 <small>Created with</small>	
<b>Company Name</b>	<b>INSDAG</b>	<b>Project Title</b>	<b>CONNECTION DESIGN, PROB 1</b>
<b>Group/Team Name</b>	<b>M M GHOSH</b>	<b>Subtitle</b>	
<b>Designer</b>	<b>M M GHOSH</b>	<b>Job Number</b>	
<b>Date</b>	<b>04 /06 /2016</b>	<b>Method</b>	<b>Limit State Design (No Earthquake Load)</b>


<b>Design Conclusion</b>	
<b>Finplate</b>	<b>Pass</b>
<b>Finplate</b>	
<b>Connection Properties</b>	
<b>Connection</b>	
Connection Title	Single Finplate
Connection Type	Shear Connection
<b>Connection Category</b>	
Connectivity	Column web-Beam web
Beam Connection	Bolted
Column Connection	Welded
<b>Loading (Factored Load)</b>	
Shear Force (kN)	160
<b>Components</b>	
<b>Column Section</b>	ISSC 200
Material	Fe 410
<b>Beam Section</b>	ISMB 400
Material	Fe 410
Hole	STD
<b>Plate Section</b>	300X100X10
Thickness (mm)	10
Width (mm)	100
Depth (mm)	300
Hole	STD
<b>Weld</b>	
Type	Double Fillet
Size (mm)	8
<b>Bolts</b>	
Type	HSFG
Grade	8.8
Diameter (mm)	20
Bolt Numbers	3
Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	110
End Distance (mm)	40

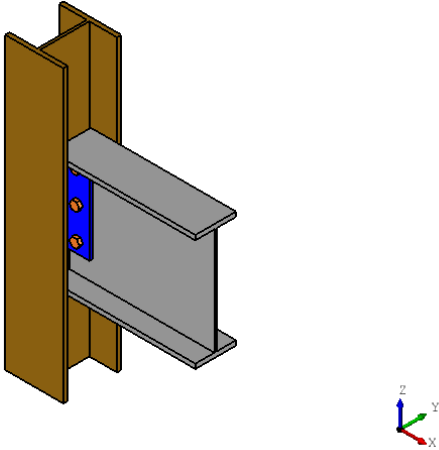
Edge Distance (mm)	40
<b>Assembly</b>	
Column-Beam Clearance (mm)	20


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Design Check			
Check	Required	Provided	Remark
<b>Bolt shear capacity (kN)</b>		$V_{dsb} = (800 \times 0.6126 \times 20 \times 20) / (\sqrt{3} \times 1.25 \times 1000)$ $= 90.529$ [cl. 10.3.3]	
<b>Bolt bearing capacity (kN)</b>		$V_{dps} = (2.5 \times 0.508 \times 20 \times 8.9 \times 410) / (1.25 \times 1000)$ $= 74.148$ [cl. 10.3.4]	
<b>Bolt capacity (kN)</b>		Min (90.529, 74.148) = 74.148	
<b>No. of bolts</b>	160/74.148 = 2.2	3	Pass
<b>No. of column(s)</b>	$\leq 2$	1	
<b>No. of bolts per column</b>		3	
<b>Bolt pitch (mm)</b>	$\geq 2.5 \times 20 = 50, \leq \text{Min}(32 \times 8.9, 300) = 285$ [cl. 10.2.2]	110	Pass
<b>Bolt gauge (mm)</b>	$\geq 2.5 \times 20 = 50, \leq \text{Min}(32 \times 8.9, 300) = 285$ [cl. 10.2.2]	0	
<b>End distance (mm)</b>	$\geq 1.7 \times 22 = 37.4, \leq 12 \times 8.9 = 106.8$ [cl. 10.2.4]	40	Pass
<b>Edge distance (mm)</b>	$\geq 1.7 \times 22 = 37.4, \leq 12 \times 8.9 = 106.8$ [cl. 10.2.4]	40	Pass
<b>Block shear capacity (kN)</b>	$\geq 160$	$V_{db} = 426$	Pass
<b>Plate thickness (mm)</b>	$(5 \times 160 \times 1000) / (300 \times 250) = 10.67$ [Owens and Cheal, 1989]	10	Pass
<b>Plate height (mm)</b>	$\geq 0.6 \times 400 = 240.0, \leq 400 - 16 - 14 - 10 = 330.0$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	300	Pass
<b>Plate width (mm)</b>		100	
<b>Plate moment capacity (kNm)</b>	$(2 \times 90.529 \times 110^2) / (110 \times 1000) = 19.916$	$M_d = (1.2 \times 250 \times Z) / (1000 \times 1.1) = 40.91$ [cl. 8.2.1.2]	Pass
<b>Effective weld length (mm)</b>		300 - 2 \times 8 = 284	
<b>Weld strength (kN/mm)</b>	$\sqrt{[(19916 \times 6) / (2 \times 284^2)]^2 + [160 / (2 \times 284)]^2}$ $= 0.793$	$f_v = (0.7 \times 8 \times 410) / (\sqrt{3} \times 1.25)$ $= 1.06$ [cl. 10.5.7]	Pass

<b>Weld thickness (mm)</b>	$\text{Max}((0.793 \cdot 1000 \cdot \sqrt{3} \cdot 1.25) / (0.7 \cdot 410), 10 \cdot 0.8) = 8.0$ [cl. 10.5.7, Insdag Detailing Manual, 2002]	8	<b>Pass</b>
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Designer	M M GHOSH	Job Number	
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

Views	
	

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<b>Additional Comments</b>			