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Group/Team Name		Subtitle	
Designer	Paresh	Job Number	1
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

Pass
Single Finplate
Shear Connection
<u> </u>
Column flange-Beam web
Bolted
Welded
<u> </u>
160
<u> </u>
ISSC 200
Fe 410
ISMB 400
Fe 410
STD
320X100X10
10
100
320
STD
•
Double Fillet
8
•
HSFG
8.8
20
3
1
3
0
120
40
40
•

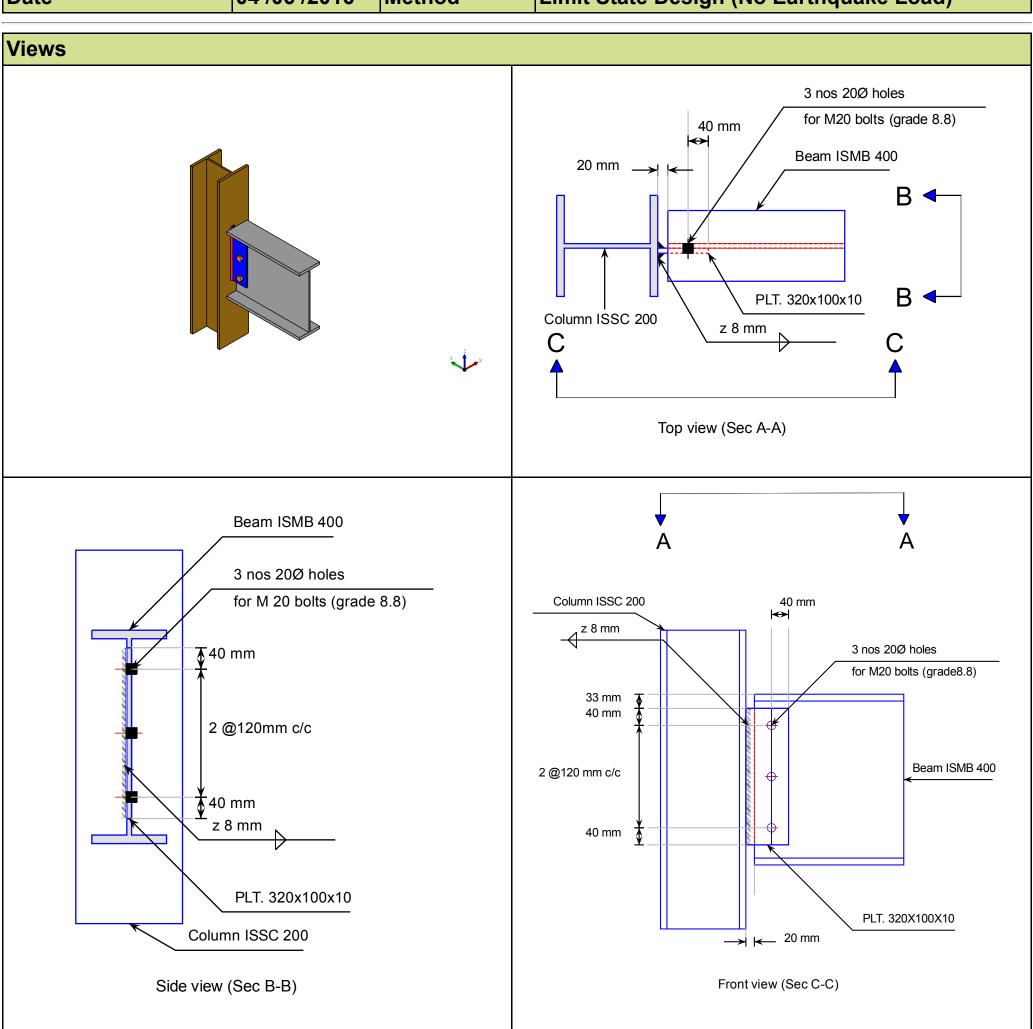
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Design Check			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{\rm dsb}$ = (800*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 90.529 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.508*20*8.9*410)/(1.25*1000) = 74.148 [cl. 10.3.4]	
Bolt capacity (kN)		Min (90.529, 74.148) = 74.148	
No. of bolts	160/74.148 = 2.2	3	Pass
No.of column(s)	≤ 2	1	
No. of bolts per column		3	
Bolt pitch (mm)	≥ 2.5* 20 = 50, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	120	Pass
Bolt gauge (mm)	≥ 2.5*20 = 50, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	0	
End distance (mm)	\geq 1.7*22 = 37.4, \leq 12*8.9 = 106.8 [cl. 10.2.4]	40	Pass
Edge distance (mm)	≥ 1.7*22 = 37.4, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	40	Pass
Block shear capacity (kN)	≥ 160	$V_{\rm db} = 453$	Pass
Plate thickness (mm)	(5*160*1000)/(320*250) = 10.0 [Owens and Cheal, 1989]	10	Pass
Plate height (mm)	≥ 0.6*400=240.0, ≤ 400-16-14- 10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002]	320	Pass
Plate width (mm)		100	
Plate moment capacity (kNm)	$(2*90.529*120^2)/(120*1000) = 14.485$	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 46.55 [cl. 8.2.1.2]	Pass
Effective weld length (mm)		320-2*8 = 304	
Weld strength (kN/mm)	$\sqrt{[(14485*6)/(2*304^2)]^2}$ + $[160/(2*304)]^2$ = 0.539	$f_V = (0.7*8*410)/(\sqrt{3}*1.25)$ = 1.06 [cl. 10.5.7]	Pass
Weld thickness (mm)	Max((0.539*1000*√3* 1.25)/(0.7 * 410),10* 0.8) = 8.0 [cl. 10.5.7, Insdag Detailing Manual,	8	Pass

2002]	

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Additional Comments	