

<b>Company Name</b>	<b>LERA</b>	<b>Project Title</b>	<b>Design prob 4</b>
<b>Group/Team Name</b>	<b>Individual</b>	<b>Subtitle</b>	
<b>Designer</b>	<b>Karthik Bandi</b>	<b>Job Number</b>	<b>P786</b>
<b>Date</b>	<b>06 /06 /2016</b>	<b>Method</b>	<b>Limit State Design (No Earthquake Load)</b>

<b>Design Conclusion</b>	
<b>Finplate</b>	<b>Pass</b>
<b>Finplate</b>	
<b>Connection Properties</b>	
<b>Connection</b>	
Connection Title	Single Finplate
Connection Type	Shear Connection
<b>Connection Category</b>	
Connectivity	Column flange-Beam web
Beam Connection	Bolted
Column Connection	Welded
<b>Loading (Factored Load)</b>	
Shear Force (kN)	200
<b>Components</b>	
<b>Column Section</b>	
Material	Fe 410
<b>Beam Section</b>	
Material	Fe 410
Hole	STD
<b>Plate Section</b>	
Thickness (mm)	16
Width (mm)	100
Depth (mm)	250
Hole	STD
<b>Weld</b>	
Type	Double Fillet
Size (mm)	13
<b>Bolts</b>	
Type	HSFG
Grade	8.8
Diameter (mm)	12
Bolt Numbers	7
Columns (Vertical Lines)	1
Bolts Per Column	7
Gauge (mm)	0
Pitch (mm)	31
End Distance (mm)	30
Edge Distance (mm)	30
<b>Assembly</b>	

Column-Beam Clearance (mm)	20
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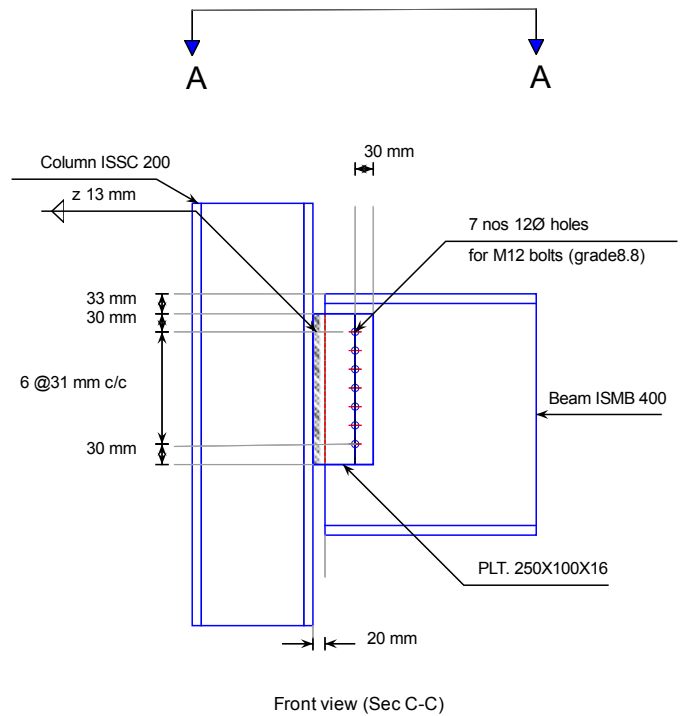
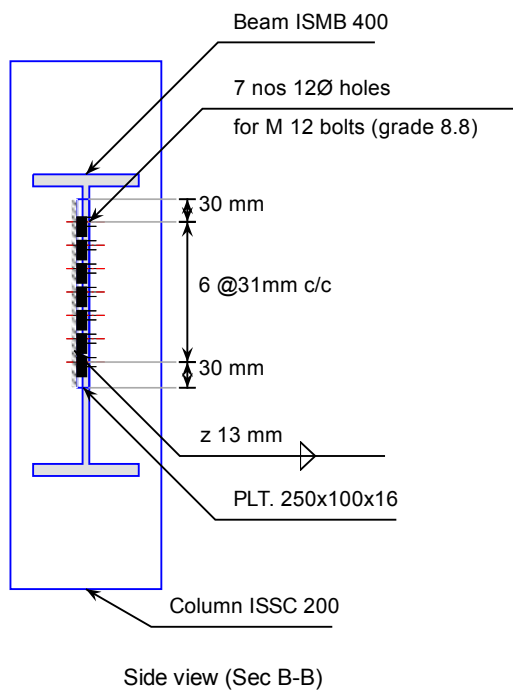
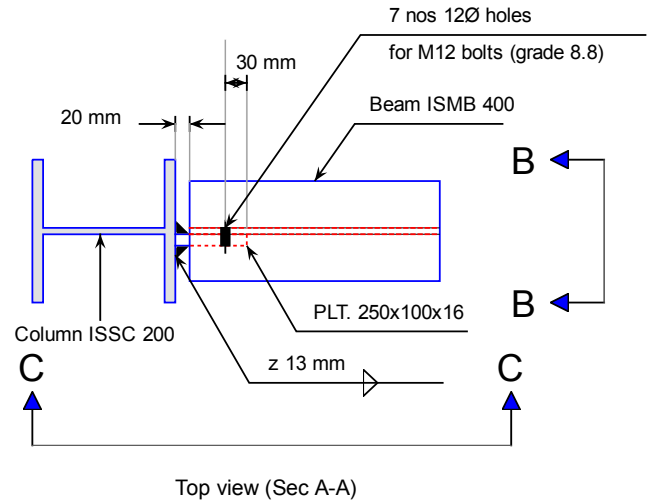
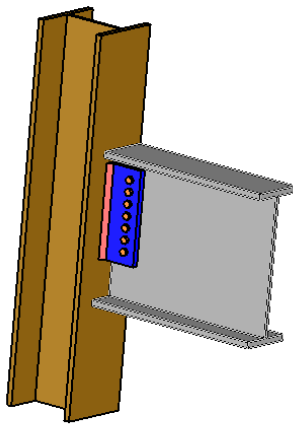
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Design Check			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb} = (800 \times 0.6126 \times 12 \times 12) / (\sqrt{3} \times 1.25 \times 1000)$ = 31.223 [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{dps} = (2.5 \times 0.519 \times 12 \times 8.9 \times 410) / (1.25 \times 1000)$ = 45.452 [cl. 10.3.4]	
Bolt capacity (kN)		Min (31.223, 45.452) = 31.223	
No. of bolts	$200 / 31.223 = 6.4$	7	Pass
No. of column(s)	$\leq 2$	1	
No. of bolts per column		7	
Bolt pitch (mm)	$\geq 2.5 \times 12 = 30, \leq \text{Min}(32 \times 8.9, 300) = 285$ [cl. 10.2.2]	31	Pass
Bolt gauge (mm)	$\geq 2.5 \times 12 = 30, \leq \text{Min}(32 \times 8.9, 300) = 285$ [cl. 10.2.2]	0	
End distance (mm)	$\geq 1.7 \times 13 = 22.1, \leq 12 \times 8.9 = 106.8$ [cl. 10.2.4]	30	Pass
Edge distance (mm)	$\geq 1.7 \times 13 = 22.1, \leq 12 \times 8.9 = 106.8$ [cl. 10.2.4]	30	Pass
Block shear capacity (kN)	$\geq 200$	$V_{db} = 467$	Pass
Plate thickness (mm)	$(5 \times 200 \times 1000) / (250 \times 250) = 16.0$ [Owens and Cheal, 1989]	16	Pass
Plate height (mm)	$\geq 0.6 \times 400 = 240.0, \leq 400 - 16 - 14 - 10 = 330.0$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	250	Pass
Plate width (mm)		100	
Plate moment capacity (kNm)	$(2 \times 31.223 \times 31^2) / (31 \times 1000) = 14.0$	$M_d = (1.2 \times 250 \times Z) / (1000 \times 1.1) = 45.45$ [cl. 8.2.1.2]	Pass
Effective weld length (mm)		$250 - 2 \times 16 = 218$	
Weld strength (kN/mm)	$\sqrt{[(14000 \times 6) / (2 \times 218^2)]^2 + [200 / (2 \times 218)]^2}$ = 0.996	$f_v = (0.7 \times 13 \times 410) / (\sqrt{3} \times 1.25)$ = 2.121 [cl. 10.5.7]	Pass
	$\text{Max}((0.996 \times 1000 \times \sqrt{3} \times 1.25) / (0.7 \times$		Pass

<b>Weld thickness (mm)</b>	410),16* 0.8) = 12.8 [cl. 10.5.7, Insdag Detailing Manual, 2002]	13	<b>pass</b>
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## Views





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Additional Comments	
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