			Created with OSdag
Company Name	ngd	Project Title	problem3
Group/Team Name	mit	Subtitle	
Designer	ngd	Job Number	
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	•
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	160.0
Components	•
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 100X100X8
Thickness (mm)	8
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD
Bolts on Beam	
Type	Black Bolt
Grade	4.8
Diameter (mm)	12
Bolt Numbers	12
Columns (Vertical Lines)	2
Bolts Per Column	6
Gauge (mm)	30
Pitch (mm)	30
End Distance (mm)	22

Edge Distance (mm)	22
Bolts on Column	
Type	Black Bolt
Grade	4.8
Diameter (mm)	12
Bolt Numbers	24
Columns (Vertical Lines)	2
Bolts Per Column	6
Gauge (mm)	30
Pitch (mm)	30
End Distance (mm)	22
Edge Distance (mm)	22
Assembly	
Column-Beam Clearance (mm)	20

			Created with OSdag
Company Name	ngd	Project Title	problem3
Group/Team Name	mit	Subtitle	
Designer	ngd	Job Number	
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Check: Se	Design Check: Secondary Beam Connectivity			
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		V_{dsb} = ((2*400*0.6126*12*12)/($\sqrt{3}$ *1.25*1000) = 31.223 [cl. 10.3.3]		
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.519*12*7.7*400)/(1.25*1000) = 38.364 [cl. 10.3.4]		
Bearing capacity of beam web (kN)		V_{dpb} = (2.5*0.519*12*7.7*410)/(1.25*1000) = 39.324 [cl. 10.3.4]		
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.519*12*8*410)/(1.25*1000) = 40.856 [cl. 10.3.4]		
Bearing capacity (kN)		Min (38.364, 39.324, 40.856) = 38.364		
Bolt capacity (kN)		Min (31.223, 38.364) = 31.223		
Critical bolt shear (kN)	≤ 31.223	14.176	Pass	
No. of bolts		12		
No.of column(s)	≤ 2	2		
No. of bolts per column		6		
Bolt pitch (mm)	$\geq 2.5^*$ 12 = 30, \leq Min(32*7.7, 300) = 247 [cl. 10.2.2]	30	Pass	
Bolt gauge (mm)	\geq ;2.5*12 = 30, \leq Min(32*7.7, 300) = 247 [cl. 10.2.2]	30		
End distance (mm)	≥ 1.7*13.0 = 22.1, ≤ 12*7.7 = 92.4 [cl. 10.2.4]	22	Pass	
Edge distance	≥ 1.7*13.0 = 22.1, ≤ 12*7.7 = 92.4	22	Pass	

(mm)	[cl. 10.2.4]		
Block shear capacity (kN)	≥ 160.0	V _{db} = 231.574 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	194	Pass
Cleat moment capacity (kNm)	(2*31.223*30 ²)/(30*1000) = 5.04	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 90.326 [cl. 8.2.1.2]	Pass

			Created with OSdag
Company Name	ngd	Project Title	problem3
Group/Team Name	mit	Subtitle	
Designer	ngd	Job Number	
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Check: Primary Beam Connectivity			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((400*0.6126*12*12)/($\sqrt{3}$ *1.25*1000) = 15.611 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V _{dpb} = (2.5*0.519*12*8.0*400)/(1.25*1000) = 39.859 [cl. 10.3.4]	
Bearing capacity of beam web (kN)		V _{dpb} = (2.5*0.519*12*9.4*410)/(1.25*1000) = 48.005 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.519*12*8*410)/(1.25*1000) = 40.856 [cl. 10.3.4]	
Bearing capacity (kN)		Min (39.859, 48.005, 40.856) = 40.856	
Bolt capacity (kN)		Min (15.611, 40.856) = 15.611	
Critical bolt shear (kN)	≤ 15.611	14.791	Pass
No. of bolts		24	
No.of column(s) per angle	≤ 2	2	
No. of bolts per column per angle		6	
Bolt pitch (mm)	$\geq 2.5^*$ 12 = 30, \leq Min(32*8.0, 300) = 256 [cl. 10.2.2]	30	Pass
Bolt gauge (mm)	\geq 2.5*12 = 30, \leq Min(32*8.0, 300) = 256 [cl. 10.2.2]	30	
End distance (mm)	≥ 1.7*13.0 = 22.1, ≤ 12*8.0 = 96.0 [cl. 10.2.4]	22	Pass
	≥1.7*13.0 = 22.1, ≤12*8.0 =		_

Edge distance (mm)	96.0 [cl. 10.2.4]	22	Pass
Block shear capacity (kN)	≥160.0	$V_{\rm db}$ = 231.574 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	194	Pass
Cleat moment capacity (kNm)	(2*15.611*30 ²)/(30*1000) = 5.348	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 90.326 [cl. 8.2.1.2]	Pass

			Created with OSdag
Company Name	ngd	Project Title	problem3
Group/Team Name	mit	Subtitle	
Designer	ngd	Job Number	
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Views	

			Created with OSdag
Company Name	ngd	Project Title	problem3
Group/Team Name	mit	Subtitle	
Designer	ngd	Job Number	
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Additional Comments	