The result is a very large system simulated at relatively small time-steps which produces great quantities of data that are difficult to digest. Where simpler models are applicable (no backwater, surcharges, or bifurcations) substantial savings in data preparation and computer solution time can be realized using the simpler routing model.

EXTRAN has limitations which, if not appreciated, can result in improperly specified systems and the erroneous computation of heads and flows. The significant limitations are these:

- Headloss at manholes, expansions, contractions, bends, etc. are not explicitly accounted for. These losses

*That is, the RUNOFF and TRANSPORT Modules from the EPA SWMM computer program.

must be reflected in the value of the Manning n specified for the channels or conduits where the loss occurs.

- Changes in hydraulic head due to rapid expansions or contractions are neglected. At expansions, the headloss will tend to equalize the heads; but at contractions, the headloss could aggravate the problem.

- At a manhole where the invert of connecting pipes are different (e.g., a drop manhole), computational errors will occur during surcharge periods if the invert of the highest pipe lies above the crown of the lowest pipe. the severity of the error increases as the separation increases.

- Computational instabilities can occur at junctions with weirs if: 1) the junction is surcharged, and 2) the weir becomes submerged to the extent that the downstream head equals or exceeds the upstream head.

- EXTRAN is not capable of simulating water quality. Any quality information input on tape to EXTRAN is ignored by the program.

Methods for dealing with these problems are discussed in Chapter 4.

Finally, a word of caution. EXTRAN is a tool, like a calculator, that can assist engineers in the examination of the hydraulic response of a drainage system to inflow hydrographs. While the model is based on scientific truth, approximations in time and space are made in order to solve these problems. While we have tried to anticipate most prototype configurations, these approximations may not be appropriate in some system configurations or unusual hydraulic situations. Therefore, persons using the computer program must be experienced hydraulicians. The computational results should never be taken for granted, but rather the computer output should be scanned for each simulation to look for suspicious results. The checking procedure should be analogous to that which would be followed in checking a backwater profile that a junior engineer had performed by hand computation. Remember that the major difference between the engineer and the computer is that the computer can't think!

For the May 1982 Second Printing, minor typographical errors have been corrected on the following pages: iii,8,19,93,94,95,99,100,102,103, 106,110,115,116,121,122,153,160,162,164. Where easily done, some changes in the program code that make the program agree with that contained on the May 1982 Version III.1 of SWMM are shown on pages: 169,183,184,188, 189,190,201,208,215. (Only those on pages 189,201 and 208 are error corrections.)

For the July 1983 Sixth Printing, minor typographical errors have been corrected on the following pages: v, 13, 19, 99, 100, 105, 110, 113, 114.

ABSTRACT

This report contains the documentation and user's manual for the Extended Transport (EXTRAN) Block of the Storm Water Management Model (SWMM). EXTRAN is a dynamic flow routing model used to compute backwater profiles in open channel and/or closed conduit systems experiencing unsteady flow. It represents the drainage system as links and nodes, allowing parallel or -looped pipe networks; weirs, orifices, and pumps; amd system surcharges to be simulated. EXTRAN is used most efficiently if it is only applied to those parts of the drainage system which cannot be simulated accurately by simpler, less costly models.

The EXTRAN manual is designed to give the user complete information in executing of the model both as a block of the SWMM package and as an independent model. Formulation of the input data is discussed in detail and demonstrated by seven example problems. Typical computer output is also discussed. Problem areas which the user may confront are described, as well as the theory on which the EXTRAN model rests. The manual concludes with a comprehensive discussion of the EXTRAN code.

This report was submitted in partial fulfillment of EPA Cooperative Agreement No. CR805664 to the University of Florida under the partial sponsorship of the U.S. Environmental Protection Agency. Camp Dresser & McKee, Inc. prepared this report as a contractor to the University of Florida. Work was completed as of August, 1981.

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CHAPTER 1

BLOCK DESCRIPTION

BACKGROUND

EXTRAN is a hydraulic flow routing model for open channel and/or closed conduit systems. The EXTRAN Block receives hydrograph input at specific nodal locations by tape transfer from the RUNOFF Block and/or by input. The model performs dynamic routing of stormwater flows through the major storm drainage system to the points of outfall to the receiving water system. The program will simulate branched or looped networks, backwater due to tidal or nontidal conditions, free-surface flow, pressure flow or surcharge, flow reversals, flow transfer by weirs, orifices and pumping facilities, and storage at on or off-line facilities. Types of channels that can be simulated include circular, rectangular, horseshoe, egge, baskethandle pipes, plus trapezoidal channels. Simulation output takes the form of water surface elevations and discharge at selected system locations.

EXTRAN was developed for the City of San Francisco in 1973([1,2]). At that time it was called the San Francisco Model and (more properly) the WRE Transport Model. In 1974, EPA acquired this model and incorporated it into the SWMM package, calling it the Extended Transport Model - EXTRAN - to distinguish it from the TRANSPORT Module developed by the University of Florida as part of the original SWMM package. Since that time, the model has been refined, particularly in the way the flow routing is performed under surcharge conditions. Also, much experience has been gained in the use and misuse of the model.

This document is the User's Manual and Program Documentation of the most recent version of EXTRAN as extended and refined by Camp Dresser & McKee Inc. (CDM)[1]. The documentation section (Chapter 5) has been expanded to include more discussions of program limitations *and* the input data descriptions have been revised to provide more guidance in the preparation of data for the model.

The remainder of this chapter discusses program operating requirements and characteristics of EXTRAN, and how it interfaces with other SWMM blocks. Chapter 2 contains instructions for data preparation. Narrative discussions of the input data requirements contain tips for developing a well defined system. Chapter 3 consists of several example problems that demonstrate how to set up EXTRAN for each of the storage/diversion options in the model.

[1]Water Resources Engineers was wholly integrated into Camp Dresser & McKee, Inc. in 1980.

Chapter 4 discusses typical problems that can occur with the use of the model and what action should be taken to correct them. A discussion of error messages contained in the program is also presented. Chapter 5 describes the conceptual, mathematical, and functional respesentation of EXTRAN; the program structure and listing is contained in Chapter 6.

PROGRAM OPERATING REQUIREMENTS

EXTRAN was originally programed for the Univac 1108 in FORTRAN V. This version of the FORTRAN compiler is essentially compatible with the IBM FORTRAN LEVEL G compiler and the extended compiler used on CDC 6600 series equipment. The model was subsequently installed on IBM, CDC, DEC 20, and several other computers. The latest refinements to the model have been performed on the DEC 20 computer.

EXTRAN is presently sized to simulate drainage systems of up to 187 channels, 187 junctions, 20 storage elements, 60 orifices, 60 weirs, 20 pumps, and 25 outfalls. The core storage and peripheral equipment to operate this program are:

High speed core: $130,000_8$ words
$45,000_{10}$ words
Peripheral storage: 2 drum, disk or tape files
One card reader or input file device
One line printer

Execution times for EXTRAN are roughly proportional to the number of system conduits and the number of time-steps in the simulation period. A summary of CDM's prior experience in running the EXTRAN on both CDC 6600 and Univac 1108 systems is presented graphically in Figure 1. Using the Univac 1108 operating data in Figure 1 as an example, it is estimated that the total compuation time for a network of 100 pipes, using a 10-second time-step over a 1-hour simulation period, would be approximately 300 system-seconds. Run time for the example problems in Chapter 3 (9 pipes, 8 hour simulation, 20 second time-step) was about 44 seconds on the DEC 20 computer. Note that the curves presented in Figure 1 become highly nonlinear for $t < 10$ seconds because of the increased frequency of internal tape transfers and output processing.

INTERFACING WITH OTHER SWMM BLOCKS

The EXTRAN Program can easily be interfaced with other SWMM Blocks, even though EXTRAN is designed to stand by itself. Figure 2 shows a schematic overview of the EXTRAN Block and its relation to SWMM system control and input data cards. The EXTRAN Block receives hydrograph input at specific nodal locations either by tape transfer from a preceding block, usually RUNOFF, or by card input, described in Chapter 2. The output tape, which contains hydrographs at all system outfall points, can be generated if desired. This output tape can then be used as input to any subsequent SWMM Block, typically RECEIV. The EXTRAN Program itself is called as a subroutine by the Executive Block. The EXTRAN Block, in turn, reads the input data it requires to perform its flow routing function.

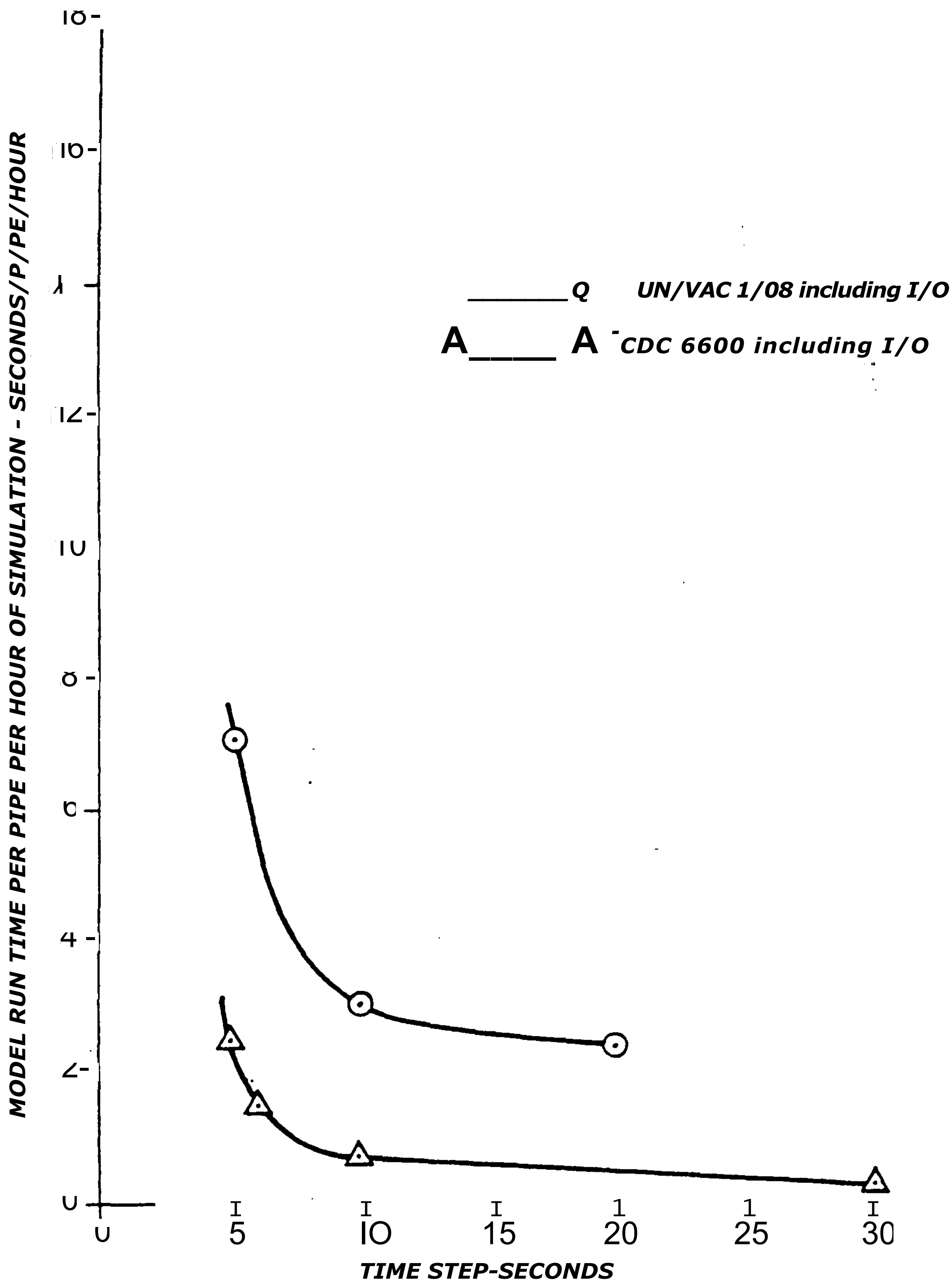

Figure 1. Summary of EXTRAN run times on CDC and UNIVAC systems.

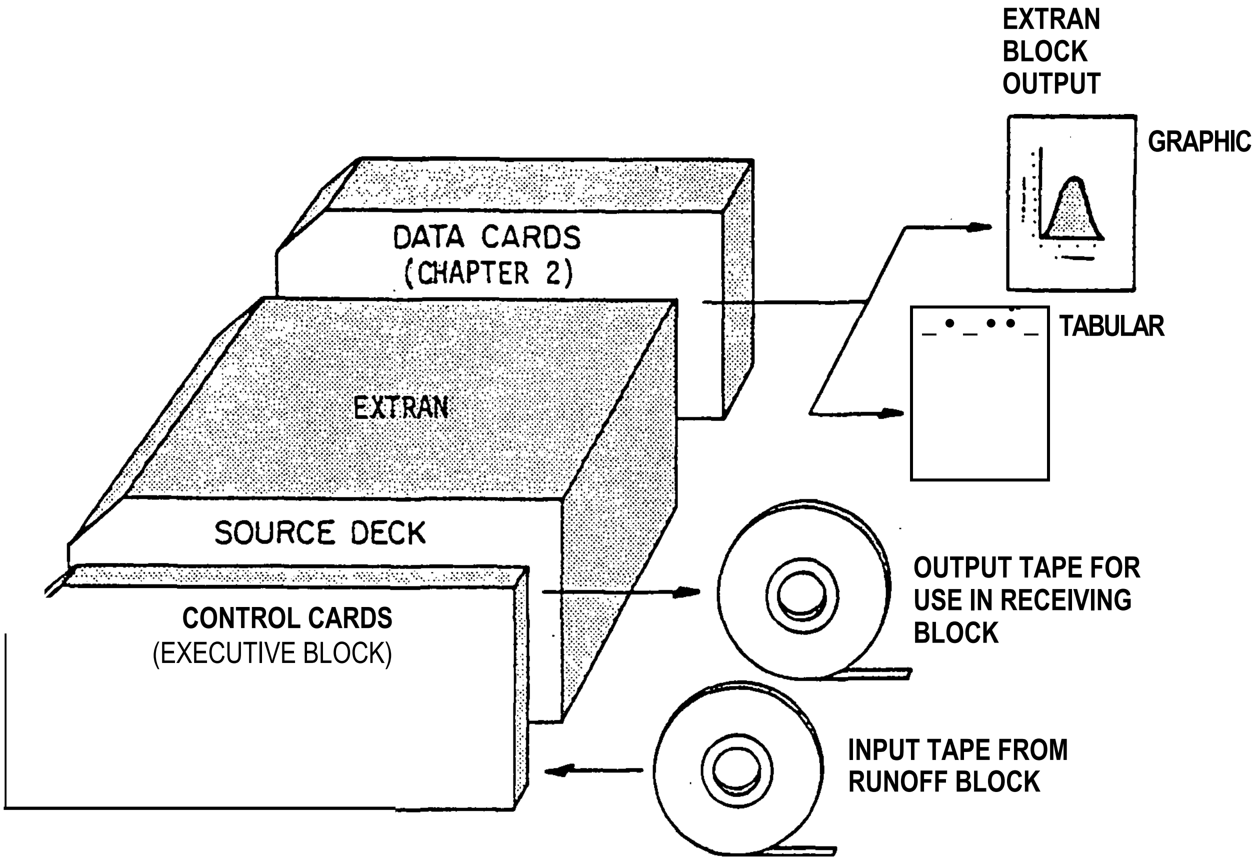

Figure 2. Typical schematic of EXTRAN block setup deck.

The version of EXTRAN shown in Appendix Table A-3 is set up to run outside the SWMM Executive Block. Chapter 6 explains the revisions to the code needed to convert the present main program of EXTRAN to Subroutine EXTRAN of the Executive Block. Although SWMM is designed to run successive blocks consecutively without user intervention, it is strongly recommended that this option not be used with EXTRAN. Simulation results should be examined before they are used as input to EXTRAN; EXTRAN results should be reviewed, in turn, for reasonableness before they are input to subsequent blocks. To bypass the inter-block review process is to invite undetected errors in the analysis results and/or to require expensive reruns of blocks that used erroneous output data from a preceeding block.

If EXTRAN is the only block called from the Executive Block, input data for the Executive Block would be structured as follows:

CARD GROUP 1 I/O tape/disk assignments

JIN = output tape/file number from, typically, the RUNOFF Block if RUNOFF hydrographs are to be used in simulation

= 0 if input hydrographs are from cards only (See Card Group 20 and 21 in EXTRAN Block input data description)

JOUT = output tape/file number that will be used to input outfall hydrographs from EXTRAN into a subsequent block such as RECEIV

= 0 if the outfall hydrographs are not required by a subsequent block

Note that there is no EXTRAN Quality Block. If pollutographs are to be routed through the drainage system, it is suggested that RUNOFF or TRANSPORT be used for this purpose.

CARD GROUP 2 - Scratch tape/disk assignment

Enter a blank card

CARD GROUP 3 - Block Control Card

Enter EXTRAN only

In this case, Card Group 1 of the EXTRAN input data set shown in Chapter 2, Table 1, is not needed and should be omitted. If, on the other hand, EXTRAN is run independently of the Executive Block usi ng the code shown in Appendix Table A-3, the input data set should be formed exactly as shown in Table 1, omitting the above Executive Block input data.

CHAPTER 2

INSTRUCTIONS FOR DATA PREPARATION

INTRODUCTION

When a drainage system is to be analyzed with EXTRAN, the first step in the study is generally to define the sewer system and the watershed (sewershed) that it drains. This information is usually available from the agency responsible for operation *and* maintenance of the system. Care should be taken in this step to insure that "as built" drawings of the system are used. Where information is suspect, a field investigation is in order.

Once the sewer system and watershed has been defined, the watershed is subdivided into subareas in accordance with the guidelines presented in the RUNOFF Block documentation. Figure 3 shows the South Boston combined sewer system and its watershed subdivided into subbasins. Figure 4 is a schematic representation of the South Boston combined sewer system. Note that "TRANSPORT" refers to EXTRAN in this case. The figure shows all pipes and channels to be simulated in the study, the location and type of all diversion structures and all system outlets and overflow points. It may be of interest to note here that the 6000-series channels at the Columbus Park Headworks represent the four-channel grit chambers in the headworks that determine the stage-discharge relationship at Junction 60101 in the sytem.

Note that conduits are distinguished on Figure 4 between those that will be simulated in RUNOFF and those to be simulated in EXTRAN. As a general rule, the upstream portions of the drainage system should be represented in RUNOFF as much as possible because the data preparation is more simple and the flow routing takes less computer time. The dividing point for the two systems is the point where backwater effects, surcharge, and/or diversion facilities affect the flow and head computation. Pipes and channels downstream of this point should be included in EXTRAN.

Junction Points should be identified at each:

- Upstream terminal point in the system,
- Outfall and discharge point,
- Pump station, storage point, orifice and weir diversion,
- Junction where inflow hydrographs will be input (either by card input or from RUNOFF),
- Pipe junction,
- Point where pipe size/shape changes significantly,
- Point where pipe slope changes significantly, and
- Point where pipe inverts are significantly different.

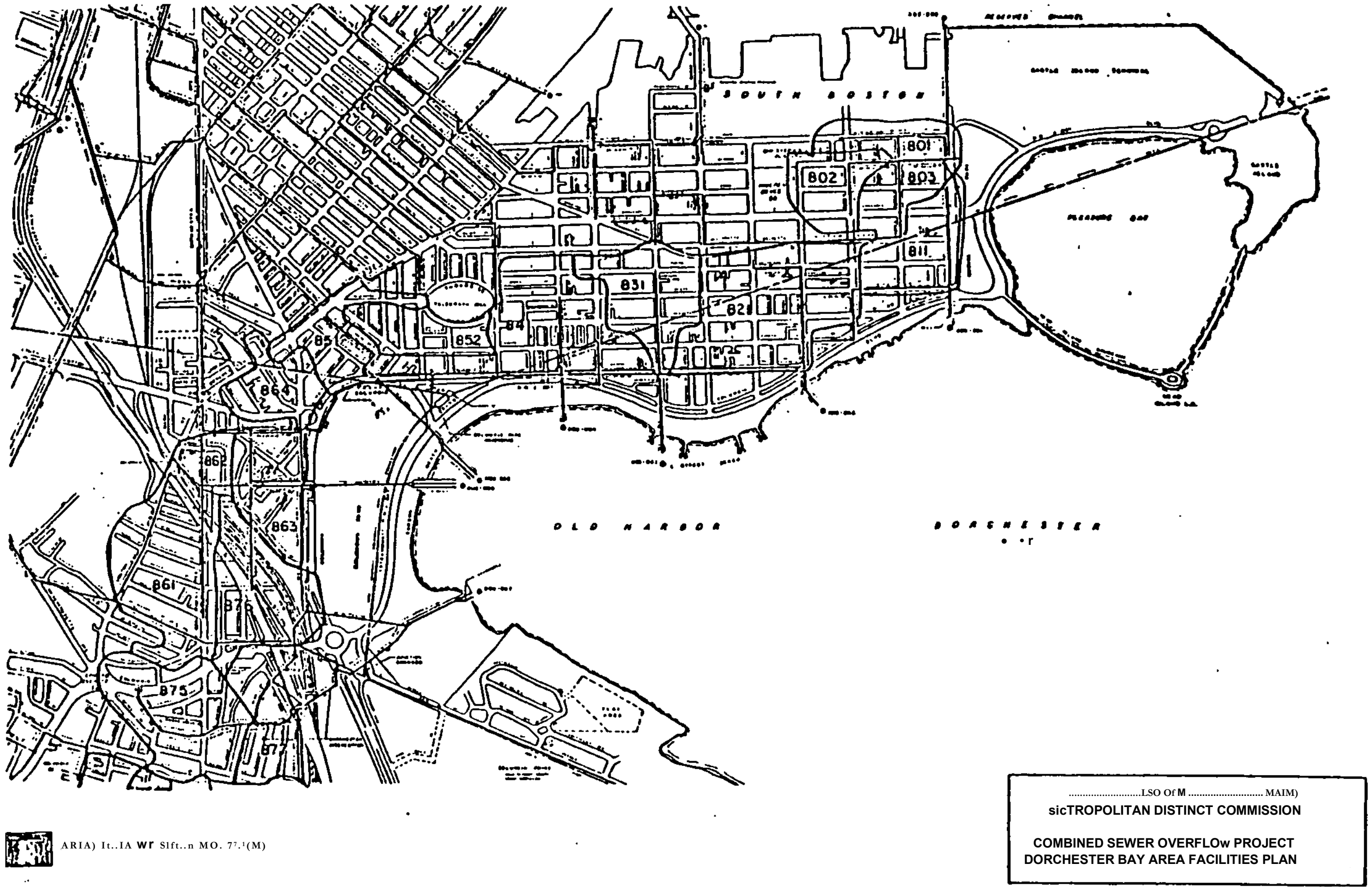

Figure 3. Runoff Subbasins Tributary to South Boston Interceptor.

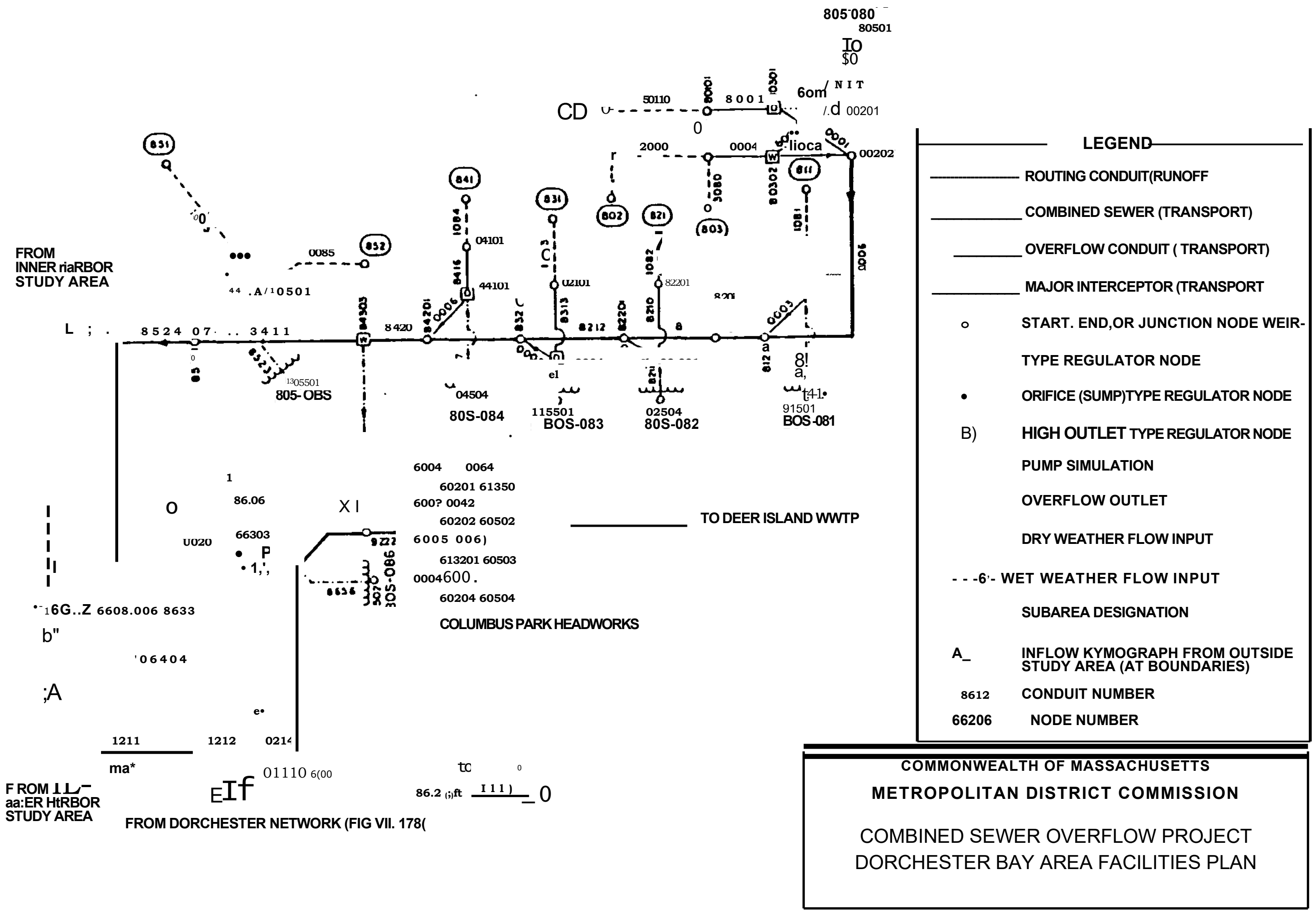

Figure 4. Schematic Representation of the South Boston Sewerage System for Use in the EXTRAN Model.

Following the preliminary identification of junction points, a check should be made to eliminate extremely long or short distances between junctions. As a rule of thumb, the longest conduit should not exceed four or five times the length of the shortest conduit. If this occurs, short conduits can be increased in length by use of equivalent pipes and long conduits can be shortened by adding intermediate junction points.

Keep in mind when setting conduits lengths (placing junctions) that the time-step is generally controlled by the wave celerity in the system. To estimate the time-step, first compute:

$$\Delta t_c = \frac{L}{\sqrt{gD}} \qquad (1)$$

where:

Δt_c = time for a surface wave to travel from one end of a conduit to the other in seconds,
L = conduit length in feet,
g = 32.2 ft/sec,
D = channel depth or pipe diameter in feet.

The time-step can usually exceed Δt_c by a factor of 1.5 to 2.0 for a few widely separated conduits. For most problems, conduit lengths can be of such length that a 15 to 30 second time-step can be used. Occasionally, a 5 to 10 second time-step is required. A time-step of 60 to 90 seconds should not be exeeded even in large open channel systems where the celerity criteria is not violated with a larger time-step.

If an extremely short pipe is included in the system, as indicated by a small Δt_c, an equivalent longer pipe can be developed using the following steps. First, set the Manning equation for the pipe and its proposed equivalent equal:

$$\frac{1.49}{n_p} A_p R_p^{2/3} S_p^{1/2} = \frac{1.49}{n_e} A_e R_e^{2/3} S_e^{1/2} \qquad (2)$$

where:

p = actual pipe,
e = equivalent pipe,
n = Manning's roughness coefficient,
A = cross-sectional area,
R = hydraulic radius, and
S = slope of the hydraulic grade line.

If we assume that the equivalent pipe will have the same cross-sectional area and hydraulic radius as the pipe it replaces, we can say:

$$S_p^{1/2}/n_p = S_e^{1/2}/n_e \qquad (3)$$

Now, since

$$S = h_L/L \qquad (4)$$

where:

h_L = the total head loss over the conduit length, and
L = conduit length,

and since the head losses are to be equal in both pipes, equation (2) can be simplified to:

$$n_e = n_p L_p^{1/2} / L_e^{1/2} \tag{5}$$

where L_e is the desired equivalent pipe length, either no smaller than four to five times smaller than the longest pipe in the system, or large enough to give a t_c within the range indicated above. The user, through experience, will be able to determine the pipe length changes required to achieve stability and an acceptable time-step for the simulation.

At this point, the system schematic should be in pretty good shape for developing model input data. The remaining sections of this chapter describe, step-by-step, how to develop the input data file for EXTRAN.

INPUT DATA DESCRIPTION

Specifications for input data preparation are contained in Table 1. The table defines input format column location and variable description and name. Table A-1 in the Appendix is a set of input data forms which can be used to facilitate encoding the data for EXTRAN. Perusal of Table 1 reveals that the input data is divided into 22 card groups. Card Groups 1-7 are control cards that identify the simulation, set the time-step and start time, and identify junctions for card input hydrograph, and junction and conduits for printing and plotting of heads and flows. The identification of conduits and junctions is done in Card Groups 8 and 9, respectively. Card Groups 10-13 identify storage and diversion junctions, while Groups 14-18 identify system outfalls and backwater conditions at the outfalls. Initial conditions for heads and flows are defined in Card Groups 19-20. Card Groups 21 and 22 define card input hydrographs. Further descriptions of the data to be entered in each card group are given below.

Card Group 1: Tape Numbers

Card Group 1 is a single card *which* specifies the identifying numbers of tapes used for input and output hydrograph storage, if used. A zero should be entered if a particular tape is not used. Card Group 1 should be included only if EXTRAN is to be run on its own, i.e., outside the SWMM Executive Block.

Card Group 2: Run Identification

Card Group 2 consists of 2 cards, each having 80 columns or less, which typically describe the system and the particular storm being simulated.

Card Group 3: Run Control

Card Group 3 is a single card defining the number of integration steps in the simulation period, the length of each time-step, output control data, the number of hydrograph input points to be supplied by cards tin addition to, or rather than, tape input generated by the Runoff Block), and control parameters for iterations on computations for surcharged areas.

The time-step, DELT, is most critical to the cost and stability of the EXTRAN model run and must be selected carefully. The time-step should be selected according to the guideline described in the Introduction to this chapter (see Equation 1). The computer program will check each conduit for violation of the surface wave criteria and will print the message:

**** WARNING **** (C*DELT/LEN) IN CONDUIT IS rrr AT FULL DEPTH

where rrr is the ratio

of = t (6)

where

6t = the time-step
g = gravity
D = conduit height or pipe diameter
L = conduit length

As already noted, if rrr is greater than 1.5 or 2.0 for any conduit, or if several conduits have rrr over 1.5, the time-step should be reduced. rrr should never exceed 1.0 in a terminal conduit (i.e., an upstream term-Ml conduit or outfall).

Another constraint to be observed carefully is the length of the total simulation period defined as the product of NTCYC and DELT. This period must not extend in time beyond the simulation period of any preceding block. Otherwise, an improper attempt to read beyond the end-of-file of the input hydrograph tape is made and execution of EXTRAN stops.

The printing interval, INTER, also must be specified carefully to insure proper output of heads, velocities, and flows. The present output capacity of EXTRAN provides for 100 values each of nodal water depth, elevation, conduit flow, and velocity to be printed as detailed output for any given simulation run. When this number is exceeded, the printing arrays are filled with extraneous results taken from other core storage locations which bear no resemblance to the desired output. As an example, if NTCYC = 1600 and we start printing in cycle 1(NSTART=1), then INTER must = 16 or more to maintain correct printing control. Alternatively, if NSTART = 801 then INTER can be 8. Also, the output looks better if NSTART and INTER are selected so that the first (and subsequent) output(s) occurs at an even minute(s) or half minute(s).

The variables ITMAX and SURTOL control the accuracy of the solution in surcharged areas. In reality, the inflow to a surcharged area should equal the outflow from it. Therefore, the flows and heads in surcharged areas are recalculated until either the difference in inflows and outflows is less than a tolerance, defined as SURTOL times the average flow in the surcharged area, or the number of iterations exceeds ITMAX. It has been found that good starting values for ITMAX and SURTOL are 30 and 0.05, respectively. The user should be careful to check the intermediate printout, which indicates whether the iteration is converging. Also, if there is more than one surcharged section of the drainage system, special rules apply. More details on checking convergence of the surcharge iterations are found in Tips For Troubleshooting (Chapter 4).

Card Groups 4 and 5: Detailed Printing for Junctions and Conduits

Card Group 4 contains the list of individual junctions (up to 20) for which water depth and water surface elevations are to be printed continuously throughout the course of the simulation period. Card Group 5 contains the list of individual conduits tup to 20) for which flows and velocities are to be printed.

Card Groups 6 and 7: Detailed Plotting for Junctions and Conduits

Card Groups 6 and 7 contain, respectively, the lists of junctions and conduits for which time histories and water surface elevation and flows are to be plotted.

Card Group 8: Conduit Data

Card Group 8 contains data input specification for conduits including shape, size, length, hydraulic roughness, connecting junctions, and invert distances referenced from the junction invert. The input data instructions, as presented in Table 1, are self-explanatory with the exception of junction/conduit invert elevations.

Basic definitions of conduit invert distances ZP(N,1) and ZP(N,2) are illustrated in Figure 5. The junction invert elevation is specified in Card Group 9. The distance ZP is height of the invert of connecting conduits above the junction floor. Note, however, that the lowest pipe connected to the junction (pipe N in Figure 5) must have a ZP of zero. If it does not, the junction will behave like a mass sink in the system. Water will flow into the junction but none will flow out.

Card Group 9: Junction Data

The explanation of ground and invert elevations is also shown in Figure 5. One junction card is required for every junction in the network including: regular junctions, storage and diversiFi-Tinctions, pump junctions, and outfall junctions. It is emphasized again that the junction invert elevation is defined as the invert elevation of the lowest pipe connected to the junction. The program execution will terminate with an error message:

**** ERROR **** ALL CONDUITS CONNECTING TO JUNCTION
LIE ABOVE THE JUNCTION INVERT

unless there is at least one pipe having a zero ZP at the junction.

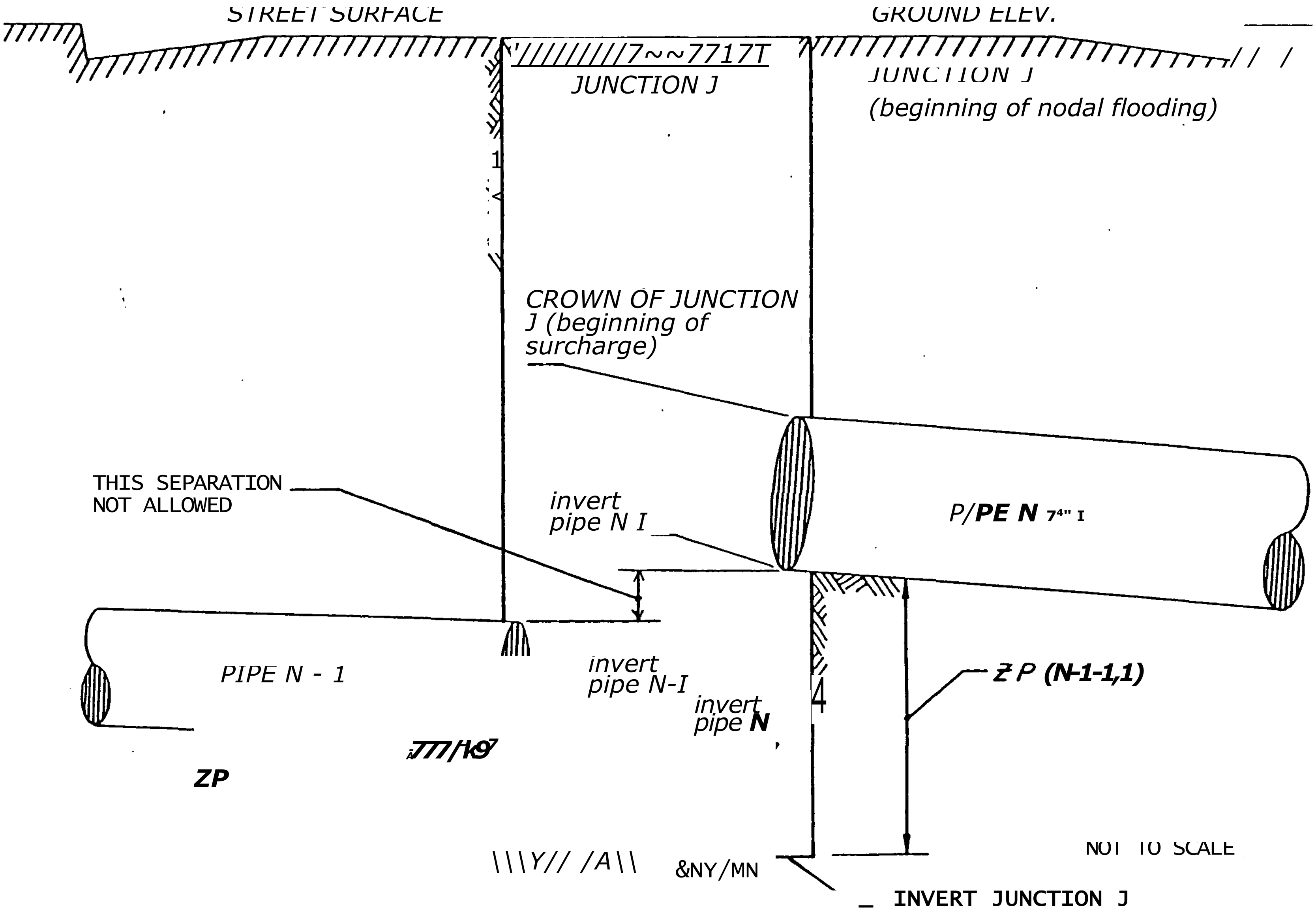

Figure 5. Definition of elevation terms for three-pipe junction.

The surcharge level or junction crown elevation is defined as the crown elevation of the highest connecting pipe and is computed automatically by EXTRAN. Note that the junction must not surcharge except when the water surface elevation exceeds the crown of the highest pipe connected to the junction. *Pipe* N+1 in Figure 5 is too *high.* This junction would go into surcharge during the period when the water surface is between the crown of pipe N-1 and the invert of pipe N+1. If a junction is specified as shown in Figure 5 and the water surface rises above the crown of pipe N-1, the program will print an error message:

```
**** ERROR **** SURFACE AREA AT JUNCTION IS ZERO,
CHECK FOR HIGH PIPE
```

and will then stop. To correct this situation, a new junction should be specified that connects to pipe N+1. A "dummy conduit is specified which connects the old junction with pipes N-1 and N to the new junction which connects to pipe N+1. The pipe diameter should be that of N+1 and the length selected to meet the stability criteria given by Equation 6. ihe Manning *n* for the "dummy pipe" is computed to reflect the energy loss that occurs during surcharge as water moves up through the manhole and into pipe N+1.

The "ground elevation", GRELEV(J), is the elevation at which the assumption of pressure flow is no longer valid. Normally, this will be the street or ground elevation; however, if the manholes are bolted down, the GRELEV(J) should be set sufficiently high so that the simulated water surface elevation does not *exceed* it. When the hydraulic head must exceed GRELEV(J) to maintain continuity at the junction, the program allows the excess junction inflow to "overflow onto the ground" and become lost from the system for the remainder of the simulation period.

QINST(J) is the net constant flow entering (positive) or leaving (negative) the junction.

Card Group 10: Storage Junctions

Conceptually, storage junctions are tanks of constant surface area, over their depth. A storage "tank" may be placed at any junction in the sytem, either in-line or off-line. The elevation of the top of the tank is specified in the storage junction data and must be at least as high as the highest pipe crown at the junction. If this condition is violated, the system will go into simulated surcharge before the highest pipe is flowing full.

Card Group 11: Orifice Cards

EXTRAN simulates orifices as equivalent pipes (see Chapter 5). Data entry is straightforward. For sump orifices, the program automatically sets the invert of the orifice one diameter below the junction invert so that the orifice is flowing full before there is any discharge (overflow) to conduits downstream of the junction containing the orifice.

Card Group 12: Weir Cards

The definition sketch for weirs is illustrated in Figure 6. The following types of weirs can be simulated:

- Internal diversions (from one junction to another via a transverse or sideflow weir.

- Outfall weirs which discharge to the receiving waters. These weirs may be transverse or sideflow types, and may be equipped with flap gates that prevent backflow.

Transverse weir and sideflow weirs are distinguished in EXTRAN by the value of the exponent to which the head on the wg-ir is taken. For transverse weirs, head is taken to the 3/2 pow (i.e., Q_w-H^4/) while for sideflow weirs the exponent is 5/3 (i.e., Q_w-Ho/J).

When the water depth at the weir junction exceeds YTOP (see Figure 6) the weir functions as an orifice (Q_w-$H^{1/2}$). The discharge coefficient for the orifice flow condition is computed internally in EXTRAN (see Chapter 5).

Stability problems can be encountered at weir junctions if the junction surcharges during the simulation. If this happens or is suspected of happening, the weir·..may be represented as an equivalent pipe. To do this, equate the pipe and weir discharge equations, e.g.:

$$1.49 \; A...K^{2/3} \; S^{1/2} = C_w W H^{3/2} \qquad (7)$$

where:

n = Manning n for the pipe,
A = cross-sectional area,
R = hydraulic radius,
S = hydraulic grade line for the pipe,
H = head across the weir,
C_w = weir discharge coefficient,
W = weir length.

In this equation, S = H/L where L is the pipe length, and A = WH. If R is set at the value of the hydraulic radius where the head is half way between YCREST and YTOP, and L is set in accordance with Equation 6, then n can be computed as:

$$n \quad \frac{R^{2/3}}{C_w L^{1/2}} \qquad (8$$

for the equivalent pipe.

Card Group 13: Pump Cards

Pumps may be of two types:

1. An off-line pump station with a wet well; the rate of pumping depends upon the volume (level) of water in the wet well.

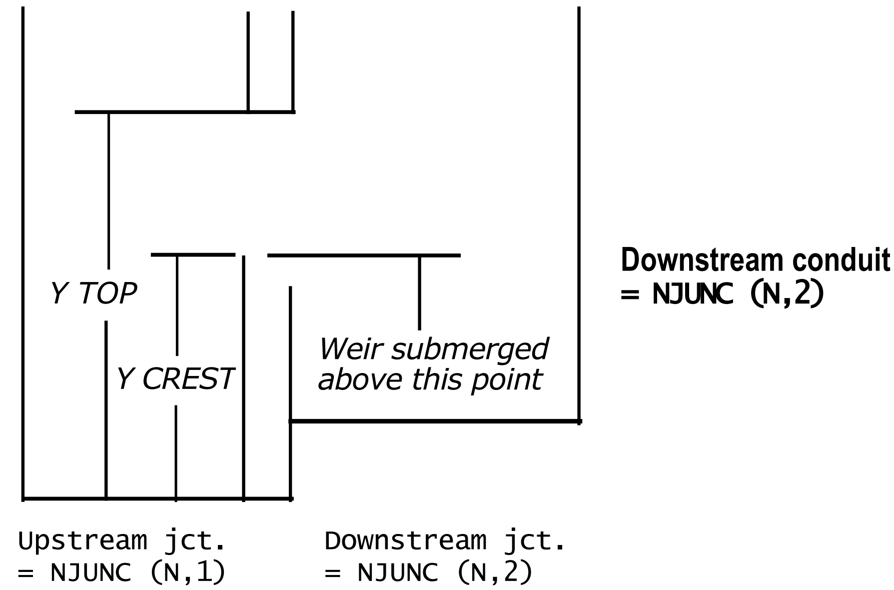

Figure 6. Definition sketch of weir input data.

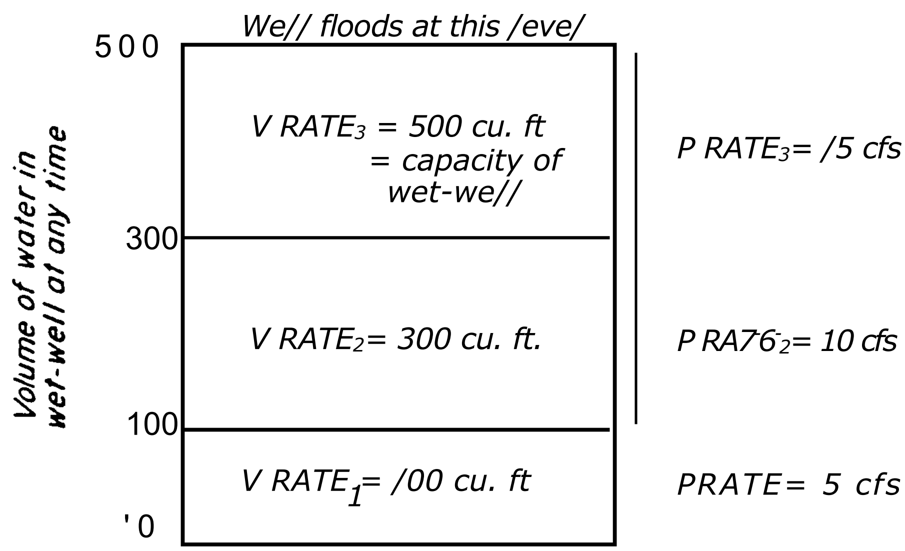

Figure 7. Definition sketch of pump input data. 16

2. An on-line lift station that pumps according to the level of the water surface at the junction being pumped.

The definition sketch in Figure 7 defines the input variable for Type 1

pump. For a Type 2 pump station, the following operating rule is used:

Y < VRATE(I,1)	Qp = Junction inflow or PRATE(I,1), whichever is less
VRATE(I,1) < Y < VRATE(I,2)	Q_p = PRATE(I,2)
VRATE(I,2) < Y	Qp = PRATE(I,3)

Note that for a Type-2 pump station VRATE is the water depth at the pump junction, while for a Type 1 station it is the volume of water in the wet well.

Note also that <u>only one conduit may be connected to a Type 1 pump station junction.</u>

<u>Card Group 14: Free Outfall Pipes</u>

Three types of outfalls can be simulated in EXTRAN:

1. A weir outfall with or without a flap (tide) gate (Card Group 12),
2. A conduit outfall without a flap gate (Card Group 14), or
3. A conduit outfall with a flap (tide) gate (Card Group 15).

Under Card Group 14, enter the outfall junction number for <u>outfall conduits without</u> flap gates.

<u>Card Group 15: Outfall Pipes With Flap Gates</u>

Enter the outfall junction number for <u>conduits with flap gates.</u>

<u>Card Groups 16, 17, and 18: Tidal Backwater Control Cards</u>

Card Groups 16, 17, and 18 describe the single tidal backwater condition which is applied at all outfalls in the drainage system. The tidal index, NTIDE, is specified according to whether there is: (1) no water surface at any outfall; (2) a water surface at constant elevation; (3) a tide whose period and amplitude are described by user supplied tide coefficients; or (4) a tide which will be computed by EXTRAN using Equation 37 which is based on a specified number of stage-time points describing a single tidal cycle.

<u>Card Groups 19 and 20: Initial Flows, Velocities, and Heads</u>

Frequently, it is desired to initialize the drainage network with starting values of flow, velocity, and water surface elevation which represent either the dry weather or antecedent flow conditions just prior to the storm to be simulated. Card Groups 19 and 20 are designed for the purpose of supplying these initial conditions throughout the drainage system at the beginning of the simulation. Card Group 19 contains discharge for each conduit in the same order as it is specified in Card Group 9. Note that initial discharge must be specified for all real conduits plus all internal links. (There is one internal link for each orifice, weir, pump, and outfall in the system. In a complex network, the total number of real plus internal conduits is best determined from the conduit connectivity summary in a trial run with EXTRAN.) As an example, in a system of 25 real conduits, 28 junctions, 2 orifices, 3 weirs, and 1 free outfall, we have a total of 31 links. The specification of initial discharges in Card Group 19 requires a total of 8 cards with 4 conduits on each. For the case where all flows and heads are zero at the start of the simulation, enter 99999. in columns 6 thru 10 of Card Group 19.

Similarly, initial depths of flow (not elevations) are keypunched according to the instruction in Card Group 20. Again the initial heads are supplied for all, real and internal junctions. The latter are specified automatically by the EXTRAN program for each weir in the system. Thus in the example above, we would have a total of 31 junctions and the initial dry-weather heads would be punched on four cards in the order the junctions appear in Card Group 10.

Card Groups 21 and 22: Hydrograph Input Cards

EXTRAN provides for input of up to 20 inflow hydrographs by cards in cases where it is desirable to run EXTRAN alone without prior use of the Runoff Block or to add additional input hydrographs, either at the same or different nodes, to those computed by the Runoff Block. The specification of individual junctions receiving hydrograph input by cards is given in Card Group 21. Note that multiple hydrographs coming into a given junction can be indicated by repeating the junction number in Group 21 for each inflow hydrograph. The order of hydrograph time discharge points in Card Group 22 now must correspond exactly with the order specified by Card Group 21. The time, TEO, of each discharge point is given in decimal clock hours; i.e., 10:45 a.m. is punched as 10.75. Hydrograph time input points can be specified at any convenient time as long as a point is included for each junction specified in Card Group 21. The hydrographs are then formed by interpolating between consecutive time input points for each time step.

TABLE I. EXTRAN DATA REQUIREMENTS

Card Group	Format	Card Columns	Description	Variable Name
			TAPE UNIT NUMBERS	
			NOTE: ONLY INCLUDE CARD GROUP 1 IF EXTRAN IS RUN STANDING ALONE. SKIP IF EXTRAN IS CALLED FROM THE EXECUTIVE BLOCK OF SWMM.	
1	2I5	1-5	Hydrograph input tape from Runoff Block	N21
		6-10	Outfall hydrograph tape for input to RECEIVE Block	N22
			RUN TITLE	
2	20A4	1-60	Description of computer run (2 cards.) Will be printed on output (2 lines).	ALPHA
			RUN CONTROL PARAMETERS	
3	I5,2F5.0, 8I5,F5.0	1-5	Number of integration steps or C time cycles desired	NTCY
		6-10	Length of integration step, seconds	DELT
		11-15	Start time of simulation, decimal hours	TZERO
		16-20	Number of junctions for detailed printing of head output (20 nodes	NHPRT
		21-25	Number of conduits for detailed printing of discharge output (20 pipes max.)	NQPRT
		26-30	Number of junctions to be plotted (20 max.)	NPLT
		31-35	Number of conduit flows to be plotted (20 max.)	LPLT
		36-40	First time-step to begin print cycle	NSTART

TABLE 1. EXTRAN DATA REQUIREMENTS
(Continued)

Card Group	Format	Card Columns	Description	Variable Name
3 (Continued)		41-45	Interval between print cycles (max. number of cycles printed is $\frac{NTCYC - NSTART}{INTER}$	INTER
		46-50	Number of input junctions, if 'card W input hydrographs are used (65 max.)	NJS
		51-55	Maximum number of iterations to adjust head and flow of surcharged junctions	ITMAX
		56-60	Segment of flow in surcharged areas to be used as the tolerance for ending surcharge iterations	SURTOL
			PRINTED HEADS	
4	8I10	1-10	First junction number for detailed printing	JPRT(1)
		11-20	Second junction number, up to number of nodes defined by NHPRT	JPRT(2)
			PRINTED FLOWS	
5	8I10	1-10	First conduit number for detailed printing	CPRT(1)
		11-20	Second conduit number up to number of nodes defined by NQPRT	CPRT(2)
			PLOTTED HEADS	
		NOTE:	IF NPLT = 0, SKIP THIS CARD GROUP	
6	8I10	1-10	First junction number for plotting	JPLT(1)
		11-20	Second junction number, up to number of nodes defined by NPLT	JPLT(2)
			PLOTTED FLOWS	
		NOTE:	IF LPLT = 0, SKIP THIS CARD GROUP	
7	8110	1-10	First conduit number for plotting	KPLT(1)

TABLE 1. EXTRAN DATA REQUIREMENTS
(Continued)

Card Group	Format	Card Columns	Description	Variable Name
7 (Continued)		11-20	Second conduit number for plotting, up to number of nodes defined by LPLT (This option is for conduit flow rate)	KPLT(2)
			CONDUIT CARDS(1 CARD/CONDUIT, 187 MAX.)	
8	4I5, 9F5.0	1-5	Conduit number (none greater than 90000)	NCOND(N)
		6-10	Junction number at upstream end of conduit	NJUNC(N,1)
		11-15	Junction number at downstream end of conduit	NJUNC(N,2)
		16-20	Type of conduit shape 1 = circular 2 = rectangular 3 = horseshoe 4 = egg 5 = baskethandle 6 = trapezoid	NKLASS(N)
		21-25	Cross sectional area of conduit, sq. ft. (necessary only for types 3, 4, and 5)	AFULL(N)
		26-30	Vertical depth of conduit, ft.	DEEP(N)
		31-35	Maximum width of conduit, ft. Bottom width for trapezoid, ft.	WIDE(N)
		36-40	Length of conduit, ft.	LEN(N)
		41-45	Distance of conduit invert above junction invert at NJUNC(N,1)	ZP(N,1)
		46-50	Distance of conduit invert above junction invert at NJUNC(N,2)	ZP(N,2)
		51-55	Mannings coefficient (includes entrance and exit losses)	ROUGH(N)
		56-60	Slope of one side of trapezoid, (horizontal/vertical; 0=vertical)	STHETA(N)

TABLE 1. EXTRAN DATA REQUIREMENTS
(Continued)

Card Group	Format	Card Columns	Description	Variable Name
8 (Continued)		61-65	Slope on other side of trapezoid, (horizontal/vertical; 0=vertical)	SPHI(N)
			(Last card must have 99999 in columns 1 to 5)	
			JUNCTION CARDS (1 CARD/JUNCTION, 187 MAX.)	
9	I5, 3F5.0	1-5	Junction number (none greater than 90000)	JUN(J)
		6-10	Ground elevation, ft.	GRELEV(J)
		11-15	Invert elevation, ft.	Z(J)
		16-20	Net constant flow into junction, cfs (may be a negative number)	QINST(J)
			STORAGE JUNCTIONS (1 CARD/JUNCTION, 20 MAX.)	
.		NOTE:	JUNCTION MUST BE IDENTIFIED IN JUNCTION DATA	
10	I5, 2F5.0	1-5	Junction containing storage facility	JSTORE(I)
		6-10	Junction crown elevation (must be higher than crown of highest pipe connected to storage facility)	ZCROWN(J)
		11-15	Storage volume per foot of depth (surface area), cu. ft/ft.	ASTORE(I)
			(Last card must have a 99999 in columns 1 to 5)	
			ORIFICE CARDS (1 CARD/ORIFICE, 60 MAX.)	
11	3I5, 3F5.0	1-5	Junction containing orifice	NJUNC(N,1)
		6-10	Junction to which orifice discharges	NJUNC(N,2)
		11-15		

TABLE 1. EXTRAN DATA REQUIREMENTS
(Continued)

Card Group	Format	Card Columns	Description	Variable Name
11 (Continued)		16-20	Orifice area in sq. ft.	AORIF(I)
		21-25	Orifice discharge coefficient	CORIF(I)
		26-30	Distance of orifice invert above junction floor (define only for side outlet orifices)	ZP(I)
			(Last card must have 99999 in columns 1 to 5)	
			WEIR CARDS (1 CARD/WEIR, 60 MAX.)	
12	3I5, 4F5.0	1-5	Junction at which weir is located	NJUNC(N,1)
		6-10	Junction to which weir discharges NOTE: To designate outfall weir, set NJUNC(N,2) equal to zero	NJUNC(N,2)
		11-15	Type of weir 1 = transverse 2 = transverse with tide gates 3 = side flow 4 = side flow with tide gates	KWEIR(I)
		16-20	Height of weir crest above invert,	YCREST(I)
		21-25	Height to top of weir opening above	YTOP(I)
		26-30	Weir length,	WLEN(I)
		31-35	Coefficient of discharge for weir	COEF(I)
			(Last card must have 99999 in columns 1 to 5)	
			PUMP CARDS (1 CARD/PUMP, 20 MAX.) NOTE: ONLY ONE PIPE CAN BE CONNECTED TO A PUMP NODE	
13	3I5, 7F5.0	1-5	Junction being pumped	NJUNC(N,1)
		6-10	Pump discharge goes to this junction	NJUNC(N,2)

TABLE 1. EXTRAN DATA REQUIREMENTS
(Continued)

Group	Card	Card Columns	Description	Variable Name
13 (Continued)		11-15	Type of pump 1 = off-line pump with wet well 2 = on-line lift pump	IPTYP(I)
		16-20	Initial wet well volume, (enter 0 for type 2 pump)	VWELL(I)
		21-25	Lower pumping rate, cfs.	PRATE(I,1)
		26-30	Mid-pumping rate, cfs.	PRATE(I,2)
		31-35	High pumping rate, cfs.	PRATE(I,3)
		36-40	Wet well volume (or junction depth) for mid rate pumps to start, cu. ft. (or ft.)	VRATE(I,1)
		41-45	Wet well volume (or junction depth) for high rate pumps to start, cu. ft.(or ft.)	VRATE(I,2)
		46-50	Total wet well capacity, cu. ft. (enter 0 for type 2 pump)	VRATE(I,3)
			(Last card must have 99999 in columns 1 to 5)	
			OUTFALL PIPES W/O TIDE GATES (1 CARD OUTFALL, 25 MAX.) NOTE: ONLY ONE CONNECTING CONDUIT IS PERMITTED TO A FREE OUTFALL NODE	
14	I5	1-5	Junction for free outfall	JFREE(I)
			OUTFALL PIPES WITH TIDE GATES (1 CARD OUTFALL, 25 MAX.) NOTE: ONLY ONE CONNECTING CONDUIT IS PERMITTED TO OUTFALL NODE	
15	I5	1-5	Junction at which gate is located	JGATE(1)
			(Last card must have 99999 in columns 1 to 5)	

TABLE 1. EXTRAN DATA REQUIREMENTS
(Continued)

Card Group	Format	Card Columns	Description	Variable Name
			1 CARD FOR TIDAL CONTROL	
16	I5, 8F5.0	1-5	Tide index: 1 = no water surface at outfalls 2 = outfall control water surface at constant elevation, A1 3 = tide coefficients provided 4 = program will compute tide coefficients	NTIDE
		6-10	First tide coefficient	A1
			NOTE: COLUMNS 11-45 NOT REQUIRED UNLESS NTIDE = 3	
		11-15	Second tide coefficient	A2
		16-20	Third tide coefficient	A3
		21-25	Fourth tide coefficient	A4
		26-30	Fifth tide coefficient	A5
		31-35	Sixth tide coefficient	A6
		36-40	Seventh tide coefficient	A7
		41-45	Tidal period in hours	
			REQUIRED IF NTIDE = 4	
17	3I5	1-5	If one, there are four information points, program will develop the coefficients	KO
		6-10	Number of information points (4 if KO above equals 1)	NI
		11-15	If one, will print information on tide coefficient development	NCHTID

TABLE 1. EXTRAN DATA REQUIREMENTS
(Continued)

Card Group	Format	Card Columns	Description	V. Name
			REQUIRED IF NTIDE = 4	
18	8F10.0	1-10	Time, first information points	TT(1)
		-11-20	Tidal stage, at time above	YY(1)
		21-30	Time, second information points	TT(2)
		31-40	Tidal stage, at time above, up to number of points as defined by NI	YY(2)
			INITIAL FLOWS	
19	8F10.0	1-10	Initialflows (cfs)	Q(1)
		11-20	Initialvelocities (fps)	V(1)
	'	21-30	**NOTE: IF ALL INITIAL FLOWS, VELOC-ITIES AND HEADS ARE ZERO, PUNCH**	**Q(2)**
		31-40	**99999. IN COLS. 1-10 OF FIRST CARD FOR Q(1). NO OTHER CARDS REQUIRED**	**V(2)**
		41-50		
			(4 conduits per card, up to NTL conduits. Includes internallinks.)	
			INITIAL DEPTHS	
20	8F10.0	1-10	Initial junction depth (ft.)	Y(1)
		11-20 -	NOTE: SKIP IF A 99999. HAS BEEN PUNCHED FOR Q(1) ABOVE.	Y(2)
			(8 junctions per card up to NJ junc-tions. Includes internal junctions.)	Y(NJ)

TABLE 1. EXTRAN DATA REQUIREMENTS
(Continued)

Card Group	Format	Card Columns	Description	Variable Name
			CARD HYDROGRAPHS	
			IF NJSW = 0, SKIP CARD GROUPS 21 and 22	
21	16I5	1-5	First input node for card hydrograph	JSW(1)
		6-10	Second input node for card hydrograph	JSW(2)
		.		.
		.		.
		.		.
			REQUIRED IF NJSW > 1	
22	8F10.0	1-1(	Clock time, in decimal hours	TEO
		11-20	Flow rate, cfs., first input node, JSW(1)	QCARD(1,1
		21-30	Flow rate, cfs., second input node, JSW(2), up to NJSW nodes	QCARD(2,1:
			(If more than one card is needed for . each time, the next QCARD(N,1) should begin in Columns 1-10 of each subsequent card, 8 QCARD(N,1) per card up to QCARD(NJSW,1). Repeat with the same number of cards for each TEO with the final TEO greater than the end time of the run.)	

CHAPTER 3

EXAMPLE PROBLEMS

INTRODUCTION

Seven test runs of EXTRAN have been made and are included in this report. They will demonstrate how to set up the input data sets for each of the flow diversions included in the model. The complete or partial results of these runs have also been included as an example of typical output and an aid in interpreting EXTRAN results.

EXAMPLE 1: BASIC PIPE SYSTEM

Figure 8 shows a typical system of conduits and channels conveying the stormwater flow. In this system, which is used in all example problems while below, conduits are designated with four-digit numbers junctions have been given five-digit numbers. There are three inflow hydrographs, which are read from cards, and one free outfall. Table 2 is the input data set for Example 1.

The complete output for Example 1 is found in Table 3. The first section is an echo of the input data and a listing of conduits created internally by EXTRAN to represent outfalls and diversion caused by weirs, orifices, and pumps.

The next section of the output is the intermediate printout. This lists system inflows as they are read by EXTRAN and gives the depth at each junction and flow in each conduit in the system at a user-input time interval. A junction in surcharge is indicated by printing an asterisk beside its depth. Also, if surcharge iterations are occurring at the time of the intermediate printout, EXTRAN prints the flow differential over all surcharged junctions and the number of iterations required. An asterisk beside a conduit flow indicates that the flow is the normal flow for the conduit. Ihe intermediate print out ends with the printing of a continuity balance of the water passing through the system during the simulation. Printed outflows from junctions not designated as outfalls in the input data set are junctions which have flooded.

The final section of the output gives the time history of depths and flows for those junctions and conduits input by the user, as well as a summary for all junctions and conduits in the system. The output ends with the user-requested plots of junction heads and conduit flows.

EXAMPLE 2: TIDE GATE

Figure 9 shows the system simulated in Example 2, which is the basic pipe system with a tide gate at the outfall and a constant receiving water depth of 94.4 feet Two changes to the input data set, shown in Table 4, are required for this situation. These, shown in Table 4, are:

1. Placing the outfall junction number (10208) in Card Group 14; and
2. Changing NTIDE in Card Group 15 to 2 and inputing A1 = 94.4.

The summary statistics for this run are in Table 5.

EXAMPLE 3: SUMP ORIFICE DIVERSION

Example 3 uses a two foot diameter sump orifice to divert flow to junction 15009 in order to relieve the flooding upstream of junction 82309. A free outfall is also used in this example. Table 6 indicates that the sump orifice is inserted simply by changing Card Group 10 as shown. A summary of the results from this example is found in Table 7.

EXAMPLE 4: WEIR DIVERSION

A weir can also be used as a diversion structure to relieve the flooding upstream of junction 82309, a shown in Figure 11. Card Group 11 has been revised as shown in Table 8 in order to input the specifications for this weir. Summary results are shown in Table 9.

EXAMPLE 5: STORAGE FACILITY WITH SIDE OUTLET ORIFICE

Inclusion of a storage facility requires several changes to the basic pipe system. Figure 12 shows that a new junction, 82308, has been inserted to receive the outflow from the orifice in the storage facility. Table 10 shows that this requires a new junction in Card Group 8, the invert of which is set to that of conduit 1602. This change, however, also requires that the invert of junction 82309 be raised to that of conduit 8060. Table 2 shows that, for the basic pipe system, conduit 8060 is 2.2 feet (ZP(N,2) above the invert of junction 82309. Thus, the invert of 82308 is set at 112.3 feet (the origin'al elevation of 82309), the invert of 82309 is 114.5 feet, and ZP(N,2) for 8060 is 0.0. Card Group 9 is revised to show the size of the storage facility, and Card Group 10 is changed to show the specifications of the two foot diameter size orifice. Table 11 gives the results of this example.

EXAMPLE 6: OFF-LINE PUMP STATION

Inclusion of an off-line pump station requires the addition of a junction to represent the wet-well and a conduit to divert the flow to it, as Figure 13 demonstrates. Examination of Card Groups 7 and 8 in Table 12 shows the specifications for junction 82310 and conduit 8061. The length and Manning's n of conduit 8061 shown here, though, have been altered for stability purposes to those of an equivalent pipe to the actual 8061, which is 20 feet long with an n of .015. Chapter 2 gives the details of the equivalent *pipe* transformation. Also, Card Group 12 now includes a card giving the pump specifications. Results from this example are found in Table 13.

EXAMPLE 7: IN-LINE PUMP STATION

The pump in Example 6 can be moved to junction 82309 to simulate an • inline pump station. Figure 14 shows that this requires no alteration to the basic pipe system of Example 1. The only change to the input data set, shown in Table 14, is the pump card in Group 12. It should be noted, though, that the VWELL variables are now water elevations at junction 82309 rather than the volume of a wet-well. Results are found in Table 15.

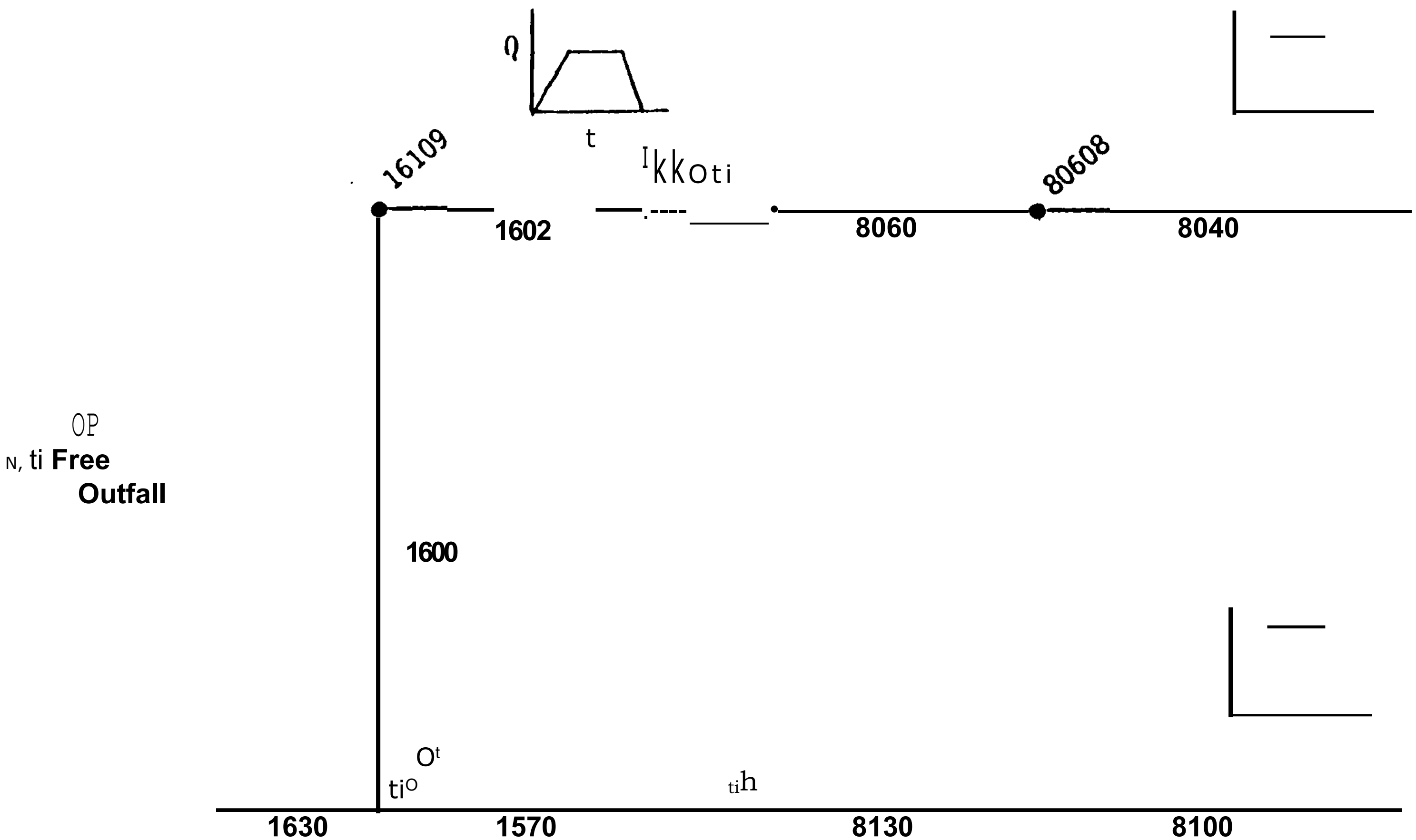

Figure 8. Basic system with free outfall.

TABLE 2

INPUT DATA. SET FOR EXAMPLE 1

```
0              0
   EXTRAN USER'S MANUAL    EXAMPLE PROBLEM 1
   BASIC PIPE I]YSTEM FROM FIGURE 8
1440        20.    .0     6     6     6     6    45    45     3    30  0.05
                      80608      16109      15009      82309       90.108
                       1030       1600       1602       1570        8130
                      80608      16109      15009      82309       80408
                       1030       1600       1602       1570        8130
             80408040880608-      4.00       1800.             .015
              80608060882309      4.00       2075.       2.2   .015
              81008100981309      4.50       5100.             .015
              81308130915009      4.50       3500.             .015
10301030910208            6       9.0        4500.             .016   3.    3
15701500916009            1       5.5        5000.            .0154
             1600160091,5109      6.0         300.             .015
1.6301600910309           6       9.0         300.             .015   7.    Z
              16028230916109      JI         5000,             .034
   99999
   80408138.0124.6 0.0
   80608135.0118.3 0.0
   31009137.0128.2 0.0
   81309130.0117.3 0.0
   82309155.0112.3 0.0
   10208100.0 39.9 0.0
   10309111.0101.6 0.0
   13009125.0111.3 0.0
   16009120.0102.0 0.0
   16109125.0102.8 0.0
   99999
   99999
   99999
   99999
   99999
   10208
   99999
   99999
       1
     999
   F323098040881009
   0.0          0.0          0.0          0.0
   0.25         40.0         45.0         50.0
   3.0          40.0         45.0         50.0
   3.25         0.0          0.0          0.0
   12.0         0.0          0.0          0.0
```

TABLE 3

OUTPUT FROM EXAMPLE 1

ENVRONNENTAL PROTECTION AGENCY WASIIIMITON, D.C.

tat EXTENDED TRANSPORT PROGRAM au
its: • its*
rtil - ANALYSIS ?MULE UBE*

WATTI) RESIJmCES DIVISION
CAMP DRESSER I KCIME INC.
NOVA ALE. VIRGINIA

EtTRAN RISER'S MURK DIME PROUD 1
BASIC PIPE SYSTV TIM FUME 8

INTEGRATION CYCLES 1440

LA VIN OF INTEGRATION STET IS 20. SECONDS

PRINTING STARTS IN CrIE 45 AND PRINTS AT INTERVALS OF 45 CYCLES

INITIAL TIME 0.00 HOURS

PRINTED OUTPUT AT TIC Fau.oune 6 JUNCTION:

80608 16009 16109 15009 82309 80408

AND FOR TIE FOLLOWING 6 CONDUITS

1030 1630 1600 1602 1570 8130

WATER SURFACE ELEVATIONS WILL BE PLOTTED FOR THE FOLLOWING 6 JUNCTIONS

80608 16009 16109 15009 82309, 80408

FLOV RATE WILL BE PLOTTED FOR TIE FOLLOWING 6 CONDUITS

1030 1630 1600 1602 1570 8130

ENVIRONMENTAL PROTECTION AGENCY   till WOOED TRANSPORT PROGRAM =ES   WATER RESOURCES
W(SNINOTON ▸ O.C.   Dv/Islam
CAN DRESSER f MCKEE INC.
0(TRAN USER'S MANUAL EXAMPLE   IUD   ANALYSIS MODULE   MOM   ANNAOMALEP
PROBLEM / BASIC PIPE SYSTEM FILM FINK 8   VIRGINIA

	CONDUIT	LOOM	CLASS .	AREA	NANNING	MAX WIDTH	DEPTH	JUNCTIONS		INVERT HEIGHT			
					TRAPEZOID								
1	8040	1800.	1	12.57	0.015	4.00	4.00	80408	80608				
2	8060	2075.	1	12.57	0.015	4.00	4.00	80608	82309	0.00	2.20		
3	8100	5100.	1	15,90	0.015	4.50	4.50	81009	81309				
4	8130	3500.	I	15.90	0.015	4.50	4.30	81309	15009				
5	1030	4500.	6	243.00	0.016	0.01	9.00	10309	10208			3.00	3.00
6	1570	5000.	1	23.76	0.015	5.50	5.50	15009	16009				
7	1600	500,	1	28.27	0.015	6.00	6.00	16009	16109				
8	1630	300.	6	243.00	0.015	0.01	9.00	16009	10309			3.00	3.00
9	1602	5000.	1	19.63	0.034	5.00	5.00	82309	16109				

an VARNDIG MD (CSOELT/LEN) IN CONDUIT 1630 IS 1.1 AT FILL DEPTH.

ENVIRONMENTAL PROTECTION AGENCY
MASHINGTON, D.C.

EXTRMI USER'S IV AN. EXAMPLE PRONOOI 1
BASIC PIPE SYSTEM FROM FIGURE 8

Int EXTENDED TR/AIRPORT PROGRAM talYAM RESOURCES DIVISION
9W flit CAMP DRESSER t MCXEE INC.
tsti ANALYSIS MODULE Kitt ANNANDALE, VIRGINIA

JUNCTION INNER	GROUND ELEV.	CRONE ELEV.	MOT ELEV.	GMT (CFS)	CONIECTIMI =NITS		
/	138.00	128.64	124.60	0.00	8040		
2	133.00	122.30	118.30	0.00	8040	8060	
3	137.00	132.70	128.20	0.00	8100		
81309	130.00	122.00	117.50	0.00	8100	8130	
S	155.00	118.50	112.30	0.00	8060	1602	
6	100.00	98.90	89.90	0.00	1030		
7 10309	111.00	110.60	101.60	0.00	1030	1630	
8	125.00	117.00	111.50	0.00	8130	1570	
9	120.00	111.00	102.00	0.00	1570	1600	1630
10	125.00	108.80	102.80	0.00	1600	1602	

FREE OUTFALL DATA

FREE OUTRUN AT JUNCTIONS10208

EXTRAN USER'S MANUAL EXAMPLE PREMED
1 BASIC PIPE SYSTEM CRON FIGURE 8

INTERIM. COMECTIVITY INFORMATION

CONDUIT	JUNCTION	JUNCTION
90010	10208	0

EXTRAN USER'S MANUAL ECANPLE PROM 1
BASIC PIPE SYSTEM FROM FIGURE

MIMI OF IRITIAL HEADS, FLOW AND VELOCITIES

INITIAL !CABS, FLOW MR VELOCITIES ORE ZERO

EXTRAN USER'S MANUAL MAPLE PROBLEM
1 BASIC PIPE SYSTEM FRO!FIORE 8

W SYSTEM INFLOW (CARDS) AT 0.00 HOURS FOR 3 JUNCTIONS

82309/ 0.00 804C4/ 0.00 81009/ 0.00

tt0tr8 SYSTEM INFLOWS (CARDS) AT 0.25 HOURS (JUNCTION / INFLOWICFS)

82309/ 40.00 80408/ 43.00 81009/ 50.00

....

BMW SYSTEM INFLOWS (CARDS) AT 3.00 HOURS (JUNCTION / INFLOW.CFS)

82309/ 40.00 80408/ 45.00 81009/ 50.00

CYCLE 45 TINE 0 1115 - 15.00 MIN

JUNCTIONS / DEPTHS

80408/	2.60	80608/	1.70	81009/	2.21	81109/	0.15	82309/	7.23	10208/	0.00	10309/	0.00	15009/	0.00
16009/	0.00	1.S109/	0.17												

CONDUITS / FLOW

8040/	42.76	8060/	15.52	8100/	13.81	8130/	0.25	1030/	0.00	1370/	0.00	1600/	0.09	1630/	0.00
1602/	6.33	90010/	0.00												

CYCLE 90 TIME 0 HRS - 10.00 AIM

JUNCTIONS / DEPTHS

80408/	2.26	80608/	3.82	81009/	3.32	81309/	2.12	82309/	9.571	10208/	0.00	10309/	0.08	15009/	0.37

16009/ 0.55 16109/ 2.05

CONDUITS / nous

8040/ 45.018 8060/ 8.34 8100/ 54.64 8130/ 13.33 1030/ 0.00 1570/ 1.08$ 1600/ 20.74 1630/ 0.51

1602/ 47.72 90010/ 0.00

CYCLE 133 TINE 0 IRS - 45.00 SIN

JUNCTIONS / DEPTHS

80408/ 12.751 80608/ 16.708 81009/ 2.72 81309/ 3.48 82309/ 21.658 10208/ 0.34 10309/ 2.16 15009/ 1.47

16009/ 2.68 16109/ 2.88

CONDUITS/FLOWS

8040/ 45.00 8060/ 28.02 8100/ 52.98$ 8130/ 44.19 1030/ 21.08 1570/ 18.932 1600/ 69.12 1630/ 88.59

1602/ 68.02 90010/ 21.08

CYCLE 180 TIME 1 NRS - 0.00 NIN

JUNCTIONS / DEPTHS

80408/ 12,75: 80608/ 16.7011 81009/ 2.63 81309/ 3.48 82309/ 21.658 10208/ 0.44 10309/ 3.41 15009/ 2.27

16009/ 3.10 16109/ 2.87

CONDUITS/FLOSS

8040/ 45.00 8060/ 27.98 8100/ 50.24$ 8130/ 54.64 1030/ 68.44 1570/ 43.73$ 1600/ 66.70 1630/ 98.88

1642/ 67.98 90010/ 68.44

....

CYCLE 223 TINE 1 HRS - 15.00 SIN

JUNCTIONS/DEPTHS

80408/ 12.75$ 80608/ 16.708 81009/ 2.62 81309/ 3.25 82309/ 21.548 10208/ 0.49 10309/ 3.90 15009/ 2.50

16009/ 3.55 16109/ 3,13

CONDUITS FLOWS

8040/ 45.00 8060/ 29.32 8100/ 30.02: 8130/ 53.80 1030/ 97.16 1370/ 52.172 1600/ 67.33 1630/113.23

1602/ 69.33 90010/ 97.16

CYCLE 270 TIME - 30.00 MIN

JUNCTIONS / CONS

80408/ 12.752 80608/16.70281009/ 2.62 81309/ 3.11 82309/21.422 10208/ 0.52 10309/ 4.11 15009/ 2.50

16009/ 3.76 16109/ 3.30

CONDUITS / FIOUS

8040/ 45.00 8060/ 30.14 8100/ 50.002 8130/ 51.72 1030/112.26 1570/ 52.442 1600/ 69.26 1630/119.11

1602/ 70.14 90010/ 112.26

CYCLE I15 TIME 1 HRS - 45.00 KIN

JUNCTIONS / DEPTHS

80408/12.752 80608/16.702 81009/ 2.62 81309/ 3.06 82209/21.46: 10208/ 0.53 10309/ 4.19 15009/ 2.47

16009/ 3.84 16109/ 3.37

CONDUITS / FLOWS

8040/ 45.00 8060/ 30.46 8100/ 30.002 8130/ 50.54 1030/118.23 1570/ 51.232 1600/ 70.16 1630/120.54

1602/ 70.46 90010/118.23

....

CYCLE 360 TIME 2 MRS - 0.00 MIN

JUNCTIONS / DEPTHS

80408/ 12.752 80608/ 16.702 81009/ 2.62 81309/ 3.05 82309/ 21.452 10208/ 0.53 10309/ 4.22 15009/ 2.45

16009/ 3.86 16109/ 3.39

CONDUITS / FLOWS

8040/ 45.00 8060/ 30.56 8100/ 50.008 8130/ 50.09 1030/ 120.08 1570/ 50.438 1600/ 70.48 1630/ 120.69

1602/ 70.56 90010/ 120.08

CYCLE 405 TINE 2 MS - 15.00 MIN

JUNCTIONS / DEPTHS

80408/12.758 90608/16.708 81009/ 2.62 81309/ 3.05 82309/21.452 10208/ 0.53 10309/ 4.22 15009/ 2.44

16009/ 3.87 16109/ 3.39

CONDUITS / FLOUS

8040/ 45.00 8060/ 30.58 8100/ 50.001 8130/ 49.97 1030/120.51 1570/ 50.098 1600/ 70.57 1630/120.62

1602/ 70.58 90010/120.51

CYCLE 450 TIM 2 MRS - 30.00 MIN

JUNCTIONS / *CONS*

80408/ 12.718 80608/16.701 81009/ 2.62 81309/ 3.05 82309/21.458 10208/ 0.53 10309/ 4.22 25009/ 2.44

16009/ 3.87 16109/ 3.39

CONDUITS / FLOM

8040/ 45.00 8060/ 30.59 8100/ 50.008 8130/ 49.97 1030/120.57 1570/ 49.998 1600/ 70.59 2630/120.58

1602/ 70.59 90010/120.57

....

VW 495 TIME 2 HRS - 45.00 MIN

JUNCTIONS / DEPTHS

80408/ 12.758 80608/ 16.708 81009/ 2.62 81309/ 3.05 82309/21.458 10208/ 0.53 10309/ 4.22 15009/ 2.44

16009/ 3.87 16109/ 3.39

CONDUITS / FLOWS

8040/ 45.00 8060/ 30.59 8100/ 50.001 9130/ 49.98 1030/120.57 1570/ '19.988 1600/ 70.59 1630/120.57

160E 70.59 90010/ 120.57

=SS MIN II3.0US (CARDS) AT 3.25 IC RS (JUNCTION / DIFLOWICP3)

82309/ 0.00 80408/ 0.00 81009/ 0.00

CYCLE 540 TINE 3 HIS - 0.00 MIN

JUNCTIONS / DEPTHS

80408/ 12.751 80608/16.701 81009/ 2.62 81309/ 3.05 82309/21.451 10201/ 0.53 10309/ 4.22 15009/ 2,44

16009/ 3.87 16109/ 3.39

CONDUITS / FLOUS

8040/ 45.00 8060/ 30.59 8100/ 50.001 8130/ 49.99 1030/120.57 1570/ 49.991 1600/ 70.59 1630/120.58

1602/ 70.59 90010/120.57

ttsnt SYSTIO4 IMAM (CARDS) AT 12.00 HOURS (JUNCTION / IN FLOVICFS)

82309/ 0.00 80408/ 0.00 01009/ 0.00

aC1E 585 TIME 3 IRS - 15.00 NDI

ARICTIONS / DEPTHS

B0408/ 0.89 80608/ 2.59 81009/ 1.56 81309/ 2.61 82309/ 5.8910208/ 0.52 10309/ 4.1115009/ 7.36

16009/ 3.75 16109/ 3.12

CONDUITS / FLOWS

8040/ 8.381 8060/ 41.03$ 8100/ 20.511 8130/ 41.93 1030/ 111.64 1570/ 47.451 1600/ 48.45 1630/ 99.70

1602/ 47.22 900t0/111.64

CYCLE 630 TINE 3 IRS 30.40 4/4.

JUNCT/ONS / DEPTHS

80408/ 0.21 80608/ 0.83 81009/ 0.71 81309/ 1.61 82309/ 3.80 10208/ 0.49 10309/ 3.87 13009/ 1.93

16009/ 3.50 16109/ 2.85

CONDUITS / FLOWS

8040/ 0.438 8060/ 5.118 8100/ 4.328 8130/ 19.608 1030/ 93420 1370/ 32.788 1600/ 40,76 1630/ 79.89

1602/ 33.03 90010/ 95.20

CYCLE 473 TINE 3 MRS 43.00 MIN

JUNCTIONS / DEPTHS

80408/ 0.10 80608/ 0.44 81009/ 0.43 81309/ 1.06 82309/ 2.32 10208/ 0.45 10309/ 3.49 15009/ 1.42

16009/ 3.12 16109/ 2.41

CONDUITS/FLOWS

8040/ 0•118 8060/ 0.88 8100/ 1.538 8130/ 8.621 1030/ 72.33 1570/ 18.171 1600/ 27.00 1630/ 53.68

1602/ 19.292 90010/ 72.13

CYCLE 720 TIME 4 ICS 0.00 MIN

JUNCTIONS / DEPTHS

80408/ 0.06 80608/ 0.33 81009/ 0,30 81309/ 0.75 82309/ 1.44 10208/ 0.41 10309/ 3.05 13009/ 1.07

16009/ 2.67 16109/ 1.93

CONDUITS/FLOWS

8040/ 0451 8060/ 0.45 8100/ 0.725 8130/ 4.334 1030/ 50.49 1570/ 10.21D 1600/ 14.68 1630/ 33.37

1602/ 7.905 90010/ 50.49

....

CYCLE 765 TIME 4 MRS - 15.00 MIN

JUNCTIONS / (GPM

80408/ 0.04 80600/ 0.25 81009/ 0.22 813091 0.37 82309/ 1.01 10208/ 0.38 10309/ 2.64 15009/ 0.83

16009/ 2.26 16109/ 1.31

CONWITS/FLOM

8040/ 0.022 8060/ 0.24 8100/ 0.38$ 8134/ 2.421 1030/ 34.73 1570/ 6.102 1600/ 9.14 1630/ 22.09

1602/ 3.902 90010/ 34.73

CYCLE810 TIME 4 MIS - 34.00 MIN

JUNCTIONS / CEPTIIS

80408/ 0.03 80608/ 0.21 81009/ 0.17 81309/ 0.45 82309/ 0.76 10208/ 0.36 10309/ 2.30 15009/ 0.66

16009/ 1.92 16109/ 1.17

CONDUITS/FLOM

8040/ 0.012 8060/ 0.14 8100/ 0.222 8130/ 1.472 1030/ 24.37 1570/ 3.842 1600/ 6.09 1630/ 15.25

1602/ 2.192 90010/ 24.37

....

CYCLE 855 TINE 4 1NIS - 45.00 MIN

ACTIONS/DEPTHS

80408/ 0.02 80604/ 0.18 81009/ 0.14 81309/ 0.36 112309/ 0.60 10208/ 0.27 10309/ 2.02 13009/ 0.55

16009/ 1.65 16109/ 0.89

CONDUITS/M088

8040/ 0.012 8060/ 0.10 8100/ 0.162 8130/ 0.94* 1030/ 16.76 1570/ 2.582 1600/ 4.12 1630/ 10.60

1602/ 1.332 90010/ 16.76

....

CYCLE 900 TINE 5 MRS - 0.00
SIN

ACTIONS/WINS

80408/ 0.01 80608/ 0.1681009/ 0.12 81309/ 0.30 82309/ 0.4910208/ 0.19 10309/ 1.81 15009/ 0.46

16009/ 1.44 16109/ 0.68

CONDUITS / FLAN(

8040/ 0.011 8060/ 0.07 8100/ 0.111 8130/ 0.661 1030/ 11.96 1570/ 1.771 1600/ 2.85 1630/ 7.37

1602/ 0.871 90010/ 11.96

CYCLE 945 TIME 5 - 15.00 MIN

JUNCTIONS / DEPTHS

80408/ 0.01 80608/ 0.1481009/ 0.10 81309/ 0.26 82309/ 0.41 10208/ 0.14 10309/ 1.63 15009/ 0.39

16009/1.26 16109/ 0.51

CONDUITS / FLOWS

8040/ 0.001 8060/ 0.06 8100/ 0.091 8130/ 0.471 1030/ 8.87 1570/ 1.251 1600/ 2.02 1630/ 3.61

1602/ 0.59$ 90010/ 8.87

CYCLE 990 TINE 5 IRS - 30.00 MIN

JUNCTIONS / DEPTHS

80408/0.01 80608/ 0.1281009/ 0.08 81309/ 0.22 82309/ 0.35 10208/ 0.11 10309/ 1.48 15009/ 0.34

16009/1.11 16109/ 0.38

CONDUITS / FLOWS

8040/ 0.008 8040/ 0.05 8100/ 0.071 8130/ 0.348 1030/ 6.73 1570/ 0.971 1600/ 1.221 1630/ 4.12

1602/ 0.438 90010/ 6.73

....

CYCLE 1035 TIME 5 HRS - 45.00 MIN

JUNCTIONS / DEPTHS

80408/ 0.01 80608/ 0.11 81009/ 0.07 81309/ 0.20 82309/ 0.30 10208/ 0.08 10309/ 1.35 15009/ 0.30

16009/ 0.98 16109/ 0.30

CONDUITS / FLOWS

8040/ 0.008 8060/ 0.04 8100/ 0.058 8130/ 0.261 1030/ 5.16 1570/ 0.731 1600/ 0.761 1630/ 3.08

1602/ 0.32S 90010/ 5.16

CYCLE 1080 TIME 6 HS - 0.00 MIN

JUNCTIONS / DEPTHS

80408/ 0.01 80608/ 0.10 81009/ 0.06 81309/ 0.18 82309/ 0.26 10208/ 0.06 10309/ 1.24 15009/ 0.26

16009/ 0.87 16109/ 0.26

CONDUITS / FLOWS

8040/ 0.001 8060/ 0.03 8100/ 0.04S 8130/ 0.20$ 1030/ 4.01 1570/ 0.56$ 1600/ 0.311 1630/ 7.37

1602/ 0.23 90010/ 4.01

CYCLE 1125 TIME 6 IRS - 15.00 MIN

JUNCTIONS / DEPTHS

80408/ 0.00 80608/ 0.09 81009/ 0.05 81309/ 0.16 82309/ 0.23 10208/ 0.05 10309/ 1.14 15009/ 0.24

16009/ 0.77 16109/ 0.22

CONDUITS / FLOVS

8040/ 0.00$ 8060/ 0.03 8100/ 0.03S 8130/ 0.17* 1030/ 3.17 1570/ 0.44S 1600/ 0.382 1630/ 1.86

1602/ 0.17 90010/ 3.17

....

CYCLE 1170 TIME 6 HRS - 30.00 MIN

MMM / DEPTHS

W)408/ 0.00 80608/ 0.08 81009/ 0.05 81309/ 0.14 82309/ 0.21 10208/ 0.04 10309/ 1.05 15009/ 0.21
16009/ 0.68 16109/ 0.19

CONDUITS / FUN

9040/ 0.008 8060/ 0.03 8100/ 0.02: 8130/ 0.15: 1030/ 2.54 1570/ 0.33: 1600/ 0.308 1630/ 1.50
1602/ 0.13 90010/ 2.54 .

CYCLE 1213 TIME 4 HAS - 45.00 MIN

JUNCTIONS/DEPTHS

80408/ 0.00 80608/ 0.08 81009/ 0.04 81309/ 0.13 82309/ 0.19 10208/ 0.03 10309/ 0.98 13009/ 0.20
16009/ 0.61 16109/ 0.17

CONDUITS / FUN

8040/ 0.001 8060/ 0.02 8100/ 0.02: 8130/ 0.13: 1030/ 2.07 1570/ 0.31: 1600/ 0.201 1630/ 1.74
1602/ 0.11 90010/ 2.07

CYCLE 1260 7 INS - 0.00 MIN
TIME

AUCTIONS / DEPTHS

80408/ 0.00 •80608/ 0.07 81009/ 0.04 81309/ 0.12 82309/ 0.18 10208/ 0.03 10309/ 0.91 15009/ 0.18
16009/ • 0.55 16109/ 0.15

CONDIITS / FLOWS

8040/ 0.00t 8060/ 0.02 8100/ 0.021 8130/ 0.11: 1030/ 1.72 1570/ 0.27: 1600/ 0.20: 1630/ 1.03
1602/ 0.09 90010/ 1.72

CYCLE 1303 TINE 7 INS - 15.00 MIN

JUNCTION / WPM

80408/ 0.00 80608/ 0.07 81009/ 0.03 81309/ 0.11 82309/ 0.17 10208/ 0.02 10309/ 0.85 15009/ 0.17

16009/ 0.50 16109/ 0.13

CTAMUITS / F1088

8040/ 0.00$ 8060/ 0.02 8100/ 0.022 8130/ 0.09$ 1030/ 1.44. 1570/ 0.232 1600/ 0.172 1630/ 0.87

1602/• 0.08 90010/ 1.44

CYCLE 1330 TN 7 I S - 30.00 MIN

JUNCTIONS / DEPTHS

80408/ 0.00 SOW/ 0.06 81009/ 0.03 81309/ 0.10 82309/ 0.16 10208/ 0.02 10309/ 0.80 15009/ 0.13

16009/ 0.45 16109/ 0.12

CONDUITS / FLOYS

SOW 0.002 8060/ 0.02 8100/ 0.018 8130/ 0.08$ 1030/ 1.21 1370/ 0.20$ 1600/ 0.141 1630/ 0.74

1602/ 0.07 90010/ 1.21

CYCLE 1395 TIME 7 HIS - 45.00 MIN

JLMCTIONS / DEPTHS

80408/ 0.00 80608/ 0.06 81009/ 0.03 81309/ 0.09 82309/ 0.15 10208/ 0.02 10309/ 0.76 15009/ 0.14

16009/ 0.41 16109/ 0.11

CONDUITS/FLOSS

8040/ 0.002 8060/ 0.01 8100/ 0.012 8130/ 0.072 1030/ 1.03 1570/ 0.182 1600/ 0.122 1630/ 0.61:

1602/ 0.06 90010/ 1.0I

....

CYCLE 1440 TIME 8 MRS - 0.00 MIN

ACTIONS DEPTHS

80408/ 0.00 80608/ 0.05 81009/ 0.02 81I09/ 0.09 82309/ 0.14 10208/ 0.01 10309/ 0.71 15009/ 0.13

16009/ 0.38 16109/ 0.10

CONDUITS / FLOWS

8040/ 0.002 8060/ 0.01 8100/ 0.01$ 8130/ 0.072 1030/ 0.88 1570/ 0.162 1600/ 0.112 1630/ 0.49*

1602/ 0.06 90010/ 0.88

.... CONTINUITY BALANCE AT END WI RUN..

TOTAL WAIN INFLOW VOLUME • 1458000. CU FT

JUNCTION OUTFLOWS AND
STREET FLOODIN8

JUICTION OUTFLOW, FT3

80408	S85.
80608	136037.
10208	1328008.

TOTAL 1404631. CU FT

VOLUME LEFT IN smog • 7902. CU FT

ERROR IN CONTINUITY. PERCENT • -1.00

ENVIRONMENTAL PROTECTICI1 ABENCT HAMM% D.C. | 8882 EXTENDED TRANSPORT PROGRAM USN DIVISION | VATFR RESOURCES
2828 8888 | CM, EIRIESSERtICXEE INC.
ETTRAN USER'S NIA COME PROM 1 | 1828 ANALYSIS NODULE | •NNOODALE ▸ VIRGINIA
BASIC PIPE MIEN FRON FIEUE

IttS8StiSSISSASSSSAS TINE HISTORY OF H. 8. L. BISSSISSttStittitti
(VALUES IN FEET)

TINE	JUICTION80608 ORM 135.00		JUICTION16009 GRAD 120.00		ARCTION16109 GRHD 121.00		JUNCTION15009 MB 125.00		JUNCTION82309 MIND155.00		JUNCTION00408 ORNO 138.00	
ITIt.HD!	ELEV	DEPTH	ELEV	DEPTH	ELEV	DEPTH	ELEV	DEPTH	ElEV	DEPTH	ELEV	DEPT
0.15	120.00	1.70	102.00	0.00	102.97	0.17	111.50	0.00	114.53	2.23	127.20	2.60
0.30	122.12	3.82	102.55	0.55	104.85	2.05	111.87	0.37	121.87	6.20	126.86	2.26
0.45	135.00	4.00	104.68	2.68	105.68	2.88	112.97	1.47	133.75	6.20	117.31	4.00
1. 0	135.00	4.00	105.10	3.10	105.67	2.87	113.77	2.27	133.95	6.20	137.35	4.00
1.15	133.00	4.00	105.55	3.55	105.93	3.13	114.00	2.54	133.84	6.20	137.35	4.00
1.30	135.00	4.00	101.76	3.76	106.10	3.30	114.00	2.50	133.71	6.20	137.35	4.00
1.45	135.00	4.00	101.84	3.84	106.17	3.37	113.97	2.47	133.76	6.20	117.33	4.00
2. 0	135.00	4.00	105.86	3.86	106.19	3.39	113.95	2.45	133.75	6.20	137.35	4.00
2.15	135.00	4.00	103.87	3.87	106.19	3.39	113.94	2.44	133.75	6.20	137.35	4.00
2.30	135.00	4.00	103.87	3.87	106.19	3.39	113.94	2.44	133.75	6.20	137.35	4.00
2.43	133.00	4.00	103.87	3.87	106.19	3.39	113.94	2.44	133.75	6.20	137.35	4.00
3. 0	135.00	4.00	105.87	3.87	106.19	3.39	113.94	2.44	133.73	6.20	137.35	4.00
3.15	120.89	2.59	105.75	3.75	103.92	3.12	113.36	2.36	118.19	5.89	125.49	0.89
3.30	119.13	0.83	105.50	3.50	105.65	2.85	113.43	1.93	116.10	3.80	124.81	0.21
3.45	118.74	0.44	105.12	3.12	105.21	2.41	112.92	1.42	114.62	2.32	124.70	0.10
4. 0	118.63	0.33	104.67	2.67	104.73	1.93	112.37	1.07	113.74	1.44	124.66	0.06
4.15	118.33	0.25	104.26	2.26	104.31	1.51	112.33	0.33	113.31	1.01	124.64	0.04
4.30	118.51	0.21	103.92	1.92	103.97	1.17	112.16	0.66	113.06	0.76	124.63	0.03
4.45	118.48	0.18	103.65	1.63	103.69	0.89	112.05	0.55	112.90	0.60	124.62	0.02
5. 0	118.46	0.16	103.44	1.44	103.48	0.68	111.96	0.46	112.79	0.49	124.61	0.01
5.15	118.44	0.14	103.26	1.26	103.31	0.51	111.89	0.39	112.71	0.41	124.61	0.01
5.30	118.42	0.12	103.11	1.11	103.18	0.38	111.84	0.34	112.65	0.35	124.61	0.01
5.45	118.41	0.11	102.98	0.98	103.10	0.30	111.80	0.30	112.60	0.30	124.61	0.01
6. 0	118.40	0.10	102.87	0.87	103.06	0.26	111.76	0.26	112.56	0.26	124.61	0.01
6.15	118.39	0.09	102.77	0.77	103.02	0.22	111.74	0.24	112.13	0.23	124.60	0.00
6.30	118.38	0.08	102.68	0.68	102.99	0.19	111.71	0.21	112.51	0.21	124.60	0.00
6.45	118.38	0.08	102.61	0.61	102.97	0.17	111.70	0.20	112.49	0.19	124.60	0.00
7. 0	118.37	0.07	102.55	0.55	102.95	0.15	111.68	0.18	112.48	0.18	124.60	0.00
7.15	118.37	0.07	102.50	0.50	102.93	0.13	111.67	0.17	112.47	0.17	124.60	0.00
7.30	118.36	0.06	102.45	0.45	102.92	0.12	111.65	0.18	112.46	0.16	124.60	0.00
7.43	118.36	0.06	102.41	0.41	102.91	0.11	111.64	0.14	112.45	0.15	124.60	0.00
8. 0	118.31	0.05	102.38	0.38	102.90	0.10	111.63	0.13	112.44	0.14	124.60	0.00

SUNNART STATISTICS FOR JUNCTIONS

AICTIIII NUM	ROWS ELEVATION (FT)	UPPERMOST PIPE CRONN ELEVATION on	mAxDLI COWS DEPTH an	TIM OF OCCURBCE NN.	MIN.	FEET OF MINIM AT MAX. DEPTH	FEET MAX. WNIS SEMIGROWS ELEVATION	LENGTH OF SURCHARGE CHIN)
80408	138.00	128.60	13.40	0	32	9.40	0.00	153.0
80608	135.00	122.30	16.70	0	30	12.70	0.00	159.3
81009	137.00	132.70	3.36	0	27	0.00	5.44	0.0
81309	130.00	122.00	3.56	0	31	0.00	8.94	0.0
82309	155.00	118.50	21.68	0	33	15.48	21.02	163.3
10208	100.00	98.90	0.53	3	1	0.00	9.57	0.0
10309	111.00	110.60	4.22	3	1	0.00	3.18	0.0
15009	125.00	117.00	2.51	1	22	0.00	10.99	0.0
16009	120.00	111.00	3.87	3	0	0.00	14.13	0.0
16109	125.00	108.80	3.39	3	0	0.00	18.81•	0.0

tSttlfitillitiltTIME HISTORY OF FLOP AND VELOCITY Iiitsittttittii
O(CFS), VEL(FPS)

TINE 1030	CONDUIT		CONDUIT 1630 FIZA	VEL	CONDUIT 1600 FLOW		CONDUIT 1602 FLON	VEL	CONDUIT 1570 FLOW		CONDUIT 8130 FLOW	
0.13	0.00	0.2	0.00	0.2	0.09	0.6	6.53	1.6	0.00	0.3	0.25	1.0
0.30	0.00	0.3	0.51	1.3	20.74	4.3	47.72	2.9	1.08	1.1	13.33	3.5
0.45	21.08	3.5	88.59	5.0	69.12	5.4	68.02	3.9	18.93	2.2	44.19	5.0
1. 0	68.44	4.7	98.88	3.1	66.70	4.8	67.98	3.9	43.73	3.8	54.64	5.1
1.15	97.16	5.2	113.23	2.7	67.35	4.2	69.33	3.9	52.17	1.9	53.80	5.0
1.30	112.26	5.4	119.11	2.6	69.26	4.0	70.14	3.9	52.40	3.8	51.72	5.0
1.45	118.23	5.4	120.54	2.3	70.16	4.0	70.46	3.9	31.23	3.6	50.54	4.9
2. 0	120.08	5.4	120.69	2.5	70.48	3.9	70.36	3.9	50.43	3.6	50.09	4.9
2.15	120.31	5.5	120.62	2.5	70.57	3.9	70.38	3.9	50.09	3.6	49.97	4.9
2.30	120.57	5.5	120.38	2.3	70.59	3.9	70.59	3.9	49.99	3.6	49.97	4.9
2.43	120.57	5.5	120.57	2.4	70.59	3.9	70.39	3.9	49.98	3.6	49.98	4.9
3. 0	120.57	5.5	120.58	2.4	70.59	3.9	70.59	3.9	49.99	3.6	49.99	4.9
3.15	111.64	5.3	99.70	2.1	48.45	2.9	47.22	2.7	47.45	3.5	41.93	4.7
3.30	93.20	5.1	79.89	2.0	40.76	2.7	35.03	2.3	32.78	2.8	19.60	3.4
3.45	72.33	4.8	53.68	1.6	27.00	2.1	19.29	2.1	18.17	2.0	8.62	2.4
4. 0	50.49	4.4	33.37	1.4	14.68	1.5	7.90	1.4	10.21	1.4	4.33	1.9
4.15	34.73	4.0	22.09	1.2	9.14	1.2	3.90	1.0	6.10	1.1	2.42	1.3
4.30	24.37	3.6	15.25	1.1	6.09	1.0	2.19	0.8	3.84	0.9	*1.47*	1.3
4.45	16.76	3.3	10.60	1.0	4.12	0.9	1.33	0.7	2.50	0.7	0.94	1.1
3. 0	11.96	3.0	7.57	1.0	2.55	0.8	0.87	0.7	1.77	0.6	0.66	1.0
5.15	0.87	2.8	5.61	0.9	2.02	0.8	0.59	0.7	1.27	0.6	0.47	0.9
MO	6.73	2.6	4.12	0.8	1.22	0.6	0.43	0.7	0.97	0.5	0.34	0.8
5.45	5.16	2.4	3.08	0.7	0.76	0.4	0.32	0.6	0.73	0.3	0.26	0.7
6. 0	4.01	2.2	2.37	0.7	0.51	0.4	0.23	0.6	0.56	0.4	0.20	0.7
6.13	3.17	2.1	1.86	0.7	0.38	0.3	0.17	0.5	0.44	0.4	0.17	0.7
6.30	2.54	2.0	1.50	0.7	0.30	0.3	0.13	0.5	0.35	0.4	0.15	0.7
6.45	2.07	1.9	1.24	0.6	0.24	0.3	0.11	0.3	0.31	0.4	0.13	0.6
7. 0	1.72	1.8	1.03	0.6	0.20	0.3	0.09	0.4	0.27	0.4	0.11	0.6
7.15	1.44	1.7	0.87	0.6	0.17	0.3	0.08	0.4	0.23	0.4	0.09	0.6
7.30	1.21	1.6	0.74	0.6	0.14	0.3	0.07	0.4	0.20	0.4	0.08	0.6
7.45	1.03	1.6	0.61	0.6	0.12	0.3	0.06	0.4	0.18	0.4	0.07	0.3
8.0	0.88	1.5	0.49	0.5	0.11	0.3	0.06	0.4	0.16	0.4	0.07	0.5

SUMMARY STATISTICS FOR CONDUITS

COMMIT MIER	DESIGN OILER FLOW (CIS)	DESIGN VELOCITY (FPS)	CONDUIT VERTICAL DEPIN (IN)	MAXIMUM COMPUTED FLOW (CFS)	TIME OF OCCUNDICE HR	MIN	MAXIMUM COMPUTED VELOCITY (FPS)	TINE OF OCCUREMCE HR	MIN	RATIO OF MAX. TO DESIGN FLOW	MAXIM DEPTH ABOVE INVERT AT CONDUIT UPSTREAM (FT)	MIDSTREAM (FT)
8040	73.6	5.9	48.0	50.8	0	19	6.4	0	18	0.7	13.40	16.70
8060	53.3	4.2	48.0	47.5	0	23	3.0	0	24	0.9	16.70	19.48
8100	78.1	4.9	54.0	61.0	0	37	5.5	0	34	0.8	3.36	3.56
8130	70.6	4.4	54.0	54.9	1	4	5.1	0	57	0.8	3.56	2.51
1030	3021.3	12.5	108.0	120.6	3	1	5.5	3	2	0.0	4.22	0.53
1370	123.6	5.2	66.0	52.7	1	22	3.9	t	10	0.4	2.51	3.87
1600	146.8	5.2	72.0	73.7	0	40	6.1	0	38	0.5	3.39	3.87
1630	2313.2	9.3	108.0	120.7	1	37	5.0	0	44	0.1	3.87	4.22
1602	43.4	2.2	60.0	70.6	2	60	4.0	0	28	1.6	21.68	3.39

EXTRAIUSEIrSAWLEXAMPLEMKS1
BASICPIPESYSTEMFRONFUME8

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        I GROUND ELEV- 133.00 FEET
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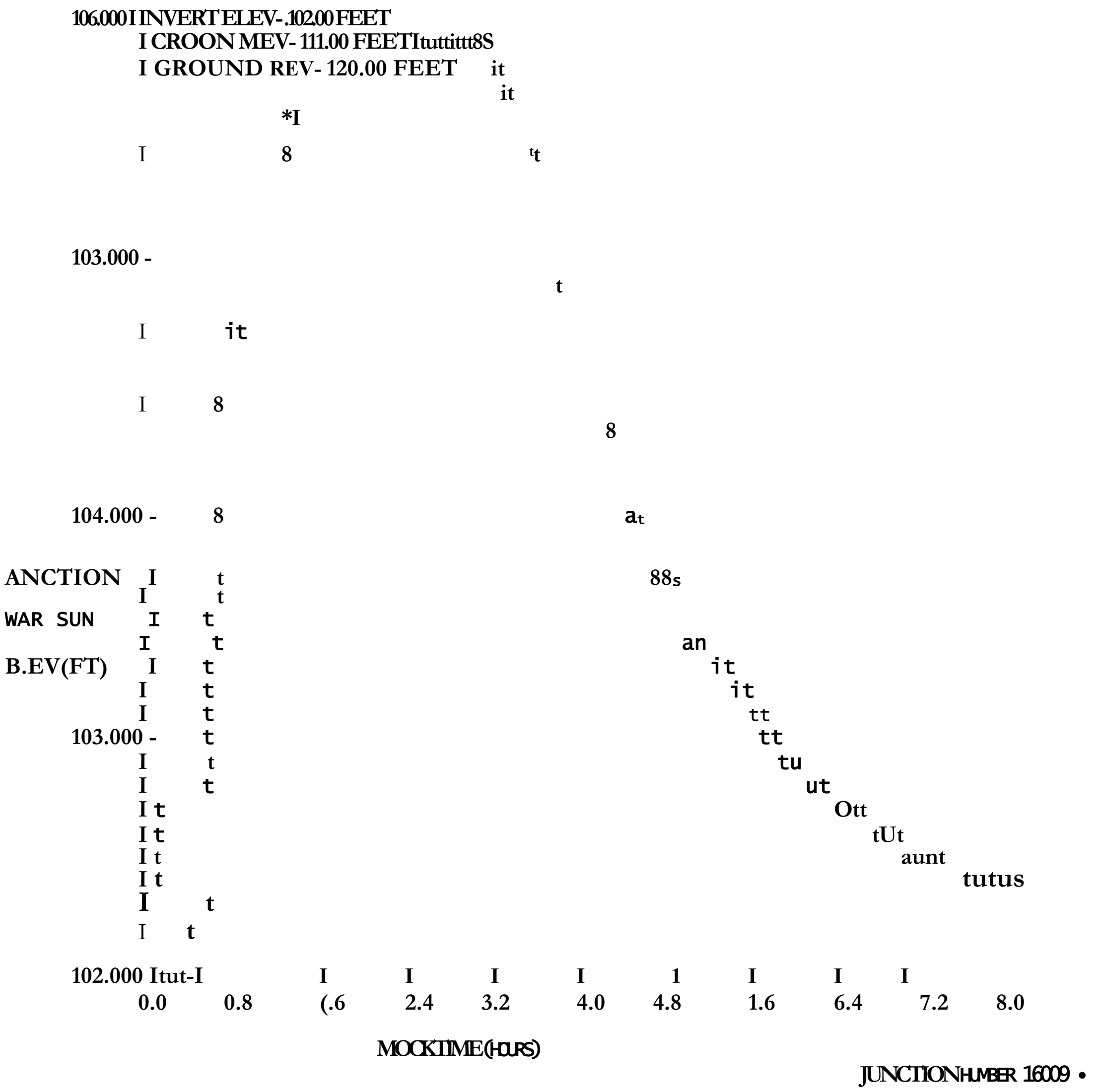
MOM TI)E MRS)

JUNCTIONNUMBER80608

CNIROMPUTM. PROTECTION AGENCY
UASIDIGIONiD.C.

MLM USER'S MANUAL EXAMPLE ?MOILER 1
BASIC PIPE SYSTEM FROM FIGURE 8

ittt EXTENDED TRANSPORT PROGRAM nit PATER REMIXES DIVISION
ittS int VAMP DRESSER I MCKEE INC.
tin ANALYSIS NODULE tut ANNANDALE, VIRGINIA



EWIRONNENTAL PROTECTION AGENCY
iASIIINGTONtO.C.

WWI USER'S MANUAL MIRE PROLITI 1
BASIC PIPE MIEN FRON FINK 8

:US DIMMED TRANSPORT PROGRAM !SU WATER MACES DIVISION
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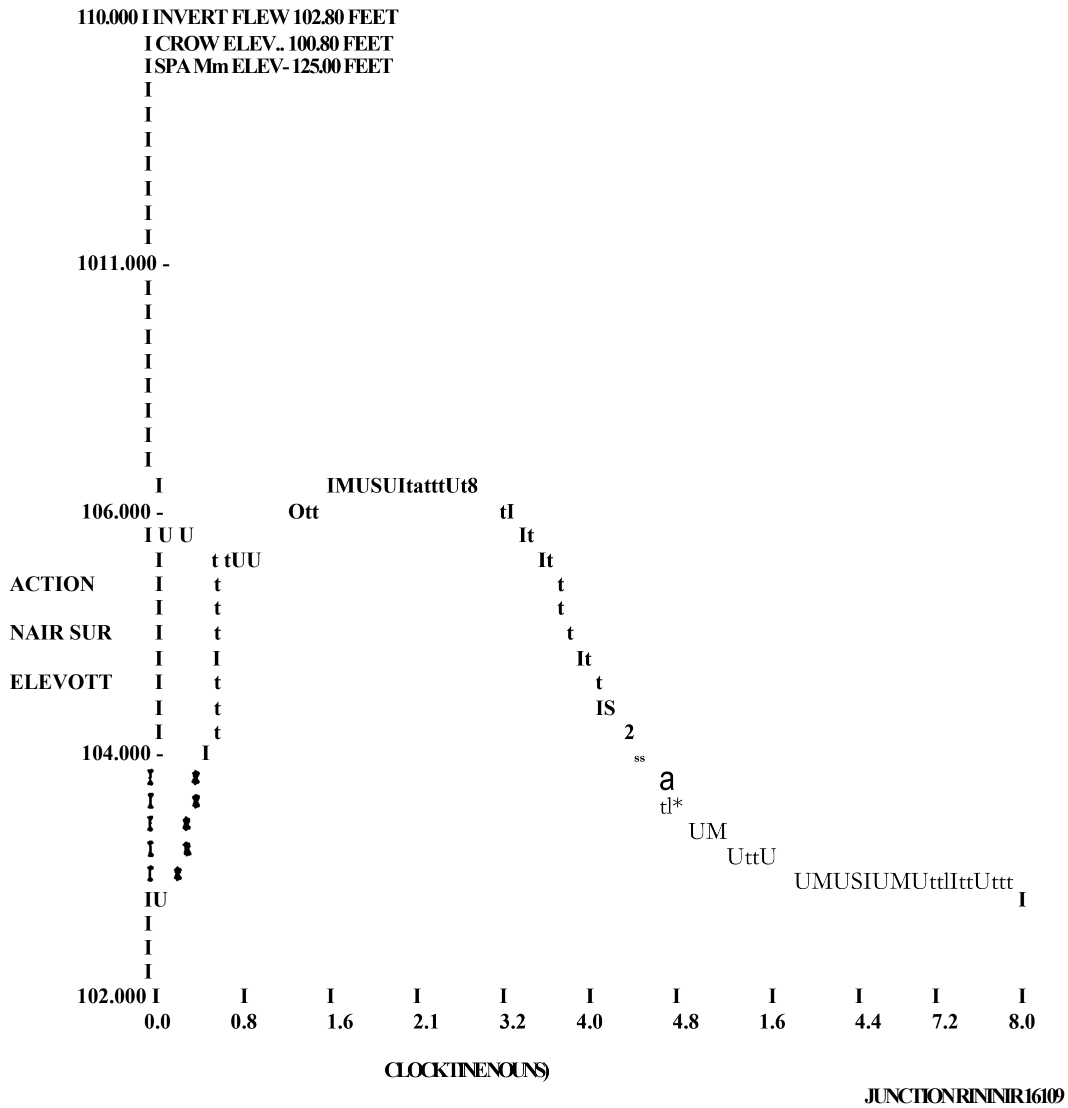
tilt ANALYSIS OW ItU ANNANONLEs VIRGINIA



EAVIRCIIINTALPROTECTIONAGENCY EU EXTEND TRANSPORT PROGRAM Mt WATER RESCUICES DIVISION
WASHINGTON,D.G. nit tES DIP DRESSER t MCKEE INC.
tttt ANALYSIS MOUE VIRGINIA

EXTIRAMUSERSMANUALEXAMPLEPROBLEM
1BASICPIPESYSTEMFROMFIGURE8

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114.400 I INERT ELEV- 111.50 FEET
        I CROW ELEV- 117.00 FEET
        I GROWN ELEV- 125.00 FEET

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                              CM ME (HOURS)
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ENVIRONNENTAL PROTECTIOM AGENCY
WASHLGITON,D.C.

EXTRAN USES MANUAL EXAMPLE PROBLEM 1
BASIC PIPE SYSTOI FROM FIGURE 8

Etn EXTENDED TRANSPORT PROGRAM Mt
DUB ttn
tut ANALYSIS MOBILE flit

HATER RESOURCE DIVISION
CAN DRESSER 1 COM INC.
WARMER VIRGINIA

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        I GROUND 0.EV 155.00 FEET


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                          CLOE TINE (HOURS)
                                                        JUNCTION HUMBER 82309
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ENVIRONMENTAL PROTECTION AGENCY
WASHIAITON, D.C.

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SUB ANALYSIS MODULE int

WATER RESOURCES DIVISION
CAMP DRESSER I MCKEE U(.
ANNANDALE, VIRGINIA

EXTRAN USER'S MANUAL COMЕ PROBLEM
1 BASIC PIPE SYSTEM FRON FIGURE 8

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                                   CLOCK TINE (HOURS)
                                                                    JUNCTION HUMBER 80408
```

EP7IRONENTAL PROTECTION AGENCY
WASHINGTON, D.C.

ORANUSER'S?MAL *EXAMPLE* PROBLEM 1
BASIC PIPE SYSTEM FROM FIGURE 8

tin MENDED Tumor PROGRAM SUB
DIVISION
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INC.
nn ANALYSIS NOXLEtin

PATER RESOURCES

CAMP DRESSER L MCKEE

ANNANDALE, VIRGINIA

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CLOCK TIME MOMS)

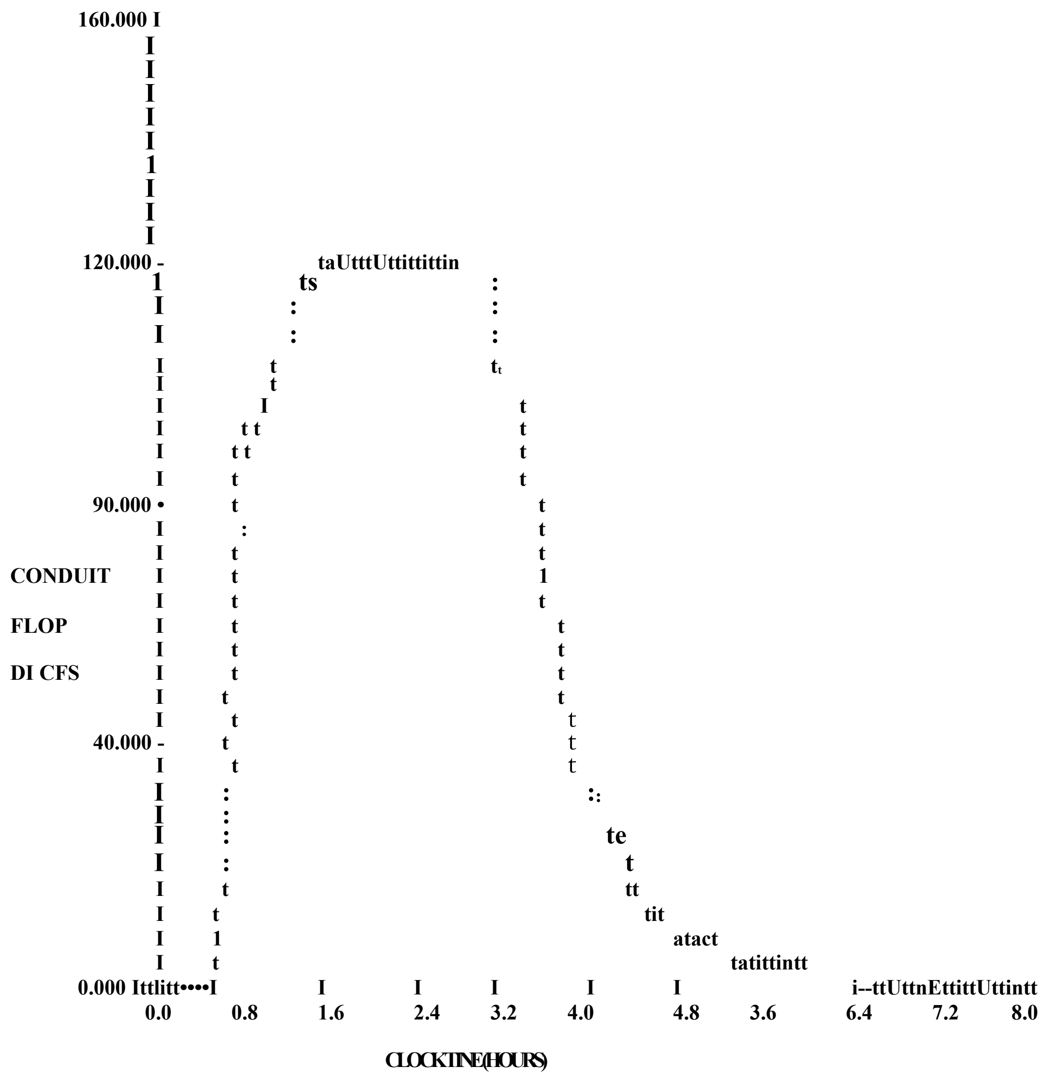
COMMIT OM 1030

ENVIRONMENTAL PROTECTION NANCY
VASNINOTON, B.C.

ORAN DM'S *RAM OMNI PR0MEN* 1
BASIC PIPE SYSTEM FROM nee 8

ttrt EXTENDED TRANSPORT FROWN an
au au
Mt NVLYSIS NODULE WV

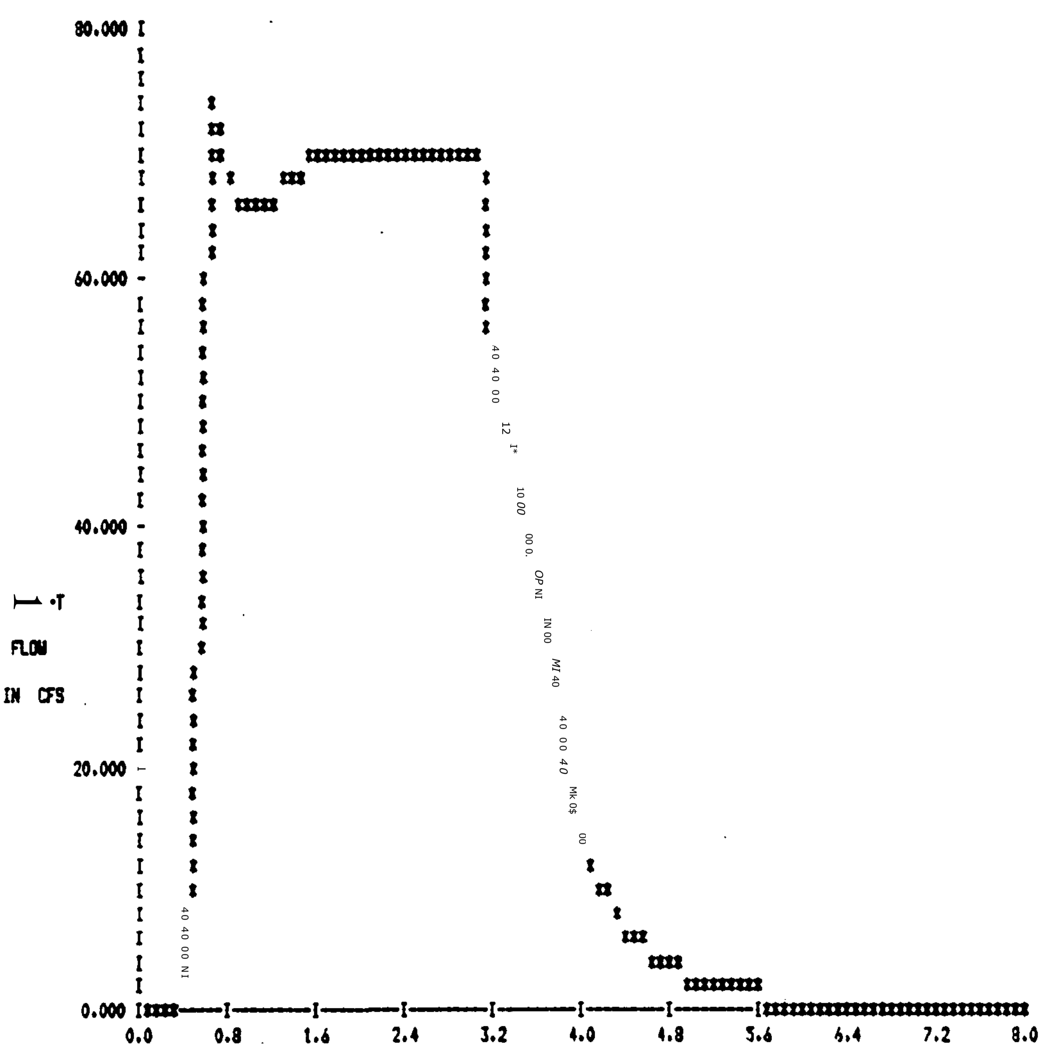
WU RESOURCES DIVISION
CANP DRESSER $ COI INC.
ANNANDALE, VIRGINIA



CONDUIT MUNRO 1630

ENVIRONMENTAL PROTECTION AGENCY **** EXTENDED TRANSPORT PROGRAM **** WATER RESOURCES DIVISION
WASHINGTON, D.C. **** **** CAMP DRESSER & MCKEE INC.
**** ANALYSIS MODULE **** ANNANDALE, VIRGINIA

EXTRAN USER'S MANUAL EXAMPLE PROBLEM 1
BASIC PIPE SYSTEM FROM FIGURE 8



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ENVIROMENTAL PROTECTION AGENT          an =KID TRANSMIT FROMM tat          WATER RESOURCES DIVISION
WASHINGTON D.C.                        an                        tat       CAN> DREFIt f MU INC.
                                       tttt      AI .ISIS NOttU  an        ANNANDALE, VIRGINIA
SITU! USER'S IDWRIAL MIRE PROW 1
BASIC PIPE SYSTEM FRON FIGLI71 8

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                              CLOCK TINE (HOURS)
                                                                      CONDUIT MIC16 1602
```
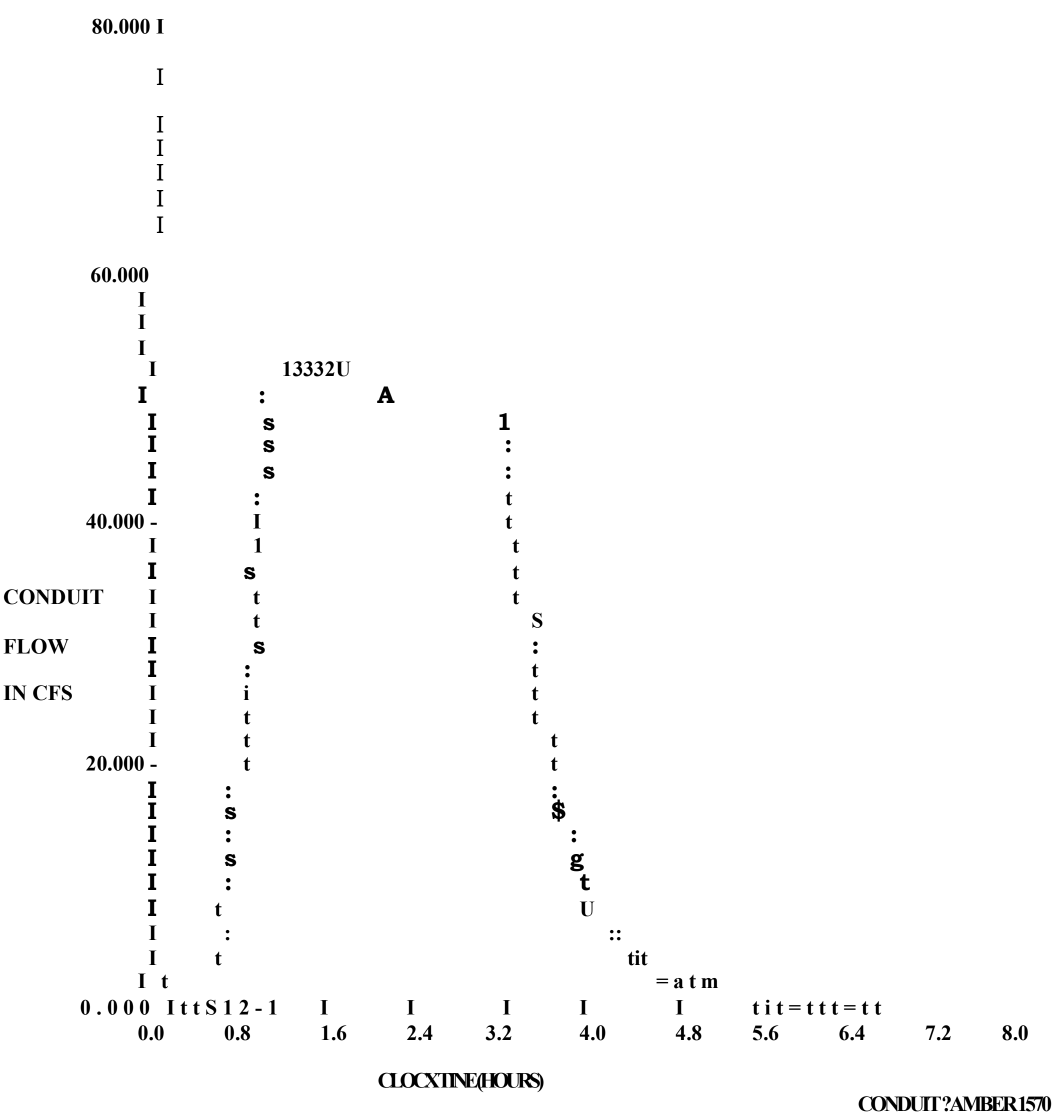

EXTRMIUSER'SNAIMALOUMPLEPROLEM
1BASICPIPESYSTEMFIXINFIGURE8



EWIRONIENTALPROTECTIONAGENCY
HASHINEITONIDC.

CawUSER'SMANUALDAMEPROILEM1BASIC
PIPESYSTEMFROMFIGURE8

12U MENDED TRANSPORT PR06RA11 tiff HATER PISUCES DIVISION
tin ttat *CON* DRESSER 1 NOV 11C.
tili ANALYSIS NODULE JUR AINIANNALE,
VIRGINIA

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CLOT(TIME(HOURS)

CONDUITNUMBER8130

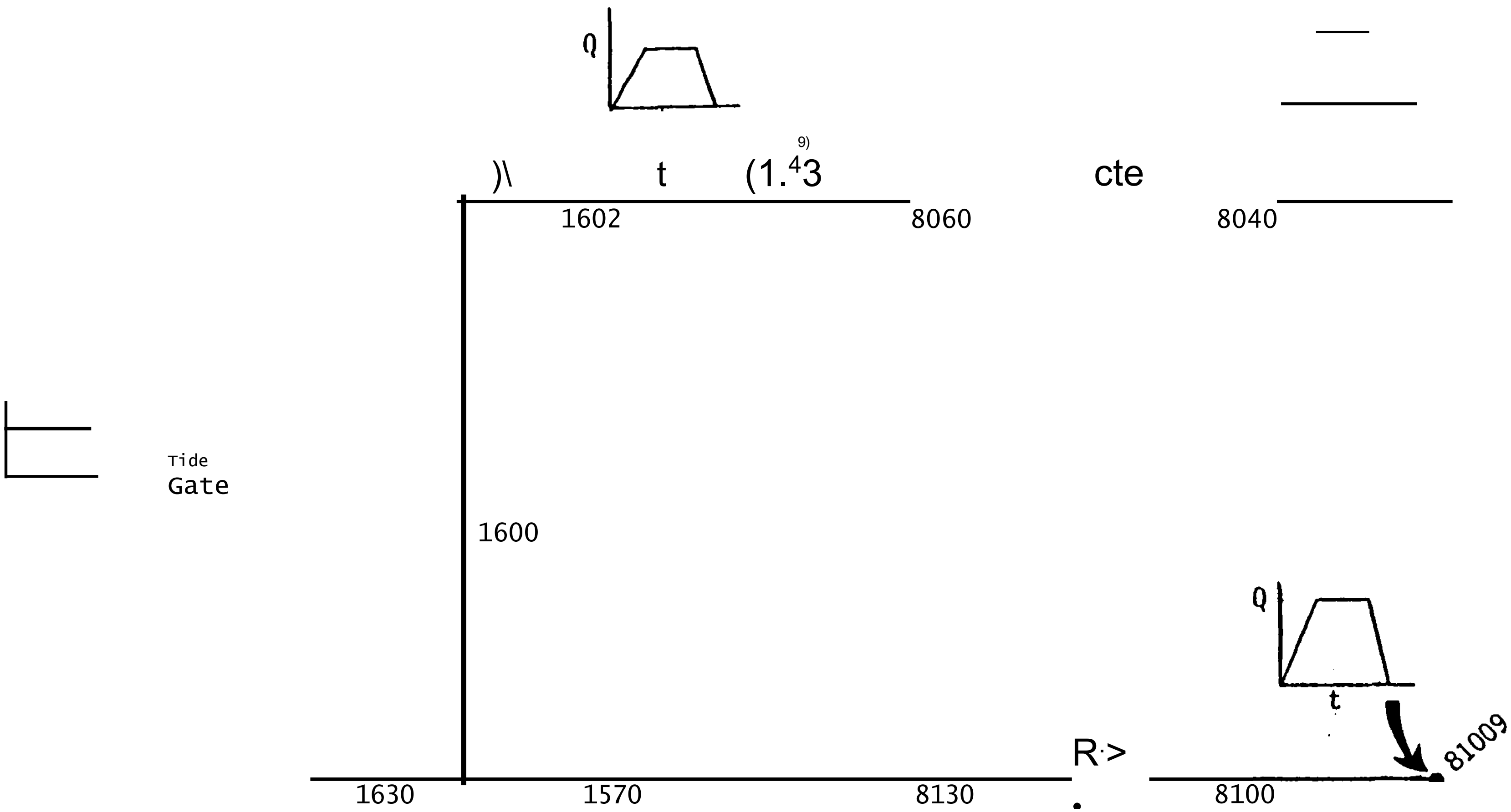

Figure 9. Basic system with tide gate.

TABLE 4

INPUT DATA SET FOR EXAMPLE 2

```
 0           0
   EXTRAN USER'S MANUAL EXAMPLE PROBLEM 2
   BASIC PIPE SYSTEM WITH TIKE CATE FROM FIGURE        9
1440     20.     .0    6     6    6     6   45    43    3    30 0.05
         80608      16009      16109      15009      82309      80408
          1030       1630       1600       1602       1570       8130
         80608      16009      16109      15009      82309      80408
          1030       1630       1600       1602       1370       3130
    80408040880603     1       4.00       1800.            .015
    80608060882309     1       4.00       2075.      2.2   .015
    81008100981309     1       4.50       5100.            .015
    81308130915009     1       4.50       3500.            .015
    10301030910208     6        9.0       4500.            .016  3.     3
    15701300916009     1        3.5       1000.            .01.34
    J6001600916109     1        6.0        500.            .015
    16301600910309     6        9.0        300.            .015  3.     3
    16028230916109     1         5.       5000.            .034
  99999
  80408138.0124.6 0.0
  80608135.0118.3 0.0
  31009137.0128.2 0.0
  81309130.0117.3 0.0
  82309155.0112.3 0.0
  10208100.0 89.9 0.0
  10309111.0101.6 0.0
  15009125.0111.3 0.0
  16009120.0102.0 0.0
  16109125.0102.8 0.0
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  0.0          0.0         0.0         0.0
  0.25         40.0        45.0        50.0
  3.0          40.0        45.0        50.0
  3.25         0.0         0.0         0.0
  12.0         0.0         0.0         0.0
```

TABLE 5

OUTPUT FROM EXAMPLE 2

ENVIRONMENTAL PROTECTION AGENCY   Utt EXTENDED TRANSPORT PROGRAM Vt12   MATER RESOURCES DIVISION
WASHINGTON, D.C.   MI   Mt   CAW DRESSER I MCXEE INC.
Mt   ANALYSIS MODULE   tfili   ANNANDALE: VIRGINIA
EXTRA" USER'S MANUAL CORFU PROBLEM 2
BASIC PIPE SYSTEM WITH TIDE GATE FROM FIGURE 9

SUMMARY STATISTICS FOR JUNCTIONS

JUNCTION NUMBER	GROUND ELEVATION (FT)	UPPERMOST PIPE CROWN ELEVATION (FT)	MAXIMUM COMPUTED DEPTH (FT)	TIME OF OCCURENCE HR.	MIN	FEET OF SURCHARGE AT MAX. DEPTH	FEET MAX. DEPTH IS BELOW GROUND ELEVATION	LENGTH OF SURCHARGE (MIN)
80408	138.00	128.60	. 13.40	0	32	9.40	0.00	153.3
80608	133.00	122.30	16.70	0	30	12.70	0.00	159.7
81009	137.00	132.70	3.36	0	27	0.00	5.44	0.0
81309	130.00	122.00	3.56	0	51	0.00	8.94	0.0
82309	155.00	118.50	21.68	0	33	15.48	21.02	166.3
10208	100.00	98.90	4.50	0	10	0.00	5.60	0.0
10309	111.00	110.60	2.68	1	33	0.00	6.72	0.0
15009	125.00	117.00	2.51	1	22	0.00	10.99	0.0
16009	120.00	111.00	1.09	0	48	0.00	14.91	0.0
16109	125.00	108.80	3.07	0	37	0.00	19.13	0.0

TABLE 5

OUTPUT FROM EXAMPLE 2
(Continued)

ENVIRONMENTAL PROTECTION AGENCY Btu EXTENDED TRANSPORT PROSRAN tm WATER RENEWS DIVISION
BASHING:Mb D.C. nu tut CAMP DRESSER I ICKEE INC.
um ANALYSIS NODULE pits ANNANDALE. VIRGINIA

EXTRAS USER'S MANUAL EXAMPLE PROBLEM 2
BASIC PIPE SYSTEM WITH TIDE GATE FRON FIGURE 9

SUMMARY STATISTICS FOR CONDUITS

CONDUIT NUMBER	DESIGN FLOW (CFS)	DESIGN VELOCITY (FPS)	CONDUIT VERTICAL DEPTH (IN)	mune(COMPUTED FLU (CFS)	TIME OF OCCLIREKE HR.	MIN.	maximum COMPUTED VELOCITY (FPS)	TINE OF OCCURENPE HR.	MIN.	RATIO OF MAX. TO DESIGN FLOW	MAXIMUM DEPTH ABOVE INVERT AT CONDUIT ENOS UPSTREAM (F1)	DOIMISTREAN (F7)
8040	73.6	5.9	48.0	30.8	0	19	6.4	0	18	0.7	13.40	16.70
8060	53.3	4.2	48.0	47.5	0	23	5.0	0	24	0.9	16.70	19.48
8100	78.1	4.9	54.0	61.0	0	37	5.5	0	34	0.8	3.36	3.56
8130	70.6	4.4	54.0	54.9	1	4	5.1	0	57	0.8	3.56	2.51-
1030	3028.3	12.5	108.0	120.4	1	35	3.0	1	35	0.0	2.68	4.50
1570	123.6	1.2	66.0	52.7	1	22	4.5	t	23	0.4	2.51	3.09
1600	146.8	s.2	72.0	74.6	0	40	6.1	0	38	o.s	3.07	3.09
1630	2313.2	9.5	108.0	120.8	1	21	5.5	0	31	0.1	3.09	2.68
1602	43.4	2.2	60.0	69.0	0	37	4,0	0	28	1.6	21.68	3.07

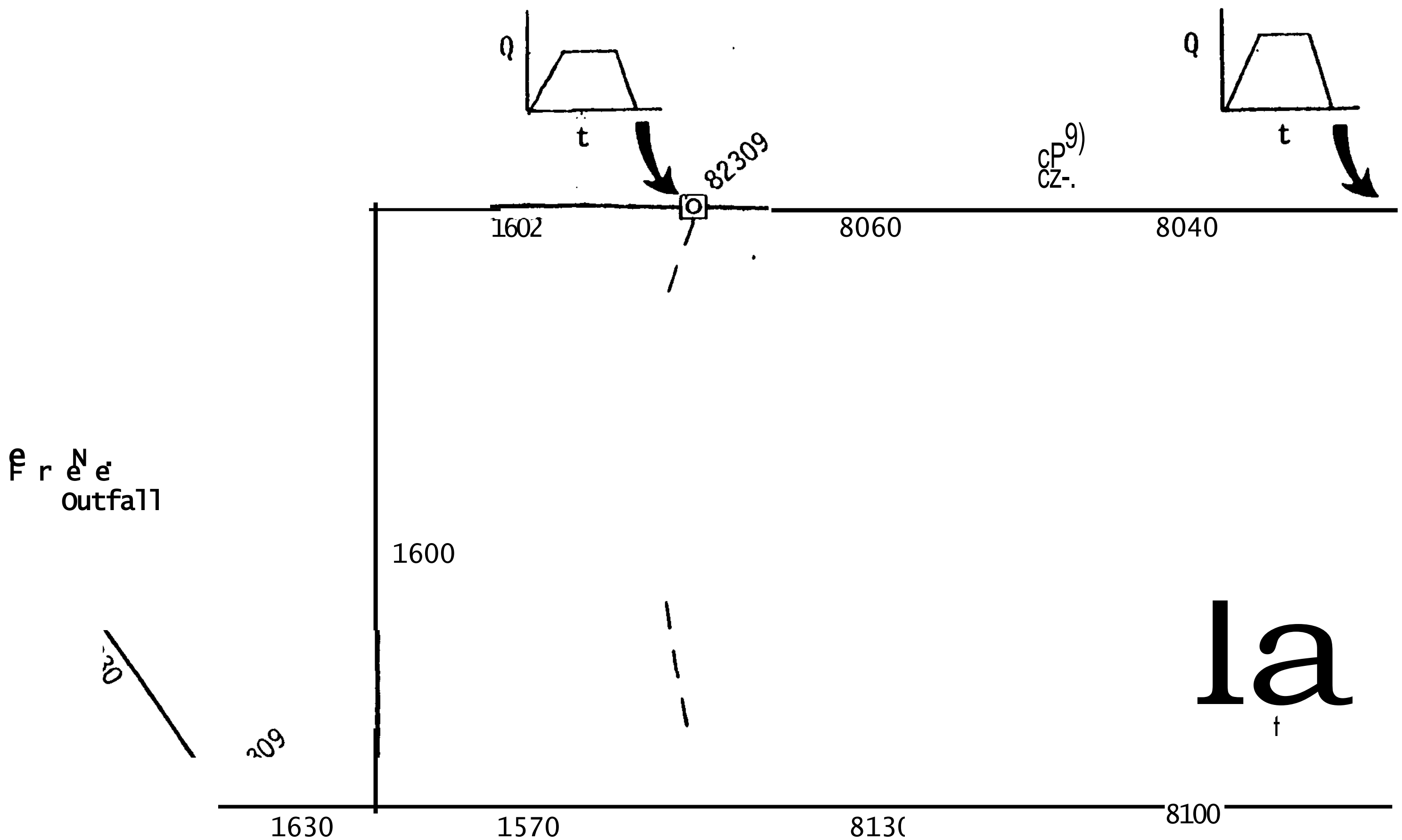


Figure 10. Sump orifice at Junction 82309..

TABLE 6

INPUT DATA SET FOR EXAMPLE 3

```
              0

              0                                                                       10
1440       20.      .0      6      6      6      6    45     45      3     70 0.05
        80608      16009      16109      15009      32309      80408
         1030       1630       1600       1602       1570      81 30
        80608      16009      16109      15009     .82309      80408
         1030       1630       1600       1602       1570       8130
    80408040880608-    1       4.00       1800.             .015
     80608060882309    1       4.00       2075.      ":,el.aL  .015
     81008100981309    1       4.50       5100.             .015
     81308130915009    1       4.10       3500.             .015
     10301030910208    6       9.0        4500.             .016   3.
     15701500916009    1       5.5        5000.             .0154
     16001600916109    1       6.0         500.             .015
     16301600910309    6       9.0         300.             .011   3.    3
     16028230916109    1        5.        5000.             .034
  99999
  80408138.0124.6   0.0
  80608135,0118.3   0.0
  81009137.0128.2   0.0
  81309130.0117.5   0.0
  82309155.0112.3   0.0
  10208100.0 89.9   0.0
  10309111.0101.6   0.0
  15009125.0111.5   0.0
  16009120.0102.0   0.0
  16109125.0102.8   0.0
  99999
  99999
  8230915009     2 3.14   .85
  99999
  99999
  99999
  10208
  99999
  99999
      1
  99999
  323098040881009
  0.0          0.0          0.0         0.0
  0.25         40.0         45.0        50.0
  3.0          40.0         45.0        50.0
  3.15         0.0          0.0         0.0
  12.0         0.0          0.0         0.0
```

TABLE 7

OUTPUT FROM EXAMPLE 3

ENVIRONMENTAL PROTECTION AGENCY — itaMENDED TRANSMITMOANfm — HATFR REsrastas DIVISION
WASHINGTON, D.C. — tut tiu — CAMP DEW 1 MCBEE INC.
lift ANALYSIS MODULE flit — ANNNIDALE: VIRGINIA

EXTRA USER'S MANUAL EXAMPLE PROBLEM 3
BASIC PIPE SYSTEM WITH SW ORIFICE AT JUNCTION 82309 FROM FIG. 10

▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪ SUMMARY STATISTICS FOR JUNCTION S ▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪▪

JUNCTION NUMBER	GROUND ELEVATION (FT)	UPPERMOST PIPE CROWN ELEVATION (FT)	MAXIMUM COMPUTED DEPTH (FT)	TIME OF OCCURRENCE HR.	MIN.	FEET OF MCMNN AT MAX DEPTH	FEET MAX DEPTH IS BELOW GROUND ELEVATION	LENGTH OF SURCHARGE (MIN)
90408	138.00	128.60	2.61	0	16	0.00	10.79	0.0
80608	135.00	122.30	3.21	0	32	0.00	13.49	0.0
81009	137.00	132.70	3.48	0	29	0.00	5.32	0.0
81309	130.00	122.00	2.86	0	58	0.00	9.64	0.0
82309	155.00	118.30	6.16	0	59	0.00	36.34	0.0
10208	100.00	98.90	0.56	2	40	0.00	9.54	0.0
10309	111.00	110.60	4.40	2	41	0.00	5.00	0.0
15009	125.00	117.00	3.60	1	2	0.00	9.90	0.0
16009	120.00	111.00	4.05	2	41	0.00	13.95	0.0
16109	123.00	108.80	3.37	2	40	0.00	18.83	0.0

TABLE 7

OUTPUT FROM EXAMPLE 3
(Continued)

ENVIROMENTAL PROTECTION AGENCY *m MUSED TRANSPORT PROGRAM ittt   8ATFR RESOURCES DIVISION
WASHINGTON. D.C.   nu   nu   *CAMP* DRESSER i MO EE
INC.
ANALYSIS MOBILE nu   AnimatE, VIRGINIA

EXTRAN USER'S AMA. EXAMPLE PROBLEM 3
BASIC PIPE SYSTEM WITH SUMP ORIFICE AT JUNCTION 82309 FROM FIG. 10

SUMMARY STATISTICS FOR CONDUITS

	DESIGNDESIGN		CONDUIT VERTICAL	MAXIMUMTIME	CONFUTED		MAXIMUM OFCOMPUTED	TIME		RATIO OF	OFWIWI DEPTH	ABOVE MAX. TO
	INVERT AT CONDUIT ENDS											
8040	73.6	5.9	48.0	50.8	0	19	6.4	0	18	0.7	2.61	3.21
8060	53.3	4.2	48.0	49.5	0	40	5.1	0	34	0.9	3.21	3.96
8100	78.1	4.9	54.0	57.2	0	41	5.5	0	37	0.7	I.48	2.86
8130	70.6	4.4	54.0	51.6	0	58	4.3	0	S2	0.7	2.86	3.60
1030	3028.3	12.5	108.0	135.0	2	40	5.6	2	40	0.0	4.40	0.56
1570	123.6	5.2	*66.0*	93.3	1	7	5.7	1	3	0.8	3.60	4.05
1600	146.8	5.2	72.0	47.6	2	31	3.3	0	90	0.3	3.17	4.03
1630	2313.2	9.5	108.0	135.1	2	13	4.8	0	44	0.1	4.05	4.40
1602	43.4	2.2	60.0	47.7	1	31	2.8	0	35	1.1	6.16	3.37

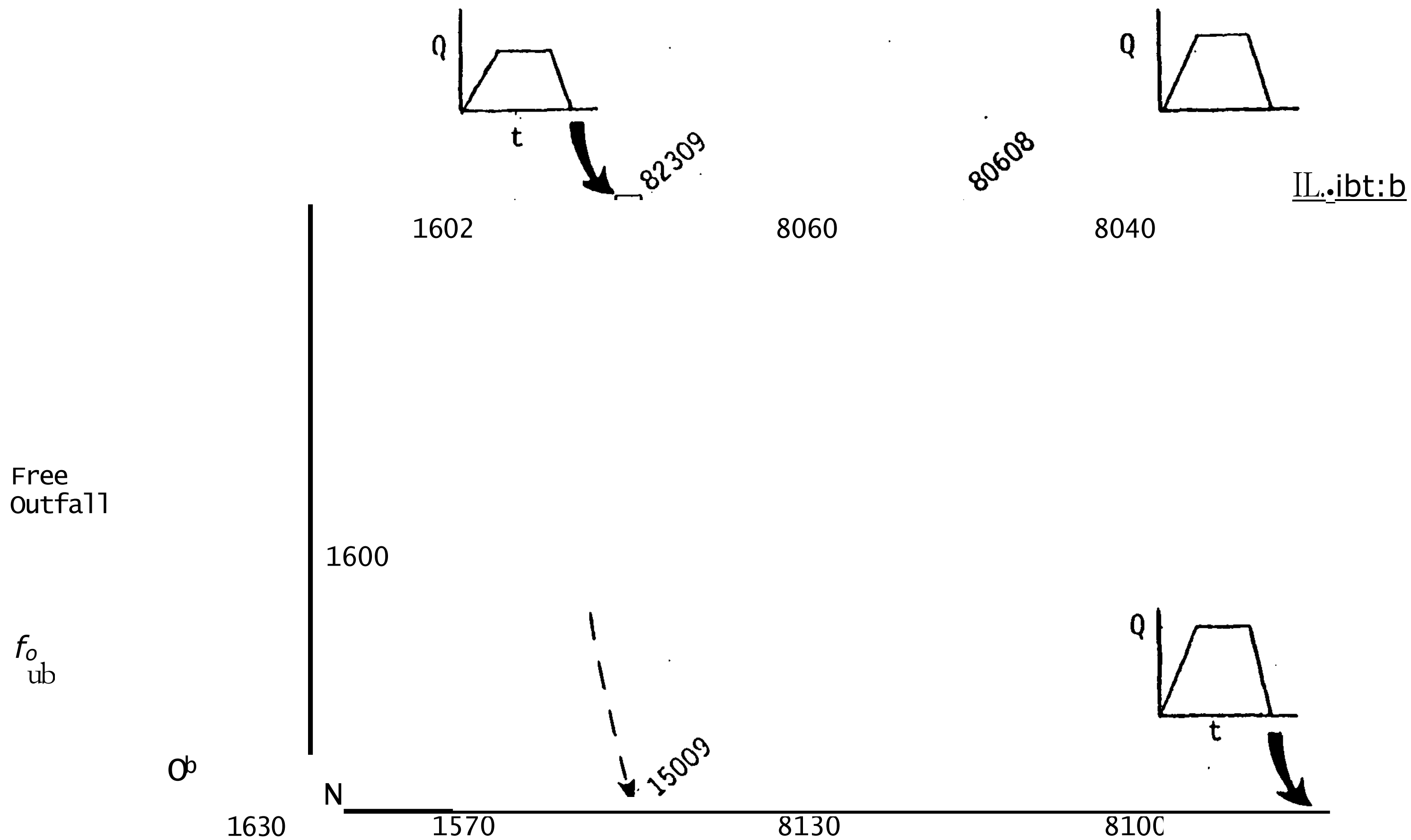


Figure 11. Weir at Junction 82309.

TABLE 8

INPUT DATA SET FOR EXAMPLE 4

```
0          0
    EXTRAN USER'S MANUAL EXAMPLE PROBLEM 4
     BASIC PIPE SYSTEM WITH A WEIR AT JUNCTION 82309 FROM FIGURE    11
1440     20.    .0    6     6     6     6    45    45     3    30 0,05
        80608.    16009      16109      1.3009      92309      80408
         1030      1630       1600       1602       1570       8130
        80608     16009      16109      15009      82309      80408
         1030.     1630       1600       1602       1570       8130
    80408040880608     1        4.00       1800.             .015
    80608060882309     1        4.00       2075.       2.2   .015
    81008100981309     1        4.50       5100.             .015
    81308130915009     1        4.50       3500.             .015
    10301030910208     6         9.0       4500.             .016   3.    3
    15701500916009     1         5.5       5000.            .0154
    16001600916109     1         6.0        500.             .015
    16301600910309     6         9.0        300.             .015   3.   3,
    16028230916109     A          5.       5000.             .034
  99999
  80408138.0124.6   0.0
  80608135.0118.3   0.0
  81009137.0128.2   0.0
  81309130.0117.5   0.0
  82309155.0112.3   0.0
  10208100.0 89.9   0.0
  10309111.0101.6   0.0
  15009125.0111.3   0.0
  16009120.0102.0   0.0
  16109125.0102.8   0.0
  99999
  99999
  99999
  8230915009     1  3.0   6.0   3.0   .80
  99999
  99999
  10208
  99999
  99999
      1
  99999
  823098040881009
  0.0           0.0        0.0        0.0
  0.25          40.0       45.0       50.0
  3.0           40.0       45.0       50.0
  3.15          0.0        0.0        0.0
  12.0          0.0        0.0        0.0
```

TABLE 9

OUTPUT FROM EXAMPLE 4

OIVIRONENTAL PROTECTION AGENCY tat EXTENDED TRANSPORT PROGRAM Mt WATER RESOURCES DIVISION
WASHINGTON, D.C. g/II NM GNP DRESSER 1 MCKEE INC.
nit ANALYSIS MODULE MI ANNANDALE/ VIRGINIA

EXTRAN USER'S MANUAL COME PROMS 4
BASIC PIPE SYSTEM WITH A WEIR AT JUNCTION 82709 FROM FIRM 11

SUMMARY STATISTICS FOR JUNCTIONS

JUNCTION HUMBER	GROUIM ELEVATION (FT)	UPPERMOST PIPE CROWN ELEVATION (FT)	MAXIMUM COMPUTED OEM (FT)	TIME OF OCCURENGE MR.	MIN.	FEET OF MINIMISE AT MAX. OEM	MT MAX DEPTH IS BELOW GROUND ELEVATION	LEANN OF SURCHARGE (MINI
80408	138.00	128.60	10.12	0	37	6.12	3.28	144.3
80608	133.00	122.30	13.32	0	38	9.32	3.38	157.3
81009	137.00	132.70	3.36	0	28	0.00	5.44	0.0
81309	130.00	122.00	3.14	0	47	0.00	9.36	0.0
82309	155.00	118.54	16.46	0	38	10.26	26.24	160.7
10208	100.00	98,90	0.56	3	1	0.00	9.54	0.0
10309	111.00	110.60	4.40	3	1	0.00	5.00	0.0
15009	125.00	117.00	3.13	1	11	0.00	10.37	0.0
16009	120.00	111.00	4.05	3	0	0.00	13.95	0.0
16109	125.00	108.80	3.46	3	0	0.00	13.74	0.0

TABLE 9

OUTPUT FROM EXAMPLE 4
(Continued)

DIVIROMENTAL PROTECTION AGM WASHIN6TON, D.C.

MX EXTENDED TRANWORT PROGRAM LUX
SUS USX
flit ANALYSIS Mani SW

WATCH REMIXES DIVISMI
CAW DRESSER I KEE INC.
ANNANDFLE, VISHNU

EXTRANUSER'S MANUAL EXAMPLE PROIWI 4
BASIC PIPE SYSTEM WITH A WEIR AT JUNCTION 82309 FROM FIGURE 11

SUMMARY STATISTICS FOR CONDUITS

CONDUIT	DESIGN FLOW	DESIGN VELOCITY	CONDUIT VERTICAL DEPTH	MAXIMUM CONUTED FLOW	TIME OF OCOIRENCE		MAXIMUM COMPUTED vaciarr	TIME OF =UWE		RATIO OF MAX. TO DESIGN	MAXIMUM DEPTH ABM INVERT AT CONDUIT ENDS UPSTREM	DOWNSTREAM
8040	73.6	5.9	48.0	50.8	0	19	6.4	0	18	0.7	10.12	13.32
8060	53.3	4.2	48.0	47.4	0	25	5.0	0	24	0.9	13.32	14.26
8100	78.1	4.9	54.0	59.5	0	38	5.3	0	34	0.8	3.36	3.14
8130	70.6	4.4	54.0	55.7	0	56	5.0	0	48	0.8	3.14	3.13
1030	3029.3	12.5	108.0	133.0	3	1	5.6	3	1	0.0	4.40	0.56
1570	123.6	5.2	64.0	76.3	1	11	3.2	0	55	0.6	3.13	4.05
1600	146.8	5.2	72.0	62.3	0	41	5.6	0	38	0.4	3.46	4.05
1630	2313.2	9.5	108.0	133.0	3	0	5.2	0	43	0.1	4.05	4.40
1602	43.4	2.2	60.0	61.2	3	0	3.6	0	29	1.4	16.46	3.46

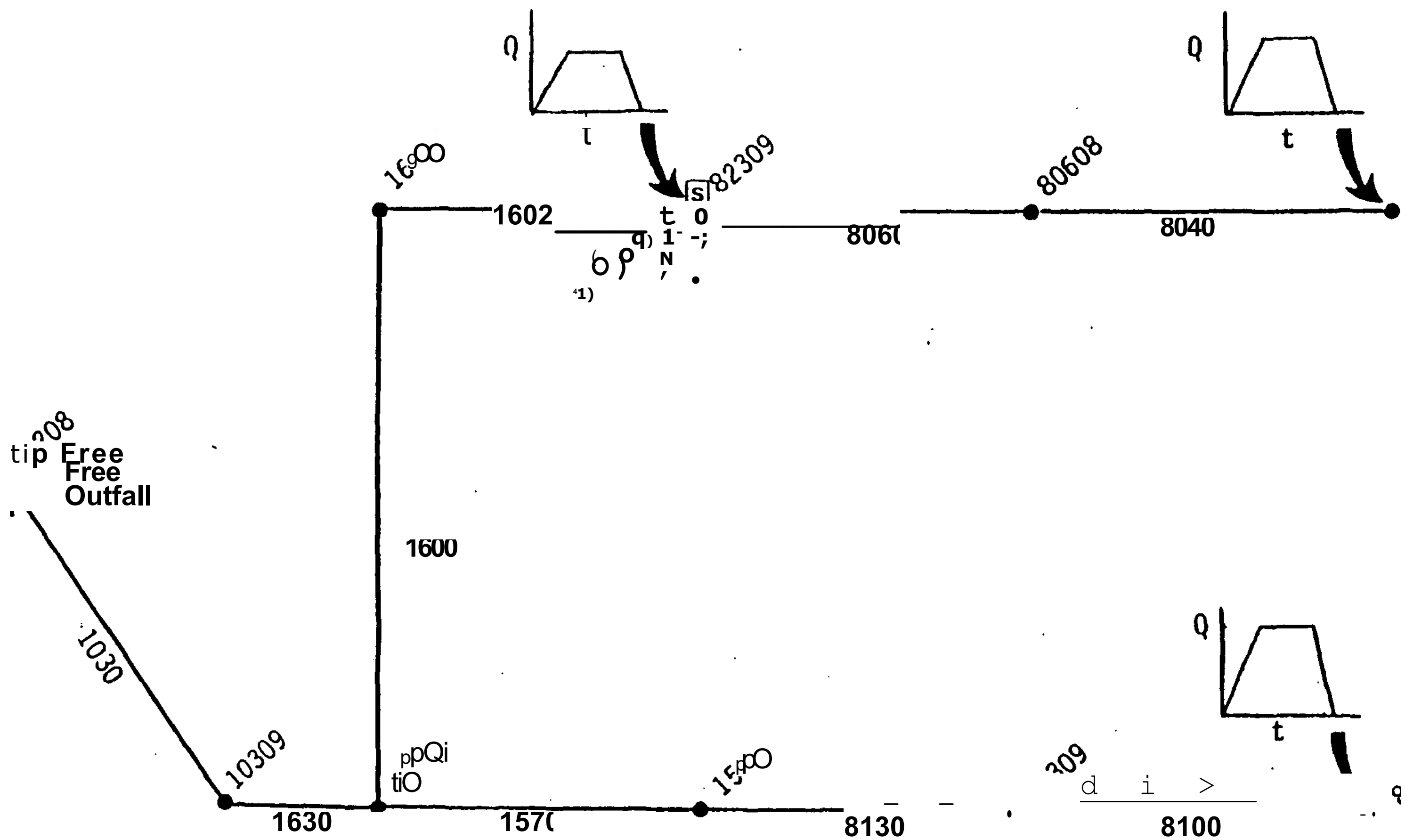


Figure 12. Storage facility and side outlet orifice at Junction 82309.

TABLE 10

INPUT DATA SET FOR EXAMPLE 5

```
     0     0
  EXTRAN USER'S MANUAL EXAMPLE PROBLEM 5
  STORAGE FACILITY AND   SIDE OUTLET  ORIF/CE AT JONC. 32309 (FIG.  12)
1440      20.    .0      6          6  6      45    45     3   70  0.05
80608               16009      16109      15009      82309      80408
1030                 1630       1600       1602       1570       8130
80608               16009      16109      15009      82309      80408
1030                 1630       1600       1602       1570       8130
80408040880608.        1        4.00       1800.              .015
80608060382309         1        4.00       2075.              .015
81008100981309         1        4.50       5100.              .015
81308130915009         1        4.50       3500.              .015
10301030910208         6        9.0        4500.              .016   7.     7
15701500916009         1        5.5        5000.             .0154
16001600916109         1        6.0         500.              .015
16301600910309         6        9.0         300.              .015   3.     3
1.5028230816109        1          5.       5000.              .034
  99999
  80408138.0124.6   0.0
  30608135.0118.3    0.0
  31009137.0128.2    0.0
  81309130.0117.5    0.0
  82309155.0114.5    0.0
  82308155.0112.3    0.0
  10208100.0  89.9   0.0
  10309111.0101.6    0.0
  15009125.0111.5    0.0
  16009120.0102.0    0.0
  16109125.0102.8    0.0
  99999
  32309      155.0      800.0
  99999
  8230982308     1 3.14.85     0.0
  99999
  9⁰999
  99999
  tO2O8
  99999
  99999
     1
  99999
  823098040881009
  0.0          0.0        0.0         0.0
  0.23         40.0       45.0       50.0
  3.0          40.0       45.0       50.0
  3.25         0.0        0.0         0.0
  12.0         0.0        0.0         0.0
```

TABLE 11

OUTPUT FROM EXAMPLE 5

DMRONSHTALPROTECTIONAGENCY SUSEXTENDEDIRANSPORTPROGRAMat WWIRESOURCESDMSION
IIASHINGTOIb D.C. ant MS CAMP DRESSER 1 ICKEE INC.
tm ANALYSIS MOUE CIS ANNANDALE, VIRGINU
CORMUSERSMANUALEXAMPLEPROBLEM3
STORAGE FACILITY AN SIDE OUTLET ORIFICE AT JUIC. 82309 (FIG. 12)

SUMMARY STATISTICS FOR JUNCTIONS

JUNCTION WIDER	GROUND ELEVATION (FT)	UPPERMOST PIPE WWI ELEVATION (n)	MAXIMS COMPUTED DUN (FT)	TIME OF OCCURDCE HR.	MI	FEETOF SURCHARGE ATMAX DEPTH	FEETMAX DEPTHIS DELONGROW ELEVATION	LENGTH OF SUMAS (300
80408	138.00	128.60	13.01	0	33	9.01	0.39	161.0
80608	133.00	122.30	16.70	0	33	12.70	0.00	172.3
81009	137.00	132.70	3.36	0	27	0.00	5.14	0.0
81309	130.00	122.00	3.53	0	II	0.00	8.97	0.0
82309	155.00	153.00	21.03	0	33	0.00	19.17	0.0
82308	133.00	117.30	16.01	0	33	11.01	26.69	163.0
10208	100.00	98.90	0.51	3	2	0.00	9.39	0.0
10309	111.00	110.60	1.08	3	2	0.00	5.32	0.0
15009	125.00	117.00	2.51	1	22	0.00	10.94	0.0
16009	120.00	111.00	3.73	3	1	0.00	11.27	0.0
16109	123.00	108.80	3.19	3	0	0.00	19.01	0.0

TABLE 11

OUTPUT FROM EXAMPLE 5
(Continued)

ONIR0MMENTAL PROTECTION AGENCY nu EMIG TRANSPORT PROGRAM Mt HAM RESOURCES DIVISION
WASHINGTON, D.C. ins ztu CAN DRESSER 1 AWE INC.
Ott ANALYSIS MOLE Ou ANNANDALE, VIRGINIA

MIN USER'S MANUAL EXAMPLE PROSIER 5
STORAGE FACILITY AND SIDE OUTLET ORIFICE AT ANC. 82301 (FIG. 12)

SUMMARY STATISTICS FOR CONDUITS

	DESIGN ENDS	DESIGN	CONDUIT ABOVE VERTICAL	MAXIMUM COMPUTED	TIME OF		MAXIMUM COMPUTED	TIME OF		RATIO OF MAX. TO	MAXIMUM DEPTH INVERT AT CONDUIT	
8040	73.6	5.9	48.0	52.6	0	19	6.4	0	20	0.7	13.01	16.70
8060	53.3	4.2	48.0	47.2	0	27	3.8	0	28	0.9	16.70	21.03
8100	78.1	4.9	54.0	61.0	0	37	5.5	0	34	0.8	3.36	3.53
8130	70.6	4.4	54.0	54.6	1	4	5.1	0	57	0.8	3.33	2.51
1030	3028.3	12.5	108.0	109.9	3	2	5.3	3	3	0.0	4.08	0.51
1570	123.6	5.2	66.0	52.6	1	n	4.1	1	7	0.4	2.51	3.73
1600	146.8	5.2	72.0	63.2	0	43	5.9	0	41	0.4	3.19	3.73
1630	2313.2	9.5	108.0	109.9	3	1	5.0	0	48	0.0	3.73	4.08
1602	43.4	2.2	60.0	61.0	0	31	3.9	0	32	1.4	16.01	3.19

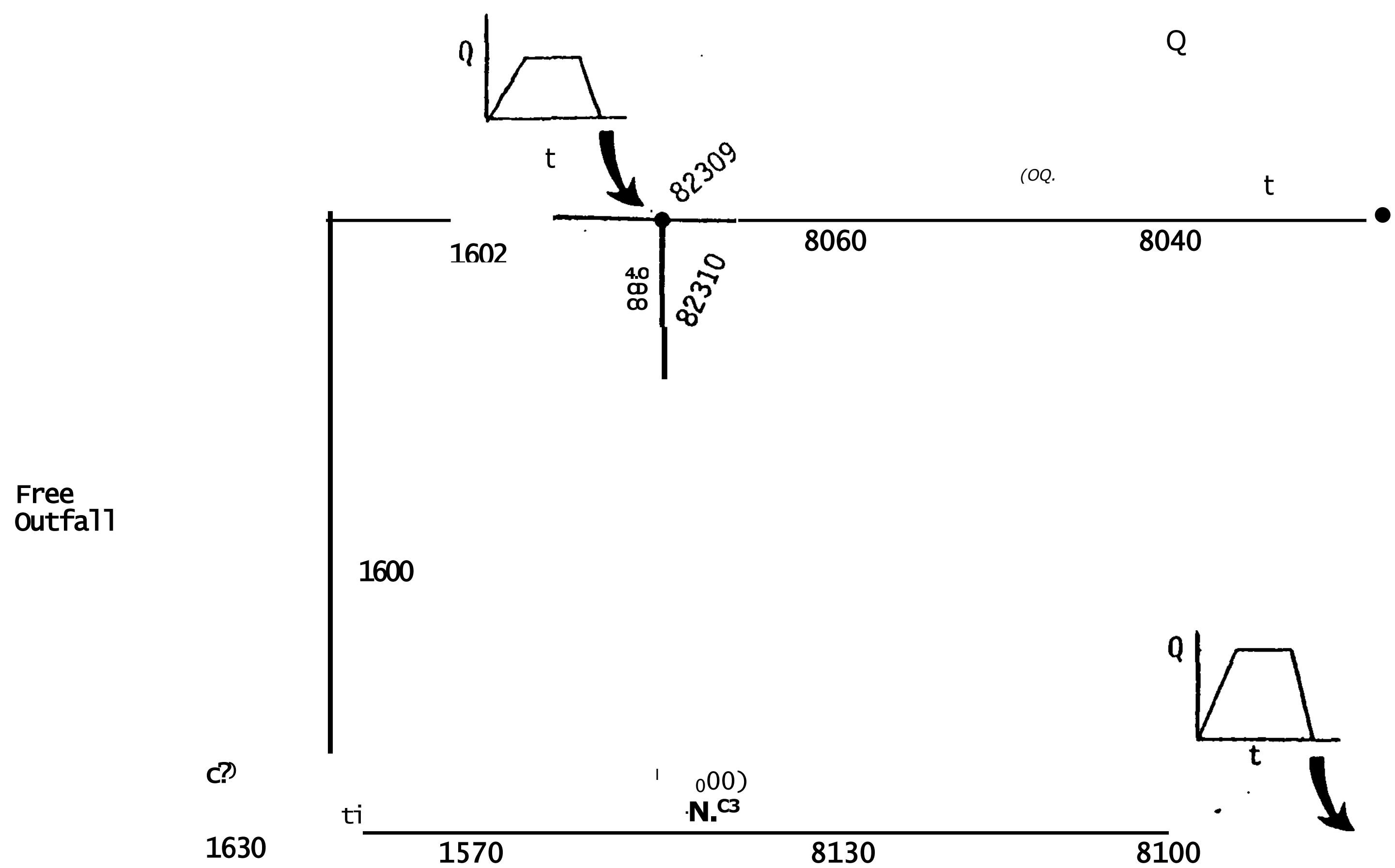


Figure 13. Off-line pump station (activitated by wet well volume) at Junction 82310.

TABLE 12

INPUT DATA SET FOR EXAMPLE 6

```
      0     0
  EXTRAN USER'S MANUAL EXAMPLE PROBLEM 6
  OFF LINE PUMP STATION AT JUNCTION 82310 FROM FIGURE      13-
1440       20.     .0     6 6         6 6        45    45     3    30 0.05
80608              16009     1.6109      15009       32309      80408
1030                1630       1600 1602               1570       8130
80608              16009      16109      15009       82309      30408
1030                1630       1600 1602               1570       8130
80408040880608         1       4.00      1800.              .015
80608060882309         1       4.00      2075.          2.2 .015
8100810098130?         1       4.50      3100.              .015
80618230982310         1      4.0        300.0              .004
81308130915009         1       4.50      3500.              .015
10301030910208         6       9.0       4500.              .016   3,      3
15701500916009         1       5.5       3000.              .0154
           16001600916109      6.0        500.              .015
16301.600910309        6       9.0        300.              .015   3.      3
           16028230916109        J.      5000.              .034
  99999
  80408138.0124.6 0.0
  82310155.0112.3    0.0
  80608135.0118.3    0.0
  81009137.0128.2    0.0
  81309130.0117.5    0.0
  82309155.0112.3    0.0
  10208100.0  89.9   0.0
  10309111.0101.6    0.0
  15009125.0111.5    0.0
  16009120.0102.0    0.0
  16109125.0102.8    0.0
  99999
  99999
  99999
  99999
231015009        1  60.0 5.0    10.0   20.0700.0600.01200.
  99999
  10208
  99999
  99999
      1
  99999
  223098040881009
  0.0           0.0         0.0         0.0
  0.25          40.0        45.0        50.0
  3.0           40.0        45.0        50.0
  3.25          0.0         0.0         0.0
  12.0          0.0         0.0         0.0
```

TABLE 13

OUTPUT FROM EXAMPLE 6

ENVIRONMENTAL PROTECTION AGENCY tut WENDED TRANSPORT PROGRAM tut WATER RESOURCES °Vista
WASHINGTON, O.C. tat tux Cur DRESSER 1 ?)CREE INC.
test ANALYSIS MODULE Int ANNANDALE VIRGINIA

IMAM USER'S ANNAN. EXAMPLE PROBLEM 6
OFF LIME P(RP STATIC!, At JUNCTION 82310 'RON FIGURE 13

S M A R T S T A T I S T I C S F O R J U N C T I O N S

JUNCTION UBER	GROUND ELEVATION (FT)	UPPERMOST PIPE CROWN ELEVATION (FT)	MAXIMUM MINTED DEPTH (FT)	TIME OF COMMENCE HR.	MIN.	FEET OF SURMISE AT MAX DEPTH	FEET NAL DEPTHS MELONGROUND ELEVATION	LENGTH OF SURINAME (MINI
80408	138.00	128.60	13.19	0	44	9.19	0.21	137.7
82310	155.00	116.30	4.00	0	37	0.00	:tut	0.0
80608	135.00	122.30	16.70	0	45	12.70	0.00	145.7
81009	137.00	132.70	3.46	0	29	0.00	5.34	0.0
81309	130.00	122.00	2.87	0	57	0.00	9.63	0.0
82309	155.00	118.50	20.12	0	46	13.92	22.58	151.7
10208	100.00	98.90	0.36	3	1	0.00	9.54	0.0
10309	111.00	110.60	4.40	3	1	0.00	5.00	0.0
15009	125.00	117.00	2.98	1	19	0.00	10.52	0.0
16009	120.00	111.00	4.05	3	1	0.00	13.95	0.0
16109	125.00	108.80	3.49	3	0	0.00	18.71	0.0

TABLE 13

OUTPUT FROM EXAMPLE 6
(Continued)

ENVIRONMENTAL PROTECTION AGENCY — Mt MEG TRANSPORT PROGRAM as — WATER RESOURCES DIVISION
WASHINGTON, D.C. — as — itt8 — CAN; DRESSER $ MCKEE INC.
LUX — ANALYSIS NODULE — tat — ANNAINIALL. VIRGINIA

EXTRA USER'S MANUAL EXAMPLE PROBLEM 6
OFF LIME PULP STATION AT JUNCTION 82310 FROM FIGLIE 13

SUMMARY STATISTICS FOR CONDUITS

CONDUIT NUMBER	DESIGN FLOW (CFS)	DESIGN VELOCITY (FPS)	CONDUIT VERTICAL DEPTH (IN)	KAXNUN COMPUTED FLOW (CFS)	TIME OF mama HR.	NIA.	MAXIMUM CONPUTED VELOCITY (FPS)	TIME OF OCCURENCE HR.	MIN.	RATIO OF MAX. TO DESIGN FLOW	MAXIMUM DEPTH ABOVE NURRT AT CONDUIT ENDS UPSTREAM (FT)	DOINISTRFAN (FT)
8040	73.6	5.9	48.0	50.8	0	19	6.4	0	18	0.7	13.19	16.70
8060	53.3	4.2	48.0	50.4	0	33	5.1	0	30	0.9	16.70	17.92
8100	78.1	4.9	54.0	57.1	0	42	5.5	0	37	0.7	3.46	2.87
8061	0.0	0.0	48.0	115.8	3	9	9;5	3	9	0.0	20.12	sun
8130	70.6	4.4	54.0	52.0	0	57	4.9	0	56	0.7	2.87	2.98
1030	3028.3	12.5	108.0	135.0	3	1	5.6	3	2	0.0	4.40	0.56
1570	123.6	5.2	66.0	70.5	1	(9	5.0	0	42	0.6	2.98	4.05
1600	146.8	5.2	72.0	63.0	3	0	4.8	0	31	0.4	3.49	4.03
1630	2313.2	9.5	108.0	135.0	2	59	5.2	0	46	0.1	4.05	4.40
1602	43.4	2.2	60.0	63.7	1	5	3.7	0	47	1.5	20.12	3.49

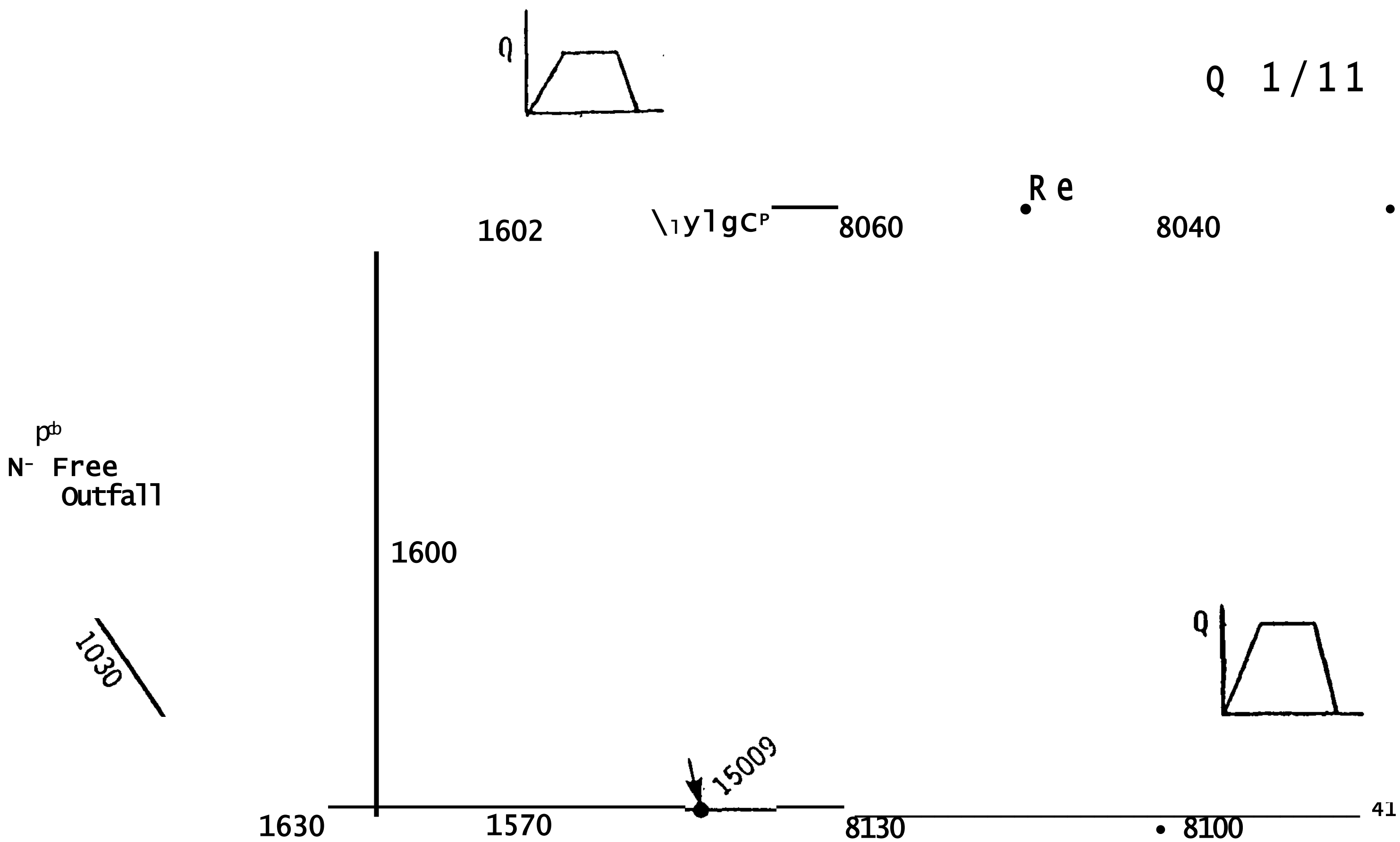


Figure 14. In-line pump (stage activated)
at Junction 82309.

TABLE 14

INPUT DATA SET FOR EXAMPLE 7

```
     0      0
  EXTRAN USER'S MANUAL EXAMPLE PROBLEM 7
  IN LINE   PUMP STATION AT .JUNCTION 82309 FROM FTGURE    14
1440      20.     .0     6 6         66         45    45     330     0.05
80608               16009     1.6109      15009      82309      80408
1030                 1630       1600       1602       1570       8130
80608               16009      16109      15009      82309      80408
1030                 1630       1600       1602       1570       8110
80408040880608          1       4.00      1800.            .015
80608060882309          1       4.00      2075.       2.2  .015
81008100981309          1       4.50      5100.            .015
H1308130915009          1       4.50      3500.            .015
10301030910208          6       9.0       4500.            .016   3.      3
 15701500916009         1       5.5       5000.            .0154
16001600916109          1       6.0        500.            .015
16301600910309          6       9.0        300.            .015   3.      3
16028;0916109           1        5.       9000.            .034
  9999Q
  H0408138.0124.6   0.0
  80608135.0118.3     0.0
  91009137.0128.2     0.0
  31309130.0117.5     0.0
  82309155.0112.3     0.0
  10208100.0   89.9   0.0
  10309111.0101.6     0.0
  15009125.0111.5     0.0
  16009120.0102.0     0.0
  16109125.0102.8     0.0
  99999
  99999
  Dr/99Q
  99999
  .,..'30915009   2   0.0 5.0    10.020.0    8.0 25.0   0.0
  99999
  10208
  9'7,D99
  49999
     1
  99999
  823098040881009
  0.0           0.0          0.0         0.0
  0.25          40.0         45.0        50.0
  7.0           40.0         45.0        50.0
  3.25          0.0          0,0         0.0
  12.0          0.0          0.0         0.0
```

TABLE 15

OUTPUT FROM EXAMPLE 7

ENVIRONMENTAL PROTECTION AREND sat tXIENCDIRANSPORIPROMJ1U UMRESOURCES DIVISION
HASHINGTOM. D.C. ttli tilt *CAN* DRESSER $ MOTE INC.
Sm ANALYSIS MODULE RO>tt ANNANOALEI VIRGINIA

EXTRAN USER'S NANUAL EXAMPLE PROBLEM 7
IN LINE PUN STATION AT JUNCTION 82309 FROM FIGURE 14

SUMMAR)· STATISTICS FOR JUNCTIONS

JUNCTION JAMS	GROUND ELEVATION (FT)	UPPERMOST PIPE CROW ELEVATION (FT)	MAXINUN COMPUTED DEPTH (FT)	TIME OF OCCURF1OE HR.	KN.	FEETOF SURCHARGE AT MAX DEPTH	FEET MAX DEPTH IS BELCH GROUND ELEVATION	LEJIGTH OF SURCHARGE (MIN)
30408	138.00	128.60	13.40	0	35	9.40	0.00	148.7
80608	135.00	122.30	16.70	0	35	12.70	0.00	153.3
31009	137.00	132.70	3.13	0	28	0.00	5.37	0.0
81309	130.00	122.00	3.17	0	53	0.00	9.33	0.0
82309	155.00	118.50	20.93	0	36	14.73	21.77	162.0
10208	100.00	98.90	0.55	3	1	0.00	9.55	0.0
10309	111.00	110.60	4.34	3	1	0.00	5.06	0.0
15009	125.00	117.00	2.75	1	21	0.00	10.75	0.0
16009	120.00	111.00	3.99	3	0	0.00	14.01	0.0
16109	125.00	108.80	3.48	3	0	0.00	18.72	0.0

TABLE 15

OUTPUT FROM EXAMPLE 7
(Continued)

ENVIRONMENTAL PROTECTION AGENCY 10:32 EXTENDED TRANSPORT PROGRAII tits am RESOURCES DIVISION
WASHINGTON ▸ D.C. UZI tlli CAMP DRESSER I =FE
Silt ANALYSIS MODULE SW *ANNANDALE, VIRGINIA*

EXTRAN USERS MANUAL CUWIE PROBLEM 7
INLINE Per STATION AT JUNCTION 82309 FROM FIGURE 14

SUMMARY STATISTICS FOR CONDUITS

	DESIGN ENDS	DESIGN	CONDUIT ABOVE VERTICAL	MAXIMUM COMPUTED	TIME OF		MAXIMUM COMPUTED	TIME OF		RATIO OF MAX. TO	MAXIM" DEPTH INVERT AT CONDUIT	
8040	73.6	5.9	48.0	50.8	0	19	6.4	0	18	0.7	13.40	16.70
8060	53.3	4.2	48.0	48.3	0	27	5.1	0	23	0.9	16.70	18.73
8100	78.1	4.9	54.0	59.3	0	40.	5.5	0	36	0.8	3.43	3.17
8130	70.6	4.4	54.0	53.4	1	3	5.0	0	54	0.8	3.17	2.73
1030	3028.3	12.5	108.0	129.8	3	1	5.6	3	1	0.0	4.34	0.55
1570	123.6	3.2	66.0	61.9	1	21	4.4	1	2	0.5	2.73	3.99
1600	146.8	5.2	72.0	69.8	3	0	5.8	0	38	0.5	3.48	3.99
1630	2313.2	7.5	108.0	129.8	3	0	5.2	0	45	0.1	3.99	4.34
1602	43.4	2.2	60.0	69.8	3	0	3.9	0	30	1.6	20.93	3.48

CHAPTER 4

TIPS FOR TROUBLE-SHOOTING

In the preceding three chapters of this user's manual, we have described in detail the individual data input elements for EXTRAN. We believe that careful study of the data input instructions, together with the example problems of the last section, will go a long way in answering the usual questions of "how to get started" in using a computerized stormwater model as intricate as this one.

Obviously, it is not possible to anticipate all problems in advance and therefore certain questions are bound to occur in the user's initial attempts at application. The purpose of this section is to offer a set of guidelines and recommendations for setting up EXTRAN which will help to reduce the number of problem areas and thereby alleviate frequently encountered start-up pains.

Most difficulties in using the EXTRAN MODEL arise from three sources: (1) improper selection of time step and incorrect specification of the total simulation period; (2) incorrect print and plot control variables; and (3) improper system connectivity in the model. These and other problems are discussed below:

- Numerical stability constraints in the EXTRAN Model require that DELT, the time-step, be no longer than the time it takes flow to travel the length of the shortest conduit in the transport system. A 10-second time-step is recommended for most wet-weather runs, while a 45-second step may be used satisfactorily for DWF conditions. The numerical stability criteria for the explicit finite-difference scheme used by the model are discussed in Chapter 2.

- Numerical instability in the EXTRAN Block is signaled by the occurrence of the following hydraulic indicators:

  (1) Oscillations in flow and water surface elevation which are undampened in time are sure signs of numerical instability. Certain combinations of pipe and weir structures may cause temporary resonance, but this is normally short lived. The unstable pipe usually is short relative to other adjacent pipes and may be subject to backwater created by a downstream weir. The correction is a shorter time-step, a longer pipe length

or combination of both. Neither of these should be applied until a careful check of system connections on all sides of the unstable pipe has been made as suggested below.

(2) A second indicator of numerical instability is a node which continued to "dry up" on each time-step despite a constant or increasing inflow from upstream sources. The cause usually is too large a time-step and excessive discharges in adjacent downstream pipe elements which pull the upstream water surface down. The problem is related to items (1) and (3) and may usually be corrected by a smaller time-step.

(3) Excessive velocities (over 20 fps) and discharges which appear to grow without limit at some point in the simulation run are manifestations of an unstable pipe element in the transport system. The cause usually can be traced to the first source above and the corrections are normally applied, as suggested in item (1) above.

(4) A large continuity error is a good indicator of either stability or other problems. A continuity check, which sums the volumes of inflow, outflow, and storage at the end of the simulation, is found at the end of the intermediary printout. If the continuity error exceeds + 10%, the user should check the intermediate printout for with zero flow or oscillating flow. These could be caused by stability or an improperly connected system.

- Systems in surcharge require a special iteration loop, allowing the explicit solution scheme to account for the rapid changes in flows and heads during surcharge conditons. This iteration loop is controlled by two variables, ITMAX, the maximum number of iterations, and SURTOL, a fraction of the flow through the surcharged area. It is recommended that ITMAX *and* SURTOL be set initially at 30 and 0.05, respectively. the user can check the convergence of the iteration loop by examining the number of iterations actually required and the size of the net difference in the flows through the surcharged area, shown in the intermediate printout. These are significant since the iterations end when either SURTOL times the average flow through the surcharged area is less than the flow differential discussed above, or when the number of iterations exceeds AMAX. If ITMAX is exceeded many times, leaving relatively large flow differentials, the user should increase ITMAX to improve the accuracy of the surcharge computation. If, on the other hand, the user finds that most or all of the iterations do converge, he may decrease ITMAX or increase SURTOL to decrease the runtime of the model and, consequently, the cost. the user should also keep an eye on the continuity error to insure that a large loss of water is not caused by the iterations.

- In some large systems, more than one area may be in surcharge at the same time.* If this occurs and the flows in these areas differ appreciably, those areas with the smallest flows may not converge, while areas with large flows will. This is because both the tolerance and flow differential are computed as sums of all flows in surcharge. It is possible, therefore, to assume convergence has occurred even when relatively large flow errors still exist in surcharge areas with small flows. If the user suspects this situation exists, he can compute a flow differential for any particular surcharge area by adding the differences between inflow to and outflow from each node in that surcharge area. Such information can be found in the intermediary printout. Whenever the flow differential computed in this way is a significantly large fraction of the average flow in this area, inaccurate results may be expected. To correct this, SURTOL can be decreased until the flow differntial for the area in question decreases to a small value over time. It should be noted, however, that large flow differentials for a short period of time are not unusual providing they decrease to near or below the established tolerance for most of the simulation.

- The simulation period is defined by the product NTCYC x DELT or the number of integration cycles times the length of each cycle. If this product exceeds the simulation period of the inflow hydrocraph tape, an illegal end-of-file is encountered and execution stops. NTCYC must then be reduced to correspond with this simulation period.

- The length of all conduits in the transport system should be roughly constant and no less than 100 feet. This constraint may be difficult to meet in the vicinity of weirs and abrupt changes in pipe configurations which must be represented in the model. However, the length of the shortest conduit does directly determine the maximum time step and the number of pipe elements, both of which in turn control the cost of simulation as indicated in Chapter 2. The use of longer pipes should be facilitiated through use of equivalent sections and slopes in cases where significant changes in pipe shape, cross sectional area and gradient must be represented in the model.

- In EXTRAN, printed output can be requested for a maximum of 20 nodes and conduits. In addition, the number of printed points for a given node or conduit is automatically set at 100 regardless of the length of simulation. This requires that the print frequency control variable INTER is defined strictly by the criterion:

$$\frac{\text{NTCYC - NSTART}}{\text{INTER}}$$

  where all variables are as defined in Chapter 2. If, for example, NTCYC = 1600 and NSTART = 9, and we had selected

INTER = 10 then the ratio (NTCYC - NSTART) - INTER = 159. Because the 100-value printing arrays would then be filled with 159 values, an overflow situation would occur thereby producing output which is badly scrambled at best and unusable at worst. Therefore, it is worthwhile to look closely at INTER prior to any major EXTRAN run.

- Prior to a lengthy run of EXTRAN for a new system, a short test run of perhaps five integration cycles should be made to confirm that the link-node model is properly connected and correctly represents the prototype. This check should be made on the echo of the input data, which show the connecting links at each node. The geometric-hydraulic data for each pipe and junction should also be confirmed. Particular attention should be paid to the nodal location of weirs, orifices, and outfalls to ensure thEse conform to the prototype system. In addition, the total number of conduits *and junctions, including internal links and nodes* created, can be determined from the Internal Connectivity Table. This information is necessary for proper specification of initial heads and flows at time zero in the simulation.

- The introduction of a ZP invert elevation difference for all pipes connecting a single junction will cause the junction invert elevation to be incorrectly specified. This, in turn, will create errors in hydraulic computation later in the simulation. The junction invert must be at the same elevation as the invert of the lowest pipe either entering or leaving the junction or it is improperly defined. This problem is readily corrected by checking the punched conduit data cards to determine where a non-zero ZP should be set to zero.

CHAPTER 5

FORMULATION OF EXTRAN

GENERAL

A conceptual overview of EXTRAN is shown in Figure 15. As shown here, the specific function of EXTRAN is to route inlet hydrographs through the network of pipes, junctions, and flow diversion structures of the main sewer system to the treatment plant interceptors and receiving water outfalls. It has been noted in Chapter 2 that the boundary between the RUNOFF and EXTRAN Models is dependent on the objectives of the simulation. EXTRAN must be used whenever it is important to represent severe backwater conditions and special flow devices such as weirs, orifices, pumps, storage basins, and tide gates. Normally, these conditions occur in the lower reaches of the drainage system when pipe diameters exceed roughly 20 inches (50 cm). The Runoff Model, on the other hand, is well suited for the simulation of overland and small pipe flow in the upper regions of the system where the kinematic assumptions of uniform flow hold.

As shown in Figure 15, EXTRAN simulates the following elements -- pipes, manholes (pipe junctions), weirs, orifices, pumps, storage basins, and outfall structures. These elements and their associated properties are summarized in Tables 16 and 17. Output from EXTRAN takes the form of 1) discharge hydrographs and velocities in selected conduits in printed and plotted form; and 2) flow depths and water surface elevations at selected junctions in printed and plotted form. This output is supplied by off-line storage (e.g., discs, tapes) to a subsequent block, e.g., the Receiving Water Block.

CONCEPTUAL REPRESENTATION OF THE TRANSPORT SYSTEM

EXTRAN uses a link-node description of the sewer system which facilitates the discrete representation of the physical prototype and the mathematical solution of the gradually-varied unsteady flow equations which form the mathematical basis of the model.

As shown in Figure 16, the conduit system is idealized as a series of links or pipes which are connected at nodes or junctions. Links and nodes have well defined properties which, taken together, permit representation of the entire pipe network. Moreover, the link-node concept is very useful in representing flow control devices. The specific properties of links and nodes have been summarized in Table 17.

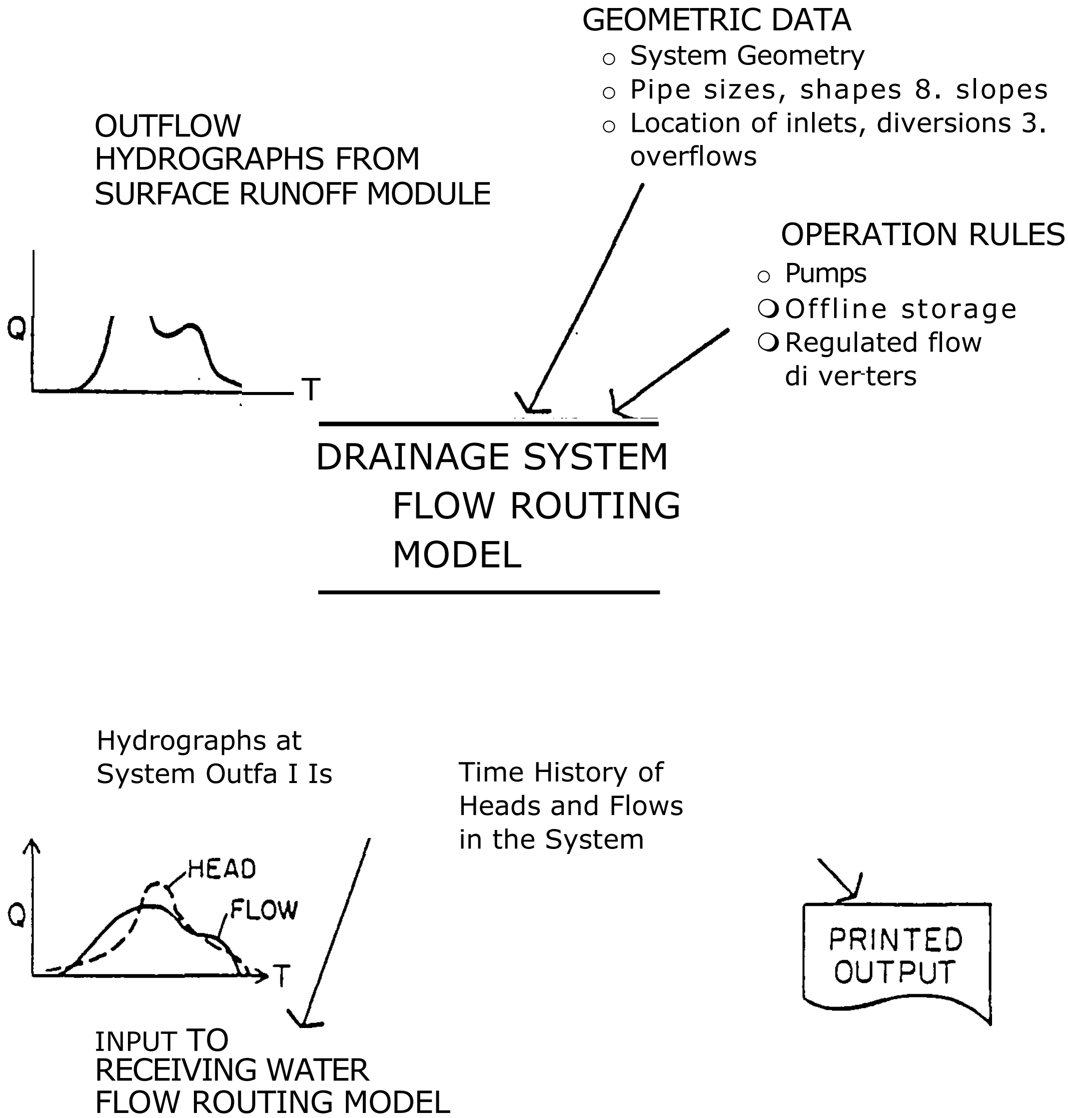


Figure 15. Schematic illustration of EXTRAN

TABLE 16

CLASSES OF ELEMENTS INCLUDED IN THE TRANSPORT MODEL

Element Class	Types
Conduits or Links	Rectangular Circular Horseshoe Baskethandle Eggshape Trapezoid
Junctions or Nodes (Manholes)	------------------------
Diversion Structures	Orifices Transverse weirs Sideflow weirs
Pump Stations	On-line or off-line pump station
Storage Basins	On-line (enlarged pipes or tunnels)
Outfall Structures	Transverse weir with tide gate Transverse weir without tide gate Sideflow weir with tide gate Sideflow weir without tide gate Outfall with tide gate Free outfall without tide gate

TABLE 17

PROPERTIES OF NODES AND LINKS IN THE TRANSPORT MODEL

	Properties and Constraints
NODES	
Constraint	zQ change in storage
Properties computed at each time-step	Volume Surface area Head
Constant Properties	Invert, crown, and ground elevations
LINKS	
Constraint	Qin = Qout
Properties computed at each time-step	Cross-sectional area Hydraulic radius Surface width Discharge Velocity of flow
Constant Properties	Head loss coefficients Pipe shape, length, slope, roughness,

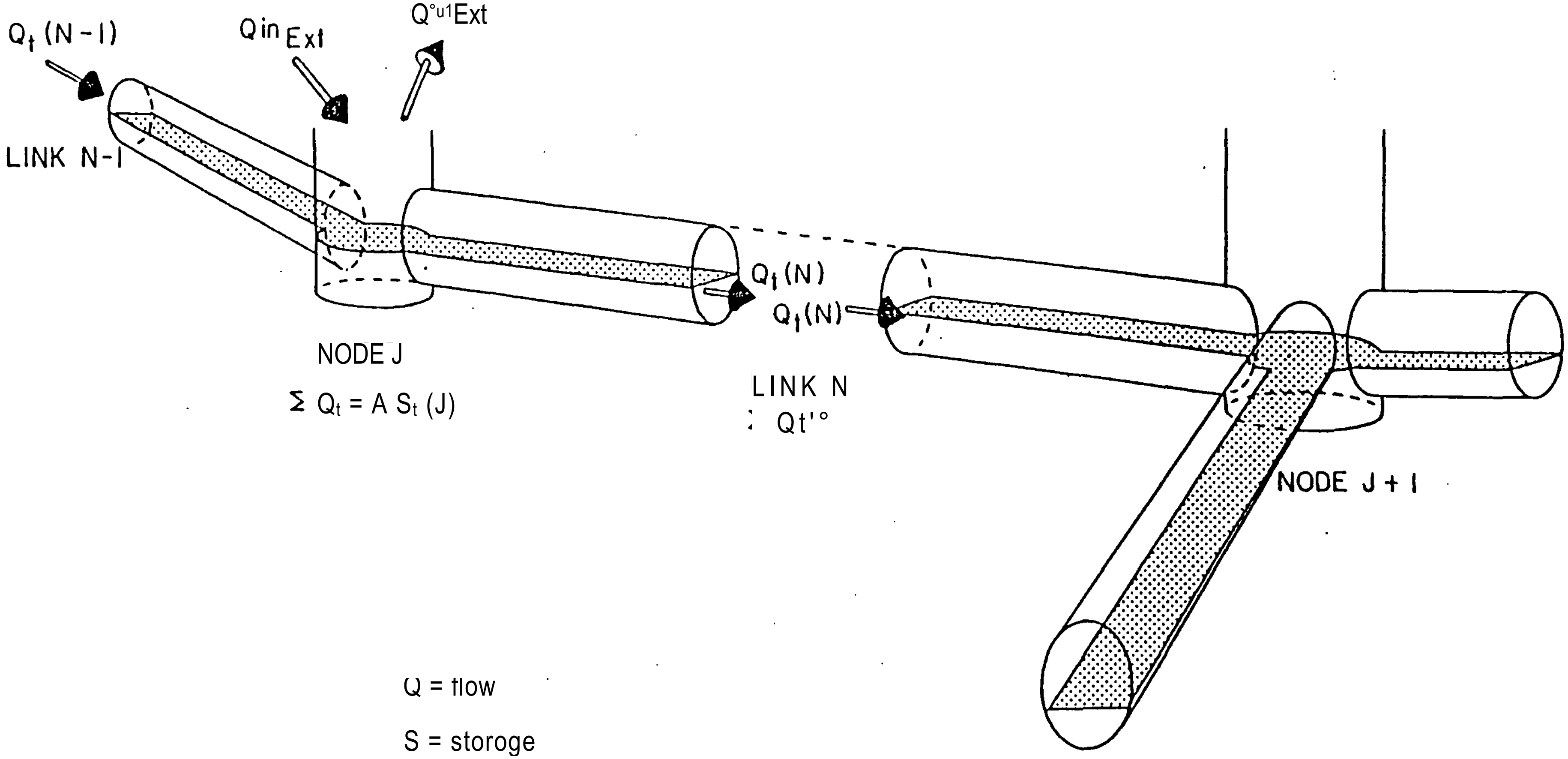


Figure 16. Conceptual representation of the EXTRAN model.

Links transmit flow from node to node. Properties associated with the links are roughness, length, cross-sectional area, hydraulic radius, and surface width. The last three properties are functions of the instantaneous depth of flow. The primary dependent variable in the links is the discharge, Q. It is assumed that Q is constant in the link, while velocity and the cross-sectional area of flow, or depth, are variable in the link. In the early development of EXTRAN, a constant velocity approach was used, but this was found later to produce highly unstable solutions.

Nodes are the storage elements of the system and correspond to manholes or pipe junctions in the physical system. The variables associated with a node are volume, head, and surface area. The primary dependent variable is the head, H, which is assumed to be changing in time but constant throughout any one node. Inflows, such as inlet hydrographs, and outflows, such as weir diversions, take place at the nodes of the idealized sewer system. The volume of the node at any time is equivalent to the water volume in the half-pipe lengths connected to any *one* node. The change in nodal volume during a given time step, At, forms the basis of head and discharge calculations as discussed below.

BASIC FLOW EQUATIONS

The basic differential equations for the sewer flow problem come from the gradually varied, unsteady flow equations for open channels, otherwise known as the Saint-Venant or shallow water equations. The equation for unsteady spatially varied discharge can be written:

3Q = -dAS 204 v23A gA1ft (9)
at f at ax ax

where

Q = discharge through the conduit
V = velocity in the conduit
A = cross-sectional area of the flow
H = hydraulic head
S_f = friction slope

The friction slope is defined by Manning's equation, i.e.

S_f - ___ /3 QIVI
gAR4 (10)

where k = $g(n/1.49)^2$. Use of the absolute value sign on the velocity terms makes Sf a directional quantity and ensures that the frictional force always opposes the flow. Substituting in equation 9 and expressing the finite difference form gives:

H_2-H_1
Qt+at = Qt Qt+At 2V41 at v2 A2-A1 At - gA L At (11)
At

Solving equation 11 for $Q_{t.I.L}$ gives the final finite difference form of the dynamic flow equation as:

$$Q_{t+At} - \frac{1}{1 + k\frac{1}{\_\_4/3} .At \; , \; [\; , \; I^{v'}} \quad Q_{.e.} + 29 \, AA + 9 \; ',. \; ' \, 2^{A}-3^{-A}] \; At - gA \frac{H \; -H}{2L} \; 1 \; At \;] \qquad (12)$$

In equation 12, the values V, R, and A are weighted averages of the conduit end values at time t.

The basic unknowns in equation 12 are Q_{t+At}, H2 and H1. The variables **T**, **R**, and **A** can all be related to Q and H. We, therefore, require another equation at a node. .

$$\frac{aH}{at} = \frac{EQ_t}{A_{S_t}} \qquad (13)$$

or in finite difference form

$$H_{t+At} = H_t + \frac{EQ_t At}{A_{S_t}} \qquad (14)$$

SOLUTION OF FLOW EQUATION BY MODIFIED EULER METHOD

Equations 12 and 14 can be solved sequentially to determine discharge in each link and head at each node over a time-step Lt. The numerical integration of equations 12 and 14 is accomplished by a modified Euler method. The results are accurate and, when certain constraints are followed, stable. Figure 17 shows how the process would work if only the discharge equation were involved. The first three operations determine the slope aQ/at at the "half-step" value of discharge. In other words, it is assumed that the slope at time t + At/2 is the mean slope during the interval. The method is extended easily to more than one equation, although graphic representation is then very difficult. The corresponding half-step and full-step calculations of head are shown below:

Half-step at node j: Time t + At/2

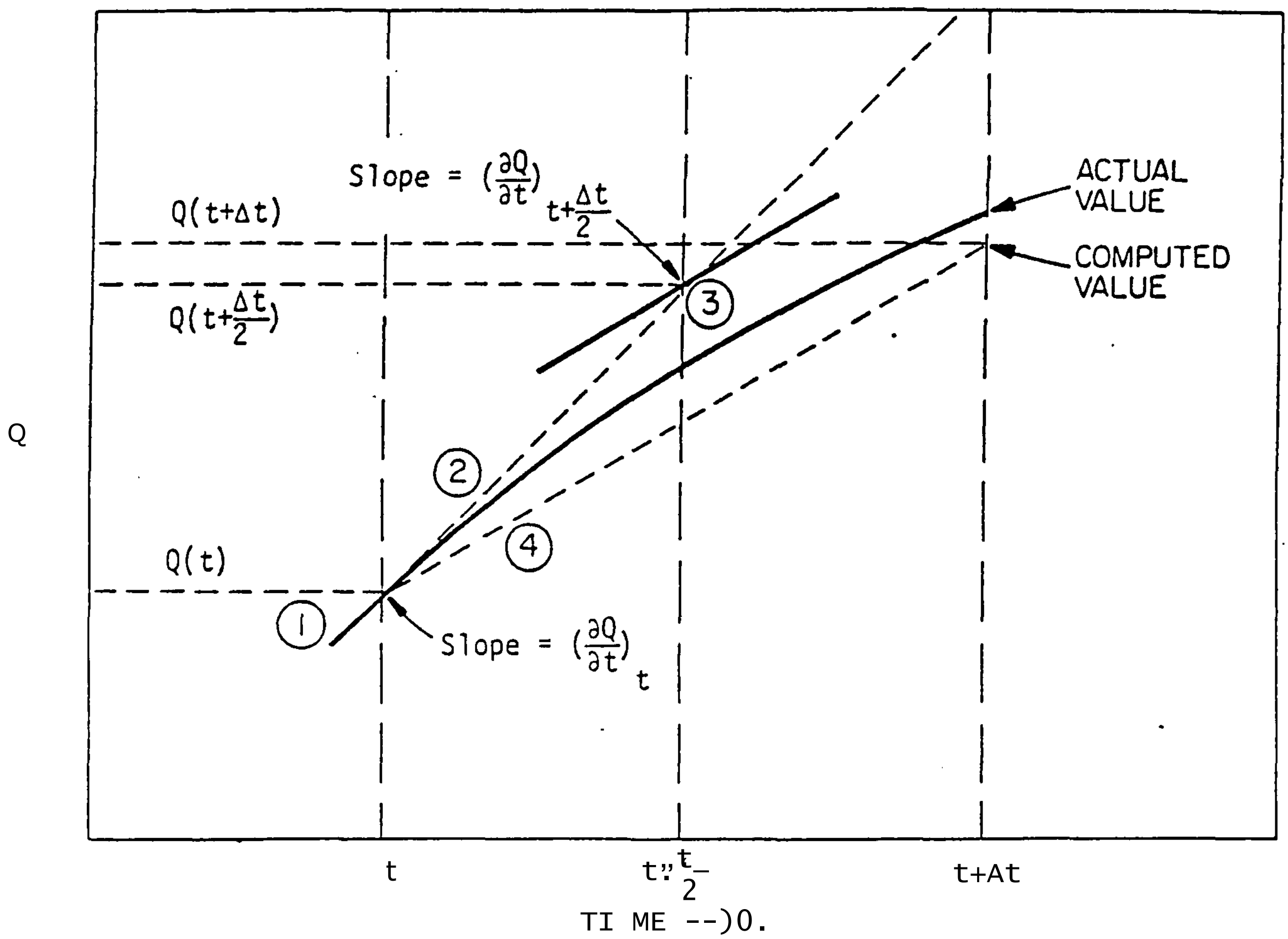
$$H_j(t + \frac{At}{2}) = H_j(t) + \frac{At}{2}[2E\,[Q(t) \quad Q(t \quad 1)] \;\; z\; Q(t + \frac{At}{2})]/AS_j(t) \qquad (15)$$

conduits diversions
surface runoff pumps
outfalls

Full-step at node j: Time t + Lt

$$H_j(t + At) = H_j(t) + Lt\,\frac{1}{2}\,E[Q(t) + Q(t + At)] + E\;Q(t + At)]/AS_j(t) \qquad (16)$$

conduits diversions
surface runoff pumps
outfalls



(f) Compute (at) from properties of system at time t
t

(.2D Project Q(t4L) as Q(t+S-'-) = Q(t) (N7) 41–
t

® a Compute system properties at t+ At

b. Form (29) At from properties of system at
time t+--
DL 2 2

(i) Project Q(t+At) as Q(t+At) = Q(t) +) t+ot t+ At
2

Figure 17. Modified Euler solution method for discharge based on half-step, full-step projection.

Note that the half-step computation of head uses the half-step computation of discharge in all connecting conduits. Similarly, the full-step computation requires the full-step discharge at time t + At for all connecting pipes. In addition, the inflows to and diversions from each node by weirs, orifices, and pumps must be computed at each half and full-step. The total sequence of discharge computations in the links and head computations in the nodes can be summarized as:

1. Compute half-step discharge at t + At/2 in all links based on preceding full-step values of head at connecting junctions.

2. Compute half-step flow transfers by weirs, orifices, and pumps at time t + at/2 based on preceding full-step values of head at transfer junction.

3. Compute half-step head at all nodes at time t + At/2 based on average of preceding full-step and current half-step discharges in all connecting conduits, plus flow transfers at the current half-step.

4. Compute full-step discharge in all links at time t + At based on half-step heads at all connecting nodes.

5. Compute full-step flow transfers between nodes at time t + At based on current half-step heads at all weir, orifice, and pump nodes.

6. Compute full-step head at time t + ,At for all nodes based on average of preceding full-step and current full-step discharges, plus flow transfers at the current full-step.

NUMERICAL STABILITY

The modified Euler method yields a completely explicit solution in which the motion equation is applied to discharge in each link and the continuity equation to head at each node entirely without implicit coupling. It is well known that explicit methods involve fairly simple arithmetic and require little storage space compared to implicit methods. However, they are generally less stable and often require very short time-steps. From a practical standpoint, experience with EXTRAN has indicated that the program is stable numerically when the following inequalities are met:

<u>Conduits:</u>

$$At < \frac{\quad}{-\ 41Y^{-}} \qquad (17)$$

where L is the pipe length in feet, g is gravity (ft/sec), D is the pipe depth, and At the time step in seconds.

<u>Nodes:</u>

$$t \underset{-}{<} \frac{C' A_s H_{max}}{EQ} \qquad (18)$$

where C' is a dimensionless constant determined by experience to be approximately 0.10, H_{max} is the maximum water-surface rise in time-step At, A_s is the corresponding surface area of the node, and EQ is the net inflow to the junction.

Examination of inequalities 17 and 18 reveals that the maximum allowable time, At, will be determined by the shortest, smallest pipe having high inflows. Based on past experience with EXTRAN, a time-step of 10 seconds is nearly always sufficiently small to produce outflow hydrographs and state-time traces which are free from spurious oscillation and also satisfy mass continuity under non-flooding conditions. In most applications, 15 to 30 second time-steps are adequate; occasionally time steps up to 60 seconds can be used.

Equivalent Pipes

An equivalent pipe is the computational substitution of an actual element of the drainage system by an imaginary conduit which is hydraulically identical to the element it replaces. Usually, an equivalent pipe is used when it is suspected that a numerical instability will be caused by the element of the drainage system being replaced in the computation. Short conduits and weirs are known at times to cause stability problems and thus occasionally need to be replaced by an equivalent pipe. (Orifices are automatically converted to equivalent pipes by the program; see the description below.)

The equivalent pipe substitution used by EXTRAN involves the following steps. First the flow equation for the element in question is set equal to the flow equation for an "equivalent pipe". This, in effect, says that the head losses in the element and its equivalent pipe are the same. The length of the equivalent pipe is computed using the numerical stability equation 17. Then, after making any additional assumptions which may be required about the equivalent pipe's dimensions, a Manning's n is computed based on the equal head loss requirement. In the case of orifices, this conversion occurs internally in EXTRAN, but in those cases where short pipes and weirs are found to cause instabilities, the user must make the necessary conversion and revise the input data set. Chapter 2 of this report outlines the steps needed to make these conversions.

SPECIAL PIPE FLOW CONSIDERATIONS

The solution technique discussed in the preceding paragraphs cannot be applied without modification to every conduit for the following reasons. First, the invert elevations of pipes which join at a node may be different since sewers are frequently built with invert discontinuities. Second, critical depth may occur in the conduit and thereby restrict the discharge. Third, normal depth may control. Finally, the pipe may be dry. In all of these cases, or combinations thereof, the flow must be computed by special techniques. Figure 18 shows each of the possibilities and describes the way in which surface area is assigned to the nodes. The options are:

1. Normal case. Flow computed from motion equation.
   Half of surface area assigned to each node.

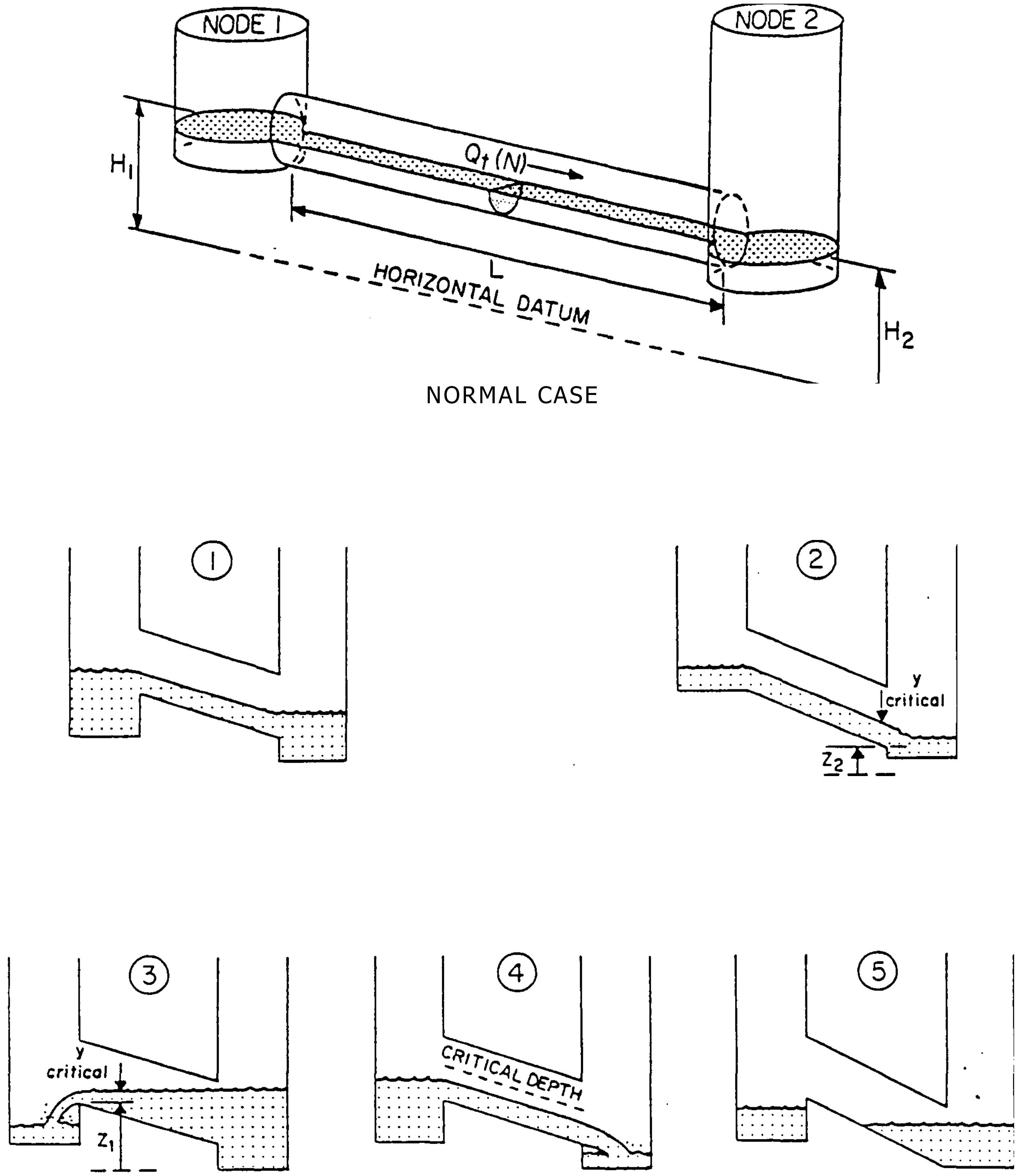


Figure 18. Special hydraulic cases in EXTRAN flow calculations.

2. **Critical depth downstream. Use lesser of critical or normal depth downstream. Assign all surface area to upstream node.**

3. **Critical depth upstream. Use critical depth. Assign all surface area to downstream node.**

4. **Flow computed exceeds flow** at critical **depth. Set flow to normal value. Assign surface area in usual manner as in (1).**

5. **Dry pipe. Set flow to zero. If any surface area exists, assign to downstream node.**

Once these depth and surface area corrections are applied, the computations of head and discharge can proceed in the normal way for the current time-step. Note that any of these special situations may begin and end at various times and places during simulation. EXTRAN detects these automatically.

HEAD COMPUTATION DURING SURCHARGE AND FLOODING

Another hydraulic situation which requires special treatment is the occurrence of surcharge and flooding. Surcharge occurs when all pipes entering a node are full or when the water surface at the node lies between the crown of the highest entering pipe and the ground surface.

Flooding is a special case of surcharge which takes place when the hydraulic grade line breaks the ground surface and water is lost from the sewer node to the overlying surface system. While it would be possible to track the water lost to flooding by surface routing, this is not done in the present version of EXTRAN.

During surcharge, the head calculation in equations 15 and 16 is no longer possible because the surface area of the surcharged node is zero. Thus, the continuity equation for node j at time t is

$$zQ(t) = 0 \qquad (19)$$

where EQ(t) is all inflows to and outflows from the node from surface runoff, conduits, diversion structures, pump, and outfalls.

Since the flow and continuity are not solved simultaneously in the model, the flows computed in the links connected to node j will not satisfy equation 19. However, computing aQ/aHj for each link conected to node j, a head adjustment can be computed such **that the continuity equation is satis fied. Rewriting equation** 19 in terms of the adjusted head gives:

$$E(Q(t) \underline{+aQHt)} AH_i(t)) = 0 \qquad (20)$$

which can be solved for Alij as

$$AH.(t) = -O(t)/E \ {}^{2}H_{a.} iLL \qquad (21)$$

This adjustment is made by half-steps during surcharge so that the half-step correction is given as:

$$_{Hj} (t + {}^{1}4L) \quad _{j} H(t) + kaH.(t + {}^{1\cdot}4 \tag{22}$$

where H.(t + At/2) is given by equation 21, while the full-step head is computed as:

$$H_{i} (t + at) = H.(t + -^{41}) + k\ aH.(t) \tag{23}$$

where AH.(t) is described by equation 21. The value of the constant k theoretically should be 1.0. However, it has been found that equation 22 tends to overcorrect the head; therefore, a value of 0.5 is used for k in the half-step computation which gives much better results. It has also been found that oscillations are triggered at upstream terminal junctions when these values of k are used. Therefore, to eliminate the oscillations, values of 0.3 and 0.6 are automatically set for k in the half-step and full-step computations, respectively, at these nodes.

Use of 30(t)/31-li, as mentioned above, satisfies continuity. Unfortunately, though, the explicit solution technique cannot meet the physical constraint of the inflows to surcharged areas of the system equaling the outflows. Because of this unmet constraint, the surcharge heads fall below their actual physical values. In order to boost these heads to their expected values, the full-step computations of flow and head in surcharge areas are repeated in an iteration loop. The iterations for a particular time-step continue until one of the following two conditions is met:

1. The net difference of inflows to and outflows from all nodes in surcharge in less than a tolerance, computed every time-step as a fraction of the average flow through the surcharge area. The fraction is input by the user.

2. The number of iterations exceeds a maximum set by the user.

The iteration loop has been found to produce accurate results with little continuity error. The user may need to experiment somewhat with the user input values in order to accurately simulate all surcharge points without incurring an unreasonably high computer cost due to extra iterations.

For various types of links connected to a node, aQ/aH is computed as follows:

<u>Conduits</u>

$$\frac{}{Q_{ai}(t)} - 3\ {}_{1\cdot7}(i^{2}{}_{T} \qquad {}_{A}t \tag{24}$$

where

K(t) ..6,t 32.2 n24/3 Iv(t)I

2 . 2 0 8 R

At = time interval
A(t) = flow cross sectional area in the conduit
L = conduit length
n = Manning n
R = hydraulic radius for the full conduit
V(t) = velocity in the conduit

System Inflows

$$\frac{aQ(t)}{aH.} - 0 \qquad (25)$$

Orifice, Weir, Pump, or Outfall Diversions

Orifices are converted to equivalent pipes (see below); therefore, equation 24 is used to compute aQ/aH. For weirs, aQ/aH in the weir link is taken as zero, i.e., the effect of the flow changes over the weir due to a change in head is ignored in adjusting the head at surcharged weir junctions. (The weir flow, of course, is computed in the next time-step on the basis of the adjusted head.) As a result, the solution may go unstable under surcharge conditions. If this occurs, the weir should be changed to an equivalent pipe as described in Chapter 2, under Card Group 12.

aQ/aH for pump junctions is also taken as zero. For off-line pumps (with a wet well), this is a valid statement since Q_{pump} is determined by the volume in the wet well, not the head at the junction. For in-line pumps, where the pump rate is determined by the water depth at the junction, a problem could occur if the pumping rate is not set at its maximum value at a depth less than surcharge depth at the junction. This situation should be avoided, if possible, because it could cause the solution to go unstable if a large step increase or decrease in pumping rate occurs while the pump junction is surcharged.

For all outfall pipe, the head adjustment at the outfall is treated as any other junction. Outfall weir junctions are treated the same as internal weir junctions (aQ/aH for the weir link is taken as zero). Thus, unstable solutions can occur at these junctions also under surcharge conditions. Converting these weirs to equivalent pipes will eliminate the stability problem.

Because the head adjustments computed in equations 22 and 23 are approximations, the computed head has a tendency to "bounce" up and down when the conduit first surcharges. This bounding can cause the solution to go unstable in some cases; therefore, a transition function is used to smooth the changeover from head computations by equations 15 and 16 to equation 22 and 23. The transition function used is:

$$\frac{a\ Q\ (\ t\ )}{DENOM} \quad A\ H\ .\ (\ t\ ) \quad - \qquad (26)$$

where

DENOM is given by

$$DEMON = \frac{\partial Q(t)}{\partial H_j} + \left(AS_j(t) - \frac{\partial Q(t)}{\partial H}\right) \exp\left(\frac{-15(Y_i - D_i)}{D_i}\right) \qquad (27)$$

and

AS = the nodal surface area at 0.96 full depth
D_i = pipe diameter
y_j = water depth.

The exponential function causes equation 27 to converge within two percent of equation 21 by the time the water depth is 1.25 times the full flow depth.

Finally, it is noted that when flooding of the node above the ground surface is detected, EXTRAN automatically resets the water surface at the ground elevation of the node. Water rising above this level under flooding conditions is then lost from the system and does not return to the EXTRAN in the present version of the program.

FLOW CONTROL DEVICES

The link-node computations can be extended to include devices which divert sanitary sewage out of the storm drainage system or relieve the storm load on sanitary interceptors. In EXTRAN, all diversions are assumed to take place at a node and are handled as internodal transfers. The special flow regulation devices treated by EXTRAN include: weirs (both sideflow and transverse), orifices, pumps, and outfalls. Each of these is discussed in the paragraphs below.

Storage Devices

Storage devices in-line or off-line act as flow control devices by providing for storage of excessive upstream flows thereby attenuating and lagging the wet weather flow hydrograph from the upstream area. The conceptual representations of a storage junction and a regular junction are illustrated in Figure 19. Note that the only difference is that added surface area in the amount of ASTORE is added to that of the connecting pipes. Note also that ZCROWN(J) is set at the top of storage "tank". When the hydraulic head at junction J exceeds ZCROWN(J), the junction goes into surcharge.

Orifices

The purpose of the orifice generally is to divert sanitary wastewater out of the stormwater system during dry weather periods and to restrict the entry of stormwater into the sanitary interceptors during periods of runoff. The orifice may divert the flow to another pipe, a pumping station or an off-line storage tank.

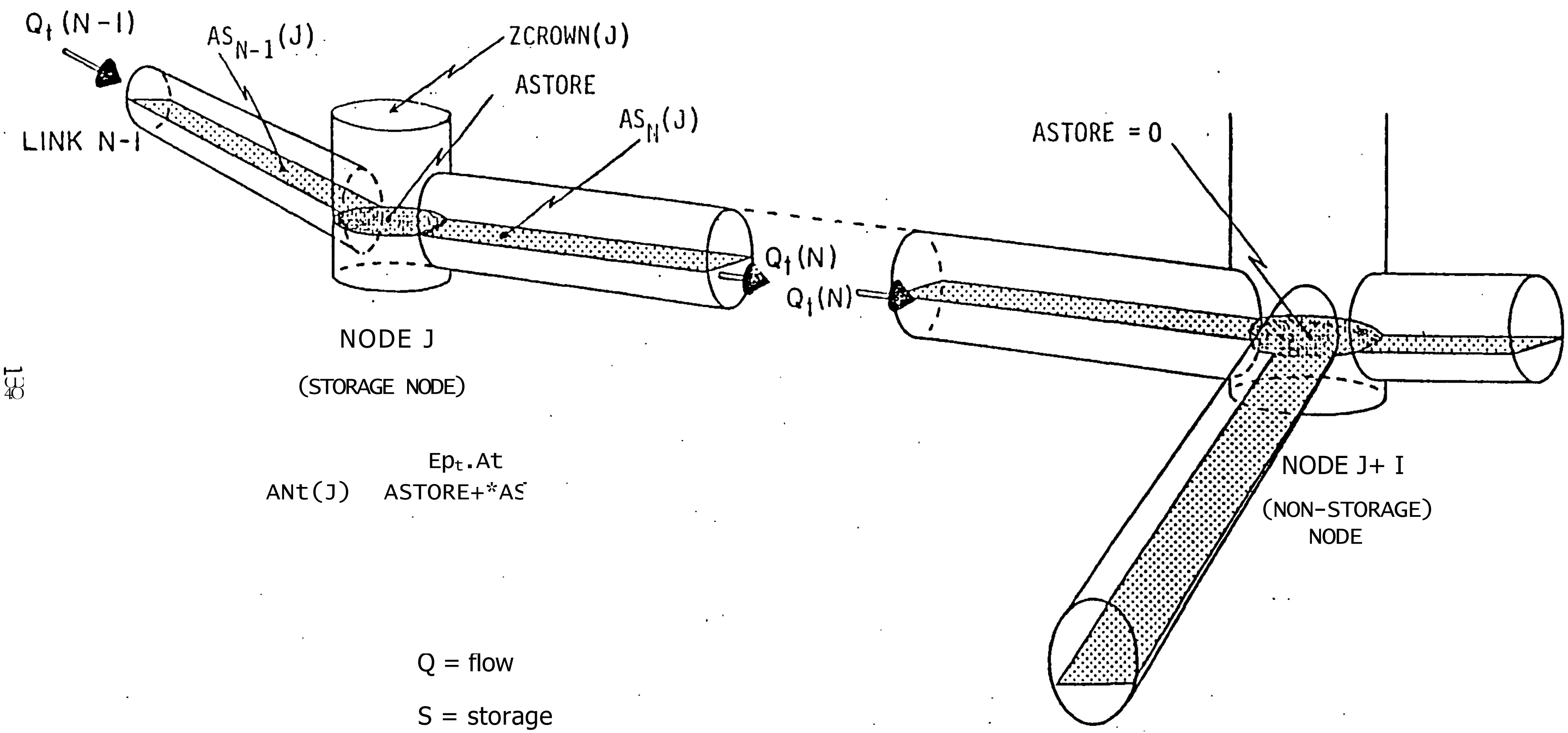


Figure 19. Conc'eptual representation of a storage junction.

Figure 20 shows two typical diversions: 1) a dropout or sump orifice, and 2) a side outlet orifice. EXTRAN simulates both types of orifice by converting the orifice to an equivalent pipe. The conversion is made as follows. The standard orifice equation is:

$$Q_o = C_o \ A/ZTT^{-} \quad (28)$$

where C_o is the discharge coefficient (a function of the type of opening and the length of the orifice tube), A is the cross-sectional area of the orifice, g is gravity, and h is the hydraulic head on the orifice. Values of C_o and A are specified by the user. To convert the orifice to a pipe, the program equates the orifice discharge equation and the Manning pipe flow equation, i.e.,

$$1.49 \ AR^{2/3} S^{1/2} = C_o \qquad (30)$$

The orifice pipe is assumed to he nearly flat, the invert on the discharge side being set 0.01 feet lower than the invert on the inlet side. In addition, for a sump orifice, the pipe invert is set by the program 0.960 <u>below</u> the junction invert so that the orifice pipe is flowing full before any outflow from the junction occurs in any other pipe. For side outlet orifices, the user specifies the height of the orifice <u>invert</u> above the junction floor.

If we write S as H_s/L where L is the pipe length, H_s will be identically equal to h when the orifice is submerged. When it is not submerged, h will be the height of the water surface above the orifice centerline while H_s will he the distance of the water surface above critical depth (which will occur at the discharge end) for the pipe. For practical purposes, we can assume that H_s = h for this case also. Thus, letting s = h/L and substituting R = D/4 (where D is the orifice diameter) into equation 29 and simplifying, we have:

$$\frac{1.49}{n} = \frac{\left(\frac{D}{4}\right)^{2/3}}{C_o} \qquad (30)$$

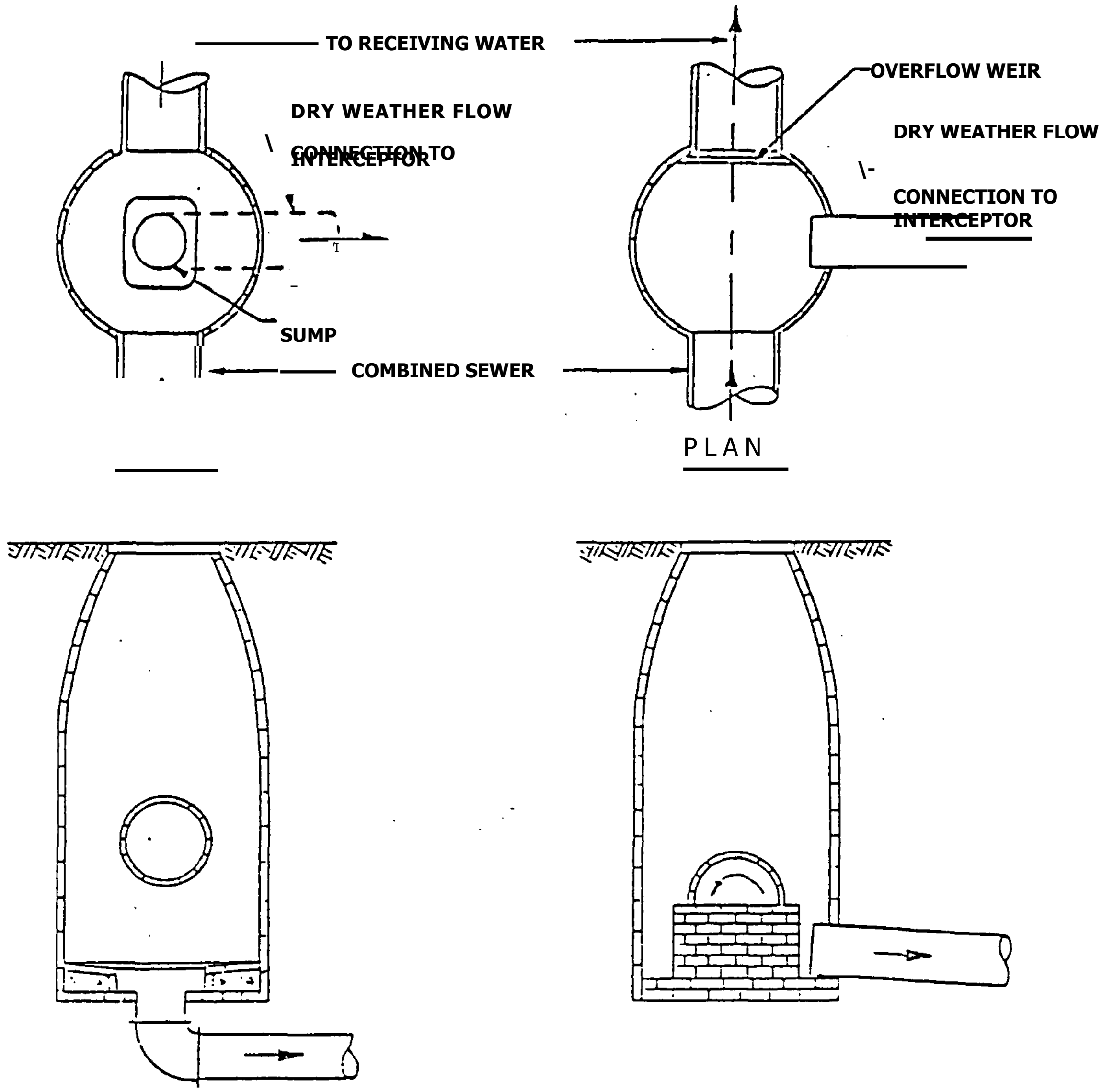
The length of the equivalent pipe is computed as the maximum of 200 feet or

$$L = 26t/515^{-} \qquad (31)$$

to insure that the celerity (stability) criteria for the pipe is not violated. n is then computed according to equation 30. This algorithm produces a solution to the orifice diversion that is not only as accurate as the orifice equation but also much more stable when the orifice junction is surcharged.

<u>Weirs</u>

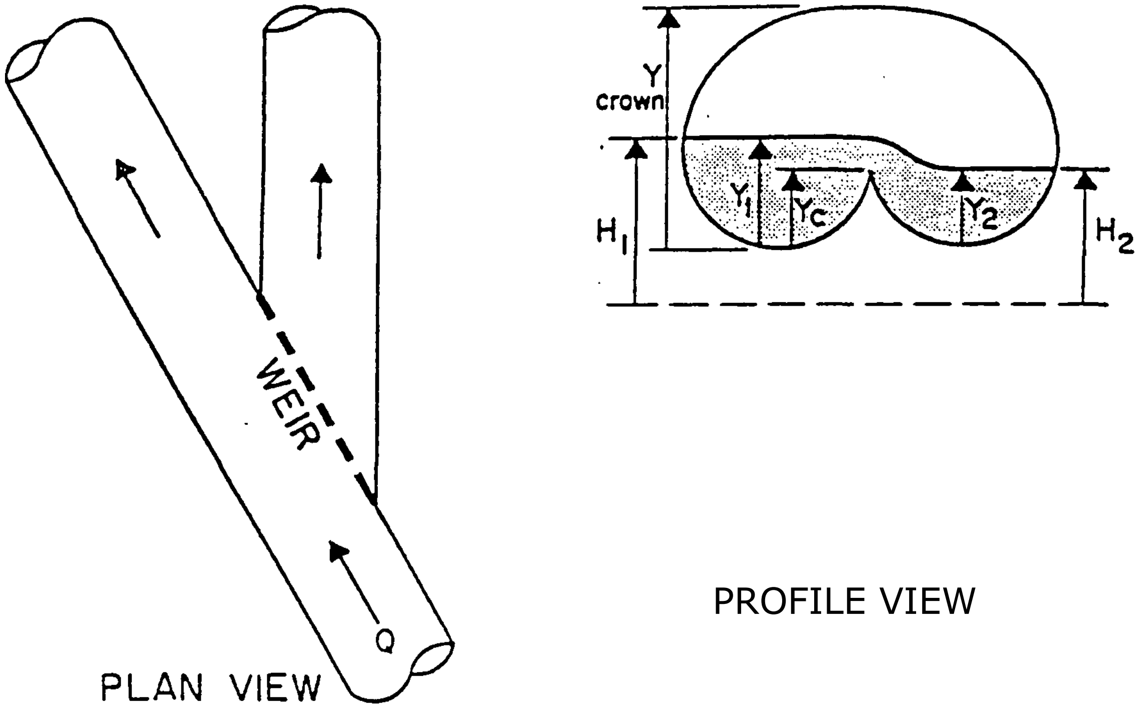
A schematic illustration of flow transfer by weir diversion between two nodes is shown in Figure 21. Weir diversions provide relief to the sanitary



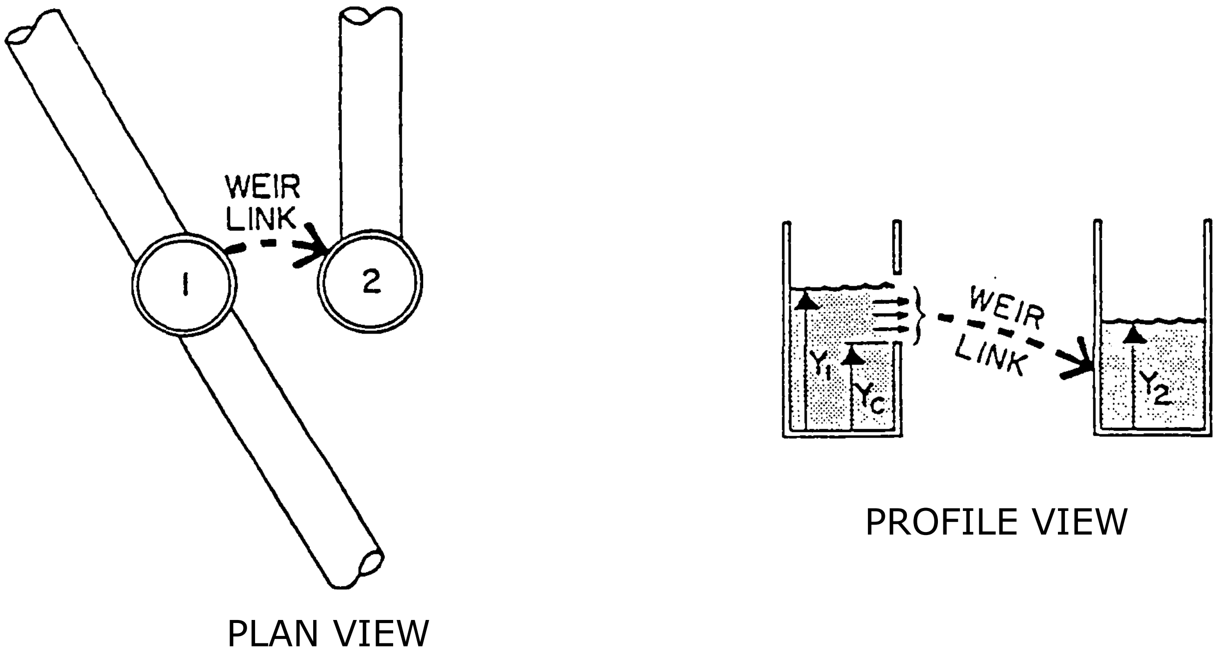
SECTION SECTION

SUMP WITH HIGH OUTLET WEIR WITH SIDE OUTLET ORIFICE

Figure 20. Typical orifice diversions.



Schematic of a Weir Diversion



Conceptual Representation of a Weir Diversion

Figure 21. Representation of weir diversions.

system during periods of storm runoff. Flow over a weir is computed by the equation:

$$Q_w = C_w L_w \left[\left(h + \frac{v^2}{2g}\right)^a - \left(\frac{v^2}{2g}\right)^a \right] \qquad (32)$$

where

C_w = discharge coefficient
L_w = weir length
h = driving head on the weir
v = approach velocity
a = weir exponent; 3/2 for transverse weirs,
  5/3 for sideflow weirs

Both C_w and L_w are input values for transverse weirs. For sideflow weirs, C_w should be a function of the approach velocity, but the present version of the program does not provide for this because of the difficulty in defining the approach velocity. For this same reason, v, which is programmed into the weir solution, is set to zero prior to computing Q_w.

Normally, the driving head on the weir is computed as the difference $h = Y1-Y_c$, where Y1 is the water depth on the upstream side of the weir and Y_c is the height of the weir crest above the node invert. However, if the downstream depth Y2 also exceeds the weir crest height, the weir is submerged and the flow is computed by equation 33.

$$Q_W = C_{SUB}\, C_W L_W (Y1 - Y_C)^{3/2} \qquad (33)$$

where CSUB is a submergence coefficient representing the reduction in driving head and all other variables are as defined above.

The submergence coefficient, CSUB, is taken from Roessert's Handbook of Hydraulics by interpolation from Table 18, where C_{RATIO} is defined as:

$$C_{RATIO} = \frac{Y_2 - Y_c}{Y_1 - Y_c} \qquad (35)$$

and all other variables are as previously defined.

The values of C_{RATIO} and CSUB are computed automatically by EXTRAN and no input data values are needed.

If the weir is surcharged it will behave as an orifice and the flow is computed as:

$$Q_w = CSUR\ L_W\ (YTOP - Y_c) \qquad (35)$$

TABLE 18

VALUES OF Csug AS A FUNCTION OF DEGREE OF WEIR SUBMERGENCE

CRATIO	CSUB
0.00	1.00
0.10	0.99
0.20	0.98
0.30	0.97
0.40	0.96
0.50	0.95
0.60	0.94
0.70	0.91
0.80	0.85
0.85	0.80
0.90	0.68
0.95	0.40
1.00	0.00

where

YTOP = distance to top of weir opening shown in Figure 6

h' = Yl - maximum (Y2, Y_c)

CSUR   weir surcharge coefficient

The weir surcharge coefficient, CSUR, is computed automatically at the beginning of surcharge. At the point where weir surcharge is detected, the preceding weir discharge just prior to surcharge is equated to Q_w in equation 34 and equation 35 is then solved for the surcharge coefficient, CSUR. Thus, no input coefficient for surcharged weirs are required.

Finally, the present version of EXTRAN detects flow reversals at weir nodes which causes the downstream water depth, Y2, to exceed the upstream depth, Yl. All equations in the weir section remain the same except that Y1 and Y2 are switched so that Yl remains as the "upstream" head. Also, flow reversal of a sideflow weir causes it to behave more like a transverse weir and consequently the exponent as in equation 32 is set to 1.5

Weirs With Tide Gates

Frequently, weirs are installed together with a tide gate at points of overflow into the receiving waters. Flow across the weir is restricted by the tide gate, which may be partially closed at times. This is accounted for by reducing the effective driving head across the weir according to an empirical factor published by Armco(3):

$$h' = h - 4_{v}2 \quad \underline{-1.15v} \qquad (36)$$
$$_{g}\ {}_{exp}(V-F--$$

where h is the previously computed head before correction for flap gate and v is the velocity of flow in the upstream conduit.

PUMP STATIONS

A pump station is conceptually represented as either an in-line lift station, or an off-line node representing a wet-well, from which the contents are pumped to another node in the system according to a progammed rule curve. For an in-line lift station, the pump rate is based on the water depth at the pump junction. The rule is as follows:

$$\begin{aligned} \text{Pump Rate} &= R1 \text{ for } 0 < Y < Yl \\ &= R2 \qquad Yl < Y < Y2 \\ &= R3 \qquad\qquad Y < Y_3 \end{aligned}$$

For Y = 0, the pump rate is the inflow rate to the pump junction.

Inflows to the off-line pump must be diverted from the main sewer system through an orifice, a weir, or a pipe. The influent to the wet-well node must be a free discharge regardless of the diversion structure. The pumping rule curve is based on the volume of water in the storage junction. A schematic presentation of the pump rule is shown in Figure 22. The rule operates as follows:

1. Up to three wet-well volumes are prespecified as input data for each pump station: $V_1 < V_2 < V_3$, where V_3 is the maximum capacity of the wet well.

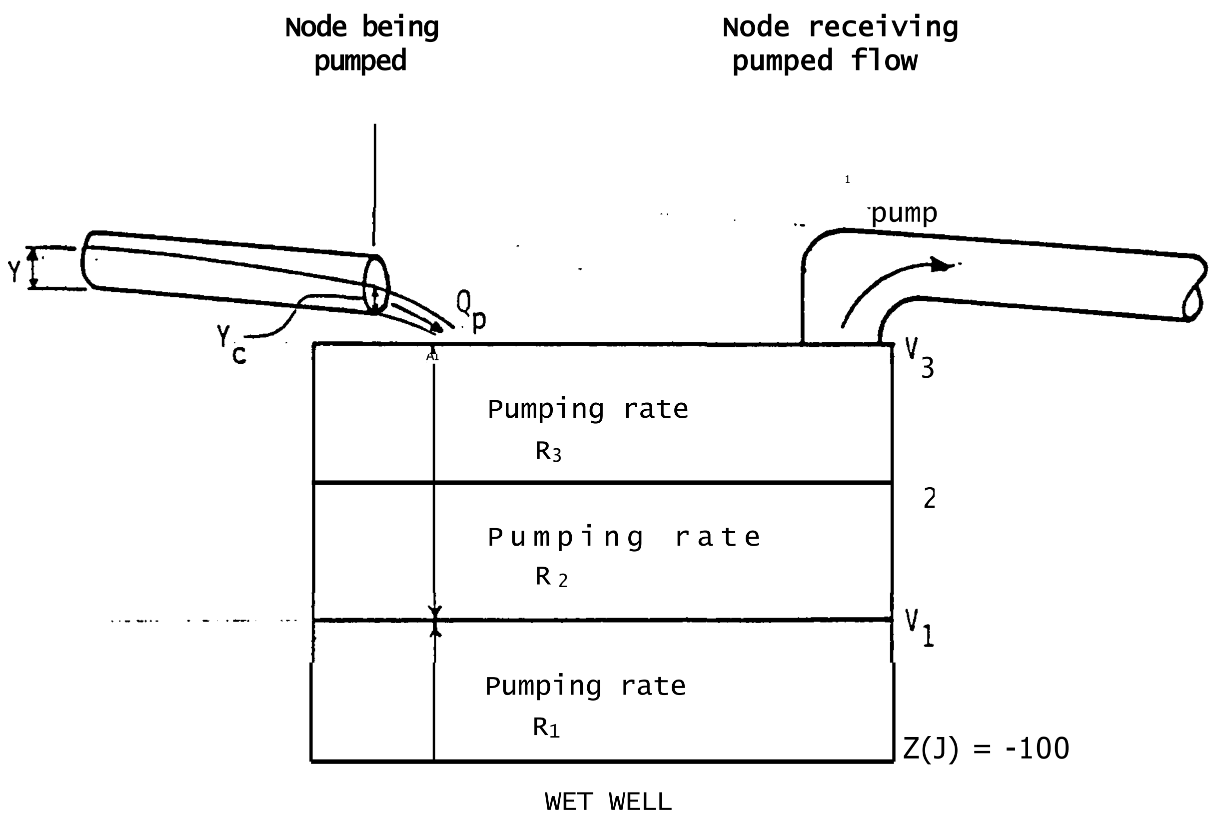
2 Three pumping rates are prespecified as input data for each station. The pump rate is selected automatically by EXTRAN depending on the volume in the wet-well, as follows:

   R1 for the volume in wet-well < V1
   R_2 for V_1 < volume in wet-well < V_2
   R3 for V2 < volume in wet-well < V_3

3. A mass balance of pumped outflow and inflow is performed in the wet-well during the model simulation period.

4. If the wet-well goes dry, the pump rate is reduced below rate R1 until it just equals the inflow rate. When the inflow rate again equals or exceeds R1, the pumping rate goes back to operating on the rule curve.

5. If V3 is exceeded in the wet-well, the inflow to the storage node is reduced until it does not exceed the maximum pumped flow. When the inflow falls below the maximum pumped flow, the inflow "gates" are opened. The program automatically steps down the pumping rate by the operating rule of (2) as inflows and wet-well volume decrease.



$$\text{Pumping rate} = R_1 \text{ for } V < V_1$$
$$= R_2 \text{ for } V < V < V_2$$
$$= R_3 \text{ for } V < V < V_3$$

V is volume in wet well

Figure 22. Schematic Presentation of Pump Diversion.

OUTFALL STRUCTURES

EXTRAN simulates both weir outfalls and free outfalls. Either type may be protected by a tide gate. A weir outfall is a weir which discharges directly to the receiving waters according to relationships given previously in the weir section. The free outfall is simply an outfall conduit which discharges to a receiving water body under given backwater conditons. The free outfall may be truly "free" if the elevation of the receiving waters is low enough, or it may consist of a backwater condition. In the former case, the water surface at the free outfall is taken as critical or normal depth, whichever is less. If backwater exists, the receiving water elevation is taken as the water surface elevation at the free outfall.

When there is a tide gate on an outfall conduit, a check is made to see whether or not the hydraulic head at the upstream end of the outfall pipe exceeds that outside the gate. If it does not, the discharge through the outfall is equated to zero. If the driving head is positive, the water surface elevation at the outfall junction is set in the same manner as that for a free outfall subjected to a backwater condition.

CHAPTER 6

PROGRAM STRUCTURE OF EXTRAN

GENERAL

The EXTRAN Block is a set of computer subroutines which are organized to simulate the unsteady, gradually-varied movement of stormwater in a sewer network composed of conduits, pipe junctions, diversion structures, and free outfalls. A program flowchart for the major computation steps in the EXTRAN Block is presented in Figure 23. A full listing of the program, together with key variable definitions, is contained in Table A-3, Appendix A.

The EXTRAN Block contains 13 subroutines in addition to the main program which controls execution. The organization of each subroutine and its relation to the main program has been diagrammed in the master flowchart of Figure 24. A description of each subroutine follows in the paragraphs below.

SUBROUTINE EXTRAN

EXTRAN is the executive subroutine of the EXTRAN Block. It sets the unit numbers of the device containing the input data and the device where printed output will be directed. The device numbers of the input and output hydrograph tapes, if used, are also set here. Then the first two lines of the input hydrograph tape, if required, are read and this information is written on the output hydrograph tape, if used. Finally, subroutine TRANSX is called to perform the computations of the EXTRAN Block.

Presently, subroutine EXTRAN is set up to run the EXTRAN Block independently of the SWMM model. It can easily be changed to operate within SWMM by:

1. Removing the comment marks (C/////) from the first line of the program, leaving SUBROUTINE EXTRAN;

2. Changing the first executable line of the program from ISKIP=1 to ISKIP=0; and

3. Removing the comment marks (C/////) from the RETURN statement at the end of the subroutine.

SUBROUTINE TRANSX

TRANSX is the main controlling subprogram of the EXTRAN Block which drives all other subprograms and effectively controls the execution of EXTRAN as it has been presented graphically in the flowchart of Figure 23.

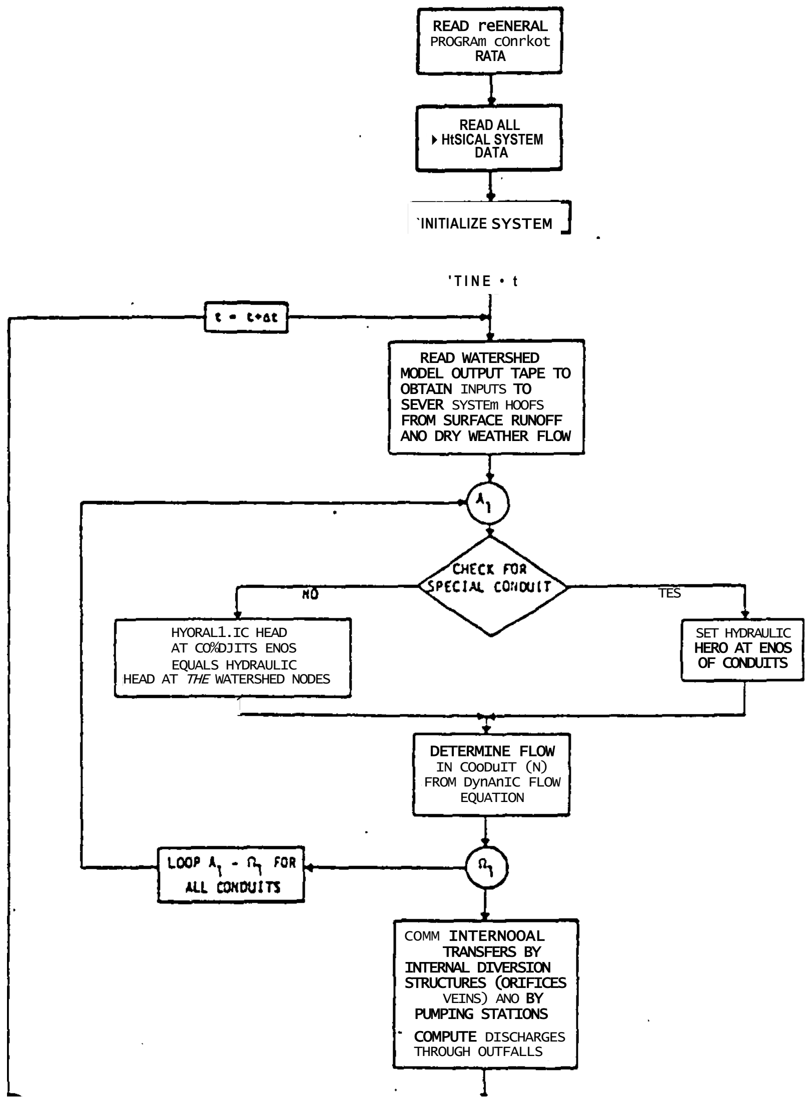


Figure 23. EXTRAN Block program flowchart

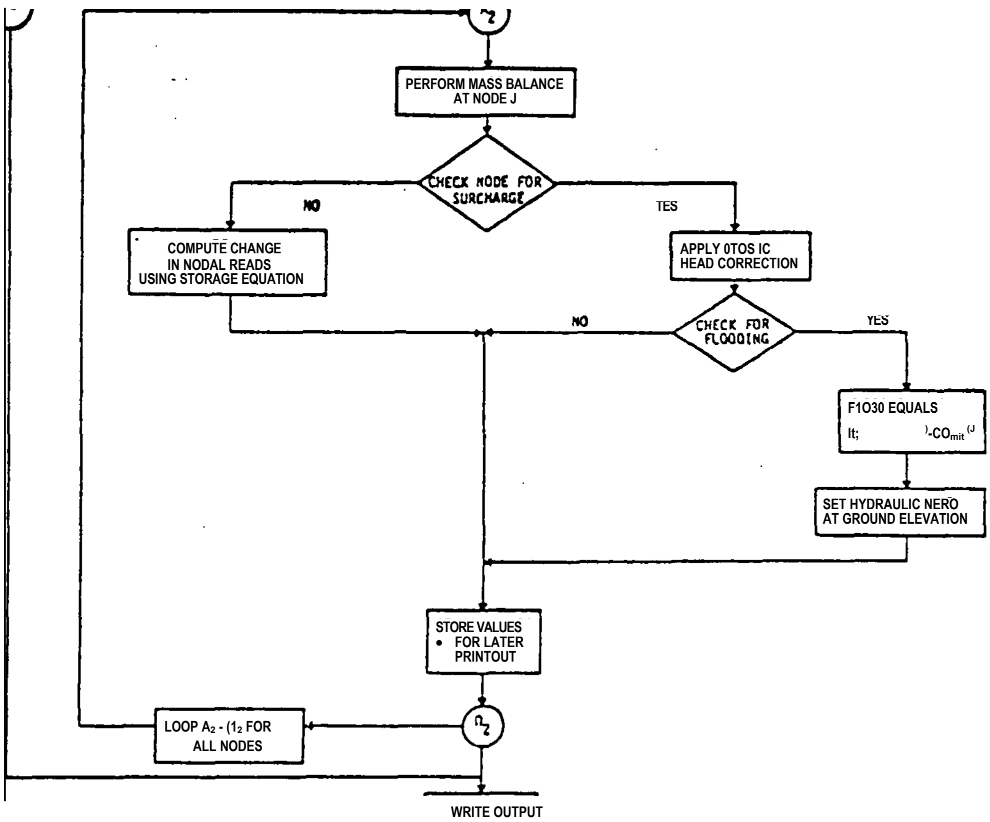


Figure 23. EXTRAN Block program **flowchart (Continued)**

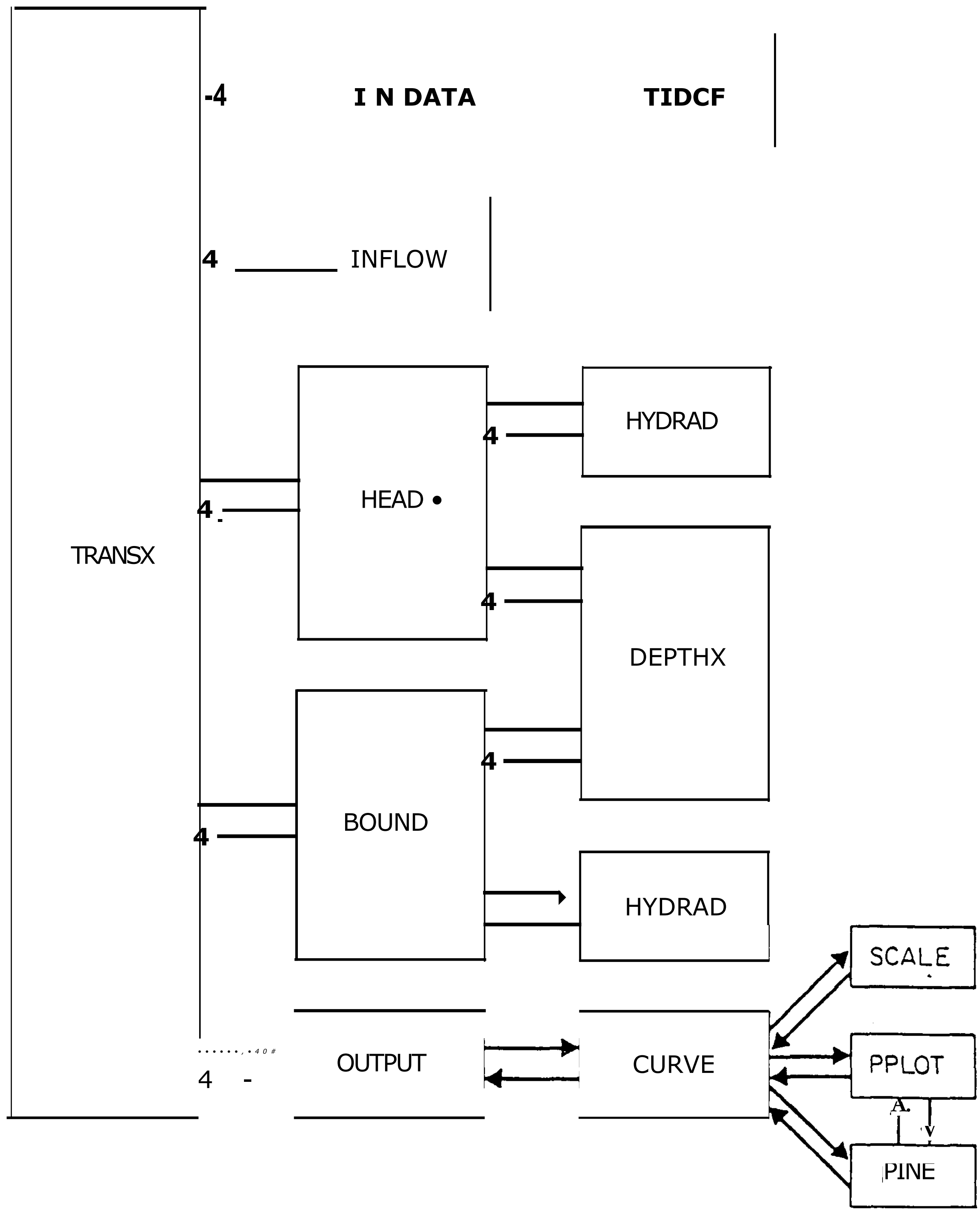


Figure 24. Master flowchart for the EXTRAN block.

Principal steps in TRANSX are outlined below in the order of their execution:

1. Call INDATA for reading all input data cards defining the length of the transport simulation run, the physical data for the transport system, and the instruction for output processing.

2. Initialize system flow properties and set time = TZERO.

3. Advance time = ti-Lt and begin main computation loop contained in steps 4 through 10 below.

4. Select current value of inflow hydrographs for all input nodes by call to INFLOW, which interpolates runoff hydrograph records either on tape unit N21 supplied by the RUNOFF Block or on data cards.

5. For all physical conduits in the system, compute the following time-changing properties based on the last full-step values of depth and flow:

   - Hydraulic head at each pipe end;

   - Full-step values of cross-sectional area, velocity, hydraulic radius, and surface area corresponding to preceding full-step flow. This is done by calling subroutine HEAD;

   - Half-step value of discharge at time t=t+At/2 by modified Euler solution;

   - Check for normal flow, if appropriate;

   - Set system outflows and internal transfers at time t+tt/2 by call to subroutine BOUND. BOUND computes the half-step flow transfers at all orifices, weirs, and pumps at time t=t+At/2. It also computes the current value of tidal stage and the half-step value of depth and discharge at all outfalls.

   - Average flow in all pipes connected to junctions in surcharge. A fraction of this value is used as the tolerance of the surcharge iteration loop.

6. For all physical junctions in the system, compute the half-step depth at time t=t+At/2. This depth computation is based on the current net inflows to each node and the nodal surface areas computed previously in step 5. Check for surcharge and flooding at each node and compute water depth accordingly.

7. For all physical conduits, compute the following properties based on the last half-step values of depth and flow (repeat step 5 for time t+At/2):

   - Hydraulic head at each pipe end;

   - Half-step values of pipe cross-sectional area, velocity, hydraulic radius, and surface area corresponding to preceding half-step depth and discharge;

   - Full-step discharge at time t+At by modified Euler solution;

   - Check for normal flow if appropriate;

   - Set system outflows and internal transfer at time t+At by calling BOUND.

8. For all junctions, repeat the nodal head computation of step 6 for time t+At. Sum the differences between inflow and outflow for each junction in surcharge.

9. Repeat steps *7* and 8 for surcharged links and nodes until the sum of the flow differences from step 8 is less than the tolerance from step 5 or a maximum number of iterations is exceeded.

10. Store nodal water depths and water surface in junction print arrays to be used later by OUTPUT. Also, store conduit discharges and velocities for later printing. Print intermediate output.

11. Return to step 3 and repeat through step 10 until the transport simulation is complete for the entire period.

12. Call subroutine OUTPUT for printing and plotting of conduit flows and junction water surface elevations.

SUBROUTINE BOUND

The function of subroutine BOUND is to compute the half-step *and* full-step flow transfers by orifices, weirs, and pump stations. BOUND also computes the current level of receiving water backwater and determines discharge through system outfalls. A summary of principal calculations follows:

1. Compute current elevation of receiving water backwater. Depending on the tidal index, the backwater condition will be constant, tidal or below the system outtails (effectively non-existent). The tidally varied backwater condition is computed by a t.ourier series about a mean time equal to the first coefficient, Al.

2. Compute depth at orifice junction for sump orifice flowing less than full.

3. Compute discharge over transverse and sideflow weirs. Check for reverse flow, surcharge, and weir submergence. If weir is surcharged, compute flow by orifice-type equation. If weir is submerged, compute the submergence coefficient and re-compute weir flow. If a tide gate is present at weir node, then compute head loss, reduce driving head on weir and re-compute weir discharge.

4. Compute pump discharges based on current junction or wet-well level and corresponding pump rate. If wet-well is flooded, set pump rate at maximum level and reduce inflow.

SUBROUTINE DEPTH

Subroutine DEPTH computes the critical and normal depths corresponding to a given discharge using the critical flow and Manning uniform flow equations, respectively. Tables of normalized values for the cross-sectional area, hydraulic radius and surface width of each pipe class are taken from a Block Data element to speed the computations of critical and normal depth. Subroutine DEPTH is used by subroutines BOUND and HEAD.

SUBROUTINE HEAD

Subroutine HEAD is used to convert a nodal water depth to the depth of flow above the invert of a connecting pipe. Based on the depths of flow at each pipe end, HEAD computes the surface width and assigns surface area to the upstream and downstream node according to the following criteria:

1. For the normal situation in which both pipe inverts are submerged and the flow is sub-critical throughout the conduit, the surface area of that conduit is assigned equally to the two connecting junctions.

2. If a critical flow section is detected at the downstream end of a conduit, then surface area for that conduit is assigned to the upstream node.

3. If a critical section occurs at the upstream end, the conduit surface area is assigned to the downstream node.

4. For a dry pipe (pipe inverts unsubmerged), the surface area is zero. The velocity, cross-sectional area and hydraulic radius are set to zero for this case.

5. If the pipe is dry only at the upstream end, then all surface area for the conduit is assigned to the downstream junction.

Note that adverse flow in the absence of a critical section is treated as in (1) above.. If a critical section occurs upstream, then all surface area for the adverse pipe is assigned downstream as in (3). The assignment of nodal surface area, based on the top width and length of conduit flow, is essential to the proper calculation of head changes computed at each node from mass continuity as discussed in Chapter 5. Following surface area assignment, HEAD computes the current weighted average values of cross-sectional area, flow velocity, and hydraulic radius for each pipe. Subroutine HEAD is called by program MAIN and it in turn uses subroutines DEPTH and HYDRAD in its surface area computations.

SUBROUTINE HYDRAD

The function of subroutine HYDRAD is to compute average values of hydraulic radius, cross-sectional area, and surface width for all conduits in the transport system. Based on the current water depth at the ends and mid-point of each conduit, HYDRAD computes from a table of normalized properties the current value of hydraulic radius, cross-sectional area, and surface width. HYDRAD is used by subroutine HEAD for computing nodal surface areas as described above. It is also called by BOUND for computing the cross-sectional area and average velocity of flow in the outfall pipe protected by a tide gate.

SUBROUTINE INDATA

INDATA is the principal input data subroutine for the EXTRAN Block which is used once at the beginning of subroutine TRANSX. Its primary function is to read all input data specifying the links, nodes, and special structures of the transport network. It also establishes transport system connectivity and sets up an internal numbering system for all transport elements by which the computations in TRANSX can be carried out. The principal operations of INDATA are listed below in the order they occur in the program:

1. Read first two title cards for output headings and run control card specifying the number of integration cycles, the length of the time-step, TZERO, and other parameters for output and run control.

2. Read external junction and conduit numbers for detailed printing and plotting of simulation output.

3. Read physical data for conduits and print a summary of all conduit data.

4. Read physical data for junctions and print summary of all junction data.

5. Set up internal numbering system for junctions and conduits and establish connectivity matrix. This matrix shows the connecting nodes at the end of each conduit and conversely the connecting links for each node in the transport system.

6. Read orifice input data and print summary. Assign internal link between orifice node and node to which it discharges.

7. Read weir input data and assign an internal link and node to each weir in the system. Print summary of all weir data.

8. Read pump data and assign an internal link number to each pump node. Print summary of all pumping input data. Set invert elevation and inflow index for pumped node.

9. Read free outfall *data* and print a data summary for outfalls. Assign an internal link for each free outfall in the internal numbering system.

10. Read tide-gated (non-weir) outfall data from cards and print a summary of tide gate data. Assign an internal link for each free outfall in the internal numbering system.

11. Print a summary of internal connectivity information showing the internal nodes and connecting links assigned to orifices, weirs, pumps, and free outfalls.

12. Read tidal boundary input data. Depending on the tidal index, one of the following four boundary conditions will exist:

    - No control water surface at the system outfalls;

    - All outfall control water surfaces at the same constant elevation, A1;

    - Tide coefficient read in by cards;

    - Tide coefficients A1 through A1 will be generated by TIDCF which are printed in subroutine TIDCF.

    Print summary of tidal boundary input data, including the tide coefficients generated by TIDCF which are printed in subroutine TIDCF.

13. Set up print and plot arrays for output variables in the internal numbering system.

14. Initialize conduit conveyance factor in Manning equation. Also, read input data defining the initial conduit flows, velocities, and junction depths at TZERO corresponding to DWF or some antecedent flow condition.

16. Read in initial system information on tape unit N21 generated by the block immediately preceding the EXTRAN Block, usually the RUNOFF Block.

17. Read first two hydrograph records either from tape unit N21 or from data input cards.

18. Write out initial transport system information on tape unit N22 which will contain the hydraulic output from the EXTRAN Block supplied as input to any subsequent block.

SUBROUTINE INFLOW

Subroutine INFLOW is called from Subroutine TRANSX on each time-step to compute the current value of hydrograph inflow to each input node in the sewer system. INFLOW reads current values of hydrograph ordinates from tape unit N21 if the RUNOFF Block (or any other block) immediately precedes the EXTRAN Block, or from card input runoff hydrographs in cases where no other block is used as a pre-processor to EXTRAN. INFLOW performs a linear interpolation between hydrograph input points and computes the discharge at each input node at the half-step time, t+At/2.

SUBROUTINE TIDCF

Subroutine TIDCF is used on a one-time basis by subroutine INDATA to compute seven tide coefficients, Al through Al, which are used by subroutine BOUND to compute the current tide elevation according to the 1-ourier series:

$$\begin{aligned} HTIDE = A1 &+ A2 \sin wT + A3 \sin 2wT \\ &+ A4 \sin 3wT + A5 \cos wT \\ &) + A6 \cos 2wT + A7 \cos 3wT \end{aligned} \quad (37$$

where

T = current time in seconds

w = 2r radians/tidal period in seconds. The tidal period is 25 hours = 90,000 sec.

The coefficients A? through A7 are developed by an interactive technique in TIDCF in which a sinusoidal series is fitted to the set of tidal stage-time points supplied as input data by subroutine INDATA.

SUBROUTINE OUTPUT

Subroutine OUTPUT is called by subroutine TRANSX at the end of the simulation run to print and plot the hydraulic output arrays generated by the EXTRAN Block. Printed output includes: 1) the water depths and water surface elevations at each junction, and 2) the discharge and flow velocity in each system conduit. The plotting of junction water surface elevation and conduit discharge is carried out by a printer-plot package labelled CURYL which is called by OUTPUT after printed output is complete.

SUBROUTINES CURVE, PINE, PPLOT, SCALE

The above subroutines form a general printer-plot package which is used in the EXTRAN Block to plot water surface elevation at selected nodes and conduit discharge in selected links. Subroutine CURVE is the executive program driving the other three subroutines of this package. CURVE is called at the conclusion of transport system simulation by OUTPUT. Inclusion of these subroutines in the EXTRAN Block allows EXTRAN to stand on its own as well as function with SWMM.

ENGLISH/METRIC CONVERSION FACTORS

All references in this manual, as well as all inputs to, outputs from, and calculations in the EXTRAN Block, are in English Units. The following conversion factors will allow the user to determine the equivalent Metric unit.

1 foot = 1 square 0.3048 meters
foot = 1 cubic 0.0929 square meters
foot = 1 0.0283 cubic meters
foot/second = 1 0.3048 meters/second
cubic foot/second 0.0283 cubic meters/seconds
= 1 inch = 2.54 centimeters

REFERENCES

1. Shubinski, R. P., and L. A. Roesner. Linked Process Routing Models, paper presented at the Symposium on Models for Urban Hydrology, American Geophysical Union Meeting, Washington, D. C., 1973.

2. Kibler, D. F., J. R. Monser, and L. A. Roesner. San Francisco Stormwater Model, User's Manual and Program Documentation, prepared for the Division of Sanitary Engineering City and County of San Francisco, Water Resources Engineers, Walnut Creek, California, 1975.

3. Armco Water Control Gates, Armco Design Manual, Metal Products Division, Middletown, Ohio.

APPENDIX A

PROGRAM LISTING AND KEY VARIABLE DEFINITION

This Appendix-is comprised of three tables. The first, Table A-1, consists of the Data Input Forms for EXTRAN. Each sheet represents one card group of the input data, as outlined in Table 1 of Chapter 2.

The second table, Table A-2, is a definition of the key variables in the EXTRAN Block. They are listed in the alphabetical order of the Common Blocks in which they are contained.

Finally, Table A-3 is a program listing of EXTRAN. The Executive Subroutine EXTRAN, which also doubles as the main program of EXTRAN when it is revised to stand alone from SWMM, is printed first. Subroutine TRANSX, which serves as the main controlling program of the EXTRAN Block, is printed next, followed by all other subprograms in alphabetical order.

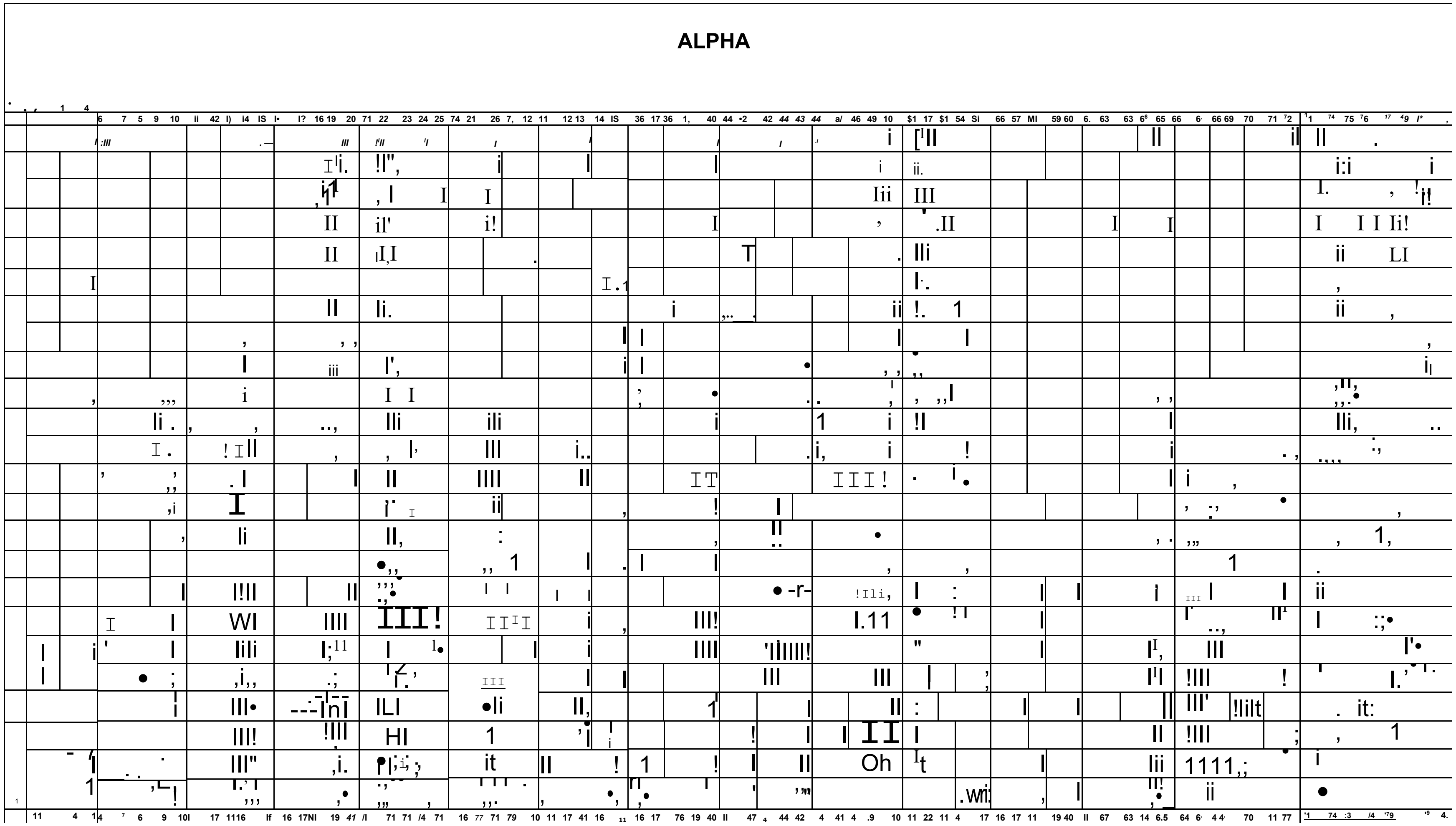
TABLE A-1 DATA
INPUT FORM
EXTRAN

Card Group 1 of 22: Input and Output Hydrograph Tape Identification
Format: 2I5

N21	N22	

NOTE: Card group 1 is only required if EXTRAN is to *be* run on its own.

Card Group 2 of 22: Title cards (2 cards)
Format: 20A4



Card Group 3 of 22: Run control card
Format: 15, 2F5.0, 815, F5.0

NTCYC	DELT	TZERO	NHPRT	NQPRT	NPLT	LPLT			NJSW	ITMAX	SURTOL	

Card Group 4 of 22: Junction print control (8 junctions per card)
Format: 8110

JPRT (1)	JPRT (2)	JPRT (3)	JPRT (4)	JPRT (5)	JPRT (6)	JPRT (7)	JPRT (8)

Card Group 5 of 22: Conduit print control (8 conduits per card)
Format: 8110

CPRT (1)	CPRT (2)	CPRT (3)	CPRT (4)	CPRT (5)	CPRT (6)	CPRT (7)	CPRT (8)

Card Group 6 of 22: Junction plot control (8 junctions per card) Format: 8110

JPLT (1)	JPLT (2)	JPLT (3)	JPLT (4)	JPLT (5)	JPLT (6)	JPLT (7)	JPLT (8)

Card Group 7 of 22: Conduit plot control (8 conduits per card)
Format: 8110

KPLT (1)	KPLT (2)	KPLT (3)	KPLT (4)	KPLT (5)	KPLT (6)	KPLT (7)	KPLT (8)

Card Group 8 of 22: Conduit cards (1 card per conduit)
Format: 4I5, 9F5.0

NCOND	NJUNC UPSTR	NJUNC DNSTR	NKLASS	AFULL SQ FT	DEEP FT	WIDE FT	LEN. FT	ZP UPSTR	ZP DNSTR	ROUGH	STHETA	SPHI

NOTE: Last card must be 99999 in columns 1-5

Card Group 9 of 22: Junction cards (1 card per junction)
Format: 15, 3F5.0

JUN	GRELEV	Z	QINST	

NOTE: Last card must be 99999 in columns 1-5

Card Group 10 of 22: Storage cards (1 card per junction)
Format: 15, 2F5.0

JSTORE	ZTOP	ASTORE ft²

NOTE: Last card must *be* 99999 in columns 1-5

Card Group 11 of 22: Orifice cards (1 card per orifice)
Format: 3I5, 3F5.0

NJUNC1	NJUNC2	NKLASS	AORIF	CORIF	ZP

NOTE: Last card must be 99999 in columns 1-5

Card Group 12 of 22: Weir cards (1 card per weir)
Format: 3I5, 4F5.0

NJUNC1	NJUNC2	KWEIR	YCREST	YTOP	WLEN	COEF

NOTE: Last card must be 99999 in columns 1-5

Card Group 13 of 22: Pump cards (1 card per pump)
Format: 3I5, 7F5.0

NJUNC	$NJUNC_2$	IPTYP	VWELL	$PRATE_1$	$PRATE_2$	$PRATE_3$	$VRATE_1$	$VRATE_2$	$VRATE_3$	

NOTE: Last card must be 99999 in columns 1-5

Card Group 14 of 22: Free outfall cards (1 card per outfall)
Format: 15

JFREE

NOTE: Last card must be 99999 in columns 1-5

Card Group 15 of 22: Outfalls with tide gates (1 card per outfall)
Format: .15

JGATE

NOTE: Last card must be 99999 in columns 1-5

Card Group 16 of 22: Tide con rol card (1 card)

Format: 15,8F5.0

Card Group 17 of 22: Tide com utation card (1 card) Format: 315

Card Gropu 18 of 22: Tide sta e card (4 points per card) Format: 8F10.0

NTIDE	A1	A2	A3	A4	A5	A6	A7	W

Card Group 19 of 22: Initial flows and velocities (4 conduits per card)
Format: 8F10.0

Q_1	V_1	Q_2	V_2	Q_3	V_3	Q_4	V_4

NOTE: If initial condition is zero flow throughout entire system, punch 99999. for Q_1.
No other cards required.

Card Group 20 of 22: Initial heads (8 junctions per card)
Format: 8F10.0

Y_1	Y_2	Y_3	Y_4	Y_5	Y_6	Y_7	Y_8

NOTE: If Q(1) is 99999 indicating zero flow, skip Card Group 19.

Card Group 21 of 22: Hydrograph input control cards (16 input nodes per card)
Format: 16I5

JSW(1)	JSW(2)	JSW(3)	JSW(4)	JSW(5)	JSW(6)	JSW(7)	JSW(8)	JSW(9)	JSW(10)	JSW(11)	JSW(12)	JSW(13)	JSW(14)	JSW(15)	JSW(16)

Card Group 22 of 22: Hydrograph inflow cards (7 flows per card at time TEO)
Format: 8F10.0

TEO	QCARD(1)	QCARD(2)	QCARD(3)	QCARD(4)	QCARD(5)	QCARD(6)	QCARD(7)

NOTE: If more than one card is required to list flows for all input nodes, QCARD(N)for all subsequent cards should begin in columns 1-10, 8 QCARD(N) per card.

TABLE A-2

DEFINITION OF KEY VARIABLES IN EXTRAN

Variable Name	Description	Unit
	COMMON/BD/	
	This common is used in the following subroutines BLOCK DATA BOUND DEPTHX HYDRAD INDATA TRANSX	
ANORM	Matrix of normalized wet cross-sectional area of of conduit, based on shape and depth	
HRNORM	Matrix of normalizd hydraulic radius of conduit, based on shape and depth	
TWNORM	Matrix of normalized conduit width at flow line, based on shape and depth	
	COMMON/BND/	
	This common is used by the following subroutines BOUND INDATA TRANSX	
JFREE	Node for free outfall	None
JGATE	Node for non-weir tide gate	None
JITDE	Not used at this time	None
NFREE	Number of free outfalls	None
NGATE	Number of non-weir tide gates	None
NTIDE	Indicator for outfall tide level control 1. No water surface at outfall 2. Outfall control water surface at constant elevation, A1 3. Tide coefficient provided 4. Program will compute tide coefficients	None

TABLE A-2

(Continued)

Variable Name	Description	
YT	Depth of water at a node at half integration step	Fe
Z	Elevation of node invert	Feet
ZCROWN	Elevation of uppermost conduit crown at a node, defined as node crown elevation	Feet

COMMON/LAB/

This common is used by the following subroutines

BLOCK DATA
CURVE
INDATA
OUTPUT
PINE
PPLOT

HORIZ	Horizontal label of curve	None
TITLE	Title printed out on curve	None
VERT	Vertical label	None
XLAB	Numerical scale labels for X	None
YLAB	Numerical scale labels for Y	None

COMMON/ORF/

This common is used by the following subroutines

BOUND
INDATA

AORIF	Cross-sectional area of orifice	Square Feet
CORIF	Orifice coefficient	None
LORIF	Internal orifice link number	None
NORIF	Number of orifices	None

TABLE A-2
(Continued)

Variable Name	Description	Unit
NTC	Number of nodal links including internal links	None
NTCYC	Number of integration cycles	None
TIME	Time counter for hydrograph input	Seconds
TIME2	TIME - DELT2	Seconds
TZERO	Zero time for the simulation Hours/Seconds	
W	Fundamental frequency of daily tidal cycle	Rad Per Sec
	COMMON/ELEV/	
	This common is used in the following subroutines OUTPUT PINE PPLOT	
IPLT	Plot control integer	None
ZCRN	Plot variable, highest crown elevation at a node	Feet
ZGRND	Plot variable, ground elevation	Feet
ZINVRT	Plot variable, node invert elevation	Feet
	COMMON/FILES	
	This common is used in all subroutines of the EXTRAN Block	
IDATEZ	Date (yr-mo-da) on which the simulation begins	None
LOCNOS	Array containing junction numbers of any outflow point in the system	None
N5	Input unit number	None
NLOCAT	Number of outflow junctions	None
NPOLL	Number of pollutants recorded on the input hydrograph tape N21	None

TABLE A-2
(Continued)

Variable Name	Description	Unit
N6	Output unit number	None
N21	Unit number for input hydrograph tape generated by preceding SWMM Block	None
N22	Unit number for output hydrograph tape to be used as input to subsequent SWMM Block	None
QCONV	Factor for converting flows on input hydrograph tape to cfs	Vary
TRIBA	Tributary area drained by the system being simulated	Acres
	COMMON/HYFLOW/	
	This common is used in the following subroutines INDATA INFLOW TRANSX	
ISW	Hydrograph input node number from tape	None
JSW	Hydrograph input node number from cards	None
NIREC	Counter for hydrograph input from tape	None
NSTEPS	Number of input records on input hydrograph file	None
QCARD	Rate of inflow, from cards	cfs
QTAPE	Rate of inflow, from tape	cfs
TE	Time of inflow for card input	Hours/Seconds
TEO	Previous value of TE	Hours/Seconds
TIMEO	TEO	Seconds
TP	TZERO	Seconds
T2	Time of inflow for tape input	Seconds

TABLE A-2
(Continued)

Variable Name	Description	Unit
T20	Previous value of T2	Seconds
WATSH	Not used at this time	None

COMMON/JUNC/

This common is used in the following subroutines
BOUND
HEAD
INDATA
INFLOW
OUTPUT
TRANSX

Variable Name	Description	Unit
AS	Surface area of a node	Square Feet
ASFULL	Surface area of a node when it enters surcharge	Square Feet
GRELEV	Ground elevation at a node	Feet
JSKIP	Internal integer control, to skip nodal head computation	None
JUN	External node number	None
NCHAN	Conduits connecting to a node	None
QIN	Flow into a node from an outside source	cfs
QINST	Dry weather flow into a node from an outside source	cfs
QOU	Flow from a node	cfs
SUMAL	Sum of ap(t)/3Hi, for all pipes at node	Feet
SUMQ	Difference between the average inflow and outflow for a node over a time-step	cfs
SUMQS	Difference between the instantaneous inflow and outflow for a node	cfs
Y	Depth of water at a node at full integration step	Feet

TABLE A-2
(Continued)

Variable Name	Description	Unit
YT	Depth of water at a node at half integration step	Feet
Z	Elevation of node invert	Feet
ZCROWN	Elevation of uppermost conduit crown at a node, defined as node crown elevation	Feet

COMMON/LAB/

This common is used by the following subroutines

BLOCK DATA
CURVE
INDATA
OUTPUT
PINE
PPLOT

HORIZ	Horizontal label of curve	None
TITLE	Title printed out on curve	None
VERT	Vertical label	None
XLAB	Numerical scale labels for X	None
YLAB	Numerical scale labels for Y	None

COMMON/ORF/

This common is used by the following subroutines

BOUND
INDATA

AORIF	Cross-sectional area of orifice	Square Feet
CORIF	Orifice coefficient	None
LORIF	Internal orifice link number	None
NORIF	Number of orifices	None

TABLE A-2
(Continued)

Variable Name	Description	Unit
	COMMON/OUT/	
	This common is used in the following subroutines INDATA OUTPUT TRANSX	
CRPT	Conduit numbers for detailed printing	None
ICOL	Not used at this time	None
IDUM	Dummy array	None
INTER	Number of integration cycles between print cycles	None
IPRT	Not used at this time	None
JPLT	Node numbers for plotting	None
JPRT	Node numbers for detailed printing	None
KPLT	Conduit numbers for plotting	None
LPLT	Number of conduits for detailed printing	None
LTIME	Counter for printed output	None
NHPRT	Number of nodes for detailed printing	None
NPLT	Number of nodes to be plotted	None
NPRT	Not used at the time	None
NPTOT	Total number of plot data points	None
NQPRT	Number of conduits for detailed printing	None
NSTART	First cycle where saved printing array will begin	None
PRGEL	Print matrix, ground elevation	Feet
PRTH	Print matrix, water surface elevation	Feet

TABLE A-2
(Continued)

Variable Name	Description	Units
PRTQ	Print matrix, flow	cfs
PRTV	Print matrix, velocity	fps
PRTY	Print matrix, water depth at node	Feet
QPLT	Matrix of flow values	cfs
TPLT	Time used for plotting	Hours
YPLT	Matrix of water surface elevations	Feet
	COMMON/PIPE/	
	This common is used by the following subroutines BOUND DEPTHX HEAD HYDRAD INDATA OUTPUT TRANSX	
A	Full-step wetted cross section	Square Feet
AFULL	Full cross-sectional area of conduit	Square Feet
AT	Half-step wetted cross section	Square Feet
DEEP	Vertical dimension of conduit	Feet
H	Depth of flow at conduit ends	Feet
LEN	Conduit length	Feet
NCOND	External conduit number	None
NKLASS	Conduit shape classification 1. circular 2. rectangular 3. horseshoe 4. eggshape 5. baskethandle	None

TABLE A-2
(Continued)

Variable Name	Description	Unit
NJUNC	External nodes at each end of conduit	None
Q	Flow in conduit at full integration step	cfs
QO	Saved flow values at beginning of each integration step	cfs
QT	Flow in conduit at half integration step	cfs
RFULL	Hydraulic radius of conduit when full	Feet
ROUGH	Manning coefficient	
V	Velocity in conduit at the full integration step	fps
VT	Velocity in conduit at the half integration step	fps
WIDE	Width of conduit	Feet
ZP	Height of conduit invert above node invert	Feet
	COMMON/PUMP/	
	This common is used in the following subroutines BOUND INDATA	
IPTYP	Type of pump 1 = Off-line pump - operates on wet-well volume 2 = In-line pump - operates on head at junction	None
JPFUL	Internal integer switch for full wet-well 0 = full 1 = not full	None
LPUMP	Internal pump linkage	None
NPUMP	Number of pumps.	None
PRATE	Pumping rate	cfs
VRATE	Volume for changing pump rates	Cubic Feet

TABLE A-2
(Continued)

Variable Name		Description Unit
VWELL	Starting volume of pump wet-well, also current wet-well volume after pumping starts	Cubic Feet
	COMMON/EXSTAT	
	This common is used in the following subroutine OUTPUT TRANSX	
DEPMAX	Maximum depth reached at a junction	Feet
IDHR	Hour at which maximum depth reached	Hours
IDMIN	Minute at which maximum depth reached	Minutes
IQHR	Hour at which maximum flow reached	Hours
IDMIN	Minute at which maximum flow reached	Minutes
IVHR	Hour at which maximum velocity reached	Hours
IVMIN	Minute at which maximum velocity reached	Minutes
QMAXX	Maximum flow reached in a conduit	cfs
SUMQIN	Total system inflow volume during simulation	Cubic Feet
SURLEN	Period during which junction surcharged	Minutes
VLEFT	Volume left in the system at the end of the simulation	Cubic Feet
VMAXX	Maximum velocity reached in a conduit	Feet/Second
	COMMON/STORE	
	This common is used in the following subroutines INDATA OUTPUT TRANSX	
ASTORE	Storage volume per foot of depth (storage facility surface area)	Square Feet

TABLE A-2
(Continued)

Variable Name	Description	Unit
JSTORE	Junction number containing storage facility	None
NSTORE	Number of storage facilities in system	None
ZTOP	Elevation of the top of the storage facility	Feet
	COMMON/SURCHG	
	This common is used in the following routines INDATA TRANSX	
ITMAX	Maximum number of iterations for surcharge computations	None
SURTOL	Fraction of surcharge flow used as tolerance on surcharge iteractions	None
	COMMON/TAPES	
	This common is passed from the SWMM executive program and is only used in subroutine EXTRAN in the EXTRAN Block	
INCNT	Counter for the JIN array	None
IOUTCT	Counter for the JOUT array	None
JIN	Array countaining input hydrograph tape unit numbers for all SWMM Blocks accessed during simulation	None
JOUT	Array containing output hydrograph tape unit numbers for all SWMM Blocks accessed during simulation	None
NSCRAT	Unit number of the scratch file	None
	COMMON/TIDE/	
	This common is used by the following subroutines INDATA TIDCF	
AA	Tidal curve fit coefficients during least square process	None

TABLE A-2
(Continued)

Variable Name	Description	Unit
SXX	Matrix used by least square process	None
SXY	Vector used by least square process	None
TT	Clock time of tidal stage	Hours/Seconds
XX	Vector used in least square tide fit	None
YY	Stage of tidal input corresponding to TT	Feet
	COMMON/TRAP/	
	This common is used in the following subroutines DEPTHX HEAD HYDRAD INDATA TRANSX	
STHETA	Side slope 1 of a trapezoidal channel	None
SPHI	Side slope 2 of a trapezoidal channel	None
	COMMON/WEIR/	
	This common is used in the following subroutines BOUND INDATA	
COEF	Coefficient of discharge for weir	None
COEFS	Coefficient of discharge for surcharged condition computed internally	None
KWEIR	Type of weir 1. transverse 2. transverse with tide gate 3. sideflow 4. sideflow with tide gate	None
LWEIR	Internal link number for weir	None

TABLE A-2
(Continued)

Variable Name	Description	Unit
NWEIR	Number of weirs	None
WLEN	Weir length	Feet
YCREST	Height of weir crest above node invert	Feet
YTOP	Distance of top of weir opening, above invert	Feet

TABLE A-3

PROGRAM LISTING

```
C/////SUBROUTINE EXTRAN
C --------------------====================================
C
C        EXTENDED TRANSPORT MODEL UPDATED APRIL, 1981
C                              BY
C                  CAMP DRESSER AND MCKEE INC.
C                      LARRY A. ROESNER
C                     ROBERT P. SHUBINSKI
C                       JOHN A. ALDRICH
C
C ------------------------------------------------- ==============
      COMMON/TAPES/INCNT,IOUTCT,JIN(10),JOUT(10),NSCRAT(5)
      COMMON/FILES/N5,N6,N21,N22,NPOLL,NLOCAT,QCONV,IDATEZ
     1,LOCNOS(100),TRIBA
      DIMENSION TITLEZ(40)
      INTEGER IFNAM(3),OFNAM(3),IHFNM(3),INOTM(3)
C
C**** TO CHANGE THIS SECTION FROM THE MAIN PROGRAM OF EXTRAN STANDING
C     ALONE TO SUBROUTINE EXTRAN OF EPA SWMM:
C          1. REMOVE C///// FROM FIRST LINE (SUBROUTINE EXTRAN)
C          2. SET ISKIP=0 IN THE NEXT EXECUTABLE LINE BELOW
C          3. REMOVE C///// FROM RETURN STATEMENT AT END OF SUBROUTINE
C
      ISKIP=1
      IF(ISKIP) 10,10,20
C/////// EPA SWMM I/O CONTROL ////////////
C**** SET UP TRANSFER TAPES
   10 INCT=INCT+1
      IOUTCT=IOUTCT+1
      N21=JIN(INCT)
      N22=JOUT(IOUTCT)
      N5=5
      N6=6
      GO TO 30
C/////////////////////////////////////////////////////////////////////
C
C/////// DEC20 I/O CONTROL////////////
C        EXTRAN STAND ALONE
C
C***** SET UP TRANSFER TAPES
   20 CONTINUE
      N5=20
      N6=21
      WRITE(5,103)
  103 FORMAT(1X,'ENTER INPUT FILE SPECIFICATIONS:',$)
      READ(5,101) IFNAM
  101 FORMAT(3A5)
      WRITE(51102)
  102 FORMAT(1X,'ENTER OUTPUT FILE SPECIFICATIONS:',$)
      READ(5,101) OFNAM
      OPEN(UNIT=N5,DEVICE='DSK',ACCESS='SEGIN'IDIALOG=IFNAM)
      OPEN(UNIT=N6rDEVICE='OSKirACCESS="SEOOUT',DIALOG=OFNAM)
C
      READ(N5,1000) N21,N22
 1000 FORMAT(2I5)
      IF(N21.LE.0)G0 TO 110
```

```
      WRITE(5,109)
  109 FORMAT(1X,'ENTER INPUT HYDROGRAPH FILE SPECIFICATIONS:',$)
      READ(5,101)IHFNM
      OPEN(UNIT=N21,DEVICE=1DSK1rACCESS="SEOIN'IDIALOG=IHFNM)
      REWIND N21
  110 IF(N22.LE.0)GO TO 30
      WRITE(5,1-11)
  111 FORMAT(1X,'ENTER FILE SPECS FOR SYSTEM INFORMATION INPUT',
     1' TO RECEIVING WATER MODEL:',$)
      READ(5,101)INOTM
      OPEN(UNIT=N22,DEVICE='DSK'▸ACCESS='SEQOUT',DIALOG=INOTM)
      REWIND N22
C///////////////////////////////////////////////////////////////////////
C
C**** READ FIRST TWO LINES OF INPUT HYDROGRAPH FILE AND WRITE TO
C**** RECEIVING WATER MODEL FILE
   30 IF(N21) 50,50,40
   40 READ(N21)(TITLEZ(I),I=1,40)
      READ(N21)IDATEZ,TZERO
   50 IF(N22) 70,70,60
   60 WRITE(N22)(TITLEZ(I),I=1,40)
      WRITE(N22) IDATEZ,TZERO
C
   70 IF(ISKIP.EO.0)WRITE(N5,100)
  100 FORMAT(/,T2,'ENTRY MADE TO EXTENDED TRANSPORT MODEL',
     1//,T2,/UPDATED BY CAMP DRESSER AND MCKEE INC APR. 1981')
      CALL TRANSX
      IF(ISKIP.E0.0) WRITE(N6,150)
  150 FORMAT(//,T11,1* * * * * EXTENDED TRANSPORT MODEL SIMULATION',
     1' ENDED * * * * *',//)
C/////RETURN
C
      END
```

```
C
      SUBROUTINE TRANSX
C                 THIS IS SUBROUTINE TRANSX OF THE SEWER MODEL
C                 IT DRIVES ALL OTHER SUBPROGRAMS AND PERFORMS THE
C                 MODIFIED EULER SOLUTION OF THE MOTION
C                 AND CONTINUITY EQUATIONS
C
      COMMON /BD/ANORM(26,5),HRNORM(26,5),TWNORM(26,5)
      COMMON/BND/ NFREEIJFREE(25),NTIDE,JTIDE(25),NGATE,JGATE(25)
C
C
      COMMON/HYFLOW/ ISW(187),(ITAPE(187,2),JSW(65),UCARD(65,2),
     1 WATSH(187),TEO,TP,T2YTE,T20,TIMEO,NSTEPS,NINREC
      COMMON/FILES/ N5,N6,N21,N22INPOLLINLOCAT,OCONV,IDATEZ,LOCNOS(100),
     1TRIBA
      COMMON/CONTR/ NTCYC,DELTO,DELT,DELT2,TZERO'ALPHA(40),
     1 NJ,NC,NTC,NTLfICYCINJSW,MJSW,TIME,TIME2,AlfA2,A31A4PA5.A6,A7,W
      COMMON/JUNC/Y(187),YT(187),NCHAN(18718),AS(187),Z(187),OIN(187),
     1 GOU(187),QINST(187),GRELEV(197),JUN(137).ZCROWN(187),JSKIP(187)
     2 ,SUMAL(187),SUMQ(187),SUMQS(137),ASFULL(187)
C
      COMMON/PIPE/LEN(187),NJUNC(18772),AFULL(187),AT(187),
     1 G(187)IV(187),VT(187),DEEP(187),A(187),WIDE(187).RFULL(187),
     2 NKLASS(187),ZP(187,2),QT(187),(10(187),H(187,2),NCOND(187),
     3 ROUGH(187)
      COMMON/TRAP/STHETA(200),SPHI(200)
C
      COMMON/STORE/ NSTORE,JSTORE(20),ZTOP(20),ASTORE(20)
C
      REAL LEN
C
      COMMON/OUT/ NPRT,IPRT,NHPRT,JPRT(20).PRTH(100,20).PRGEL(20).
   ..1) 1 NOPRT,CPRT(20),PRTV(100,20),PRTO(100,20),IDUM(12),ICOL(10),
   co 2 LTIME,NPLT,JPLT(20),YPLT(102,20),LFLT.KPLT(20),OPLT(102,20),
• 7'4,3 TPLT(14:7OT,NSTART,INTER,PRTY(100,20)
cu  ' d COMMON/ TAT VMAXX(187),OMAXX(187),DEPMAX(187),IVHR(187),
   .'t,-42IVMIN(1    (IHR(187),IOMIN(187),IDHR(187)tIDMIN(187),SURLEN(137),
   cnx3SUMOIN,VLEFT
   E.:14 COMMON/SURCHG/SURTOL,ITMAX
   H >, INTEGER CPRT
   c11-° DIMENSION ICHECK(187),JCHECK(137),IND(2),OOUT(100)
      DATA IND/1H ,1H*/
C
C
C                            E X E C U T I O N
C
      COUNT=0.0
      NDIM=187
      DO 5 N=1,NDIM
      ICHECK(N)=IND(1)
      DO 5 M=1,8
    5 NCHAN(N,M)=0
C
C****r*** INPUT DATA
      CALL INDATA
C
```

```
C******** INITIALIZATION
      ICYC=0
      LTIME=0
      NPTOT=0
      NERROR=0
      TIME=TZERO
      IDATE=IDATEZ
      TIMDAY=TZERO/3600.
      DO 901 N=1,NC
      VMAXX(N)=0.0
      OMAXX(N)=0.0
      IVHR(N)=0
      IVMIN(N)=0
      ICIHR(N)=0
      IOMIN(N)=0
  901 CONTINUE
      SUMOIN = O.
      VLEFT = O.
      DO 911 J=1,NJ
      SUMOS(J)=0.
      JCHECK(J)=IND(1)
      SURLEN(J)=0.0
      DEPMAX(J)=0.0
      IDHR(J)=0
      IDMIN(J)=0
      OOU(J) = O.
  911 CONTINUE
C
C ****** INITIALIZATION FOR DRY WEATHER FLOWS
C        IS NOW DONE IN INDATA (BEFORE READING INFLOW HYDROGRAPHS)
C
C
      DO 20 N=1,NTL
      QT(N)=Q(N)
      AT(N)=0.
      IF(N.GT.NTC.OR.QT(N).EQ.0.) GO TO 20
      NL=NJUNC(Np1)
      NH=NJUNC(Np2)
      HNL=Y(NL)+Z(NL)
      HNH=Y(NH)+Z(NH)
```

```
          CALL HEAD(NtNL,NHIHNL,HNHPOT(N),AT(N),VT(N)tHRADtANLtANH,RNLITZERO
         .0)

       20 CONTINUE
          DO 30 J=1,NJ
          YT(J)=Y(J)
       30 CONTINUE
C
C******** MAJOR PROGRAM LOOP THROUGH TIME
          MP=(NTCYC4-99)/100
          DO 760 MCY=1tNTCYC,MP
          NPTOT=NPTOT+1
          DO 640 MCYY=1,MP
          TIME=TIME+DELT
          TIME2=TIME-DELT2
          TIMDAY=TIMDAY+DELT/3600.
          IF(TIMDAY-24,) 34,32,32
       32 TIMDAY=TIMDAY-24.
          IDATE=IDATE+1
       34 ICYC=ICYCtl
          ERROR=0.
          IT=0
          TOL=1.
          NSUR=1
C
C******** SELECT INPUT HYDROGRAPHS
          CALL INFLOW
C
C******** STORE OLD FLOW VALUES
          DO 60 N=11NTL
       60 QO(N)=Q(N)
C
C******** INITIALIZE CONTINUITY PARAMETERS
          DO 30 J=1,NJ
          AS(J)=0.
          SUMO(J)=GIN(J)
          SUMOS(J)=QIN(J)
       80 SUMAL(J)=0.
C
C******** FULL-STEP AREA, RADIUS : VELOCITY
C******1* HALF-STEP FLOW
       90 DO 120 N=1,NTC
          NL=NJUNC(Ntl)
          NH=NJUNC(Nt2)
          H(Nr1)=Y(NL)+Z(NL)
          H(Nr2)=Y(NH)+Z(NH)
          CALL HEAD(NtNLINH,H(Nt1),H(Nt2),G(N),A(N)IV(N)tHRADtANHIANLtRNL,
         1TIME,ICYC)

          DELO4=4-DELT2*V(N)**2*(ANH-ANL)/LEN(N)
          DEL03=2.*V(N)*(A(N)-AT(N))
          DELP2=-(DELT23(32.2*(H(Nr2)-H(N11))/LEN(N))*A(N)
          ONEW=00(N)+DEL02+DEL03+DEL(14
          AKON=-DELT2*(ROUGH(N)/HRAD**1.33333)*ABS(V(N))
          DELQ1=AKONCINEW/(1.-AKON)
          OT(N)=CINEW+DELQ1
```

```
C******** CHECK FOR NORMAL FLOW
      IF(H(N,1) .GT. ZCROWN(NL)) GO TO 101
      DELH=H(N,1)-H(N,2)
      DELZP=ZP(N11)-ZP(N,2)
      IF(OT(N).LE.0.) GO TO 101
      IF(DELH-DELZP) 100,101,101
  100 ONORM=SORT(32.2*(ZP(N,1)-ZP(Nr2))/(LEN(N)*ROUGH(N)))
     1 *ANL*RNL**0.6667
      IF(ONORM.GT.OT(N)) GO TO 101
      QT(N)=ONORM
C******** COMPUTE CONTINUITY PARAMETERS
  101 DODH=1./(1.-AKON)*32.2*DELT2*A(N)/LEN(N)
      SUMO(NL)=SUMO(NL)-0.5*(0T(N)+00(N))
      SUMOS(NL)=SUMOS(NL)-QT(N)
      SUMAL(NL)=SUMAL(NL)-00DH
      SUMO(NH)=SUMO(NH)+0.5*(0T(N)+00(N))
      SUMOS(NH)=SUMOS(NH)+0T(N)
      SUMAL(NH)=SUMAL(NH)+DODH
  120 CONTINUE
C
C******** SET HALF STEP OUTFLOWS AND INTERNAL TRANSFERS
      CALL BOUND(YtYTtOTITIME2tDELT2,IT)
      N1=NTC-1-1
      DO 130 N=N1,NTL
      NL=NJUNC(N,1)
      SUMO(NL)=SUMO(NL)-0,5*(0T(N)+00(N))
      SUMOS(NL)=SUMOS(NL)-0T(N)
      NH=NJUNC(Nt2)
      IF(NH.E0.0)G0 TO 130
      SUMO(NH)=SUMO(NH)+0.5*(0T(N)+00(N))
      SUMOS(NH)=SUMOS(NH)+QT(N)
  130 CONTINUE
C
C******** HALF-STEP HEAD
      DO 320 J=1,NJ
      IF(JSKIP(J)) 140,1401300
  140 IF(AS(J).GT.0,0 .OR. Y(J).GE.(ZCROWN(J)-Z(J))) GO TO 135
      IF(NERROR.LE.10) WRITE(N6,2400) ICYC,JUN(J)
 2400 FORMAT(' ***** WARNING ***** ICYC=',IS,' ZERO SURFACE AREA COMPUT
     .ED AT JUNCTION'tI61 CHECK INPUT DATA FOR HIGH PIPE ')
      NERROR=NERROR+1
      YT(J)=0.0
      GO TO 300
  135 CONTINUE
      YCROWN=0.96*(ZCROWN(J)-Z(J))
      IF(Y(J)-YCROWN) 240,240,260
C
C******** COMPUTE YT FOR FREE SURFACE JUNCTIONS
  240 YT(J)=Y(J)+SUMO(J)*DELT2/AS(J)
      IF(YT(J).LT.0.) YT(J)=0.
C****** WHEN JUNCTION SURCHARGES? 'ASFULL' WILL BE THE LAST
C        VALUE OF 'AS' UNDER FREE SURFACE FLOW
      ASFULL(J) = AS(J)
      GO TO 300
C
C******** ADJUST HEAD AT SURCHARGED JUNCTIONS
```

```
C....... APPLY 1/2 OF COMPUTED CORRECTION
   260 DENOM=SUMAL(J)
       IF(Y(J).LT.1.25*YCROWN) DENOM=SUMAL(J)+(ASFULL(J)/DELT2-SUMAL(J))
              *EXP(-15.*(Y(J)-YCROWN)/YCROWN)
       CORR=0.50
C**** DECREASE.THE HEAD CORRECTION FACTOR FOR UPSTREAM TERMINAL JUNCTIONS
       IF(NCHAN(J,2).E0.0) CORR=0.30
       YT(J)=Y(J)+CORR*SUMQS(J)/DENOM
       IF((YT(J)+Z(J)).GT.GRELEV(J)) YT(J)=GRELEV(J)-Z(J)
       IF(YT(J).LT.YCROWN) YT(J)=YCROWN-0.001
C**** COMPUTE THE CONVERGENCE CRITERIA FOR FLOW ERRORS IN SURCHARGED AREAS
       OAVE=0.
       DO 280 K=1,8
       IF(NCHAN(J+K)) 290+290,280
   280 OAVE=OAVE+ABS(O(NCHAN(J,K)))
   290 K=K-1
       OAVE=SURTOL*OAVE/K
       TOL=(TOL*(NSUR-1)+OAVE)/NSUR
       NSUR=NSUR+1
C
C******** INITIALIZE FOR FULL STEP FLOWS
   300 AS(J)=0.
       SUMO(J)=OIN(J)
       SUMOS(J)=QIN(J)
       SUMAL(J)=0.
   320 CONTINUE
C
C******** HALF-STEP AREA, RADIUS : VELOCITY
C******** FULL-STEP FLOW
   330 ERROR=0.
       DO 360 N=1,NTC
       NL=NJUNC(NI1)
       NH=NJUNC(N12)
C**** CHECK WHETHER SURCHARGE ITERATIONS OCCURRING
       IF(IT) 335,335,333
   333 IF(JCHECK(NH).EO.IND(2)) GO TO 335
       IF(JCHECK(NL).NE.IND(2)) GO TO 360
   335 H(N,1)=YT(NL)+Z(NL)
       H(N,2)=YT(NH)+Z(NH)
       CALL HEAD(N,NL,NH,H(N,1)+H(N12),OT(N),AT(N),VT(N),HRAD,ANH,ANL,
      1RNLYTIMEIICYC)

       DEL04=+DELT*VT(N)**2*(ANH-ANL)/LEN(N)
       DELO3=4.*VT(N)*(AT(N)-A(N))
       DELO2=-(DELT*32.2*(H(N,2)-H(N+1))/LEN(N))*AT(N)
       ONEW=00(N)+DEL02+DEL03+DELO4
       AKON=-DELT*(ROUGH(N)/HRAD**1.33333)*ABS(VT(N))
       DEL01=AKON*ONEW/(1.-AKON)
       O(N)=ONEW+DELO1
C******** DO NOT ALLOW A FLOW REVERSAL IN ONE TIME STEP
       DIROT=SIGN(1.10T(N))
       DIRO=SIGN(1.+O(N))
       IF(DIROT/DIRO.LT.0.) 0(N)=0.001*DIRO
C******** CHECK FOR NORMAL FLOW
       ICHECK(N)=IND(1)
       IF(H(N+1) .GT. ZCROWN(NL)) GO TO 341
```

```
          DELH=H(N,1)-H(N72)
          DELZP=ZP(Nr1)-ZP(N,2)
          IF(O(N).LE.0.) GO TO 341
          IF(DELH-DELZP) 340,341,341
      340 ONORM=SORT(32.2*(ZP(Nr1)-ZP(Nr2))/(LEN(N)*ROUGH(N)))
         1 *ANL*RNL**0.6667
          IF(ONORM.GT.O(N)) GO TO 341
          ICHECK(N)=IND(2)
          O(N)=ONORM
C******** COMPUTE CONTINUITY PARAMETERS
      341 DODH=1./(1.-AKON)*32.2*DELT*AT(N)/LEN(N)
          SUMO(NL)=SUMG(NL)-0.5*(O(N)+OO(N))
          SUMOS(NL)=SUMGS(NL)-G(N)
          SUMAL(NL)=SUMAL(NL)+DGDH
          SUMG(NH)=SUMO(NH)+0.5*(O(N)+OO(N))
          SUMQS(NH)=SUMOS(NH)+Q(N)
          SUMAL(NH)=SUMAL(NH)+DODH
      360 CONTINUE
C
C******** SET FULL STEP OUTFLOWS AND INTERNAL TRANSFERS
          CALL BOUND(YT,Y,OrTIME,DELT,IT)
          N1=NTC+1
          DO 370 N=NlyNTL
C******** DO NOT ALLOW FLOW REVERSAL IN ONE TIME STEP
          DIROT=SIGN(1.,OT(N))
          DIRO=SIGN(1.1O(N))
          IF(DIROT/DIRO .LT. 0.) O(N)=0.001*DIRO
          NL=NJUNC(Npl)
          SUMO(NL)=SUMO(NL)-0.5*(O(N)+OO(N))
          SUMQS(NL)=SUMQS(NL)-O(N)
          NH=NJUNC(Nr2)
          IF(NH.EO,0)OO TO 370
          SUMO(NH)=SUMQ(NH)+0.5*(O(N)+OO(N))
          SUMOS(NH)=SUMOS(NH)1-O(N)
      370 CONTINUE
C
C******** FULL-STEP HEAD
          DO 560 J=1,NJ
          IF(JSKIP(J)) 380/330/560
      380 IF(AS(J).GT.0.0 .OR. YT(J).GE.(ZCROWN(J)-Z(J))) GO TO 375
          IF(NERROR.LE.10) WRITE(N612400) ICYC,JUN(J)
          NERROR=NERROR+1
          11(.1)=0.0
          GO TO 560
      375 CONTINUE
          YCROWN=0.96*(ZCROWN(J)-Z(J))
          IF(YT(J)-YCROWN) 480,480,500
C
C**** CHECK WHETHER SURCHARGE ITERATIONS OCCURRING
      480 IF(IT) 490,490/560
C
C******** COMPUTE Y FOR FREE SURFACE JUNCTIONS
      490 Y(J)=Y(J)+SUMG(J)*DELT/AS(J)
          JCHECK(J)=IND(1)
          IF(Y(J).LT.0.) Y(J)=0.
C******** AT INCIPIENT SURCHARGE, 'AS(J)' WILL BE THE ACTUAL
```

```
C          VALUE OF 'ASFULL'
        ASFULL(J) = AS(J)
C       GO TO 560
C******** ADJUST HEAD AT SURCHARGED JUNCTIONS
C .......APPLY 1/2 OF COMPUTED CORRECTION
    500 DENOM=SUMAL(J)
        IF(YT(J).LT.1.25*YCROWN) DENOM=SUMAL(J)+(ASFULL(J)/DELT-SUMAL(J))
       •      *EXP(-15.*(YT(J)-YCROWN)/YCROWN)
        CORR=1.00
C**** DECREASE THE HEAD CORRECTION FACTOR FOR UPSTREAM TERMINAL JUNCTIONS
        IF(NCHAN(J12).E0.0) CORR=0.60
        Y(J)=YT(J)+CORR*SUMQS(J)/DENOM
        IF((Y(J)+Z(J)).GT.GRELEV(J)) Y(J)=GRELEV(J)-Z(J)
        IF(Y(J).LT.YCROWN) Y(J)=YCROWN-0.001
        JCHECK(J)=IND(2)
C**** COMPUTE SURCHARGE FLOW ERROR IN JUNCTIONS NOT FLOODED
        IF((Y(J)+Z(J)).LT.GRELEV(J)) ERROR=ERROR+SUMOS(J)
    560 CONTINUE
C**** CHECK CONVERGENCE OF THE FLOW ERROR IN SURCHARGED AREAS
        IF(ABS(ERROR)-TOL) 565,565,561
    561 IT=IT+1
        IF(IT-30)562,562,564
C**** INITIALIZE FOR NEXT ITERATION
    562 DO 563 J=1,NJ
        IF(JCHECK(J).EQ.IND(1)) GO TO 563
        YT(J)=Y(J)
        SUMQ(J)=QIN(J)
        SUMOS(J)=OIN(J)
        SUMAL(J)=0.
    563 CONTINUE
        GO TO 330
    564 IT=IT-1
C
C******** COMPUTE CONTINUITY PARAMETERS
C
    565 DO 950 J=1,NJ
        SUMOIN = SUMQIN + OIN(J)*DELT
        IF(Y(J).EQ.GRELEV(J)-Z(J)) 00U(J)=00U(J)+SUMOS(J)*DELT
    950 CONTINUE
        NL = NTC + 1
C******** SYSTEM OUTFLOWS
        I=0
        DO 960 N=NL,NTL
        J=NJUNC(N,1)
        IF(NJUNC(N,2).NE.0) GO TO 960
        QOU(J)=QOU(J)+0(N)*DELT
        I=I+1
        OOUT(I)=Q(N)
    960 CONTINUE
C
C******** WRITE HYDRAULIC DATA FOR INPUT TO QUALITY TRANSPORT MODEL
        DUM=0.
        NHOUR=TIME/3600.
        IF(N22.GT.0) WRITE(N22) NHOUR,IDATE,TIMDAY,
                               (QOUT(N),DUMIN=1,NLOCAT)
```

```
C
C******** CHECK FOR MAXIMUM FLOW AND VELOCITY IN CONDUITS
      TMIN=(TIME-NHOUR*3600.)/60.
      DO 902 N=1INC
      IF(Q(N).GT.QMAXX(N)) GO TO 903
      GO TO 904
  903 OMAXX(N)=Q(N)
      IQHR(N)=NHOUR
      IOMIN(N)=TMIN+0.5
  904 IF(V(N).GT.VMAXX(N)) GO TO 905

      GO TO 902
  905 VMAXX(N)=V(N)
      IVHR(N)=NHOUR
      IVMIN(N)=TMIN+0.5
  902 CONTINUE
C
C******** CHECK FOR SURCHARGE AND MAXIMUM DEPTH AT JUNCTIONS
      DO 906 J=1,NJ
      IF((Z(J)+Y(J)).GTaCROWN(J)) SURLEN(J)=SURLEN(J)+DELT/60.0
      IF(Y(J).GT.DEPMAX(J)) GO TO 907
      GO TO 906

  907 DEPMAX(J)=Y(J)
      IDHR(J)=NHOUR
      IDMIN(J)=TMIN+0.5
C 906 CONTINUE
C******** CHECK PRINTOUT REQUIREMENTS
C.... INTERMEDIATE PRINTOUT
      IF(MOD(ICYC,INTER).NE.0) GO TO 570
      WRITE(N6,1504)
      IF(IT) 566,566,568
  566 WRITE(N6,1499) ICYC,NHOUR,TMIN
 1499 FORMAT(1X,'CYCLE ',I5,6X,'TIME         HRS - ',F5.2,
     *' MIN',//)
      GO TO 569
  568 WRITE(N6,1500)ICYC,NHOURITMINyERROR,IT
 1500 FORMAT(1X,'CYCLE ',I5,6X,'TIME         HRS - ',F5.21
     *' MIN FLOW DIFFERENTIAL IN SURCHARGED AREA=',F6.2,
     *'CFS ITERATIONS REQUIRED=',I2r//)
  569 WRITE(N6,1501)
 1501 FORMAT(1X,'JUNCTIONS / DEPTHS ',/)
      WRITE(N6r1502)((JUN(J),Y(J),JCHECK(J)),J=1OJ)
 1502 FORMAT(2X,I5,1/1,F7.2,A1r7(2X1I5,'/',F7.2,A1)/)
      WRITE(N6,1503)
 1503 FORMAT(/,1X,'CONDUITS / FLOWS 'r/)
      WRITE(N6,1502)((NCOND(N),Q(N),ICHECK(N)),N=1,NTL)
 1504 FORMAT(/,64(2H- )r//)
  570 CONTINUE
      IF(ICYC-NSTART) 640,580,580
  580 NSTART=NSTARTtINTER
      LTIME=LTIME+1
C******** STORE HGL FOR PRINTOUT
      DO 600 I=1,NHPRT
      J=JPRT(I)
```

```
          YMAX=ZCROWN(J)-Z(J)
          PRTY(LTIMEtI)=AMIN1(Y(J)tYMAX)
      600 PRTH(LTIMEtI)=Y(J)+Z(J)
C******** STORE FLOW * VELOCITY FOR PRINTOUT
          DO 620 I=1,NQPRT
          L=CPRT(I)
          NL=NJUNC(Lt1)
          NH=NJUNC(Lt2)
          PRTO(LTIME,I)=O(L)
      620 PRTV(LTIMEtI)=V(L)
      640 CONTINUE

C
C********.STORE HGL * FLOW FOR PRINTER PLOT ROUTINE

          TPLT(NPTOT)=TIME/3600.
          IF(NPLT) 700,700,660
      660 DO 680 N=1,NPLT
          J=JPLT(N)
      680 YPLT(NPTOT,N)=Y(J)+Z(J)
      700 IF(LPLT) 760,760,720
      720 DO 740 N=1,LPLT
          L=KPLT(N)
      740 OPLT(NPTOT,N)=0(L)
      760 CONTINUE
C******** COMPUTE WATER VOLUME LEFT IN STORAGE
          IF(NSTORE.EQ.0) GO TO 801
          DO 800 I=1,NSTORE
          J=JSTORE(I)
      800 VLEFT=VLEFT+Y(J)*ASTORE(J)
      801 CONTINUE
          DO 810 N=1,NC
          NL = NJUNC(N,1)
          NH = NJUNC(Nt2)
C**** VOLUME REMAINING IN CONDUIT WITH TIDE GATE NOT INCLUDED IN VLEFT
          IF(NGATE) 807,807,803
      803 DO 805 I=1,NGATE
          IF(JGATE(I).EO.NH.OR.JGATE(I).EO.NL) GO TO 810
      805 CONTINUE
      807 H1 = Y(NL)   Z(NL)
          H2 = Y(NH)   Z(NH)
          CALL HEAD(NINL,NH,H1tH2,O(N),A(N),V(N),HRAD,ANH,ANL,
         *RNL,TIME,ICYC)

          VLEFT = VLEFT   0.5*(ANH   ANL)*LEN(N)
      810 CONTINUE
C
C******** PRINT * PLOT OUTPUT
          CALL OUTPUT
          STOP

          END
```

```
      BLOCK DATA
C
      COMMON /BD/ANORM(26,5),HRNORM(26,5),TWNORM(26,5)
      COMMON/LAB/ TITLE(40),XLAB(11),YLAB(6),HORIZ(5),VERT(6)
C
C******** NORMALIZED CROSS-SECTIONAL AREA
      DATA ANGRM/
     1 .0000,.0134,.0374,.0680,.1033,.1423,.1845,.2292,.2759,.3242,
     2 .3736,.4237,.4745+.5255,.5763,46264,.6758,.7241,.7708,.8154,
     3 .85761.8967,.9320,.9626,.9866,1.000,
     4 .0000,.0400,.0800,.1200t.16001.2000,424001.2800,.3200,.3600,
     5 .4000,.4400,.4800,45200,.5600,.6000,.6400/.6800,.7200,.7600,
     6 .8000,.8400,.88007.9200,.9600,1.000,
     7 .0000,.0181,.0508,.0908,.13269.1757,.22011.2655,.31181.3587,
     8 .4064,.4542,.5023,.5506,.5987,.6462,.6931,.7387,.7829,.8253,
     9 .86522.9022,.9356,.9645/.9873,1.000,
     1 .00009.0150,.0400,.0550,.08501.12001.1555,.1900,.2250,.2750,
     2 .3200,.3700+.42009.4700,.5150,.5700,.6200,.68001.7300,.7800,
     3..83507.8850,.9250,.9550,.9800,1.000,
     4 .0000,.0173,.0457,.0828,.1271,.1765,.2270,.2775,.3280,.3780,
     5 .4270,.47651.5260,.5740,•6220/.6690,.7160,.7610,.8030,.83901
     6 .8770,.9110,.9410,.9680,.9880,1.000/
C
C******** NORMALIZED HYDRAULIC RADIUS
C******** SECOND SHAPE IS RECTANGULAR - BUT DO NOT USE - CANNOT NORMALIZ
C          A GENERAL RECTANGULAR HYDRAULIC RADIUS
      DATA HRNORM/
     1 .01009.1048,.2052,.3016,.3944,.4824,.5664,.6456,.7204,.79121
     2 .8568,.9176,.9736,1.024,1.070,1.11011.144,1.174,1.194,1.210,
     3 1.217,1.215,1.203,1.178,1.132,1.0001
     4 .0000,.0400,.0800,.1200,.1600,.2000,.2400,.2800,.32009.36007
     5 .40001•4400,,4800,.52007.56001.6000,.64007.6800,.7200,.7600,
     6 48000,.8400,.8800,.9200,.9600,1,000,
     7 .01001.1040,.2065,.3243,.4322,.5284,.61477.6927,.7636,.8268,
     8 .8873,.94171.9905,1.036,1.077,1.113,1.143,1•169,1•189,1.2021
     9 1.20811.20691.195,1.170,1.126,1.000,
     1 .0100+.0970,.2160,.30207.3860,44650,65360,.6110,.6760,.73501
     2 .79109.8540,.9040,.9410,1.008,1.045,1.076,1.115,1.146,1,162,
     3 1.186,1.193,1.186,1.162,1.107,1.000,
     4 .0100,#0952,•1890,.2730,+3690,.4630,•5600,46530r.7430,48220,
     5 .8830,.9490,.999091.055,1.095,1.14111.161,1.188,1.206,1.206,
     6 1.206,1.205,1.196,1.168,1.127,1.000/
C
C******** NORMALIZED SURFACE WIDTH
      DATA TWNORM/
     1 .39191.3919,.5426,.6499,.73327.8000,.8542,.89809.9330,.9600,
     2 .9798,.99281.9992,.9992,.9928,.9798,.9600,.9330,.8980,.8542,
     3 .8000,.73321.6499,.5426'.3919,.39199
     4 1.000,1.000,1.000,1.000,1.000,1.000,1.00091.000,1.000,1,000,
     5 1.000,1.00091.000,1.000,1.000,1.000,1.000,1.000,1,000,1.000,
     6 1.000,1.000,1.000,1.000,1.000,1.000,
     7 .5878,.5878,.87727,8900,.9023,.9156,.9284,.9412,.9540,.9668,
     8 .9798,.9928,.9992,.9992,.9928,.9798,.9600,.9330,.89807.8542,
     9 .8000,.7332,.6499,.5426'.3919,.3919,
     1 .2980,.2980,.4330,.5080'.5820,.6420,.6960,.7460,.7910,.8360,
     2 .8660,.8960,.9260,.9560,.9700,.985071.000,.9850,.97007.9400,
```

```
     3 .8960,.8360,.76409.6420,43100,.3100'
     4 .4900,.4900,.6670,.8200,.9300,1.000,1.000,1.000,.9970,.9940,
     5 ,9880,.9820,,9670,,9480,.9280,.9040,.8740,.8420,.7980,.7500,
     6 .6970,66370,.5670,.4670,.3420,.3420/
C
      DATA VERT.          /4HJUNCY4HTIONI4NWATRP4H SUR,4HELEV,4H(FT)/
C
      END
```

```
      SUBROUTINE BOUND(YDEP,YDEPT,OP,TIDT,IT)
C
C                THIS SUBROUTINE COMPUTES THE LINK FLOW 'QP(LINK)' FOR
C                EACH EXTERNAL * INTERNAL NODE TO NODE TRANSFER
C
      COMMON /BD/ANORM(26,5),HRNORM(26,5),TWNORM(26,5)
      COMMON/TRAP/STHETA(200),SPHI(200)
      COMMON/FILES/ N5,N6,N21,N22,NPOLLINLOCAT,OCONV,IDATEZPLOCNOS(100),
     1TRIBA
      COMMON/CONTR/ NTCYC,DELTO,DELT,DELT2,TZERO,ALPHA(40),
     1 NJINC,NTCPNTLIICYC,NJSW,MJSWPTIMEITIME2pAllA2rA3,A4,A5,A6,A7,W
C
      COMMON/JUNC/Y(187),YT(187),NCHAN(187,8),AS(187),Z(187),OIN(187),
     1 00U(187),OINST(187),GRELEV(187),JUN(187),ZCROWN(187),JSKIP(187)
     2 ,SUMAL(187),SUMO(187),SUMOS(187),ASFULL(187)
C
      COMMON/PIPE/LEN(187),NJUNC(187,2),AFULL(187),AT(187),
     1 0(187)O(187),VT(187),DEEP(187),A(187),WIDE(187),RFULL(187),
     2 NKLASS(187),ZP(187,2),OT(187)I00(187),H(187,2),NCOND(187),
     3 ROUGH(187)
      REAL LEN
C
      DIMENSION YDEP(187),YDEPT(187)i0P(187)
C
      COMMON/STORE/ NSTORE,JSTORE(20),ZTOP(20),ASTORE(20)
      COMMON/ORF/ NORIF,LORIF(60),AORIF(60),CORIF(60)
      COMMON/WEIR/ NWEIRPLWEIR(60),KWEIR(60),YTOP(60),YCREST(60),
     2 WLEN(60),COEF(60),COEFS(60)
      COMMON/PUMP/ NPUMP,LPUMP(20),PRATE(20,3),VRATE(20,3),VWELL(20),
     1 JPFUL(20),IPTYP(20)
      COMMON/END/ NFREE,JFREE(25),NTIDEIJTIDE(25),NGATEIJGATE(25)
C
C                          E X E C U T I O N
C
C**** CHECK WHETHER SURCHARGE ITERATIONS OCCURRING
      IF(IT)105,105,200
  105 HTIDE=-9999.
C
C
C******** COMPUTE NEW ELEVATION OF TIDE
      GO TO (110,109,108,108), NTIDE
  108 HTIDE=A1-14,2*SIN(W*T)+A3*SIN(2,*W*T)+A4*SIN(3.*W*T)
     1        +A5*COS(W*T)+A6*COS(2.*W*T)+A7*COS(3.*W*T)
      IF(MOD(ICYCP30).E0.0) WRITE(N6,1234) ICYC,HTIDE
 1234 FORMAT(' CYCLE',I5,' HTIDE=',F10.2)
      GO TO 110
  109 HTIDE=A1
  110 CONTINUE
C
C******** ASSIGN SURFACE AREA TO STORAGE JUNCTIONS
C
      IF (NSTORE) 116,116,112
  112 DO 114 I=1,NSTORE
      J = JSTORE(I)
      AS(J) = AS(J)    ASTORE(I)
  114 CONTINUE
```

```
   116 CONTINUE
C
C******** COMPUTE HEAD AT JUNCTIONS WITH SUMP ORIFICES WHERE
C         DEPTH IS BELOW JUNCTION INVERT
      IF(NORIF)200,200,120
   120 DO 180 I=1,NORIF
      LINK=LORIF(I)
      J1=NJUNC(LINKFL)
      JSKIP(J1)=0
      IF(NKLASS(LINK) .E0. 7 .OR. YDEP(J1) .GT. 0.) GO TO 180
      JSKIP(J1)=1
      YNL=0.96*DEEP(LINK)i-YDEP(J1)
      CALL HYDRAD(LINK,NKLASS(LINK),YNL,RNL,ANL,BNL)
      YDEPT(J1)=Y(J1)+SUMG(J1)*DT/(BNL*LEN(LINK)/2.)
      IF(YDEPT(J1).GT.O.) YDEPT(J1)=0.001
   180 CONTINUE
C
C******** COMPUTE DISCHARGE OVER TRANSVERSE AND SIDEFLOW WEIRS
   200 IF(NWEIR) 580+580,220
   220 DO 560 I=1,NWEIR
C**** INITIALIZE
      WK=COEF(I)
      POWER=1.5
      V2=0.0
      LINK=LWEIR(I)
      DIR=4.1.
      J1=NJUNC(LINK,1)
      J2=NJUNC(LINK,2)
      Y1=YDEP(J1)
      IF(J2) 240,240,260
   240 Y2=AMAX1((HTIDE-Z(J1)),YCREST(I))
      HEADW=Y1-YCREST(I)
      IF(HEADW) 480,480,320
   260 Y2=YDEP(J2)
      HEADW=AMAX1(Y1,Y2)-YCREST(I)
      IF(HEADW) 480,480,280
C**** CHECK FOR BACKFLOW
   280 IF(Y1-Y2) 300,320,320
   300 DIR=-1.
      Y1=Y2
      Y2=YDEP(J1)
      J1=J2
      J2=NJUNC(LINKr1)
C**** CHECK WHETHER SURCHARGE ITERATIONS OCCURRING
      IF(IT)320,320,310
   310 IF(Y1.GT.0.96*(ZCROWN(J1)-Z(J1))) GO TO 320
      IF(Y2.LT.0.96*(ZCROWN(J2)-Z(J2))) GO TO 560
C**** CHECK FOR SURCHARGE
   320 IF(Y1.GT.YTOP(I)) GO TO 440
      IF(DIR) 380,340,340
   340 IF(KWEIR(I)-3) 380,360,360
C**** WK IS A FUNCTION OF APPROACH VELOCITY FOR SIDEFLOW WEIRS
   360 WK=COEF(I)
      V2=0.0
      POWER=1.67
C**** WEIR DISCHARGE
```

```
  380 GWEIR=WK*WLEN(I)*((HEADW+V2/64.4)**POWER-(V2/64.4)**POWER)
      KW=KWEIR(I)
      GO TO (420,400,420,400) KW
C**** APPLY ARMCO TIDE GATE CORRECTION
C**** (ARMCO WATER CONTROL GATES CATALOG)
  400 IF(HTIDE.GE.(YDEP(J1)+Z(J1))) GO TO 480
      VEL1=COEF(I)*HEADW**(POWER-1.)
      HLOSS=(4./32.2)*VEL1**2*EXP(-1.15*VEL1/SORTCYTOP(I)-YCREST(I)))
      HEADW=HEADW-HLOSS
      IF(HEADW.LE.0) GO TO 480
      IF((YCREST(I)+Z(J)+HEADW).LE.HTIDE) GO TO 480
      OWEIR=COEF(I)*WLEN(I)*HEADW**POWER
C
C********* SUBMERGED WEIR COMPUTATIONS, DFK, 8/74
C
  420 RATIO=(Y2-YCREST(I))/(Y1-YCREST(I))
      IF((Y2-YCREST(I)).LE.0) GO TO 500
      IF(RATIO.LE.0.3) GO TO 421
      IF(RATIO.LE.0.75) GO TO 422
      IF(RATIO.LE.0.85) GO TO 423
      IF(RATIO.LE.0.95) GO TO 424
      CONST=0.4-0.3*(RATIO-0.95)/0.05
      GO TO 430
  421 CONST=1.
      GO TO 430
  422 CONST=1.0-0.1*(RATIO-0.3)/0.45
      GO TO 430
  423 CONST=0.9-0.1*(RATIO-0.75)/0.1
      GO TO 430
  424 CONST=0.8-0.4*(RATIO-0.85)/0.1
  430 OWEIR=CONST*COEF(I)*WLEN(I)*(Y1-YCREST(I))**1.5
      GO TO 500
C**** OUTFLOW IN SURCHARGED CONDITION
  440 IF(Y1-12) 480,480,460
  460 HEADW=Y1-AMAX1(Y2,YCREST(I))
      IF(COEFS(I).GT.0.0) GO TO 470
      ARE=(YTOP(I)-YCREST(I))*WLEN(I)
      COEFS(I)=ABS(OP(LINK))/(ARE*SORT(64.4*HEADW+V2))
  470 OWEIR=COEFS(I)*WLEN(I)*(YTOP(I)-YCREST(I))*SORT(64.4*HEADW+V2)
      GO TO 500
  480 GWEIR=0.
  500 OP(LINK)=DIR*OWEIR
  560 CONTINUE
C
C******** COMPUTE PUMP DISCHARGES
C
C******** NOTE -- ONLY ONE INFLUENT PIPE CAN BE CONNECTED TO A PUMP NODE
C
  580 IF(NPUMP) 920,920,600
  600 DO 900 I=1,NPUMP
      LINK=LPUMP(I)
      J1=NJUNC(LINK,1)
      J2=NJUNC(LINK,2)
      GO TI) (710,880) IPTYP(I)
C******** COMPUTE INFLOW TO WET WELL FOR GATES OPEN CONDITION
  710 U=NCHAN(J1,1)
```

```
          OINJ=OP(N)
          JUP=NJUNC(N,1)
          IF(JUP.NE.J1) GO TO 711
          JUP=NJUNC(N,2)
          OINJ=-OP(N)
C**** CHECK WHETHER SURCHARGE ITERATIONS OCCURING .
  711 IF(IT) 715,715,712
  712 IF(Y(JUP).GT.0.96*(ZCROWN(JUP)-Z(JUP))) GO TO 715
      IF(Y(J1).LE.0.96*(ZCROWN(J1)-Z(J1))) GO TO 900
  715 IF(OINJ.LTJO.) OINJ=0.
.         CALL DEPTH;NrNKLASS(N)I0P(N)rYCRITIYNORM,TIMErICYC)
. C****** SET CRITICAL DEPTH AT WET WELL FOR OFF-LINE PUMP
          YDEPT(J1)=AMIN1(YCRITrYNORM)
          VWELL(I)=VWELL(I)+OINJ*DELT2
C******** SET PUMP RATE
  720 OOUT=0.0
          IF(VWELL(I)) 800,800,740
  740 OOUT=PRATE(I,1)
          IF(VWELL(I)-VRATE(I11)) 800,760,760
  760 OOUT=PRATE(II2)
          IF(VWELL(I)-VRATE(Ir2)) 800'7809780
  780 OOUT=PRATE(I13)
C******** COMPUTE NEW WET WELL VOLUME
  800 VNEW=VWELL(I)-OOUT*DELT2
C******** CHECK FOR DRY WELL
          IF(VNEW) 820,820,840
  820 OOUT=VWELL(I)/DELT2
          VWELL(I)=0.0
          OP(LINK)=GOUT
          JPFUL(I)=1
          GO TO 900
C******** CHECK FOR FLOODED WELL
  840 IF(VRATE(I,3)-VNEW) 860,860,870
  860 DIFF=(VNEW-VRATE(I,3))/DELT2
          VWELL(I)=VRATE(I,3)
          OOUT=PRATE(Ir3)
          QP(LINK)=€OUT
          N=NCHAN(J191)
C....THROTTLE PUMP STATION INFLOW
•         OP(N)=OP(N)-SIGN(DIFFrOP(N))
          SUM€(JUP)=SUMO(JUP)+0.5*DIFF
          SUMOS(JUP)=SUMOS(JUP)+DIFF
          GO TO 900
C******** NORMAL WET WELL CONDITION
  870 VWELL(I)=VNEW
        .OP(LINK)=OOUT
          GO TO 900
C
C****** SET PUMP RATE FOR IN-LINE PUMP
C**** CHECK WHETHER SURCHARGE ITERATIONS OCCURRING
  880 IF(IT) 875,875'872
  872 IF(Y(J1).LE.0.96*(ZCROWN(J1)-Z(J1))) GO TO 900
  875 JSKIP(J1)=0
          IF(YDEP(J1)-0.001) 882'882,885
  882 QINJ=0.
          DO 883 K=1,8
```

TECHNICAL REPORT DATA

(Please read Instructions on the reverse before completinv

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16. ABSTRACT -

This report contains the documentation and user's manualfor the Extended Transport (EXTRAN) Block of the Storm Water Management Model (SWMM).EXTRAN is a dynamic flow routing modelused to compute backwater profiles in open channel and/or closedconduit systems experiencing unsteady flow. Ii represents the drainage system as links and nodes, allowing parallel or looped pipe networks; weirs, orifices, and pumps; and system surcharges to be simulated. EXTRAN is used most efficiently if it is only applied to those parts of the drainage system which cannot be simulated accurately by simpler, less costly models.

The EXTRANmanual is designed to give theuser complete information on executing of the model both as a block of the SWMM package and as an independent model. Formulation of the input data is discussed in detailand demonstrated by seven example problems. Typical computer output is also discussed. Problem areas which the user may confront are described, as well as the theory on which the EXTRAN

,17. KEY WORDS *AND* DOCUMENT ANALYSIS

a. DESCRIPTORS	b.IDENTIFIERS/OPEN ENDED TERMS	C. CCSATI Field/Group
Water pollution, Combined sewers, Water quality, Mathematical models	Storm Drainage, Flood Routing, Computer Modelin, Hydraulic Systems, Gradually Varied Flow	138

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(9.73)

```
      N=NCHAN(J1rK)
      IF(N.GT.NC) GO TO 884
  883 CIONTF(NJUNEC(N,2).E0.J1) GINJ=GINJ+GP(N)
  884 GOUT=OINJ
      GO TO elm
C****** SET PUMP RATE
  885 GOUT = PRATE(I,1)
      IF(YDEP(J1).0T.VRATE(Ir1)) GOUT = PRATE(I,2)
      IF(YDEP(J2).GT.VRATE(I,2)) GOUT = PRATE(I,3)
  888 OP(LINK) = GOUT
  900 CONTINUE
C
C******** SET DEPTH AT FREE OUTFALL * TIDAL NODES (ONE PIPE/NODE)
  920 IF(NFREE) 980,980,940
  940 DO 960 I=1rNFREE
      J=JFREE(I)
      N=NCHAN(J,1)
      LINK=NCHAN(J,2)
      OP(LINK)=0P(N)
C..** CHECK FOR OUTFALL PIPE ON AN ADVERSE SLOPE
      IF(NJUNC(N,1).EG.J)OP(LINK)=-GP(LINK)
      CALL DEPTH:(N,NKLASS(N),OP(N),YCRIT,YNORMITIME,ICYC)
      YDEPT(J)=AMIN1(YCRIT,YNORM)
C******** CHECK FOR FULL PIPE OR SURCHARGE
      IF(YDEPT(J).GT.DEEP(N)) YDEPT(J)=DEEP(N)
C******** CHECK FOR TIDAL INFLUENCE
      IF((YDEPT(J)+Z(J)).LT.HTIDE) YDEPT(J)=HTIDE-Z(J)
  960 CONTINUE
C
C******** SET DEPTH AT TIDE GATE OR CLOSE GATE
  980 IF(NGATE) 1080,1080,1000
 1000 DO 1060 I=1,NGATE
      J=JGATE(I)
      N=NCHAN(J,1)
      LINK=NCHAN(J,2)
      GP(LINK)=QP(N)
C**** CHECK FOR OUTFALL PIPE ON AN ADVERSE SLOPE
      JUP=1
      JDN=2
      IF(NJUNC(NI2).EQ.J) GO TO 1010
      OP(LINK)=-0P(LINK) •
      JUP=2
      JDN=1
 1010 IF(H(N,JUP)-HTIDE) 1020,1020,1030
C******** GATE CLOSED
 1020 YDEPT(J)=H(N,JUP)-Z(J)
      OP(LINK)=0.0
      IF(YDEPT(J).LT.0.)YDEPT(J)=0.
      GO TO 1060
C******** GATE.QfEN
 1030 CALL DEPTIZN,NKLASS(N),QP(N),YCRITrYNORM,TIME,ICYC)
      YDEPT(J)=AMIN1(YCRIT,YNORM)
C******** CHECK FOR FULL PIPE OR SURCHARGE
      IF(YDEPT(J).GT.DEEP(N)) YDEPT(J)=DEEP(N)
C******** CHECK FOR TIDE ELEVATION
```

```
      IF(CYDEPT(J)+Z(J)).LT.HTIDE) YDEPT(J)=HTIDE-Z(J)
 1060 CONTINUE
C
 1080 RETURN
      END
```

```
CURVE    NTRNJA   FORTRAN V.5A(621) /KI/C/L        20-MAY-81       15:57  PAGE 1

         SUBROUTINE CURVE(X,Y,NPT,NCVINPLOT)
         COMMON/FILES/ N5046,N21,N229NPOLL,NLOCAT,GCONV,IDATEZ,LOCNOS(100),
        1TRIBA
         DIMENSION X(202,5),Y(202,5),NPT(5)
        11DUMX(4),DUMY(4)
         COMMON/LAB/ALPHA(40)O(LAB(11),YLAB(6),H0RIZ(5),VERT(6)
C
C                                                 SET UP X AND Y
SCALES C
         XMAX = -1.0E30
         XMIN = 1.0E30
         YMAX = -1.0E30
         YMIN = 1.0E30
         DO 100 K = 1, NCV
         N = NPT(K)
         DO 100 J = 1, N
         IF( X(J,K) 'GT. XMAX ) XMAX = X(J,K)
         IF( X(J,K)  .LT. XMIN ) XMIN = X(J,K)
         IF( Y(J,K) .GT. YMAX ) YMAX = Y(J,K)
         IF( Y(J,K) .LT. YMIN ) YMIN = Y(J,K)
     100 CONTINUE
         DUMX(1) = XMIN
         DUMX(2) = XMAX
         CALL SCALE(DUMX,10.0,2.1)
         DUMY(1) = YMIN
         DUMY(2) = YMAX
         CALL SCALE  (DUMY,4.0,2,1)
         DO 120 K = 1, NCV
         N = NPT(K)
         X(N+1,K) = DUMX(3)
         X(N+2,K) = DUMX(4)
         Y(N+1,K) = DUMY(3)
         Y(N+2,K) = DUMY(4)
     120 CONTINUE
C
C                                                 FORM X LABELS AND FACTORS
C
         XMIN= DUMX(3)
         DELTX= DUMX(4)
         XLAB(1)=XMIN
         DO 140 I=1,10
     140 XLAB(I+1)=XLAB(I)+DELTX
         XSCAL=100./(XLAB(11)-XMIN)
C
C                                                 FORM Y LABELS AND FACTORS
C
         YMIN= DUMY(3)
         DELTY= DUMY(4)
         YLAB(5)=YMIN
         DO 160 I=1.4
     160 YLAB(5-I)=YLAB(6-I)+DELTY
         YSCAL=40./(YLAB(1)-YMIN)
C
C                                                 INITIALIZE PLOT OUTLINE
C
         NCD=100
```

```
        CALL PPLOT(OtO,NCD,NPLOT)
        K = 1
C
C                                            DRAW IN EACH CURVE
C
        DO 240 L=ltNCV
         IF(NPT(L).E0.0) GO TO 220
C
C                                            JOINING XO YO AND XT YT
C
        X0=XSCAL*(X(1lL)-XMIN)
        YO=YSCAL*(Y(1,L)-YMIN)
        NPOINT = NPT(L)
        DO 180 N = 2,NPOINT
        XT = XSCAL*(X(NtL)  - XMIN)
        YT = YSCAL*(Y(N,L)  - YMIN)
        CALL PINE(X0tY0tXTtYT,KtNPLOT)
        XO = XT
        YO = YT
    180 CONTINUE
    200 CONTINUE
    220 K = K   1
    240 CONTINUE
C
C                                            OUTPUT FINAL PLOT
C
        NC=99
        CALL PPLOT(OtOtNCINPLOT)
        RETURN
        END
```

```
      SUBROUTINE
DEPTHX(N,KLASSPOPOIYC,YNORMPTIMEIICYC)
C
C                       THIS SUBROUTINE FINDS THE CRITICAL DEPTH
C                       AND THE NORMAL DEPTH CORRESPONDING TO THE FLOW
OP
C
      COMMON /BD/ANORM(26,5),HRNORM(26,5),TWNORM(26,5)
C
      COMMON/FILES/ N5,N6IN210422,NPOLL,NLOCATPOCONV,IDATEZILOCNOS(100),
     1TRIBA
      COMMON/PIPE/LEN(187),NJUNC(187,2),AFULL(187),AT(187),
     1 O(187),V(187),VT(187),DEEP(187),A(187),WIDE(187),RFULL(187),
     2 NKLASS(187),ZP(18772),QT(187)OO(187),H(187,2),NCOND(187),
     3 ROUGH(187)
      COMMON/TRAP/STHETA(200),SPHI(200)
      DIMENSION KCRIT(187)
      REAL LEN
C
C                              E X E C U T I O N
C
      QP=ABS(QPO)
      YC=0.
      YNORM=0.
      IF(QP.LE.0.) RETURN
      NDIM=187
      NTYPE=KLASS
      IF(NTYPE.EG.6) GO TO 640
C******** SPECIFY NTYPE FOR ORIFICES
      IF(NKLASS(N).EO. 7 .OR. NKLASS(N).EO. 8) NTYPE=1
C*********INITIALIZE KCRIT
C
      IF(ICYC.GT.1) GO TO 100
      DO 50 I=1,NDIM
      KCRIT(I)=0
   50 CONTINUE
C******** SEARCH AREA * WIDTH TABLES FOR PROPER LOCATION
  100 000=0.
      DO 300 I=2,26
      AREA=AFULL(N)*ANORM(I,NTYPE)
      WIDTH=WIDE(N)*TWNORM(I,NTYPE)
      OC=AREA*SORT(32.2*AREA/WIDTH)
      IF(QC-QP) 250,200,200
  200 DELTA=(OP-OCO)/(OC-OCO)
      YC=0.04*(FLOAT(I-2)+DELTA)*DEEP(N)
      GO TO 400
  250 OCO=OC
  300 CONTINUE
C
C******** PIPE SURCHARGED AT THIS SECTION
      YC=DEEP(N)
C
C******** SEARCH AREA * RADIUS TABLES FOR PROPER LOCATION
  400 ONORM0=0.
      DO 600 I=2,26
      AREA=AFULL(N)*ANORM(I,NTYPE)
      HRAD=RFULL(N)*HRNORM(I,NTYPE)
      IF(NTYPE.EO. 2) HRAD=WIDE(N)+2.*(I-1)/25.*DEEP(N)
```

```
          ONORM=SORT(32.2*(ZP(N,1)-ZP(N,2))/(LEN(N)*ROUGH(N)))
         I *AREA*HRAD**0.6667
          IF(ONORM-QP) 550,500,500
      500 DELTA=(0P-ONORMO)/(ONORM-ONORMO)
          YNORM=0.04*(FLOAT(I-2)+DELTA)*DEEP(N)
          GO TO 620
      550 ONORMO=GNORM
      600 CONTINUE
C******** PIPE SURCHARGED AT THIS SECTION
          YNORM=DEEP(N)
C
      620 RETURN
C
C******** YC AND YNORM FOR TRAPEZOIDAL CHANNELS
C
C**** COMPUTE YC
      640 000=0.
          DO 660 I=2,26
          YI=0.04*FLOAT(I-1)*DEEP(N)
          WIDTH=YI*(STHETA(N)+SPHI(N))+WIDE(N)
          AREA=0.5*YI*(WIDTH+WIDE(N))
          OC=AREA*SORT(32.2*AREA/WIDTH)
          IF(QC-QP) 650',645,645
      645 DELTA=(0P-OCO)/(0C-OCO)
          YC=0,04*(FLOAT(I-.2)+DELTA)*DEEP(N)
          GO TO 665
      650 QCO=QC
      660 CONTINUE
C
C**** PIPE SURCHARGED AT THIS SECTION
          YC=DEEP(N)
C
C**** COMPUTE YNORM
      665 ONORM0=0.
          SROOTS=SORT(litSTHETA(N)**24)+SORT(1.+SPH1(N)**2.)   Replace DTEMP
          DO 680 I=2,26                                          by YI
          YI=0.04*FLOAT(I-1)*DEEP(N)
          AREA=YI*(WIDE(N)+YI/2.*(STHETA(N)+SPHI(N)))
          HRAD=AREA/(WIDE(N)+5;t*EASROOTS)
          ONORM=SORT(32.2*(ZP(N,1)-ZP(N,2))/(LEN(N)*ROUGH(N)))*AREA*HRAD**0.
         2677
          IF(ONORM-QP) 675,670,670
      670 DELTA=(P-ONORMO)/(ONORM-ONORMO)
          YNORM=0.04*(FLOAT(I-2)+DELTA)*DEENN)
          RETURN
      675 QNORMO=ONORM
      680 CONTINUE
C**** PIPE SURCHARGED AT THIS SECTION
          YNORM=DEEP(N)
          RETURN
          END
```

```
      SUBROUTINE HEAD(N,NL,NH,HEAD1,HEAD2,OP9AREAFVEL,HRAD,ANH,ANL,RNLI
     1TIME,ICYC)
C
C                     THIS SUBROUTINE CONVERTS NODAL DEPTHS TO PIPE DEPTHS
C                     IT ALSO ASSIGNS SURFACE AREAS TO THE PROPER NODES
C                     SURFACE AREA IS NOT ASSIGNED TO ORIFICE OR WEIR LINKS
C
      COMMON /BD/ANORM(26,5),HRNORM(26/5),TWNORM(2615)
      COMMON/FILES/ N5046,N211N22/NPOLL,NLOCAT,OCONV,IDATEZ,LOCNOS(100),
     1TRIBA
      COMMON/TRAP/STHETA(200),SPHI(200)
      COMMON/JUNC/Y(187),YT(187),NCHAN(187,8),AS(187),Z(187),OIN(137),
     1 OOU(187),OINST(187),GRELEV(187),JUN(187),ZCROWN(187),JSKIP(187)
     2 ,SUMAL(187),SUMO(187),SUMOS(187),ASFULL(187)
C
      COMMON/PIPE/LEN(187),NJUNC(187,2),AFULL(187),AT(187),
     1 O(187),V(187),VT(187),DEEP(187),A(187),WIDE(187),RFULL(187),
     2 NKLASS(187),ZP(187,2)1OT(187),OO(187),H(187,2),NCOND(187),
     3 ROUGH(187)
      REAL LEN
C
C                               E X E C U T I O N
C
      YNL=HEAD1-ZP(Ntl)
      YNH=HEAD2-ZP(Nr2)
C
C******** CHECK FOR DRY PIPE
      IF(YNL.LE.0..AND.YNH.LE.0.) GO TO 220
      IF(YNL)10,10,20
C******** YNL.LE.0, YNH..GT.0 (CRIT OR NORM UPSTRM OR STORAGE DWNSTRM)
   10 IF(HEAD2-ZP(N,1)) 240,15,15
   15 IF(ZP(N,1).LE.Z(NL)) GO TO 160
      CALL DEPTH(N,NKLASS(N),OP,YC,YNORM,TIME,ICYC)
      GO TO 200
C******** YNH LE 0, YNL GT 0, CRITICAL OR NORM DOWNSTREAM
   20 IF(YNH) 25,25,30
   25 IF(ZP(Nr2) LE.Z(NH)) GO TO 160
      CALL DEFININKLASS(N),OPPYC,YNORM,TIME,ICYC)
      Y2=AMIN1( CtYNORM)
      GO TO 180
C******** YNL AND YNH GT 0
   30 IF(OP) 35,50,50
C******** ADVERSE FLOW
   35 IF(ZP(N,1)-Z(NL)) 160,160,40
   40 CALL DEPTHZN,NKLASS(N),OPrYC,YNORM,TIMEtICYC)
      IF(YC-YNL) 160,160,200
C******** POSITIVE FLOW
   50 IF(ZP(N,2)-Z(NH)) 160,160,55
   55 CALL DEPTHI(N,NKLASS(N),OPtYC,YNORMITIMErICYC)
      Y2=AMIN1(YC,YNORM)
      IF(Y2-YNH) 1207120,180
  120 IF(YNH-AMAX1(YCIYNORM)) 140,140'160
  140 FASNH=(YNH-Y2)/ABSYfNORM-YC)
      GO TO 165
C
C******** NORMAL SITUATION' HALF SURFACE AREA AT EACH END
```

```
  160 FASNH=1.0
  16S YMID=0.5*(YNL+YNH)
      IF (YMID.LE.0.0) YMID=0.0
      CALL HYDRAD(N9NKLASS(N),YNL,RNL,ANL,BNL)
      CALL HYDRAD(N,NKLASS(N),YMID,RMIDPAMID,BMID)
      CALL HYDRAD(NPNKLASS(N),YNHYRNH,ANHIBNH)
      IF(NKLASS(N).GT. 6) GO TO 260
      AS(NL)=AS(NL)+0.25*(BNLJBMID)*LEN(N)
      AS(NH)=AS(NH)+0.25*(BMID+BNH)*LEN(N)*FASNH
      GO TO 260
C
C******** CRITICAL SECTION DOWNSTREAM' SURFACE AREA UPSTREAM
  180 YNH=Y2
      HEAD2=YNH42P(NI2)
      YMID=0.5*(YNL+YNH)
      IF (YMID.LE.0.0) YMID=0.0
      CALL HYDRAD(NINKLASS(N),YNLIRNLPANL,BNL)
      CALL HYDRAD(NeNKLASS(N),YMID,RMIDIAMID,BMID)
      CALL HYDRAD(N,NKLASS(N),YNH,RNH,ANH,BNH)
      IF(NKLASS(N).GT. 6) GO TO 260
      AS(NL)=AS(NL)+0.25*(BNLI-BMID)*LEN(N)
      GO TO 260
C
C******** CRITICAL SECTION UPSTREAM' SURFACE AREA DOWNSTREAM
  200 HEAD1=YC+ZP(N11)
      YNL=YC
      YMID=0.5*(YNL-FYNH)
      IF (YMID.LE.0.0) YMID=0.0
      CALL HYDRAD(N,NKLASS(N),YNLYRNL,ANL,BNL)
      CALL HYDRAD(N,NKLASS(N),YMIDIRMIDIAMID,RMID)
      CALL HYDRAD(N,NKLASS(N),YNI,I,RNHFANHIBNH)
      IF(NKLASS(N).GT..6) GO TO 260
      AS(NH)=AS(NH)+0.25*(BMID+BNH)*LEN(N)
      GO TO 260
C
C******** DRY PIPE' NO SURFACE AREA FOR ENDS WITH NEGATIVE Y
  220 HEAD1=HEAD2
      YMID=0.
      CALL HYDRAD(NrNKLASS(N),YMID,RMIDFAMID,BMID)
      ANH=0.
      ANL=0.
      RNL=0.
      AREA=0.
      VEL=0.
      HRAD=.01
      OO(N)=0.0
      IF(NKLASS(N).GT. 6) RETURN
      IF(YNL.LT.-0.001) GO TO 230
      AS(NL)=AS(NL)+BMID*LEN(N)/2.
  230 IF(YNH.LT.-0.001) RETURN
      AS(NH)=AS(NH)+BMID*LEN(N)/2.

      RETURN
C
C******** DRY UPSTREAM' SURFACE AREA DOWNSTREAM
  240 HEAD1=HEAD2
```

```
      YNL=0.
      YMID=HEAD2-0.5*(ZP(N,1)+ZP(N12))
      IF(YMIILLT.0.) YMID=0.
      CALL HYDRAD(NrNKLASS(N),YNLIRNLIANL,BNL)
      CALL HYDRAD(N,NKLASS(N),YMID,RMID,AMID,BMID)
      CALL HYDRAD(NrNKLASS(N)tYNHIRNH!ANH,BNH).
      AREA=0.25*(ANL+2.*AMID+ANH)
      VEL=0.0
      HRAD=0.5*(RMID+RNH)
      00(N)=0.0
      IF(NKLASS(N).GT. 6) RETURN
      AS(NH)=AS(NH)+0.25*(BMIDtBNH)*LEN(N)
      IF(ZP(N,1)-Z(NL).LT.0.001) AS(NL)=AS(NL)+.25*(BNL+BMID)*LEN(N)
      RETURN
C
C******** COMPUTE CROSS-SECTION AREA, VELOCITY * HYDRAULIC RADIUS
  260 AREA=0.25*(ANL+2.*AMID+ANH)
      VEL=0.
      IF(AREA.GT.0.) VEL=OP/AREA
      HRAD=0.25*(RNL+2,*RMID+RNH)
      IF (AREA.LE.0.0) 00(N)=0.0
      RETURN
      END
```

```
      SUBROUTINE HYDRAD (N,KLASS,DEPTH,HRADIAREA,WIDTH)
C
C                    THIS SUBROUTINE COMPUTES THE HYDRAULIC RADIUS,
C                    SURFACE WIDTH, * CROSS-SECTION AREA FOR PIPE 'N'
C
      COMMON/FILES/ N5,N6rN211N22,NPOLL,NLOCAT,OCONV,IDATEZ,LOCNOS(100),
     1TRIBA
      COMMON /BD/ANORM(26,5),HRNORM(26,5),TWNORM(26,5)
C
      COMMON/PIPE/LEN(187),NJUNC(187,2),AFULL(187),AT(187),
     1 0(187),V(187),VT(187),DEEP(187),A(187),WIDE(187),RFULL(187),
     2 NKLASS(187),ZP(187,2),OT(187),00(187),H(187,2),NCOND(187),
     3 ROUGH(187)
      COMMON/TRAP/STHETA(200),SPHI(200)
      REAL LEN
C
C                           E X E C U T I O N
C
      NTYPE=KLASS
      IF(DEPTH) 200,100,100
C**** SPECIFY NTYF'E FOR ORIFICES
  100 IF(NKLASS(N).EG. 7 .OR. NKLASS(N).E0. 8) NTYPE=1
      GO TO (120,180,120,120,120,190),NTYPE
  120 FDEPTH=DEPTH/DEEP(N)
      IF(FDEPTH-1.) 140,140,160
C
C******** INTERPOLATE TABLE OF PROPERTIES
  140 I=1+IFIX(FDEPTH/0.04)
      DELTA=(FDEPTH-0.04*FLOAT(I-1))/0.04
      WIDTH = WIDE(N)*(TWNORM(I,NTYPE)+(TWNORM(I+1,NTYPE)-TWNORM(IINTYPE
     1))*DELTA)
      AREA = AFULL(N)*(ANORM(I,NTYPE)   (ANORM(I+1tNTYPE)-ANORM(I,NTYPE)
     1)*DELTA)
      HRAD = RFULL(N)*(HRNORM(I,NTYPE)+(HRNORM(I+1,NTYPE)-HRNORM(I,NTYPE
     1))*DELTA)
      RETURN
C
C******** FULL PIPE
  160 WIDTH = 0.
      AREA=AFULL(N)
      HRAD=RFULL(N)
      RETURN
C******** RECTANGULAR SECTION (SPECIAL CASE)
C
  180 WIDTH=WIDE(N)
      AREA=WIDTH*DEPTH
      HRAD=AREA/(WIDTH+2.*DEPTH)
      HRAD=AMAX1(HRAD,0.01)
      RETURN
C
C******** TRAPEZOIDAL SECTION (SPECIAL CASE)
  190 CONTINUE
      DEPTT=DEPTH
      FDEP=DEPTH-DEEP(N)
      IF(FDEP) 196,196,194
  194 DEPTT=DEEP(N)
```

```
   196 CONTINUE
       WIDTH=WIDE(N)+DEPTT*(STHETA(N)+SPHI(N))
       AREA=DEPTT*(WIDE(N)4(DEPTT/2.)*(3THETA(N)+SPHI(N)))
       WETPER=WIDE(N)+DEPTT*(SORT(1.+STHETA(N)**2.)+SORT(1,+SPHI(N)**2.))
       HRAD=AREA/WETPER
       HRAD=AMAX1(HRADr0.01)
       RETURN
C
C******** NEGATIVE DEPTH
   200 WRITE(N6,5000) NCOND(N),DEPTH
  5000 FORMAT('ONEGATIYE DEPTH ENTERED TO HYDRAD, COND.',I6,E16.4)
       DEPTH=0.
       GO TO 100
       END
```

```
      SUBROUTINE INDATA
C
C                 THIS SUBROUTINE READS AND PRINTS ALL INPUT DATA
C                 EXCEPT FOR HYDROGRAPH CARDS IN 'INFLOW'
C                 IT ALSO PERFORMS SOME INITIALIZATION
C                 ALL NODE-CONDUIT LINKAGES ARE SET UP AND
C                 CONVERTED TO THE INTERNAL NUMBER SYSTEM
C
      INTEGER IFNAM(3),OFNAM(3),INFNM(3),INGTM(3)
      COMMON /BD/ANORM(26,5),HRNORM(26,5),TWNORM(26,5)
C
      COMMON/FILES/ N5,N6,N21,N22,NPOLLPNLOCAT,OCONV,IDATEZeLOCNOS(100),
     1TRIBA
      COMMON/CONTR/-NTCYCIDELTOrDELT,DELT2rTZERO,ALPHA(40),
     1 NJ,NC,NTC,NTL'ICYC,NJSW,MJSW,TIME,TIME2yAltA2rA3rA4,A5rA6,A7rW
C
      COMMON/JUNC/Y(187),YT(187),NCHAN(18778),AS(187),Z(187),GIN(187),
     1 0OU(187),QINST(187),GRELEV(187),JUN(187),ZCROWN(187),JSKIP(187)
     2 PSUMAL(187),SUMQ(187),SUMOS(187),ASFULL(187)
C
      COMMON/PIPE/LEN(187),NJUNC(187,2),AFULL(187),AT(187),
     1 G(187),V(187),VT(187),DEEP(187),A(187)?WIDE(187)tRFULL(187),
     2 NKLASS(187),ZP(187,2),GT(187),0O(187),H(187,2),NCOND(187),
     3 ROUGH(187)
      COMMON/TRAP/STHETA(200),SPHI(200)
C
      REAL LEN
C
      COMMO.N/STORE/ NSTORE,JSTORE(20),ZTOP(20),ASTORE(20)
      COMMON/ORF/ NORIF,LORIF(60),AORIF(60),CORIF(60)
      COMMON/WEIR/ NWEIR,LWEIR(60),KWEIR(60),YTOF(60),YCREST(60),
     2 WLEN(60),COEF(60),COEFS(60)
      COMMON/PUMP/ NPUMPPLPUMP(20),PRATE(20,3)IVRATE(20,3),VWELL(20),
     1 JPFUL(20),IPTYP(20)
      COMMON/BND/ NFREE,JFREE(25),NTIDEtJTIDE(25),NGATE,JGATE(25)
C
      COMMON/OUT/ NPRT,IPRT,NNPRT,JPRT(20),PRTN(100,20),PRGEL(20),
     1 NOPRT,CPRT(20),PRTV(100,20),PRT0(100,20),IDUM(12),ICOL(10))
     2 LTIMEINPLT,JFLT(20),YPLT(102,20),LFLT,KFLT(20),OPLT(102,20)t
     3 TPLT(102),NPTOT,NSTART,INTER,PRTY(100,20)
      INTEGER CPRT
C
      COMMON/TIDE/ YY(50) ,TT(50) pAA(10),XX(10),SXX(10710),SXY(10)
C
      COMMON/HYFLOW/ ISW(187).OTAPE(187,2),JSW(65).0OARD(6512),
     1 WATSH(187),TEO,TP,T2ITE,T20,TIMEOPNSTEPSININREC
C
      COMMON/LAB/ TITLE(40),XLAB(11),YLAB(6)tHORIZ(5),VERT(6)
C
      COMMON/SURCHG/SURTOL,ITMAX
C
      DIMENSION OTYPE(2),EXTRAN(5),SOURCE(5),GOUT(100)
C
      DATA OTYPE/'SIDE''''SUMP'/
      DATA ENOTE/6HERROR ,6HERRORS/
              DATA EXTRAN/4HEXTE,4HNDED,4H TRA94HNSP0,4HRT /
```

```
C
C                              E X E C U T I O N
C
      NSTOP=0
      NDIM=187
C
C******** HEADING (TITLE) CARDS
C
      READ(N5,5040) ALPHA
 5040 FORMAT(20A4)
      WRITE(N6,2999)
2999 FORMAT('1',64(2H--)/",'ENVIRONMENTAL PROTECTION AGENCY',13X,40H*
     2***    EXTENDED TRANSPORT PROGRAM       ****,8X,'WATER RESOURCES DIVISI
     3ON'/",'WASHINGTON, D.C.                    1,16)04H****,32X14H****,8X, .
     4'CAMP DRESSER & MCKEE INC.'!'I'                  ',28X,4H
     5****,6X,'   ANALYSIS MODULE ',6X,4H****,8X,'ANNANDALE, VIRGINIA
     6')
      WRITE(N6,060) ALPHA
 5060 FORMAT(' ',20A4/' '20A4//)
C
C******** GENERAL CONTROL PARAMETERS
C
      READ(N595080) NTCYCIDELT,TZERO.NHPRT,NOPRT,NPLT.LPLT,NSTART,INTER,
     1 NJSW,ITMAX,SURTOL
 5080 FORMAT(I5,2F5.0,8I5PF5.0)
      DELT2=DELT/2.
      IF(N22.EU.0)MJSW=0
      WRITE(N6r5100) NTCYC
 5100 FORMAT (19HOINTEGRATION CYCLES,I5)
      WRITE(N6,5120) DELT
 5120 FORMAT (30HOLENGTH OF INTEGRATION STEP IS,F6.0.8H SECONDS)
      IF (NSTART.LE.0) NSTART = 1
      INTEMP=(NTCYC-NSTART)/100 + 1
      IF (INTEMP.GE.INTER) INTER=INTEMP
      WRITE(N6,5140) NSTART,INTER
 5140 FORMAT('OPRINTING STARTS IN CYCLE',IS,' AND PRINTS AT INTERVALS OF
     1',I4,' CYCLES')
      WRITE(N6r5160) TZERO
 5160 FORMAT (13HOINITIAL TIMEIF6.2,6H HOURS)
      TZER0=3600.*TZERO
C
C******** PRINT AND PLOT DATA
C
C******** JUNCTION NUMBERS FOR DETAILED PRINTOUT
      READ(N5,5180)(JPRT(I),I=1.NHPRT)
 5180 FORMAT(8I10)
      WRITE(N6,5200)NHPRTI(JPRT(I),I=1,NHPRT)
 5200 FORMAT (32HOPRINTED OUTPUT AT THE FOLLOWING,I3/10H JUNCTIONS,//
     1 (10X,9I10))
C******** CONDUIT NUMBERS FOR DETAILED PRINTOUT
      READ(N5,5180)(CPRT(I),I=1,NOPRT)
      WRITE(N6,5220)NOPRT,(CPRT(I),I=1,NOPRT)
 5220 FORMAT (/,11X,'AND FOR THE FOLLOWING',I3,' CONDUITSW(10X19I10))
C******** JUNCTION NUMBERS FOR PLOTTING
      IF (NPLT.LE.0) GO TO 100
      READ (N5,5180) (JPLT(N),N=1.NPLT)
```

```
      WRITE(N6,5240) NPLT,(JPLT(N),N=1,NPLT)
 5240 FORMAT (10WATER SURFACE ELEVATIONS WILL BE PLOTTED FOR THE FOLLOWI
     1NG       JUNCTIONSW(10X,9I10))
C******** CONDUIT NUMBERS FOR PLOTTING
  100 IF (LPLT.LE.0) GO TO 120
      READ (N5,5180) (KPLT(N),N=1rLPLT)
      WRITE(N6,5260) LPLT,(KPLT(N),N=1,LPLT)
 5260 FORMAT('OFLOW RATE WILL BE PLOTTED FOR THE FOLLOWING',I&I' CONDUIT
     1S'//(10)(t9I10))
  120 CONTINUE

C******** CONDUIT DATA
C
      DO 260 N=1,NDIM
      READ (N5,5280) NCOND(N),(NJUNC(N,K),K=1,2) NKLASS(N),AFULL(N)
     1 ,DEEP(N),WIDE(N),LEN(N),(ZP(NIK),K=1,2),ROUGH(N),STHETA(N),
     2 SPHI(N)
 5280 FORMAT (4I5,9F5.0)
      IF (NCOND(N).GT.90000) GO TO 280
      IF(ROUGH(N) .LE. 0.0) ROUGH(N) = 0.014
      KLASS=NKLASS(N)
C                 NKLASS=1 CIRCULAR PIPE
C                 NKLASS=2 RECTANGULAR PIPE
C                 NKLASS=3 HORSESHOE PIPE
C                 NKLASS=4 EGGSHAPED PIPE
C                 NKLASS=5 BASKETHANDLE PIPE
C                 NKLASS=6 TRAPEZOIDAL CHANNEL
C                 NKLASS=7-8 ORIFICES (SEE BELOW)
      GO TO (140,160,180,200,220,230),KLASS
  140 RFULL(N)=DEEP(N)/4.
      AFULL(N)=(3.1415926/4.)*DEEP(N)**2
      WIDE(N)=DEEP(N)
      GO TO 240
  160 RFULL(N)=(WIDE(N)*DEEP(N))/(2.*WIDE(N)+2.*DEEP(N))
      AFULL(N)=WIDE(N)*DEEP(N)
      GO TO 240
  180 RFULL(N)=0.25381*DEEP(N)
      GO TO 240
  200 RFULL(N)=0.19311*DEEP(N)
      GO TO 240
  220 RFULL(N)=0.28800*DEEP(N)
      GO TO 240
  230 AFULL(N)=DEEP(N)*(WIDE(N)+DEEP(N)/2.*(STHETA(N)+SPHI(N)))
      RFULL(N)=AFULL(N)/(WIDE(N)+DEEP(N)*(SORT(1.+STHETA(N)**2.)
     1 +SORT(1.+SPHI(N)**2.)))
      IF(WIDE(N).LE.O.) WIDE(N) = 0.01
  240 CONTINUE
  260 CONTINUE
  280 NC=N-1
      NTC=NC
C******** PRINT CONDUIT DATA
```

```
          WRITE(N6,2999)
          WRITE(N6,5060) ALPHA
          WRITE(N6,5300)9
     5300 FORMAT(1H            CONDUIT    LENGTH    CLASS      AREA    MANNING
         1   MAX WIDTH          DEPTH    JUNCTIONS          INVERT HEIGHT
         2RAPEZOID//r
         27X,'NUMBER      (FT)               (SO FT)    COEF.            (FT)
         3      (FT)      AT ENDS         ABOVE JUNCTIONS       SIDE SLOPE')
          NSPRT=-1
          DO 300 N=1,NC
          IF((ZP(Nr1).E0.0.).AND.(ZP(Nr2).E0.0.)) GO TO 296
          GO TO 297
      296 IF(NKLASS(N).E0.6) WRITE(N6,5320)NrNCOND(N)ILEN(N),NKLASS(N),
         *AFULL(N)PROUGH(N),WIDE(N),DEEP(N),(NJUNC(NIK),K=1,2),
         *STHETA(N),SPHI(N)
          IF(NKLASS(N).NE.6) WRITE(N6,5321)N,NCOND(N),LEN(N),NKLASS(N),
         *AFULL(N),ROUGH(N),WIDE(N),DEEP(N),(NJUNC(NrK),K=1,2)
          GO TO 300
      297 IF(NKLASS(N).EQ.6) WRITE(N615322)NrNCOND(N),LEN(N),NKLASS(N),
         *AFULL(N)FROUGH(N),WIDE(N),DEEP(N),(NJUNC(N,K),K=1,2),
         *(ZP(NrK),K=1,2),STHETA(N),SPHI(N)
          IF(NKLASS(N).NE.6) WRITE(N6,5323)NOC0ND(N)tLEN(N),NKLASS(N),
         *AFULL(N),ROUGH(N),WIDE(N),DEEP(N),(NJUNC(NIK),K=1,2),
         *(ZP(N,K),K=1,2)
     5320 FORMAT(I4,I9,F9.0,I7,F12.2,F9.3,F15.2,F13.2,2X,2I6,
         *28X,2F5.2)
     5321 FORMAT(I4,I9,F9.0,I7,F12.2rF9.3,F15.2,F13.2,2X,2I6)
     5322 FORMAT(14,I9,F9.0,I7rF12.2,F9.3,F15.2,F13.2,2X,216,8X,F5.2,
         *2X,F5.2,8X,2F5.2)
     5323 FORMAT(I4rI9rF9.0,171F12.24·9.3,F15.2,F13.2,2X,2I6r8X,F5.2,
         *2X,F5.2)
    C 300 CONTINUE
    C******** CHECK FOR VIOLATION OF WAVE TRAVEL/CONDUIT LENGTH RATIO
          DO 320 N=1,NC
          RATIO=SORT(DEEP(N)*32.2)*DELT/LEN(N)
          IF(RATIO.GT.1.)WRITE(N6,5335)NCOND(N),RATIO
     5335 FORMAT(' **** WARNING **** (C*DELT/LEN) IN CONDUIT':
                        I6r' IS',F5.1,' AT FULL DEPTH.')
      320 CONTINUE
    C
    C******** JUNCTION DATA
    C
          DO 380 J=1,NDIM
          READ (N5,5340) JUN(J),GRELEV(J),Z(J),OINST(J)
     5340 FORMAT (I5r3F5.0)
          IF (JUN(J).GT.90000) GO TO 400
          ZCROWN(J)=Z(J)
          JSKIP(J)=0
    C******** SET UP JUNCTION CONNECTIVITY ARRAY FROM PIPE DATA
          LOC=1
          SUMAL(J)=0.
          DO 360 N=1,NC
          DO 360 K=1,2
          IF(NJUNC(NrK)-JUN(J)) 360,340,360
      340 NCHAN(J,LOC)=N
```

```
      LOC=LOC+1
  360 CONTINUE
      IF(LOC.GT.1) GO TO 380
      WRITE(N6t5350) JUN(J)
 5350 FORMAT('0**** WARNING **** JUNCTION',I6,' IS NOT ASSOCIATED WITH
             'ANY PIPE')
      JSKIP(J)=1
  380 CONTINUE
  400 NJ=J-1
C
C******** CONVERT CONDUIT CONNECTIVITY NUMBERS TO INTERNAL SYSTEM
C******** ASSIGN POSITIVE DOWNSTREAM FLOW CONVENTION
      DO 600 N=1,NC
      DO 540 K=1/2
      DO 500 J=1/NJ
      IF(NJUNC(N,K)-JUN(J)) 500,520,500
  500 CONTINUE
      WRITE(N6,5390) NJUNC(N,K),NCOND(N)
 5390 FORMAT('0**** ERROR **** JUNCTION',I6,' ON CONDUIT',I6,' IS NOT
             'CONTAINED IN JUNCTION DATA')
      NSTOP=NSTOP+1
  520 NJUNC(NtK)=J
  540 CONTINUE
      NL=NJUNC(Nt1).
      NH=NJUNC(N,2)
      ZP(N,1) = Z(UL) + ZP(N,1)
      ZP(Nt2)=Z(NH)+ZP(Nt2)
      IF(ZP(N,1)-ZP(N,2)) 560,580/580
  560 TEMP=ZP(Ntl)
      ZP(N11)=ZP(Nt2)
      ZP(Nr2)=TEMP
      NJUNC(N,1) = NH
      NJUNC(N,2)=NL
      NL=NJUNC(Ntl)
      NH=NJUNC(N,2)
  580 IF((ZP(Nt1)+DEEP(N)).GT.ZCROWN(NL)) ZCROWN(NL)=ZP(Nt1)+DEEP(N)
      IF((ZP(Nt2)+DEEP(N)).GT.ZCROWN(NH)) ZCROWN(NH)=ZP(Nr2)+DEEP(N)


      IF(ZCROWN(NL).LE.GRELEV(NL)+0.001) GO TO 590
      WRITE(N6,5395) NCOND(N),JUN(NL)
      ZCROWN(NL)=GRELEV(NL)-0.01
      NSTOP=NSTOP+1
  590 IF(ZCROWN(NH).LE.GRELEV(NH)+0.001) GO TO 600

      WRITE(N6,5395) NCOND(N),JUN(NH)
 5395 FORMAT('0**** ERROR **** CONDUIT',I6,' HAS CAUSED ZCROWN OF '
             'JUNCTION',I6,' TO LIE ABOVE THE SPECIFIED GROUND ELEV.')
      ZCROWN(NH)=GRELEV(NH)-0.01
      NSTOP=NSTOP+1
  600 CONTINUE
C******** PRINT JUNCTION DATA
      WRITE(N6,2999)
      WRITE(N6,5060) ALPHA
      WRITE(N6,5360)
 5360 FORMAT(1H ,5X,'JUNCTION     GROUND     CROWN      INVERT     QINST'
```

```
        1,15)WCONNECTING CONDUITSV7X,'NUMBER',7)(l'ELEV.',5WELEV.',6X,
        l'ELEV.',5X,i(CFS)V)
         DO 460 J=1,NJ
         MPT=0
         NZP = 0
         DO 420 1=1,8
         K1 = NCHAN(JiI)
         IF(Kl.E0.0) GO TO 440
         IDUM(I) = NCOND(K1)
         MPT=MPT+1
C******** = CHECK0   FOR ALL CONDUITS ABOVE JUNCTION INVERT
         J J
         IF(NJUNC(K171).E0.J) JJ = 1
         IF(JJ.NE.1) JJ =
         IF(ZP(KlIJJ).GT.Z(J)) NZP = NZP 4. 1
  420 CONTINUE
  440 CONTINUE
C
         WRITE(N6/5380) J,JUN(J),GRELEV(J),ZCROWN(J),Z(J),OINST(J)/
        1(IDUM(K),K=1,MPT)
 5380 FORMAT(I4,I9,F12t2,F10.2,F11.2,F10.2,15X,8.17)
         IF(NZP.LT.MPT) GO TO 450
         WRITE(N6,5381) JUN(J)
 5381 FORMAT(1X/I**** ERROR **** ALL CONDUITS CONNECTING',
        *' TO JUNCTION ',I6,' LIE ABOVE THE JUNCTION INVERT')
         NSTOP = NSTOP + 1
  450 CONTINUE
         OINST(J)=OINST(J)*DELT
  460 CONTINUE
  480 CONTINUE
         WRITE(N6,5382)
 5382 FORMAT(///,64(2H--)//)
C******** CHECK FOR HIGH PIPE
         DO 495 N=1,NC
         DO 495 K=1,2
         J = NJUNC(N,K)
         IF(ZP(N,K),EQ.Z(J)) GO TO 495
         DO 490 Kt< = 1,8
         NKK = NCHAii(J,KK)
         IF(NKK.EO.N) GO TO 490
         IF(NKK.EO.O.OR.NKK.GT.NC) GO TO 495
         JJ = 0
         IF(NJUNC(NKK,1).EO.J) JJ = 1
         IF(JJ.NE.1) JJ =
         IF(ZP(N,K).LE.ZP(NKK,JJ)   DEEP(NKK)) GO TO 495
  490 CONTINUE
  491 WRITE(N6,5392) NCOND(N),JUN(J)
 5392 FORMAT(' ****** ERROR ****** THE INVERT OF
        *'CONDUIT',I6,' LIES ABOVE THE CROWN OF ALL OTHER 1,
        *'CONDUITS AT JUNCTION',I6)
         NSTOP = NSTOP   1
  495 CONTINUE
C
C******** STORAGE JUNCTION DATA
C
         PO 640 1=1120
```

```
      READ(N5,5391) JSTORE(I),ZTOP(I),ASTORE(I)
 5391 FORMAT(I5,2F10.0)
      IF.5TORE(I).GT.90000) GO TO 645
  640 CONTINUE
  645 NSTORE=I-1
      IF(NSTORE) 647,647,644
  644 WRITE(N6,2999)
      WRITE(N6r5060) ALPHA
      WRITE(N6,5398)
 5398 FORMAT('0',27(2H- ),'STORAGE JUNCTION DATA',27(2H -),/)
      WRITE(N6,5495)
 5495 FORMAT(1X,'STORAGE JUNCTION',6X,'SURFACE AREA',6X,'VOLUME',
     *6X,'CROWN ELEVATION',/,26X,'(FT2)1,11X,'(CF)1,12X11(FT)')
C******** CONVERT TO INTERNAL NUMBER SYSTEM
      DO 646 I=1,NSTORE
      DO 648 J=1,NJ
      IF(JSTORE(I)-JUN(J)) 648,650,648
  648 CONTINUE
      WRITE(N6,5494) JSTORE(I)
 5494 FORMAT('0**** ERROR **** STORAGE JUNCTION           IS. NOT
     * CONTAINED IN JUNCTION DATA')
      NSTOP=NSTOP+1
  650 JSTORE(I)=J
      ZCROWN(J) = ZTOP(I)
      IF(ZCROWN(J).GT.GRELEV(J)) GRELEV(J) = ZCROWN(J) + 0.1
      JSKIP(J)=0
      CF = ASTORE(I)*(ZTOP(I)-Z(J))
      WRITE(N6r5399)(JUN(JSTORE(I))),ASTORE(I),CF,ZTOP(I)
 5399 FORMAT(6X,I5113X,F8.2,7X,F8.2,10X,F6.2)
  646 CONTINUE
      NTL=NTL+NSTORE
  647 CONTINUE
C
C******** INITIALIZE NTC AND NTL
      NTC=NC
      NTL=NC
C
C******** ORIFICE DATA
C
      DO 690 I=1,60
      N=NTC+I
      READ(N5,5400) (NJUNC(N,K),K=1,2),NKLASS(N),AORIF(I),CORIF(I),
     *ZP(N,1)
 5400 FORMAT(3I5,3F5.0)
      IF(NJUNC(N,1).GE.90000) GO TO 695
  690 CONTINUE
  695 NORIF = I-1
      NTC = NTC + NORIF
      NTL = NTL + NORIF
  790 IF(NORIF) 696,696,697
  697 WRITE(N6,5420)
      DO 730 I=1,NORIF
      N = NTC - NORIF + I
      WRITE(N6,5440)(NJUNC(NYK),K=1,2)INKLASS(N),AORIF(I),
     *CORIF(I),ZP(N,I)
C                     '1\......____Replace I by 1
```

```
C******** CONVERT TO INTERNAL NUMBER SYSTEM
      LORIF(I)=N
      NCOND(N)=N+90000
      DEEP(N)=SQRT(4.*AORIF(I)/3.14159)
      WIDE(N)=DEEP(N)
      AFULL(N)=AORIF(I)
      RFULL(N)=DEEP(N)/4.
      CLEN=2.*DELT*SORT(32.2*DEEP(N))
      LEN(N)=AMAX1(200.tCLEN)
      ROUGH(N)=1.49*RFULL(N)**.67/(CORIF(I)*SORT(LEN(N)*64,4))
      NKLASS(N)=NKLASS(N)+6
C                NKLASS(N)=1, NKLASS(N)=7 - SIDE OUTLET
C                NKLASS(N)=2, NKLASS(N)=3 - BOTTOM OUTLET (SUMP)
C******** SET ZP(N,1) FOR BOTTOM OUTLET
      IF(NKLASS(N).EO. 8) ZP(Nt1)=-0,96*DEEP(N)
      DO 770 K=1,2
      DO 700 J=1tNJ
      IF(NJUNC(N,K)-JUN(J)) 700,720,700
  700 CONTINUE
      WRITE(N6,5450) NJUNC(N,K)
 5450 FORMAT('0**** ERROR **** ORIFICE JUNCTION',I6p' IS NOT CONTAINED
             'IN JUNCTION DATA')
      NSTOP=NSTOP+1
  720 NJUNC(N,K)=J
C******** SET ZP(N,1) AND ZP(N,2) ELEVATIONS
      IF(K.EQ.2) GO TO 725
      ZP(N,K)=ZP(N,K)+Z(J)
      ZP(N,2) = ZP(N,1) - 0.1
  725 CONTINUE
C
C.... CHECK GROUND ELEVATION
      IF(ZP(N,K)+DEEP(N).LT. GRELEV(J)) GO TO 730
      WRITE(N6,5455) JUN(J)
 5455 FORMAT('0**** ERROR **** ORIFICE TOP LIES ABOVE GROUND ELEVATION'
        AT JUNCTION',I7)
      NSTOP=NSTOP+1
C
  730 CONTINUE
      DO 740 KK=1,8
      IF(NCHAN(J,KK)) 760,760,740
  740 CONTINUE
  760 NCHAN(J,KK)=N
  770 CONTINUE
C******** CHECK GRAVITY FLOW DIRECTION
      IF(ZP(N,1) .GT. ZP(N,2)) GO TO 780
      J2=NJUNC(N12)
      WRITE(N6,5458) JUN(J2)
 5458 FORMAT('0**** ERROR **** ORIFICE OUTLET AT JUNCTION',I7, 'IS
     1HIGHER THAN INLET')
      NSTOP=NSTOP+1
  780 CONTINUE
 5420 FORMAT(101,28(2H- ),'ORIFICE DATA',28(2H -),/,
     *14X,'JUNCTION',18X,'TYPE',20X,'AREA',18X,'DISCHARGE'!
     *13)WHEIGHT ABOVE',/,10X,'FROM',9X,'TO',39X7'(FT2)',
     *18X,'COEFF.',17X,'JUNCTION'i
 5440 FORMAT(9X,I5,7X,I5,14X,I4,18X,F7.2,18X,F6.4,
```

```
          *18X,F6#3,4X,F6.3)
   696 CONTINUE
C
C******** WEIR DATA
C
C**** THIS ROUTINE HAS BEEN MODIFIED TO TRANSFER
C**** WEIR DISCHARGES FROM NODE TO NODE RATHER
C**** THAN FROM NODE TO CONDUIT
           DO 820 I=1,60
           N=NTC+I
           READ(N5,5460) (NJUNC(N,K),K=1,2),KWEIR(I),YCREST(I)PYTOP(I),
          2 WLEN(I),COEF(I)
  5460 FORMAT(3I5,4F5.0)
           IF(NJUNC(N▸1).GE,90000) GO TO 840
   820 CONTINUE
   840 NWEIR=I-1
           IF(NWEIR) 1040,1040,860
   860 WRITE(N6,5480)
  5480 FORMAT(//,'0','29(2H- ),'WEIR DATA',29(2H -),//,
          *8Xi/JUNCTION'117X,'LINK',11X,'TYPE',11:6'CREST',11X,'WEIR'Y
          *11X,'WEIR'19X,'DISCHARGE',/,2X,'FROM',12X,'TO',12X,
          *'NUMBER',23X,'HEIGHT(FT)',7X,'TOP(FT)',6X,'LENGTH(FT)',
          *8X,'COEFF.')
  5487 FORMAT(1X,I5,10X,I5112X,I5111X,I2912X,F5.2,10X,F5.2,
          *10:0F5.2,10X,F5.2)
           DO 1020 I=1,NWEIR
           N1=NTC+I
           LWEIR(I)=N1
           NCOND(N1)=900001.N1
           COEFS(I)=0.
           WRITE(N6,5487)(NJUNC(N1,K),K=1,2),NCOND(N1),KWEIR(I),
          *YCREST(I)YYTOP(I),WLEN(I),COEF(I)
           DO 875 K=1,2
           IF(NJUNC(N1,K).E0.0) GO TO 375
           DO 870 J=1,NJ
           IF(NJUNC(N1,K).EO.JUN(J)) GO TO 371
   870 CONTINUE
           WRITE(N6,5490) NJUNC(N1,K)
  5490 FORMAT('0**** ERROR **** WEIR JUNCTION',I6,' IS NOT CONTAINED IN J
          2UNCTION DATA')
           NSTOP=NSTOP+i
   871 NJUNC(N1,K)=J
           DO 873 NK=1,8
           IF(NCHAN(J,KK)) 874,874,873
   873 CONTINUE
   374 NCHAN(J,KK) = N1
   875 CONTINUE
  1020 CONTINUE
           NTL=NTL+NWEIR
  1040 CONTINUE
C
C****t*** PUMP DATA
C
C***'t NOTE .-- ONLY ONE INFLUENT PIPE MAY BE CONNECTED TO AN OFF-LINE
C              PUMP NODE
           DO 1060 I=1,20
```

```
         N=NTL+I
         READ(N5,5540) (NJUNC(NIK),K=1,2),IPTYP(I),VWELL(I),
        *(PRATE(I,K),K=1,3),(VRATE(IIK),K=1,3)
   5540 FORMAT(3I5,7F5.0)                                  •
C
C         IPTYP =-1 OFF-LINE PUMP OPERATES ON WET WELL VOLUME
C
C         IPTYP = 2 IN-LINE PUMP OPERATES ON HEAD AT JUNCTION
C
         IF(NJUNC(N,1).GE.90000) GO TO 1080
   1060 CONTINUE
   1080 NPUMP=I-1
C******** PRINT PUMP NODES
         IF(NPUMP) 1260,1260,1100
   1100 WRITE(N6,5560)
   5560 FORMAT('0',30(2H- ),'PUMP DATA',30(2H
        *10X,'JUNOTIONS',8X,'TYPE',9X,'INITIAL VOLUME',14X,
        *'PUMP RATE, CFS',15X,'VOL STAGES, FT31,11X,'WET WELL',
        */,8X,'FROM',5X,'TO',21X,'IN WELL, FT3',11X,'1',11X+12',
        *11X,'31,10X1'1',11X,12',11X, VOLUME? FT3')
         DO 1120 I=1,NPUMP
         N=NTL+I
   1120 WRITE(N6,5580) II(NJUNC(N,K),K=1,2),IPTYP(i),VWELL(I),
        *(PRATE(I,K),K=1,3),(VRATE(I,K),K=1,3)
   5580 FORMAT(1X,I3,2(3X,I5),6X,I5,10X,F10.0,8X,F10.0,1X,
        *F10.0,1X,F10.0,3X,F10.0,1X,F10.0,7X,F10.0)
C******** CONVERT TO INTERNAL NUMBER SYSTEM
         DO 1240 I=1,NPUMP
         N=NTLtI
         LPUMP(I)=N
         NCOND(N)=Nt99,000
         DO 1220 K=1,z
         DO 1140 J=1,NJ
         IF(NJUNC(N,K)-JUN(J)) 1140,1160,1140
   1140 CONTINUE
         WRITE(N6,5590) NJUNC(N,K)
   5590 FORMAT('0**** ERROR **** PUMP JUNCTION'I6,' IS NOT CONTAINED IN
                'JUNCTION DATA')
         NSTOP=NSTOP+1
   1160 NJUNC(N,K)=J
         DO 1180 NK=1,8
         IF(NCHAN(J,KK)) 1200,1200,1180
   1130 CONTINUE
   1200 NCHAN(J,KK)=N
         IF(IPTYP(I).E0.2) GO TO 1220
         IF(KK.LE.2) GO TO 1220
         IF(K.EQ.2) GO TO 1220
         WRITE(N6,5595) JUN(J)
   5595 FORMAT('0**** ERROR **** MORE THAN ONE PIPE IS INFLUENT TO PUMP JU
        .NCTION ',I6)
         NSTOP=NSTOP+1
   12~0 CONTINUE
C******** SET JSKIP AND INFLOW INDEX FOR PUMP NODE
         JP=NJUNC(Npl)
         JSKIP(JP) = 0
         IF(IPTYP(I).EG.2) GO TO 1235
```

```
     -    JSKIP(JP) = 1
          Z(JP) = -100.
 1235 CONTINUE
          JPFUL(I)=1
C
 1240 CONTINUE
          NTL=NTL+NPUMP
 1260 CONTINUE
C
C******** OUTFLOW DATA FOR OUTFALLS WITHOUT TIDE GATES
C
          DO 1280 1=1,25
          READ(N5,5600) JFREE(I)
 5600 FORMAT(I5)
          IF(JFREE(I).GE.90000) GO TO 1300
 1280 CONTINUE
 1300 NFREE=I-1
C******** PRINT OUTFLOW NODES
          IF(NFREE) 1400,1400,1320
 1320 WRITE(N6,5616)
 5616 FORMAT(//,'0',27(2H- ),'FREE OUTFALL DATA',27(2H -)//)
          WRITE(N6,5620) (JFREE(I),I=1,NFREE)
 5620 FORMAT (10X,'FREE OUTFLOW AT JUNCTIONS',4X,917/(39X,9I7))
C******** CONVERT TO INTERNAL NUMBER SYSTEM
 1340 DO 1390 I=1,NFREE
          DO 1360 J=10J
          IF(JFREE(I)-JUN(J)) 1360,1380,1360
 1360 CONTINUE
          WRITE(N615630) JFREE(I)
 5630 FORMAT('0**** ERROR **** FREE OUTFALL JUNCTION',I6,' IS NOT
     •          'CONTAINED IN JUNCTION DATA')
          NSTOP=NSTOPt1
 1380 JFREE(I)=J
          N=NTEJI
          NJUNC(Nr1)=J
          NJUNC(Nr2)=0
          NCHAN(.1,2)=N
          NCOND(N)=N+90000
          JSKIP(J)=1
 1390 CONTINUE
          NTL=NTL+NFREE
 1400 CONTINUE
C
C******** OUTFALL DATA FOR OUTFALLS WITH TIDE GATES
C
          DO 1420 I=1,25
          READ(N5,5640) JGATE(I)
 5640 FORMAT(I5)
          IF(JGATE(I).GE.90000) GO TO 1440
 1420 CONTINUE
 1440 NGATE=1-1
C****.*** PRINT TIDE GATE NODES
          IF(NGATE) 1520,1520,1460
          WRITE(N6,5656)
 5656 FORMAT(//,'0',25(2H- ),'TIDE GATE OUTFALL BATA',25(2H -),//)
 1460 WRITE(N6,5660) (JGATE(I),I=1,NGATE)
```

```
 5660 FORMAT(10X,'PIPE OUTFALLS WITH TIDE GATES AT JUNCTIONS',8I7/
     *(52X,8I7))
C******** CONVERT TO INTERNAL NUMBER SYSTEM
      DO 1510 I=1,NWATE
      DO 1480 J=1,NJ
      IF(JGATE(I)-JUN(J)) 1480,1500,1480
 1480 CONTINUE
      WRITE(N6,5662) JGATE(I)
 5662 FORMAT('0**** ERROR **** TIDE GATE JUNCTION',I6,' IS NOT '
     .            'CONTAINED IN JUNCTION DATA')
      NSTOP=NSTOP+1
 1500 JGATE(I)=J
      N=NTL+I
      NJUNC(N,1)=J
      NJUNC(Nt2)=0
      NCHAN(J,2)=N
      NCOND(N)=N+90000
      JSKIP(J)=1
 1510 CONTINUE
      NTL=NTLrNGATE
 1520 CONTINUE
C******** INTERNAL CONNECTIVITY INFORMATION
      WRITE(N6,2999)
      WRITE(N6,5060) ALPHA
      WRITE(N6,5665)
 5665 FORMAT (////'0',23(2H- ),' INTERNAL CONNECTIVITY INFORMATION',
     *23(2H -)//)
      WRITE(N6,5670)
 5670 FORMAT ('          CONDUIT     JUNCTION      JUNCTION'/)
      N1=NCtl
      DO 1525 N=N1,NTL
      J1=NJUNC(N,1)
      J2=NJUNC(Nt2)
      IF(J2.GT,O) J2 = JUN(J2)
      WRITE(N6,5675) iiCOND(N),JUN(J1),J2
 5675 FORMAT(4XII11,2I13)
 1525 CONTINUE
 1527 CONTINUE
      IF(NJ.LE.NDIM) GO TO 1530
      WRITE(N6,5676)
 5676 FORMAT('0**** ERROR **** TOTAL NUMBER OF jUNCTIONS(INCLUDING WEIRS
     .) EXCEED PROGRAM DIMENSIONS, NJ=',I4)
      NSTOP=NSTOP+1
 1530 CONTINUE
      IF(NTL.LE.NDIM) GO TO 1535
      WRITE(N6,5677) NTL
 5677 FORMAT('0***)! ERROR **** TOTAL NUMBER OF LINKS EXCEEDS PROGRAM DIM
     .ENSIONS,NTL= '14)
      NSTOP=NSTOP+1
 1535 CONTINUE
C
C******** TIDAL BOUNDARY DATA
C
      READ(N5,5720) NTIDE,A1,A2,A3,A4,A5,A6,A7,W
 5720 FORMAT (I5,8F5.0)
      GO TO (1S00,1790,1780,1760),NTIDE
```

```
C
C         NTIDE=1 NO CONTROL WATERSURFACE AT THE OUTFALLS
C               2 OUTFALL CONTROL WATERSURFACE AT CONSTANT ELEVATION=A1
C               3 TIDE COEFFICIENTS READ IN
C               4 COMPUTE TIDE COEFFICIENTS
C
 1760 READ(N5,5740) KO,NI,NCHTID
 5740 FORMAT (3I5)
      READ (N5,5760) (TT(I),YY(I)▸I=1▸NI)
 5760 FORMAT (8F10.0)
      CALL TIDCF(KOINI,NCHTID)
      GO TO 1800
 1780 WRITE(N6r5780) AltA2,A304,A5,A6,A7,W
      W=2.*3.14159/W
 5780 FORMAT('OTIDAL COEFFICIENTS.',7F10.4/'OTIDAL PERIOD (HRS).', F8.2)
      GO TO 1800
 1790 WRITE(N6,5790) Al
 5790 FORMAT('OOUTFLOW CONTROL WATER SURFACE ELEVATION IS',F7.2,' FEET')
 1800 CONTINUE
      W=W/3600.
C
C******** SET PRINT : PLOT ARRAYS IN INTERNAL NUMBER SYSTEM
      DO 1550 K=1,NOPRT
      DO 1540 N=1,NTC
      IF(NCOND(N)-CPRT(K)) 1540,1545,1540
 1540 CONTINUE
      WRITE(N615678) CPRT(K)
 5678 FORMAT('0**** ERROR **** CONDUIT',I6,' REQUESTED FOR PRINTOUT IS '
              'NOT CONTAINED IN CONDUIT DATA')
      NSTOP=NSTOP+1
 1545 CPRT(K)=N
 1550 CONTINUE
      IF(LPLT) 1640,1640,1560
 1560 DO 1620 K=1,LPLT
      DO 1580 N=1,NTL
      IF(NCOND(N)-KPLT(K)) 1580,1600,1580
 1580 CONTINUE
      WRITE(N6,5680) KPLT(K)
 5680 FORMAT('0**** ERROR **** CONDUIT',I6,' REQUESTED FOR PLOTTING IS '
               'NOT CONTAINED IN CONDUIT DATA')
      NSTOP=NSTOP+1
      GO TO 1620
 1600 KPLT(K)=N
 1620 CONTINUE
 1640 DO 1660 I=1,NHPRT
      DO 1650 J=1,NJ
      IF(JUN(J)-JPRT(I)) 1650,1655,1650
 1650 CONTINUE
      WRITE(N615690) JPRT(I)
 5690 FORMAT('0**** ERROR **** JUNCTION',I6,' REQUESTED FOR PRINTOUT
              'IS NOT CONTAINED IN JUNCTION DATA')
      NSTOP=NSTOP+1
 1655 JPRT(I)=J
 1660 CONTINUE
      IF(NPLT.LE.0) GO TO 1740
      DO 1720 N=1,NPLT
```

```
         DO 1680 J=1,NJ
         IF (JUN(J).EQ.JPLT(N)) GO TO 1700
  1680   CONTINUE
         WRITE(N6,5700) JPLT(N)
  5700   FORMAT('0**** ERROR **** JUNCTION',I6,' REQUESTED FOR PLOTTING
                'IS NOT CONTAINED IN JUNCTION DATA')
         NSTOP=NSTOP+1
         GO TO 1720
  1700   JPLT(N) = J
  1720   CONTINUE
  1740   CONTINUE
C
C******** CONDUIT INITIALIZATION
         DO 1820 N=1,NTC
  1820   ROUGH(N)=32.2*ROUGH(N)**2/2.208
  1824   CONTINUE
C
C**** READ AND WRITE INITIAL FLOWS,VELOCITIES, AND HEADS
C****      FOR ALL CONDUITS AND JUNCTIONS (INCLUDING INTERNAL).

c    WRITE(N6,2999)
         WRITE(N6,5060) ALPHA
         WRITE(N6,11)
    11   FORMAT(1X,20(2H- ),' SUMMARY OF INITIAL HEADS, FLOWS AND
        * VELOCITIES ',22(2H- ),/)
         READ(N5,10) (O(N),V(N),N=1,4)
         IF (O(1).LT.99999.) GO TO 5
         DO 7 I = 1   NTL
         Q(I)=0.
         V(I)=0.
     7   CONTINUE
         DO 8 J = 1 ,NJ
         Y(J) = O.
     8   CONTINUE
         WRITE(N6,31)
         GO TO 32                   Replace NTC by NTL
     5   CONTINUL,---
         IF(NTC:CT.5) GO TO 6
         READ(N5,10) (G(N),V(N),N=5,NTL)
     6   READ(N5,10) (Y(J),J=1,NJ)
    10   FORMAT(8F10.0)
         WRITE(N6,12)
    12   FORMAT(1H0p/CONDUIT NO.',3X,'FLOW(CFS)',3X,'VELOCITY(FPS)',7X,'CON
        1DUIT NO.',3X1'FLOW(CFS)',3)(t'VELOCITY(FPS)',7X,'CONDUIT NO.',3X,'F
        2LOW(CFS)',3X,'VELOCITY(FPS)'/' -----------------------------------
        3----',          7X,' ------------------------------------- ',
        47X,' ---------------------------------------- '///)
         DO 15 KKK=1,NTL,3
         KSTOP=KKK+2
         IF(KSTOP.GT.NTL) KSTOP=NTL
    15   WRITE(N6,16)(NCOND(KK), O(KK),V(KK),KK=KKK,KSTOP)
    16   FORMAT(4X,I5,8X,F5,119X,F5.1,14X,I5,3X,F5.1t.9X,F5.1,14X,I513X,F5.1
        2,9X,F5.1)
         WRITE(N6,2999)
         WRITE(N6,5060) ALPHA
         WRITE(N6720)
```

```
   20 FORMAT(1X,26(2H- )t' SUMMARY OF INITIAL DEPTHS ',26(2H- ),/)
      WRITE(N6p22)
   22 FORMAT(1H0,7X,'JUNCTION NO•',3Xp'DEPTH(FT)1,7X,'JUNCTION NO.',
     *3Xt'DEPTH(FT)1,7X,'JUNCTION NO.',3X,'DEPTH(FT)117X,
     *'JUNCTION NO.',3X,'DEPTH(FT)',/,8X,'--------------
------------------'t3(7X,'------------------------')///)
      DO 25 KKK=1,NJ,4
      KSTOP=KKK+3
      IF(KSTOP.GT.NJ) KSTOP=NJ
   25 WRITE(N6,26)(JUN(KK),Y(KK),KK=KKK,KSTOP)
   26 FORMAT(4(11X,I5,9X,F5.1))
   31 FORMAT(///,1X,'INITIAL HEADS, FLOWS AND VELOCITIES ARE ZERO')
   32 CONTINUE
C
C******** HYDROGRAPH INPUT INITIALIZATION
      TP=TZERO
      TEO=TZERO
    . DO 1840 L=1,NDIM
      ISW(L)=0
      DO 1840 K=1,2
 1840 QTAPE(L,K)=0.
      DO 1841 L=1,20
      JSW(L)=0
      DO 1841 K=1,2
 1841 OCARD(L,K)=0.
C
C******** INPUT HYDROGRAPH INFORMATION (TAPE)
C
      IF(N21) 1940,1940,1860
 1860 CONTINUE
      REWIND N21
      READ(N21) TITLE
      READ(N21)(SOURCE(I),I=1,5),NSTEPS,DUM,MJSW,NPOLL,TRIBA
      READ(N21) (ISW(L),L=1,MJSW)
      READ(N21)(DUM,DUM,J=1,NPOLL)
      READ(N21)(DUM,DUM,J=1,NPOLL)
      READ(N21)(DUM,J=1,NPOLL)
      READ(N21) QCONV
      WRITE(N6,2999)
      WRITE(N6,5060) ALPHA
      WRITE(N6,5840)(SOURCE(I),I=1,5),MJSW
 5840 FORMAT(' TAPE INPUT HYDROGRAPHS FROM '.5A4,' BLOCK AT
     1I6.' JUNCTIONS')
C******** CONVERT TO INTERNAL NUMBERS
      DO 1920 L=1,MJSW
      DO 1880 J=1,NJ
      IF(ISW(L)-JUN(J)) 1880,1900,1880
 1880 CONTINUE
      WRITE(N6,5820) ISW(L)
 5820 FORMAT('OPROGRAM CANNOT MATCH HYDROGRAPH AT NODE',I7,' TO JUNCTION
     1 DATA')
      NSTOP=NSTOP+1
      GO TO 1920
 1900 ISW(L)=J
 1920 CONTINUE
C******** READ FIRST TWO HYDROGRAPH RECORDS
```

```
          READ(N21) T20,DUM,DUM,(OTAPE(L,1),(DUM,J=1,NPOLL),L=1:MJSW)
          WRITE(N6,5800) T20,MJSW
     5800 FORIAT('0****** SYSTEM INFLOWS (TAPE) AT ',F8.2,' HOURS FOR
         *I5, JUNCTIONS',/,50X,'JUNCTION/INFLOW(CFS)',/)
          WRITE(N6,5830)((JUN(ISW(L))IGTAPE(LI1)),L=1/MJSW)
          T2OtT2O*3600.
          READ(N21) T21DUM,DUMP(OTAPE(Lt2).(DUM,J=1,NPOLL)PL=1,MJSW)
          DO 1930 L=1rMJSW
          DO 1930 K=1,2
     1930 OTAPE(L,K)=OTAPE(L.K)*OCONV
          WRITE(N6,5810) 12
     5810 FORMAT('0****** SYSTEM INFLOWS (TAPE) AT 'rF8,2,' HOURS':
         *1        ( JUNCTION / INFLOW, CFS)',/)
          WRITE(N6,5830) ((JUN(ISW(L)),QTAPE(L,2)),L=1,MJSW)
          T2=T2*3600.
          NINREC=2
    C
    C******** INPUT HYDROGRAPH DATA (CARDS)' TYPE L
    C
     1940 IF(NJSW) 2040,2040,1960
     1960 READ(N5,5860) (JSW(L),L=1tNJSW)
     5860 FORMAT(16I5)
          WRITE(N6,2999)
          WRITE(N6,5060) ALPHA
    C******** CONVERT TO INTERNAL NUMBERS
          DO 2020 L=1,NJSW
          DO 1980 J=1,NJ
          IF(JSW(L)-JUN(J)) 1980,20001980
     1980 CONTINUE
          WRITE(N6r5820) JSW(L)
          NSTOP=NSTOPt1
          GO TO 2020
     2000 JSW(L)=J
     2020 CONTINUE
    C******** READ FIRST TWO HYDROGRAPH RECORDS
          READ(N5,5900) TEO,(OCARD(L,1),L=1tNJSW)
     5900 FORMAT(8F10.0)
          WRITE(N6r5829) TEO,NJSW
          WRITE(N6,5830) ((JUN(JSW(L)),OCARD(L,1)),L=10JS9)
     5829 FORMAT('0****** SYSTEM INFLOWS (CARDS) AT',F8.2t HOURS',
         *' FOR',I5,' JUNCTIONS',//)
     5830 FORMAT(1XtI51'/',F7.2,7(3X,I5,'/',F7.2))
          READ(N5,5900) TE,(GCARD(L,2),L=1tNJSW)
          WRITE(N6,5831) TE
     5831 FORMAT('0****** SYSTEM INFLOWS (CARDS) AT',F8,2,' HOURS',
         *'     ( JUNCTION / INFLOW,CFS )',/)
    C
          WRITE(N6,5830)((JUN(JSW(L)),OCARD(L,2))1L=1,NJSW)
          TEO = TEO*3600.
          TE=TE*3600.
          TIMEO=TEO
     2040 CONTINUE
    C******** OUTPUT HYDROGRAPH INITIALIZATION
          IF(N22.E0.0) GO TO 2050
          WRITE(N22)(ALPHA(I),I=1,40)
    C**** DETERMINE OUTFLOW NODES
```

```
      N1=NTC+1
      I=0
      DO 2045 N=N1INTL
      IF(NJUNC(N/2).NE.0) GO TO 2045
      I=I+1
      GOUT(I)=11(N)
      LOCNOS(I)=JUN(NJUNC(Nr1))
 2045 CONTINUE
      NLOCAT=I
      IF(NLOCAT.LE.100) GO TO 2048
      WRITE(N6,5850)
 5850 FORMAT(1X,'****ERROR--MORE THAN 100 OUTFALL JUNCTIONS****')
      NSTOP=NSTOP+1
 2048 DUM=0.
      NDUM=0
      WRITE(N22)(EXTRAN(I),I=195),NTCYC,DELT,NLOCATyNDUM,TRIBA
      WRITE(N22)(LOCNOS(K),K=1,NLOCAT)
      WRITE(N22) DUMDUM
      WRITE(N22) DUMDUM
      WRITE(N22) DUM
      CONV=1.
      WRITE(N22) CONV
      NHOUR=TZER0/3600.
      WRITE(N22) NHOURTIDATEZ,NHOURY(QOUT(K),DUM,K=1,NLOCAT)
 2050 IF(NSTOP.E0.0) GO TO 2060
      WRITE(N6,5920)NSTOP
 5920 FORMAT('0******** EXECUTION TERMINATED BECAUSE OF if
     *I2, DATA ERROR(S) ********')
      STOP
 2060 CONTINUE
C
      RETURN
      END
```

```
      SUBROUTINE INFLOW
C
C                 THIS SUBROUTINE SELECTS THE INPUT HYDROGRAPH
C                 ORDINATE FROM TAPE AND/OR CARDS
C
      COMMON/FILES/ N5IN6,N21IN22,NPOLLyNLOCATy000NVtIDATEZ,LOCNOS(100),
     1TRIBA
      COMMON/CONTR/ NTCYCYDELTOyDELTYDELT2YTZEROYALPHA(40),
     1 NJYNCYNTCYNTLyICYCYNJSWIMJSW,TIMEYTIME2tAltA2,A31A4.A5yA6,A7YW
C
      COMMON/JUNC/Y(187),YT(187),NCHAN(187,8),AS(187),Z(187),OIN(187),
     1 00U(187),OINST(187),GRELEV(187),JUN(187),ZCROWN(187),JSKIP(187)
     2 ISUMAL(187),SUM0(187)1SUMOS(187)1ASFULL(187)
C
C
C
      COMMON/HYFLOW/ ISW(187),QTAPE(187,2),JSW(65),OCARD(65,2),
     1 WATSH(187),TEO,TPIT2YTE,T2OrTIMEOYNSTEPSYNINREC .
C
C                          E X E C U T I O N
C
      DO 100 J=1,NJ
  100 QIN(J)=QINST(J)
C
C******** TAPE VALUES FROM WATERSHED MODEL ARE INTERPOLATED
      IF(MJSW) 280,2307120
  120 CONTINUE
      IF (TZERO-T2) 135,125,125
C******** NEW INPUT DATA REQUIRED
  125 CONTINUE
      T20=T2
      TP=T20
      DO 130 L=1,MJSW
  130 OTAPE(L,1)=QTAPE(LY2)
      IF (NINREC-NSTEPS) 132,132,131
  131 WRITE(N6,4980)
 4980 FORMAT ('O'y' TZERO IS LATER IN TIME THAN LAST RECORD ON TAPE FROM
     1 WATERSHED')
      STOP
  132 CONTINUE
      READ(N21) T2YDUMIDUMP(OTAPE(Ly1)1(DUMYJ=1,NPOLL).L=1yMJEW)
      DO 133 L=1,MJSW
      OTAPE(Lt2)=QTAPE(LY2)*QCONV
  133 CONTINUE
      NINREC=NINREC+1
      WRITE(N6,4999)
 4999 FORMAT(/y1XY64(2H- ),//)
      WRITE(N6,5000)T2
      WRITE(N6,5330)((JUN(ISW(L)),QTAPE(L,2)),L=1,MJSW)
      12=12*3600.
      GO TO 120
  135 CONTINUE
      IF (TIME-12) 220,140,140
  140 DO 160 L=1,MJSW
      J=ISW(L)
      SLOFE=CGTAPE(L72)-01APE(Ly1))/(12-120)
```

```
          01=QTAPE(L,1)+SLOPE*(TP-T20)
          02=QTAPE(L,2)
      160 OIN(J)=OIN(J)+0.5*(01+02)*(T2-TP)
          120=12
          TP=T20
          DO 180 L=1,MJSW
      180 QTAPE(L,1)=QTAPE(L,2)
          IF(NINREC-NSTEPS) 200,220,220
      200 READ(N21) T2t(QTAPE(Lr2),L=1,MJSW)
          TPT=T2/3600.
          NINREC=NINREC+1
          WRITE(N6,4999)
          WRITE(N6r5000) TPT
     5000 FORMAT(/,'0****** SYSTEM INFLOWS (TAPE) AT',F8.2,' HOURS',
         *'       ( JUNCTION / INFLOW, CFS)',/)
          WRITE(N6,5830)((JUN(ISW(L)),GTAPE(L,2)),L=1,MJSW)
          GO TO 120
    C******** NO NEW INPUT DATA REQUIRED
      220 DO 240 L=1rMJSW
          J=ISW(L)
          SLOPE=0.
          IF(T2•GT.T20) SLOPE=(QTAPE(L12)-QTAPE(L11))/(T2-120)
          Q1=QTAPE(L,1)+SLOPE*(TP-T20)
          02=QTAPE(L,1)+SLOPE*(TIME-T20)
      240 QIN(J)=QIN(J).1-0.5*(01+02)*(TIME-TP)
          TP=TIME
    C
    C******** CARD INPUT' VALUES ARE INTERPOLATED
      280 IF(NJSW) 420,420,300
      300 CONTINUE
          IF (TZERO-TE) 335,320,320
    C******** NEW INPUT DATA REQUIRED
      320 CONTINUE
          TEO=TE
          TIMEO=TE0
          DO 325 L=1,NJSW
      325 OCARD(L,1)=QCARD(L,2)
          READ(N5,5020) TE,(OCARD(L12),L=1,NJSW)
    C
          WRITE(N6,4999)
          WRITE(N6,5831) TE
     5831 FORMAT('0****** SYSTEM INFLOWS (CARDS) AT/7F3.2,' HOURS',
         *'    ( JUNCTION / INFLOW,CFS )',/)
    C
          WRITE(N6,5830)((JUN(JSW(L)),OCARD(L,2)),L=1,NJSW)
     5830 FORMAT(3X,I5,';',F7.2,7(3X,I5,'/',F7.2))
          WRITE(N615832)
     5832 FORMAT(//)
          TE=3600.*TE
          GO TO 300
      335 CONTINUE
          IF (TIME-TE) 380,338,338
      338 CONTINUE
          DO 340 L=1,NJSU
          J=JSW(L)
          SLOPE.OOARD!Lt2)-OCARD(L71))/(TE-TE0)
```

```
          01=0CARD(Lf1)+SLOPE*(TIMEO-TED)
          02=0CARD(L12)
      340 OIN(J)=0IN(J)1.0.5*(01+02)*(TE-TIMEO)
          TEO=TE
          TIMEO=TE0
          DO 360 L=1,NJSW
      360 OCARD(Lf1)=OCARD(L,2)
          READ(N5,5020) TE,(OCARD(L,2),L=1INJSW)
          WRITE(N6,4999)
          WRITE(N6,5831) TE
          WRITE(N6,5830)((JUN(JSW(L)),OCARD(Lf2)),L=1,NJSW)
          TE=3600.*TE
          WRITE(N6,5832)
     5020 FORMAT(8F10.0)
          GO TO 300
C******** NO NEW INPUT DATA REQUIRED
      380 DO 400 L=1,NJSW
          J=JSW(L)
          SLOPE=COCARD(Lt2)-0CARD(L,1))/(TE-TED)
          01=OCARD(Ly1)+SLOPE*(TIMEO-TED)
          02=OCARD(L11)+SLOPE*(TIME-TED)
          TTT=TIME-TIMED
          IF (TTT.GT.DELT) TTT=FELT
      400 OIN(J)=OIN(J)+0,5*(01+02)*TIT
          TIMEO=TIME
C
      420 DO 440 J=lyNJ
      440 OIN(J)=OIN(J)/DELT
          RETURN
          END
```

```
      SUBROUTINE OUTPUT
C
C                    THIS SUBROUTINE PRINTS OUTPUT
C                    * CONTROLS THE PRINTER PLOT ROUTINES
C
      COMMON/FILES/ N57N6,1421,N22,NPOLL,NLOCATtOCONV,IDATEZPLOCNOS(100),
     1TRIBA
      COMMON/CONTR/NTCYC,DELTO,DELTPDELT2rTZERO,ALPHA(40),
     1 NJ,NC,NTC,NTL'ICYC,NJSW,MJSW,TIME,TIME2pAllA2pA3rA4FA5,A6,A7,W
C
      COMMON/JUNC/Y(187),YT(187),NCHAN(187,8),AS(187),Z(187),OIN(187),
     1 OOU(187),OINST(187),GRELEV(187),JUN(187),ZCROWN(187),JSKIP(187)
     2 ISUMAL(187),SUMO(187),SUMOS(187),ASFULL(187)
C
      COMMON/PIPE/LEN(187),NJUNC(187,2),AFULL(187),AT(187),
     1 O(187),V(187),VT(187),DEEP(187)O(187),WIDE(187),RFULL(187),
     2 NKLASS(187),ZP(187,2),QT(187),OO(187),H(187,2),NCOND(187),
     3 ROUGH(187)
      REAL LEN
C
      COMMON/STORE/ NSTORE,JSTORE(20),ZTOP(20),ASTORE(20)
C
      COMMON/OUT/ NPRT,IPRT,NHPRT,JPRT(20),PRTH(100,20),PRGEL(20),
     1 NOPRT,CPRT(20),PRTV(100,20),PRTO(100,20),IDUM(12),ICOL(10),
  b  2 LTIME,NPLT,JPLT(20),YPLT(102,20),LPLT,KPLT(20),OPLT(102,20),
     3 TPLT(102),NPTOT,NSTART,INTER,PRTY(100,20)
  0   COMMON/ELEV/ ZINVRT,ZCRN,ZGRND,IPLT
  eg,,, INTEGER CPRT
C 0-0
      COMMON/      TLE(40),XLAB(11),YLAB(6),HORIZ(5),VERT(6)
  m x COMMON TAT VMAXX(187),OMAXX(187),DEPMAX(187),IVHR(187),
  .,E0          r OHR(187),IOMIN(187),IDHR(137),IDMIN(187),SURLEN(137),
     3SUMOIN,VLEFT
C - "f"
      DIMENSION VERTO(6)
      DATA VERTO/4HCONDI4HUIT 14H FLO,4HW      ,4HIN ,4HCFS /
C
C                              E X E C U T I O N
C******** PRINT CONTINUITY SUMMARY
C
C
      WRITE(N6,5002)
 5002 FORMAT(////,' ,23(2H- ),' CONTINUITY BALANCE AT END OF RUN
     *23(2H- ),/)
 5001 FORMAT(' TOTAL SYSTEM INFLOW VOLUME =',F12,0,' CU FT',/)
      WRITE(N6,5001) SUMOIN
      WRITE(N6,5004)
 5004 FORMAT(' JUNCTION OUTFLOWS AND',/,' STREET FLOODING',/)
      WRITE(N6,5005)
 5005 FORMAT(4X,'JUNCTION',2X,'OUTFLOW, FT3',/)
      DO 119 J=1,NJ
      IF(OOU(J).GT+0.) WRITE(N6,5003) JUN(J),OOU(J)
 5003 FORMAT(7X,I5,2X,F12.0)
      SUMOUT = SUMOUT t OOU(J)
  119 CONTINUE
      WRITE(N6,5007)
```

```
  5007 FORMAT(15:67(2H--))
        WRITE(N6,5008) SUMOUT
  5008 FORMAT(13WTOTAL',11X,F12.0,' CU FT',/)
        WRITE(N6,5009) VLEFT
  5009 FORMAT(' VOLUME LEFT IN SYSTEM =',5X,F12.0,' CU FT',/)
        PCTERR = ((SUMGIN-SUMOUT-VLEFT)/SUMOIN)*100.
        WRITE(N6,5006) PCTERR
  5006 FORMAT(' ERROR IN CONTINUITY, PERCENT =',F6.2)
 C******** PRINT H.G.L. AND WATER DEPTH AT NODES
 C
        NSTART=NSTART-LTIME*INTER
        TIMEO=TZERO+FLOAT(NSTART)*DELT
        DO 100 I=1,NHPRT
        MJPRT=JPRT(I)
        JPRT(I)=JUN(MJPRT)
   100 PRGEL(I)=GRELEV(MJPRT)
  5000 FORMAT('',20A4/'',20A4//).
        DO 120 I=1,NHPRT,6
        WRITE(N6,2999)
 2999 FORMAT('1',64(2H--)/'','ENVIRONMENTAL PROTECTION AGENCY',13X,40H*
     2***    EXTENDED TRANSPORT PROGRAM    ****,3X,'WATER RESOURCES DIVISI
     3ON'/'','WASHINGTON, D.C.              ':16X,4H****,32X,4H****,8X,
     4'CAMP DRESSER & MCKEE INC.'!':'                ',23X,4H
     5****,6X,'     ANALYSIS MODULE ',6X,4H****,8X,'ANNANDALE, VIRGINIA
     6')
        WRITE(N6,5000) ALPHA
        WRITE(N6,5020)


  5020 FORMAT (1HO' * * * * * * * * * * * * * * * * * * * * 
     1'TIME HISTORY OF H. G. L.                  * * * * * * * * * *',
           * * * * * * * * *1)
        WRITE(N6,5030)
  5030 FORMAT (56X,' (VALUES IN FEET)')
        IT=I+5
        IF(IT.GT.NHPRT) IT=NHPRT
        WRITE(N6,5040) (JPRT(L),L=IfIT)
  5040 FORMAT (1H0, 8X,6(7X,'JUNCTION',I5))
        WRITE(N6,5060) (PRGEL(L),t=IIIT)
  5060 FORMAT(' TIME', 2X,6(8X,' GRND',F7.2),/,' HR . MIN',6(7WELEV
     1 DEPTH'),/)
        LT=MINO(I+5,NHPRT)
        DO 120 L=1,LTIME
        TIME=(TIMEO+FLOAT((L-1)*INTER)*DELT)/3600.
        LTIMEH=IFIX (TIME)
        LTIMEM=IFIX((TIME-FLOAT(LTIMEH))*60.0+0.5)
   120 WRITE(N6,5080) LijMEHYLTIMEM,(PRTH(L,K),PRTY(L,K),K=I.LT)
  5080 FORMAT ('',I3,'. ,I2,2X,6(F12.2,F8.2))
 C
 C******** COMPUTE AND PRINT SUMMARY STATISTICS FOR JUNCTIONS
 C
        DO 700 J=1,NJ
        IF(J.E0.1.0Ri(J/39*39).EO.J) GO TO 701
        GO TO 702
   701 WRITE(N6,2999)
        WRITE(N6,5000) ALPHA
```

```
         WRITE(N6,750)
     750 FORMAT(//",16(2H' ),2X,'S U M M A R Y STATISTICS FO
        2R JUNCTIONS',2)016(2H ')//)
         WRITE(N6,751)
     751 FORMAT('   ,36X,'UPPERMOST'19X,IMAXIMUM1,5X,'TIME1111X,'FEET OF',
        211X,'FEET MAX.',11X,'LENGTH'/",20X,'GROUND'19X,'PIPE CROWN',8X,
        31COMPUTED',6X,'OF'911X,'SURCHARGE',10X,'DEPTH IS',14X,'OF'/",
        42X,'JUNCTION't8X,'ELEVATION't9X,'ELEVATION',10X,'DEPTH',3X,
        51OCCURENCE'r9X,'AT MAX.',9X,'BELOW GROUND',8XP'SURCHARGE1/",
        63X,'NUMBER',12X,1(FT)1,2(13X,'(FT)'),4X,'HR.1,2X,'MIN.',10Xt
        7'DEPTH',12X,'ELEVATION',11X,'(MIN)'/"12X,8('-'),8X,9('-'),8X,
        810('-'),8X119('-'),8X,9('-'),8X912('-'),8X19('-')//
C
C******** COMPUTE FEET MAXIMUM DEPTH IS BELOW GROUND ELEVATION
     702 FTBLG=GRELEV(J)-(DEPMAX(J)+Z(J))
         IF(FTBLG.LE.0.0)FTBLG=0.
C
C******** COMPUTE FEET OF SURCHARGE AT MAXIMUM DEPTH
         SURMAX=DEPMAX(J)+Z(J)-ZCROWN(J)
         IF(SURMAX.LE.0.0) SURMAX=0.0
C
C******** PRINT JUNCTION STATISTICS
         WRITE(N6,752) JUN(J),GRELEV(J).ZCROWN(J),DEPMAX(J),IDHR(J),
        2IDMIN(J)/SURMAX,FTBLS,SURLEN(J)
     752 FORMAT("r4X,I5110X/F7.2,11X,F7.2,10X,F6.2,3X,I3,3X7I2,11X,F5.2,
        214X,F5.2113X,F5.1)
c 700 CONTINUE

C******** PRINT FLOWS * VELOCITIES IN PIPES
C
         DO 140 I=1,NQPRT
         L=CPRT(I)
     140 CPRT(I)=NCOND(L)
         DO 160 I=1,NOPRTP6
         WRITE(N6,2999)
         WRITE(N6,5000) ALPHA
         WRITE(N6,5100)
    5100 FORMAT (125H0 * * * * * * * * * * * * * *** TIME HISTOR
        1Y OF FLOW AND VELOCITY ************
        1* * * /,49X,'Q(CFS), VEL(FPS)')
         IT=I+5
         IF(IT.GT.NGPRT) IT=NQPRT
         WRITE(N6r5120) (CPRT(L),L=IyIT)

    5120 FORMAT (1H07' TIME',6(4X,'CONDUIT',I5,4X),/
        1 ' HR . MIN',6(2X,'FLOW    VEL         '))
         LT=MINO(I+5,NOPRT)
         DO 160 L=1,LTIME
         TIME=(TIMEO+FLOAT((L-1)*INTER)*DELT)/3600.
         LTIMEH=IFIX (TIME)
         LTIMEM=IFIX((rIME-FLOAT(LTIMEH))*60.04.0.5)
     160 WRITE(N6,5140) LTIMEH,LTIMEMF(PRTO(L,K),PRTV(L,K),K=I,LT)
    5140 FORMAT (1H II3,1.',I2,2)(16(F7.2,F5.1.3X))
C
C******** COMPUTE AND PRINT SUMMARY STATISTICS FOR CONDUITS
C
```

```
      DO 900 N=1,NC
      IF(N.E0.1.0R.(N/39*39).EO.N) GO TO 901
  901 GO TO 902
     WRITE(N6r2999)
      WRITE(N615000) ALPHA
      WRITE(N6,800)
  800 FORMAT(//",16(2H' ),1H1,2Xt'SUMMARY STATISTICS
 .  2F OR CONDUITS't2Xt1H'116(2H ')//)
      WRITE(N61801)
  801 FORMAT(' t35:61CONDUIT'15X,'MAXIMUM't5Xt'TIME',7,X1'MAXIMUM'y
     25X,'TIME'r6X,'RATIO OF/t6WMAXIMUM DEPTH ABOVE',     ',12X,
     32('DESIGN't5X)t'VERTICAL't4Xt'COMPUTED'r6Xt'OF't7Xt'COMPUTED't
     46Xt'OF't8Xt'MAX. TO't4X,'INVERT AT CONDUIT ENDS'/'   t1Xt'CONDUIT't
     55Xt'FLOW'r5Xt'VELOCITY'16Xt'DEPTH'17X,'FLOW't4Xt'OCCURENCE'14Xt
     6/VELOCITY'r2Xt'OCCURENCE'r5WDESIGN't5Xt'UPSTREAM't4X,
     7'DOWNSTREAM'/",2Xt'NUMBER't5Xt1(CFS)',6X11(FPS)117X,'(IN)1,2X,
     8/(CFS)/t3Xt'HR.',2Xt'MIN.',6)01(FPS)/t3Xt'HR.'12Xt'MIN.',6Xt
     9'FLOW't8X11(FT)',9X,'(FT)'/"t1Xt7('-')/4Xt6('-'),2(4Xt8('-')),
     12(4X,19('-')),4X,8('-'),4X,22('-')/)
C
C******** COMPUTE DESIGN VELOCITY AND FLOW IN CONDUIT
  902 NL=NJUNC(Nr1)
      NH=NJUNC(N72)
      SLOPE=(ZP(N,1)-ZP(N,2))/LEN(N)
      VDSGN=SORT(32.2*SLOPE/ROUGH(N))*RFULL(N)**0.6666667
      ODSGN=AFULL(N)*VDSGN
C
C******** COMPUTE RATIO OF MAX TO DESIGN FLOW IN CONDUIT
      ORATIO=0.
      IF(ODSGN.GT.0.) ORATIO=OMAXX(N)/ODSGN
C
C******** COMPUTE MAX WATER DEPTH ABOVE CONDUIT INVERT AT BOTH ENDS
      DMAXNL=DEPMAX(NL)-(ZP(Nt1)-Z(NL))
      DMAXNH=DEPMAX(NH)-(ZP(Nr2)-Z(NH))
      VHGHT=DEEP(N)*12.0
C
C******** PRINT CONDUIT STATISTICS
      WRITE(N6,802) NCOND(N)rODSGN,VDSGMHGHTIOMAXX(N),IOHR(N),
     2IOMIN(N),VMAXX(N)tIVHR(N),IVMIN(N)tORATIO,DMAXNLIDMAXNH
  802 F0RMAT("12X,I5t2(5X,F6.1)OX,F5.112(6X,F6.1,3X,I3t3XtI2)76X,
     2F6.1,7X,F5.2,8X,F5.2)
  900 CONTINUE
C
C"''  PRINTER PLOT PACKAGE
      IF(NPLT) 220,20,180
  180 DO 200 N=1tNPLT
      IPLT=1
      J=JPLT(N)
      ZINVRT=Z(J)
      ZCRN=ZCROWN(J)
      ZGRND=GRELEV(J)
      NJUN=JUN(J)
      CALL CURVE(TPLT,YPLT(1,N),NPTOT,1,NJUN)
  200 WRITE(N6,5160) NJUN
 5160 FORMAT(100X,'JUNCTION NUMBER',I7)
  220 IF(LPLT) 300,300,240
```

```
  240 DO 260 L=1,6
  260 VERT(L)=VERTQ(L)
      DO 280 N=1,LPLT
      IPLT=2
      L=KPLT(N)
      NKON=NCOND(L)
      CALL CURVE(TPLT,OPLT(1,N),NPTOTI1,NKON)
  280 WRITE(N615180) NKON
 5180 FORMAT(100X,'CONDUIT NUMBER',I7)
  300 RETURN
      END
```

```
      SUBROUTINE PINE(X1,Y1,X2,Y2,NSYM,NCT)
      COMMON/FILES/ N5,N6,N21,N22INPOLL,NLOCAT/OCONV,IDATEZILOCNOS(100),
     1TRIBA
      COMMON/CONTR/ NTCYCIDELTO,BELT,DELT2ITZEROPALPHA(40),
     1 NJ,NCINTCYNTL'ICYC,NJSWIMJSWITIME,TIME2tAlfA2,A3,A4,A5,A6rA7,W
      COMMON/ELEV/ ZINVRTaCRNaGRNIIIIPLT
C-
      COMMON/LAB/TITLE(40),XLAB(11),YLAB(6),HORIZ(5),VERT(6)
C
      AXA=X1
      AXB=X2
      AYA=Y1
      AYB=Y2
      IF((AXB.EO.AXA).AND.(AYB.E0.AYA))   RETURN
      N=1
      IF(ABS(AXB-AXA).LT.ABS(AYB-AYA))   GO TO 160
C
C     SET PARAMETERS FOR X DIRECTION
C
      IF(AXB.GT.AXA)  GO TO  100
      AXA=X2
      AXB=X1
      AYA=Y2
      AYB=Y1
  100 CONTINUE
      IXA=AXA+.5
      IXB=AXBi-.5
      IYA=AYA+.5
      IYB=AYB+.5
  120 CONTINUE
      IF(IXA.LT.O.OR.IXA.GT.100)   GO TO  140
      IF(IYA.LT.O.OR.IYA.GT.40)   GO TO 140
      CALL PPLOT(IXA,IYA,NSYM,NCT)
  140 CONTINUE
      IXA=IXAtl
      YA=(N*(AYB-AYA))/(AXB-AXA)
      IYA=AYA+YA+0.5
      N=N+1
      IF(IXA.LE.IXB)  GO TO 120
      GO TO 260
C
C     SET PARAMETERS FOR Y DIRECTION
C
  160 CONTINUE
      IF(AYB.GT.AYA)    GO  TO 180
      AYB=Y1
      AYA=Y2
      AXB=X1
      AXA=X2
  180 CONTINUE
      IXA=AXAt.5

      IYA=AYA+.5
      Ivri=AYB+.5
  200 CONTINUE
      IF(IXA.LT.O.OR.IXA.GT.100)   GO TO 220
```

```
      IF(IYA.LT.O.OR.IYA.GT.40) GO TO 220
      CALL PPLOT(IXA,IYA,NSYM,NCT)
  220 CONTINUE
      IYA=IYA+1
      XA=(N*(AXB-AXA))/(AYB-AYA)
      IXA=XA+AXA+0.5
      N=N+1
      IF(IYA-IYB) 200,240,260
  240 IXA = IXB
      GO TO 200
  260 RETURN
      END
```

```
      SUBROUTINE PPLOT(IX,IY,K,NCT)
      DIMENSION A(51,101),SYM(9)
C
      COMMON/FILES/ N5tN6tN211N22,NPOLL,NLOCATtGCONVPIDATEZPLOCNOS(100)t
     1TRIBA
      COMMON/CONTR/ NTCYCtDELTarDELT,DELT2ITZEROtALPHA(40),
     1 NJ/NC,NTC/NTLtICYCtNJSWIMJSWtTIME,TIME2tAltA2tA3tA4tA5tA6tA7tW
C
      COMMON/ELEV/ ZINVRT,ZCRN,ZGRND,IPLT
C
      COMMON/LAB/TITLE(40),XLAB(11),YLAB(6),HORIZ(5)tVERT(6)
C
      DATA SYM / 4H****,4H++++, 4H'"'t 4HXXXX, 4H....1 4H2222,
     1 4H.     4HIIIII 4H---- /
      IF(K-99) 100,120,260
  100 A(41-IY,IX+1)=SYM(K)
      RETURN
  120 CONTINUE
      I=0
      J2=1
      WRITE(N6,2999)
 2999 FORMAT('1',64(2H--)/",'ENVIRONMENTAL PROTECTION AGENCY',13X,400:
     2***    EXTENDED TRANSPORT PROGRAM    ****,8X,'WATER RESOURCES DIVISI
     SON'/"P'WASHINGTON, D.C.              ',16X,4H****,32X,4H****,8X,
     4'CAIIP DRESSER & MCKEE INC.'!',                   ',23X14H
     5****,6X,'     ANALYSIS MODULE ',6X,4H****,BX,'ANNANDALE, VIRGINIA
     6')
      WRITE(N691300) ALPHA
      DO 220 II=1,5
      I=I+1
      IF(IPLT.E0.2)G0 TO 130
  125 IF (II.NE.1) GO TO 130
      WRITE(N6,1050) YLAB(II),A(tt1),ZINVRT,(A(1,J),J=29,101)
      WRITE(N6,1051) A(2t1),ZCRNt(A(2,J)t J=29,101)
      WRITE(N6,1052) A(3,1),ZGRNIWA(3,J),J=29,101)
      I=3
      J2=3
      GO TO 135
  130 WRITE(N6,1100) YLAB(II),(A(I,J),J=1,101)
      IF(II.E0.5) GO TO 240
  135 DO 200 JJ=J2t9
      I=I+1
      IF(I.NE.28) GO TO 140
      WRITE(N611500) VERT(5),VERT(6),(A(I,J),J=1,101)
      GO TO 200
  140 IF(I.NE.24) GO TO 160
      WRITE(N6,1500) VERT(1),VERT(2),(A(I,J),J=1,101)
      GO TO 200
  160 IF(I.NE.26) GO TO 180
      WRITE(N6,1500) VERT(3),VERT(4),(A(I,J),J=1,101)
      GO TO 200
  1:30 WRITE(N6,1000) (A(I,j),J=1,101)
  200 CONTINUE
      J2=1
  220 CONTINUE
  240 CONTINUE
```

```
      WRITE(N6,1200) XLAB
      WRITE(N6,1400) HORIZ
 1000 FORMAT(18X,101A1)
 1050 FORMAT (F17.3,1)(1A112X,'INVERT ELEV-',F8.21' FEET',73A1)
 1051 FORMAT (18X,A1,2X,' CROWN ELEV-',F8.2,' FEET',73A1)
 1052 FORMAT (18X,A1,2Xt'GROUND ELEV-',F8.2,' FEET',73A1)
 1100 FORMAT(F17.3,1X,101A1)
 1200 FORMAT(F20.1,10F10.1)
 1300 FORMAT(' r20A4/",20A4//)
 1400 FORMAT(/45X,20A4)
 1500 FORMAT(3)O2A4,7X,101A1)
  260 DO 300 I=1,40
      DO 280 J=1,101
  280 A(I,J)=SYM(7)
      A(I,1)=SYM(8)
  300 CONTINUE
      DO 320 J=1,101
  320 A(41,J)=SYM(9)
      DO 340 I=1,101,10
  340 A(41,I)=SYM(8)
      DO 360 1=11,31,10
      A(I,1)=SYM(9)
  360 CONTINUE
      RETURN
      END
```

```
      SUBROUTINE SCALE (ARRAY,AXLEN,NPTS,INC)
      COMMON/FILES/ N5IN6,N21O22,NPOLL,NLOCAT,OCONVPIDATEZPLOCNOS(100),
     1TRIBA
      DIMENSION ARRAY(NPTS),INT(5)
      DATA INT /2,4,5,8,10/
      INCT=IABS(INC)
C
C                                          SCAN FOR MAX AND MIN
C
      AMAX=ARRAY(1)
      AMIN=ARRAY(1)
      DO 100 N=1,NPTSYINCT
      IF(AMAX.LT.ARRAY(N))  AMAX=ARRAY(N)
      IF(AMIN.GT.ARRAY(N))  AMIN=ARRAY(N)
  100 CONTINUE
      IF( AMAX - AMIN  ) 180,120,180
C
C                                          RESET MAX AND MIN FOR ZERO RANGE
C
  120 IF(  AMIN )  160,320,140
  140 AMIN = 0.0
      AMAX = 2.0 * AMAX
      GO TO 180
  160 AMAX = 0.0
      AMIN  =  2.0 :t AMIN
  180 CONTINUE
C
C                                          COMPUTE UNITS/INCH
C
      RATE=(AMAX-AMIN)/AXLEN
C
C                                                SCALE INTERVAL TO
C                                                LESS THAN 10
      A=ALOG10(RATE)
      N=A
      IF(A.LT.0) N=A-0.9999
      RATE=RATE/(10.**N)
      L=RATE+1.00
C
C                                          FIND NEXT HIGHER INTERVAL
C
  200 DO 220  I=1,5
      IF(L-INT(I))  240:240,220
  220 CONTINUE
C
C                                          L IS NEXT HIGHER INTERVAL
C                                          RANGE IS SCALED BACK TO FULL SET
C
  240 L=INT(I)
      RANGE=FLOAT(L)*10.**N
      IF(INC.LT.0)  GO TO 300
C
C                                          SET UP POSITIVE STEPS
C
      K=AMIN/RANGE
      IF(AMIN.LT,O.)   K=K-1
```

```
C
C                                              CHECK FOR MAX VALUE IN RANGE
C
          IF(AMAX.GT.(K+AXLEN)*RANGE) GO TO 260
          I=MPTS*INCT+1
          ARRAY(I)=K*RANGE
          I=I+INCT
          ARRAY(I)=RANGE
          RETURN
C
C                                              IF OUTSIDE RANGE RESET L AND N
C
      260 L=L+1
          IF(L.LT.11) GO TO 200
          L=2
          N=N+1
      280 GO TO 200
C
C                                              SET UP NEGATIVE STEPS
C
      300 K=AMAX/RANGE
          IF(AMAX.GT.0.) K=K+1
          IF(AMIN.LT.(K+AXLEN)*RANGE) GO TO 260
          I=INCT*NPTS+1
          ARRAY(I)=K*RANGE
          I=I+INCT
          ARRAY(I)=-RANGE
          RETURN
      320 WRITE(N611000)
     1000 FORMAT( // 10X1 'RANGE AND SCALE ARE ZERO ON PLOT ATTEMPT' )
          RETURN
          END
```

```
      SUBROUTINE TIDCF(KO,NI,NCHTID)
C
C                  THIS SUBROUTINE COMPUTES SEVEN COEFFICIENTS
C                  FOR A FOURIER EXPANSION OF THE DIURNAL TIDE STAGE
C
      COMMON/FILES/ N5rN6IN21,N22,NPOLLALOCATrOCONV,IDATEZ,LOCNOS(100),
     1TRIBA
      COMMON/CONTR/ NTCYC,DELTOIDELT,DELT2ITZEROPALPHA(40),
     1 NJ,NC,NTC,NTLrICYCYNJSW,MJSW,TIME,TIME2tAlrA2rA3rA4rA5rA61A7rW
C
      COMMON/TIDE/ YY(50) ,TT(50) rAA(10)rXX(10),SXX(10,10),SXY(10)
C
C                                             TIDE COEFFICIENTS
C                                             TIDAL CURVE FIT, 7 TERM
C                                             SINUSOIDAL EQUATION
C
      WRITE(N6.140) KOrNI.NCHTID
  140 FORMAT (7H0 KO IS,I3,19H NUMBER OF POINTS =rI4,35H MAXIMUM NUMBER
     1 OF ITERATIONS IS 50.21H TIDE CHECK      ITCH IS.I2)
C
C                                             IF AO EQUALS ONE, PROGRAM WILL
C                                             READ FOUR POINTS OF INFORMATION
C                                             AND EXPAND THEM FOR A FULL TIDE
C
C                                             NT IS THE NUMBER OF INFORMATION
C                                             POINTS
C                                             IF NCHTID EQUALS ONE, TIDAL
C                                             INPUT-OUTPUT ;JILL BE PRINTED
C
C                                             MAXIT IS THE MAXIMUM NUMBER OF
C                                             ITERATIONS
C                                             DELTA IS THE ACCURACY
C                                             LIMIT IN FEET
C
      PERIOD = 25.
      MAXIT = 50
      DELTA = 0.005
      NIT=7
      V = 2.*3.14159 /PERIOD
      IF(KO.EQ.0) GO TO 225
      TT(50) =TT(1)+PERIOD
      YY(50)=YY(1)
      DO 220 1=1,4
      j=I+1
      IF (J.GT.4) J=50
      NI=NI+1
      TT(NI)=(3.*TT(I)+TT(J))/4.
      YY(NI)=0.8535*YY(I)+0.1465*YY(J)
      NI=NI+1
      TT(NI)=(TT(I)+TT(J))/2.
      YY(NI)=(YY(I)tYY(J))/2.
      NI=NI+1
      TT(NI)=(TT(I)+3.*TT(J))/4,
      YY(NI)=0,1465*YY(I)+0.8535*YY(J)
  120 CONTINUE
  '225 CONTINUE
```

```
      IF (NCHTID.NE.1) GO TO 240
      WRITE(N6,146)
  146 FORMAT (29H0 NO.     TIME        VALUE )
      WRITE(N6,148) (I,TT(I), YY(I), I=1,NI)
  148 FORMAT (I4, 2F12.3 )
  240 CONTINUE
      DO 280 J=1,NTT
      DO 260 K=1,NTT
  260 SXX(K,J) = 0.
      AA(J) = 0.
  280 SXY(J) = 0.
      NJ2 = NTT/2 + 1
      DO 360 I = 1,NI
      DO 320 J = 1,NTT
      FJ1 = FLOAT(J-1)
      FJ3 = FLOAT ( J-NJ2 )
      IF ( J.LE.NJ2 ) GO TO 300
      XX(J) = COS(FJ3*W*TT(I))
      GO TO 320
  300 XX(J) = SIN(FJ1*W*TT(I))
      IF( J.EQ.1 )XX(J) = 1.
  320 SXY(J) = SXY(J) +XX(J) *YY(I)
      DO 340 J = 1,NTT
      DO 340 K = 1,NTT
  340 SXX(K,J) = SXX(K,J) +XX(K) *XX(J)
  360 CONTINUE
      IT = 0
  380 IT = IT + 1
      DELMAX = 0.
      DO 420 K = 1,NTT
      SUM = 0.
      DO 400 J = 1,NTT
      IF (J.EQ.K) GO TO 400
      SUM = SUM -AA(J)*SXX(K,J)
  400 CONTINUE
      SUM = (SUM+SXY(K))/SXX(K,K)
      DEL = ABS(SUM-AA(K))
      IF (DEL.GT.DELMAX ) DELMAX = DEL
  420 AA(K) = SUM
      IF ( IT.GE.MAXIT ) GO TO 440
      IF (DELMAX.GT.DELTA ) GO TO 380
      GO TO 460
  440 WRITE(N6,150)
  150 FORMAT (' CANNOT REACH DESIRED DELTA, INCREASE EITHER NI OR DELTA
     1 AND TRY AGAIN')
      STOP
  460 CONTINUE
      A1 = AA(1)
      A2 = AA(2)
      A3 = AA(3)
      A4 = AA(4)
      A5 = AA(5)
      A6 = AA(6)
      A7 =  AA(7)
      IF (NCHTID.NE.1) GO TO 540
      WRITE(N6,152)
```

```
    152 FORMAT (46H0       TIME     OBSERVED     COMPUTED       RIFF )
          RES = O.   •
          DO 520 I = 1,NI
          SUM = 0.
          DO 500 J = 2,NTT
          FJ1 = FLOAT ( J-1 )
          FJ3 = FLOAT ( J-NJ2 )
          IF ( J.LE.NJ2 ) GO TO 480
          SUM = SUM +AA(J) *COS(FJ3*W*TT(I))
          GO TO 500
    480 SUM = SUM +AA(J) *SIN(FJ1*W*TT(I))
    500 CONTINUE
          SUM = SUM +AA(1)
          DIFF = SUM -YY(I)
          RES = RES + ABS(DIFF)
    520 WRITE(N6,154) TT(I),YY(I),SUM,DIFF
    154 FORMAT ( 4F12.4 )
          WRITE(N60156) RES
    156 FORMAT (6HOTOTAL  ,  30X, F12.4 )
    540 CONTINUE
C
C                                        CONSTANTS FOR INPUT WAVE FORM
C
          WRITE(N6,158)AltA2tA3tA4,A5,A6,A7
    158 FORMAT(///46H COEFFICIENTS FOR TIDAL STAGE ARE                  //85H
       1      Al          A2          A3          A4 .        A5        A6
       2A7                     //7F10.3,F12.2///31H WHERE THE WAVEFORM IS GIVE
       3N BY//92H H(J) = Al + A2*SIN(WT) + A3*SIN(2WT) + A4*SIN(3WT) + A.5*
       4COS(WT) + A6*COS(2WT) + A7*COS(3WT))
          RETURN
          END
```