foss e t t e d u c a t	e r		Created with
Company Name	KLECET	Project Title	cleat connection
Group/Team Name	Civil	Subtitle	
Designer	Deepak	Job Number	123456789
Date	05 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	'
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	100.0
Components	
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 100X100X10
Thickness (mm)	10
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD
Bolts on Beam	
Туре	Black Bolt

cleat angle.html

Grade	4.8	
Diameter (mm)	20	
Bolt Numbers	4	
Columns (Vertical Lines)	1	
Bolts Per Column	4	
Gauge (mm)	0	
Pitch (mm)	50	
End Distance (mm)	37	
Edge Distance (mm)	37	
Bolts on Column		
Туре	Black Bolt	
Grade	4.8	
Diameter (mm)	20	
Bolt Numbers	6	
Columns (Vertical Lines)	1	
Bolts Per Column	3	
Gauge (mm)	0	
Pitch (mm)	50	
End Distance (mm)	62	
Edge Distance (mm)	37	
Assembly		
Column-Beam Clearance (mm)	20	

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Design Chec	Design Check: Secondary Beam Connectivity			
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((2*400*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 90.529 [cl. 10.3.3]		
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.508*20*7.7*400)/(1.25*1000) = 62.586 [cl. 10.3.4]		
Bearing capacity of beam web (kN)		V_{dpb} = (2.5*0.508*20*7.7*410)/(1.25*1000) = 64.15 [cl. 10.3.4]		
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.508*20*10*410)/(1.25*1000) = 83.312 [cl. 10.3.4]		
Bearing capacity (kN)		Min (62.586, 64.15, 83.312) = 62.586		
Bolt capacity (kN)		Min (90.529, 62.586) = 62.586		
Critical bolt shear (kN)	≤ 62.586	22.66	Pass	
No. of bolts		4		
No.of column(s)	≤ 2	1		

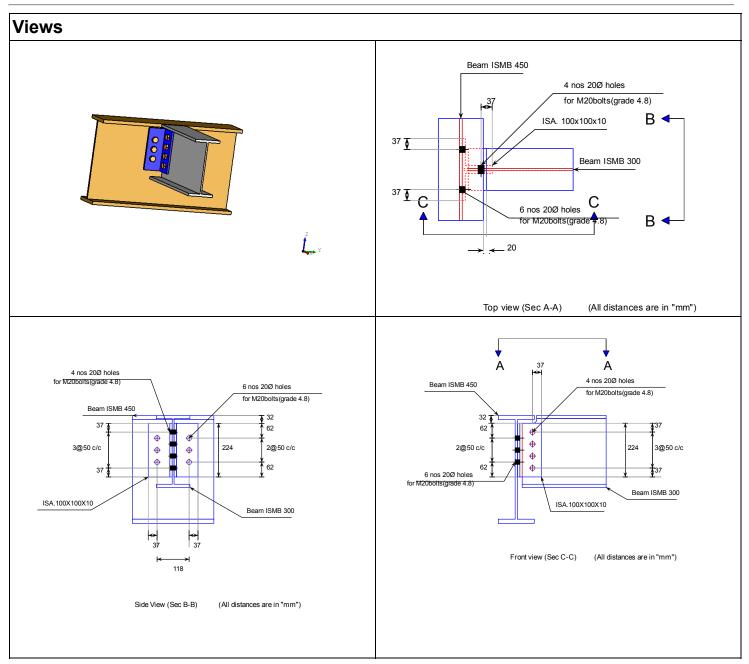
5/2016		cleat angle.html	
No. of bolts per column		4	
Bolt pitch (mm)	≥ 2.5* 20 = 50, ≤ Min(32*7.7, 300) = 247 [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	≥ ;2.5*20 = 50, ≤ Min(32*7.7, 300) = 247 [cl. 10.2.2]	0	
End distance (mm)	\geq 1.7*22.0 = 37.4, \leq 12*7.7 = 92.4 [cl. 10.2.4]	37	Pass
Edge distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*7.7 = 92.4 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥ 100.0	V _{db} = 271.568 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0-13.1-14.0-17.4-15.0-5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	(2*90.529*50 ²)/(50*1000) = 3.15	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 150.528 [cl. 8.2.1.2]	Pass

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Design Check	Design Check: Primary Beam Connectivity			
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((400*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 45.264 [cl. 10.3.3]		
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.508*20*9.4*400)/(1.25*1000) = 76.403 [cl. 10.3.4]		
Bearing capacity of beam web (kN)		V_{dpb} = (2.5*0.508*20*9.4*410)/(1.25*1000) = 78.313 [cl. 10.3.4]		
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.508*20*10*410)/(1.25*1000) = 83.312 [cl. 10.3.4]		
Bearing capacity (kN)		Min (76.403, 78.313, 83.312) = 83.312		
Bolt capacity (kN)		Min (45.264, 83.312) = 45.264		
Critical bolt shear (kN)	≤ 45.264	37.35	Pass	
No. of bolts		6		
No.of column(s) per angle	≤ 2	1		

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No. of bolts per column per angle		3	
Bolt pitch (mm)	≥ 2.5* 20 = 50, ≤ Min(32*9.4, 300) = 300 [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	\geq 2.5*20 = 50, \leq Min(32*9.4, 300) = 300 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*9.4 = 112.8 [cl. 10.2.4]	62	Pass
Edge distance (mm)	≥1.7*22.0 = 37.4, ≤12*9.4 = 112.8 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥100.0	$V_{\rm db}$ = 266.455 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	(2*45.264*50 ²)/(50*1000) = 3.342	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 150.528 [cl. 8.2.1.2]	Pass

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Additional Comments	