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Company Name	Nandadeep Designers And Valuers Pvt.Ltd	Project Title	job3b
Group/Team Name	NDVPL	Subtitle	
Designer	Aditya	Job Number	
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	100.0
Components	
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 100X100X8
Thickness (mm)	8
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD
Bolts on Beam	
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	4
Columns (Vertical Lines)	1
Bolts Per Column	4

Gauge (mm)	0		
Pitch (mm)	50		
End Distance (mm)	37		
Edge Distance (mm)	37		
Bolts on Column			
Туре	Black Bolt		
Grade	4.8		
Diameter (mm)	20		
Bolt Numbers	6		
Columns (Vertical Lines)	1		
Bolts Per Column	3		
Gauge (mm)	0		
Pitch (mm)	50		
End Distance (mm)	62		
Edge Distance (mm)	37		
Assembly			
Column-Beam Clearance (mm)	20		

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Check	Required	Provided	Remarl
Bolt shear capacity (kN)	•	$V_{\rm dsb}$ = ((2*400*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 90.529 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.508*20*7.7*400)/(1.25*1000) = 62.586 [cl. 10.3.4]	
Bearing capacity of beam web (kN)		V_{dpb} = (2.5*0.508*20*7.7*410)/(1.25*1000) = 64.15 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.508*20*8*410)/(1.25*1000) = 66.65 [cl. 10.3.4]	
Bearing capacity (kN)		Min (62.586, 64.15, 66.65) = 62.586	
Bolt capacity (kN)		Min (90.529, 62.586) = 62.586	
Critical bolt shear (kN)	≤ 62.586	22.66	Pass
No. of bolts		4	
No.of column(s)	≤ 2	1	
No. of bolts per column		4	
Bolt pitch (mm)	\geq 2.5* 20 = 50, \leq Min(32*7.7, 300) = 247 [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	\geq ;2.5*20 = 50, \leq Min(32*7.7, 300) = 247 [cl. 10.2.2]	0	
	≥ 1.7*22.0 = 37.4, ≤ 12*7.7 =		

End distance (mm)	92.4 [cl. 10.2.4]	37	Pass
Edge distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*7.7 = 92.4 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥ 100.0	V _{db} = 217.254 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	(2*90.529*50 ²)/(50*1000) = 3.15	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 120.422 [cl. 8.2.1.2]	Pass

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Design Check: Pri	Design Check: Primary Beam Connectivity				
Check	Required	Provided	Remark		
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((400*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 45.264 [cl. 10.3.3]			
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.508*20*8.0*400)/(1.25*1000) = 65.024 [cl. 10.3.4]			
Bearing capacity of beam web (kN)		V_{dpb} = (2.5*0.508*20*9.4*410)/(1.25*1000) = 78.313 [cl. 10.3.4]			
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.508*20*8*410)/(1.25*1000) = 66.65 [cl. 10.3.4]			
Bearing capacity (kN)		Min (65.024, 78.313, 66.65) = 66.65			
Bolt capacity (kN)		Min (45.264, 66.65) = 45.264			
Critical bolt shear (kN)	≤ 45.264	37.35	Pass		
No. of bolts		6			
No.of column(s) per angle	≤ 2	1			
No. of bolts per column per angle		3			
Bolt pitch (mm)	\geq 2.5* 20 = 50, \leq Min(32*8.0, 300) = 256 [cl. 10.2.2]	50	Pass		
Bolt gauge (mm)	\geq 2.5*20 = 50, \leq Min(32*8.0, 300) = 256 [cl. 10.2.2]	0			

End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*8.0 = 96.0 [cl. 10.2.4]	62	Pass
Edge distance (mm)	≥1.7*22.0 = 37.4, ≤12*8.0 = 96.0 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥100.0	V _{db} = 213.164 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	(2*45.264*50 ²)/(50*1000) = 3.342	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 120.422 [cl. 8.2.1.2]	Pass

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Views	

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Additional Comments	