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| <b>Company Name</b>    | Aker Powergas Pvt Ltd | Project Title | YDP                                     |
| <b>Group/Team Name</b> | Pune                  | Subtitle      |   |
| Designer               | Yogesh Pisal          | Job Number    | Problem No 2                            |
| Date                   | 04 /06 /2016          | Method        | Limit State Design (No Earthquake Load) |

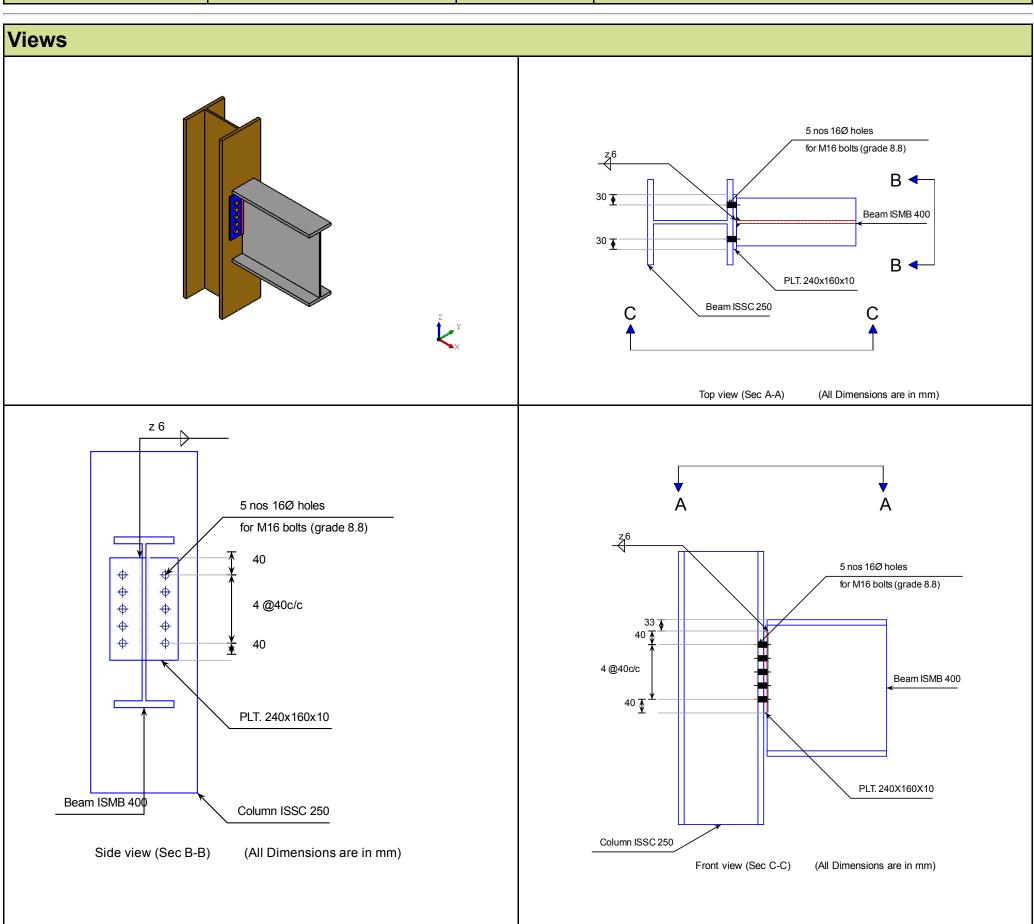
| Date   04 700 720 10       | metriod Emili State Besign (NO Earthquake Esta) |
|----------------------------|---|
| Design Conclusion          |   |
| Endplate                   | Pass  |
| Endplate                   |   |
| Connection Properties      |   |
| Connection                 |   |
| Connection Title           | Flexible Endplate                               |
| Connection Type            | Shear Connection                                |
| Connection Category        | <u> </u>  |
| Connectivity               | Column flange-Beam web                          |
| Beam Connection            | Welded  |
| Column Connection          | Bolted  |
| Loading (Factored Load)    | <u>.</u>  |
| Shear Force (kN)           | 160   |
| Components                 | <u> </u>  |
| Column Section             | ISSC 250  |
| Material                   | Fe 410  |
| Beam Section               | ISMB 400  |
| Material                   | Fe 410  |
| Hole                       | STD   |
| Plate Section              | 240X160X10                                      |
| Thickness (mm)             | 10  |
| Width (mm)                 | 160   |
| Depth (mm)                 | 240   |
| Hole                       | STD   |
| Weld                       | <u>.</u>  |
| Туре                       | Double Fillet                                   |
| Size (mm)                  | 6   |
| Bolts                      | <u>.</u>  |
| Туре                       | HSFG  |
| Grade                      | 8.8   |
| Diameter (mm)              | 16  |
| Bolt Numbers               | 10  |
| Columns (Vertical Lines)   | 2   |
| Bolts Per Column           | 5   |
| Gauge (mm)                 | 0   |
| Pitch (mm)                 | 40  |
| End Distance (mm)          | 40  |
| Edge Distance (mm)         | 30  |
| Assembly                   | ·   |
| Column-Beam Clearance (mm) | 10  |

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| Designer               | Yogesh Pisal          | Job Number    | Problem No 2                            |
| Date                   | 04 /06 /2016          | Method        | Limit State Design (No Earthquake Load) |

| Design Check                                  |  |   |        |  |
|---|--|---|--------|--|
| Check   | Required   | Provided  | Remark |  |
| Bolt shear capacity (kN)                      |  | $V_{\text{dsb}}$ = ((800.0*0.6126*16*16)/( $\sqrt{3}$ *1.25*1000) = 33.724 [cl. 10.3.3] |        |  |
| Bolt bearing capacity (kN)                    |  | V <sub>dpb</sub> = (2.5*0.491*16*10.0*410)/(1.25*1000) = 64.419 [cl. 10.3.4]            |        |  |
| Bolt capacity (kN)                            |  | Min (33.724, 64.419) = 33.724   | Pass   |  |
| Critical bolt shear (kN)                      | ≤ 33.724   | 25.612  | Pass   |  |
| No. of bolts                                  |  | 10  |        |  |
| No.of column(s)                               | ≤ 2  | 2   |        |  |
| No. of bolts per column per side of end plate |  | 5   |        |  |
| Bolt pitch (mm)                               | $\geq$ 2.5*16 = 40, $\leq$ Min(32*8.9, 300) = 285 [cl. 10.2.2]   | 40  | Pass   |  |
| Bolt gauge (mm)                               | $\geq 2.5*16 = 40, \leq Min(32*8.9, 300) = 285$ [cl. 10.2.2]   | 0   |        |  |
| End distance (mm)                             | ≥ 1.7*18.0 = 30.6, ≤ 12*8.9 = 106.8 [cl. 10.2.4]   | 40  | Pass   |  |
| Edge distance (mm)                            | ≥ 1.7*18.0 = 30.6, ≤ 12*8.9 = 106.8 [cl. 10.2.4]   | 30  | Pass   |  |
| Block shear capacity (kN)                     | ≥ 160  | V <sub>db</sub> = 191<br>[cl. 6.4.1]  |        |  |
| Plate thickness (mm)                          | ≥ 8  | 10  | Pass   |  |
| Plate height (mm)                             | ≥ 0.6*400.0=240.0, ≤ 400.0-<br>16.0-14.0-16.0-14.0- 10=330.0<br>[cl. 10.2.4, Insdag Detailing<br>Manual, 2002] | 240   | Pass   |  |
| Plate Width (mm)                              | ≥ 160, ≤ 250.0   | 160   | Pass   |  |
| Effective weld length (mm)                    |  | 240-2*6 = 228   |        |  |
| Weld strength (kN/mm)                         | 0.351  | $f_{V} = (0.7*6*410)/(\sqrt{3}*1.25*1000)$<br>= 0.795<br>[cl. 10.5.7]                   | Pass   |  |

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| Designer               | Yogesh Pisal          | Job Number    | Problem No 2                            |
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| Designer            | Yogesh Pisal          | Job Number           | Problem No 2                            |
| Date                | 04 /06 /2016          | Metdod               | Limit State Design (No Earthquake Load) |

| Additional Comments |  |
|---------------------|--|