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Company Name	NPCIL	Project Title	Project3
Group/Team Name	Civil Engineering	Subtitle	
Designer	Apurba Mondal	Job Number	1234567
Date	05 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	100.0
Components	
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 100X100X10
Thickness (mm)	10
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD
Bolts on Beam	
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	4

Columns (Vertical Lines)	1
Bolts Per Column	4
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	37
Edge Distance (mm)	37
Bolts on Column	
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	6
Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	62
Edge Distance (mm)	37
Assembly	
Column-Beam Clearance (mm)	20

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<b>Design Chec</b>	Design Check: Secondary Beam Connectivity			
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		$V_{dsb}$ = ((2*400*0.6126*20*20)/( $\sqrt{3}$ *1.25*1000) = 90.529 [cl. 10.3.3]		
Bolt bearing capacity (kN)		$V_{dpb}$ = (2.5*0.508*20*7.7*400)/(1.25*1000) = 62.586 [cl. 10.3.4]		
Bearing capacity of beam web (kN)		$V_{dpb}$ = (2.5*0.508*20*7.7*410)/(1.25*1000) = 64.15 [cl. 10.3.4]		
Bearing capacity of cleat (kN)		V <sub>dpb</sub> = (2.5*0.508*20*10*410)/(1.25*1000) = 83.312 [cl. 10.3.4]		
Bearing capacity (kN)		Min (62.586, 64.15, 83.312) = 62.586		
Bolt capacity (kN)		Min (90.529, 62.586) = 62.586		
Critical bolt shear (kN)	≤ 62.586	22.66	Pass	
No. of bolts		4		
No.of column(s)	≤ 2	1		
No. of bolts per column		4		
Bolt pitch (mm)	$\geq$ 2.5* 20 = 50, $\leq$ Min(32*7.7, 300) = 247 [cl. 10.2.2]	50	Pass	

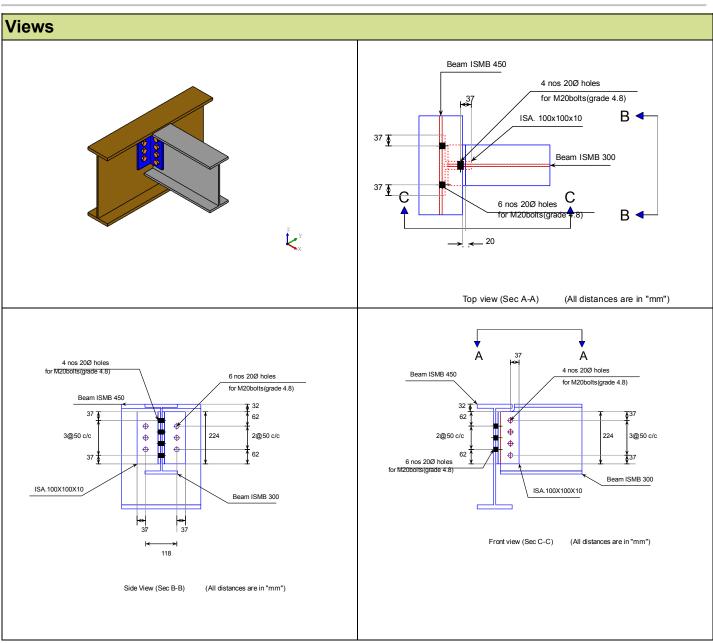
Bolt gauge (mm)	≥ ;2.5*20 = 50, ≤ Min(32*7.7, 300) = 247 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*7.7 = 92.4 [cl. 10.2.4]	37	Pass
Edge distance (mm)	$\geq$ 1.7*22.0 = 37.4, $\leq$ 12*7.7 = 92.4 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥ 100.0	V <sub>db</sub> = 271.568 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0-13.1-14.0-17.4-15.0-5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	(2*90.529*50 <sup>2</sup> )/(50*1000) = 3.15	$M_{\rm d}$ = (1.2*250* $Z$ )/(1000*1.1) = 150.528 [cl. 8.2.1.2]	Pass

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<b>Design Check</b>	: Primary Beam Connectiv	ity	
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{\text{dsb}}$ = ((400*0.6126*20*20)/( $\sqrt{3}$ *1.25*1000) = 45.264 [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*9.4*400)/(1.25*1000) = 76.403 [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*9.4*410)/(1.25*1000) = 78.313 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*10*410)/(1.25*1000) = 83.312 [cl. 10.3.4]	
Bearing capacity (kN)		Min (76.403, 78.313, 83.312) = 83.312	
Bolt capacity (kN)		Min (45.264, 83.312) = 45.264	
Critical bolt shear (kN)	≤ 45.264	37.35	Pass
No. of bolts		6	
No.of column(s) per angle	≤ 2	1	
No. of bolts per column per angle		3	
Bolt pitch (mm)	≥ 2.5* 20 = 50, ≤ Min(32*9.4, 300) = 300	50	Pass

	[cl. 10.2.2]		
Bolt gauge (mm)	≥ 2.5*20 = 50, ≤ Min(32*9.4, 300) = 300 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*9.4 = 112.8 [cl. 10.2.4]	62	Pass
Edge distance (mm)	≥1.7*22.0 = 37.4, ≤12*9.4 = 112.8 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥100.0	$V_{\rm db}$ = 266.455 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	(2*45.264*50 <sup>2</sup> )/(50*1000) = 3.342	$M_{\rm d}$ = (1.2*250* $Z$ )/(1000*1.1) = 150.528 [cl. 8.2.1.2]	Pass

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Additional Comments	
Additional Committee	