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<b>Company Name</b>	L&T	Project Title	End Plate Connection
<b>Group/Team Name</b>	HCIC	Subtitle	ISSC250 Flange & ISMB400
Designer	<b>Engineering Manager</b>	Job Number	040616
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

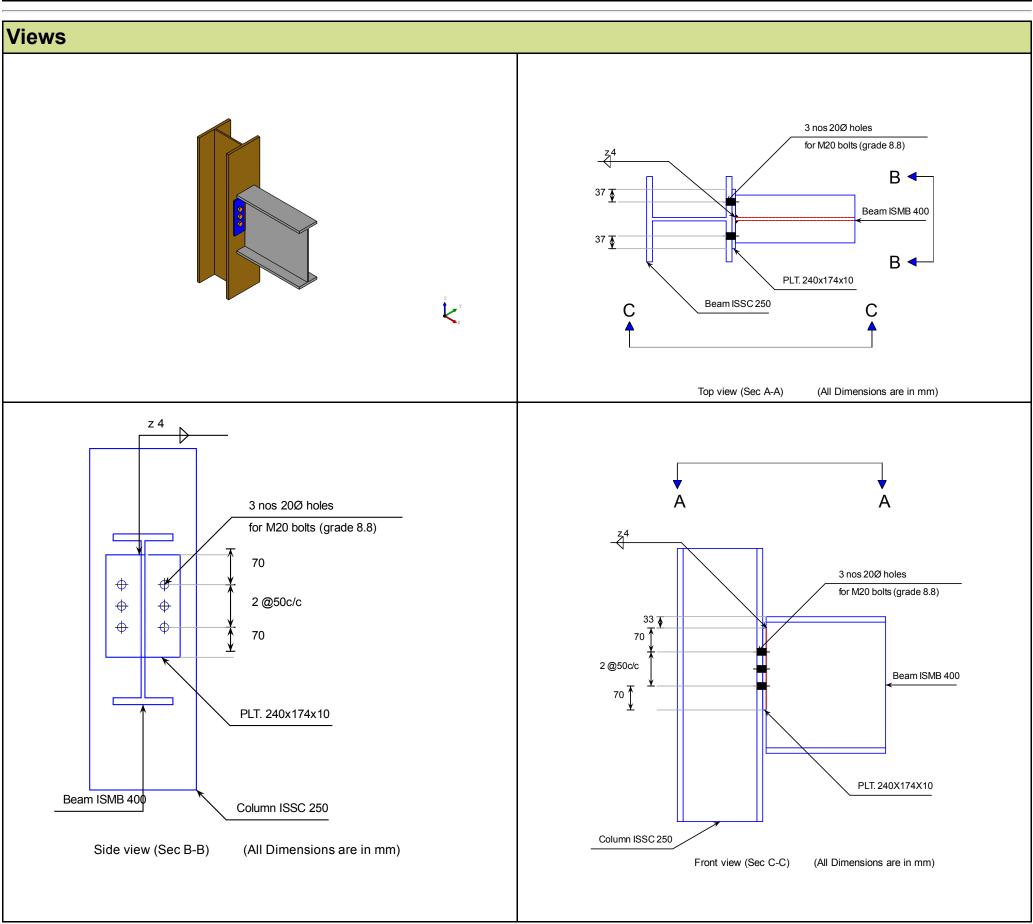
Design Conclusion	Do a c
Endplate	Pass
Endplate	
Connection Properties	
Connection	
Connection Title	Flexible Endplate
Connection Type	Shear Connection
Connection Category	
Connectivity	Column flange-Beam web
Beam Connection	Welded
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	160
Components	
Column Section	ISSC 250
Material	Fe 410
Beam Section	ISMB 400
Material	Fe 410
Hole	STD
Plate Section	240X174X10
Thickness (mm)	10
Width (mm)	174
Depth (mm)	240
Hole	STD
Weld	•
Туре	Double Fillet
Size (mm)	4
Bolts	1
Туре	HSFG
Grade	8.8
Diameter (mm)	20
Bolt Numbers	6
Columns (Vertical Lines)	2
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	70
Edge Distance (mm)	37
Assembly	<b>L</b>

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Design Check				
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((800.0*0.6126*20*20)/( $\sqrt{3}$ *1.25*1000) = 52.694 [cl. 10.3.3]		
Bolt bearing capacity (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*10.0*410)/(1.25*1000) = 83.312 [cl. 10.3.4]		
Bolt capacity (kN)		Min (52.694, 83.312) = 52.694	Pass	
Critical bolt shear (kN)	≤ 52.694	48.074	Pass	
No. of bolts		6		
No.of column(s)	≤ 2	2		
No. of bolts per column per side of end plate		3		
Bolt pitch (mm)	$\geq 2.5*20 = 50, \leq Min(32*8.9, 300) = 285$ [cl. 10.2.2]	50	Pass	
Bolt gauge (mm)	$\geq 2.5*20 = 50, \leq Min(32*8.9, 300) = 285$ [cl. 10.2.2]	0		
End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	70	Pass	
Edge distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	37	Pass	
Block shear capacity (kN)	≥ 160	$V_{db} = 203$ [cl. 6.4.1]		
Plate thickness (mm)	≥ 8	10	Pass	
Plate height (mm)	≥ 0.6*400.0=240.0, ≤ 400.0- 16.0-14.0-16.0-14.0- 10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002]	240	Pass	
Plate Width (mm)	≥ 174, ≤ 250.0	174	Pass	
Effective weld length (mm)		240-2*4 = 232		
Weld strength (kN/mm)	0.345	$f_{V} = (0.7*4*410)/(\sqrt{3}*1.25*1000)$ = 0.53 [cl. 10.5.7]	Pass	

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Additional Comments	