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Group/Team Name	LERA Team	Subtitle	
Designer	Raju	Job Number	P003
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	100.0
Components	
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 90X90X8
Thickness (mm)	8
Cleat Leg Size B (mm)	90
Cleat Leg Size A (mm)	90
Hole	STD
Bolts on Beam	
Туре	Black Bolt

	1
4.8	
16	
4	
1	
4	
0	
46	
30	
30	
Black Bolt	
4.8	
16	
6	
1	
3	
0	
70	
30	
30	
20	
	16 4 1 4 0 46 30 30 30  Black Bolt 4.8 16 6 1 3 0 70 30 30 30

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Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb}$ = ((2*400*0.6126*16*16)/( $\sqrt{3}$ *1.25*1000) = 58.012 [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{\text{dpb}}$ = (2.5*0.491*16*7.7*400)/(1.25*1000) = 48.393 [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{\text{dpb}}$ = (2.5*0.491*16*7.7*410)/(1.25*1000) = 49.603 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{\text{dpb}}$ = (2.5*0.491*16*8*410)/(1.25*1000) = 51.535 [cl. 10.3.4]	
Bearing capacity (kN)		Min (48.393, 49.603, 51.535) = 48.393	
Bolt capacity (kN)		Min (58.012, 48.393) = 48.393	
Critical bolt shear (kN)	≤ 48.393	22.982	Pass
No. of bolts		4	
No.of column(s)	≤ 2	1	

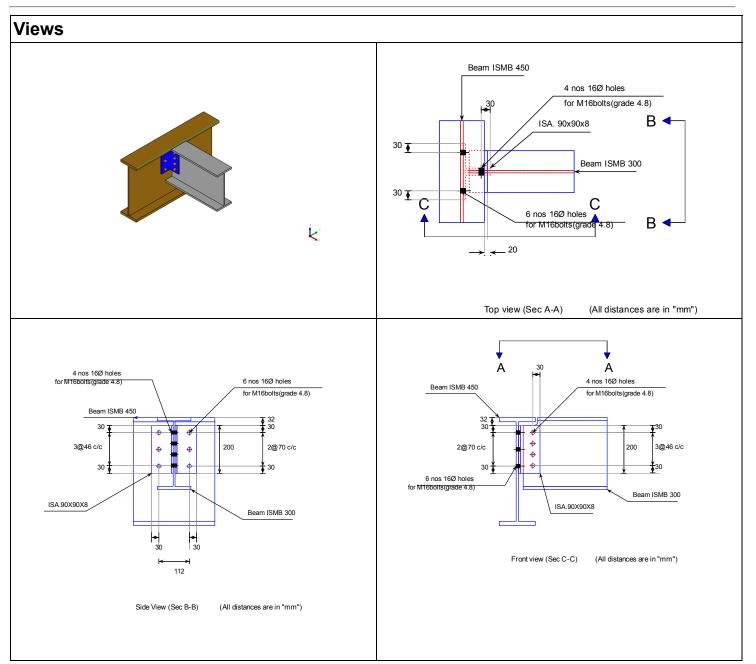
6/2016		P003.html	
No. of bolts per column		4	
Bolt pitch (mm)	$\geq$ 2.5* 16 = 40, $\leq$ Min(32*7.7, 300) = 247 [cl. 10.2.2]	46	Pass
Bolt gauge (mm)	≥ ;2.5*16 = 40, ≤ Min(32*7.7, 300) = 247 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*18.0 = 30.6, ≤ 12*7.7 = 92.4 [cl. 10.2.4]	30	Pass
Edge distance (mm)	$\geq$ 1.7*18.0 = 30.6, $\leq$ 12*7.7 = 92.4 [cl. 10.2.4]	30	Pass
Block shear capacity (kN)	≥ 100.0	V <sub>db</sub> = 200.437 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0-13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	200	Pass
Cleat moment capacity (kNm)	(2*58.012*46 <sup>2</sup> )/(46*1000) = 3.0	$M_{\rm d}$ = (1.2*250*Z)/(1000*1.1) = 96.0 [cl. 8.2.1.2]	Pass

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Design Check	: Primary Beam C	onnectivity	
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb}$ = ((400*0.6126*16*16)/( $\sqrt{3}$ *1.25*1000) = 29.006 [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{\text{dpb}}$ = (2.5*0.491*16*8.0*400)/(1.25*1000) = 50.278 [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{\text{dpb}}$ = (2.5*0.491*16*9.4*410)/(1.25*1000) = 60.554 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		V <sub>dpb</sub> = (2.5*0.491*16*8*410)/(1.25*1000) = 51.535 [cl. 10.3.4]	
Bearing capacity (kN)		Min (50.278, 60.554, 51.535) = 51.535	
Bolt capacity (kN)		Min (29.006, 51.535) = 29.006	
Critical bolt shear (kN)	≤ 29.006	28.245	Pass
No. of bolts		6	
No.of column(s) per angle	≤ 2	1	
No. of bolts			

6/2016		P003.html	
per column per angle		3	
Bolt pitch (mm)	≥ 2.5* 16 = 40, ≤ Min(32*8.0, 300) = 256 [cl. 10.2.2]	70	Pass
Bolt gauge (mm)	$\geq$ 2.5*16 = 40, $\leq$ Min(32*8.0, 300) = 256 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*18.0 = 30.6, ≤ 12*8.0 = 96.0 [cl. 10.2.4]	30	Pass
Edge distance (mm)	≥1.7*18.0 = 30.6, ≤12*8.0 = 96.0 [cl. 10.2.4]	30	Pass
Block shear capacity (kN)	≥100.0	$V_{\rm db}$ = 224.979 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	200	Pass
Cleat moment capacity (kNm)	(2*29.006*70 <sup>2</sup> )/(70*1000) = 3.192	$M_{\rm d}$ = (1.2*250* <i>Z</i> )/(1000*1.1) = 96.0 [cl. 8.2.1.2]	Pass

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