operdight. N. P.			Created with OSdag
Company Name	NPCIL	Project Title	Project2
Group/Team Name	Civil Engineering	Subtitle	
Designer	Apurba Mondal	Job Number	123456
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Endplate	Pass
Endplate	
Connection Properties	
Connection	
Connection Title	Flexible Endplate
Connection Type	Shear Connection
Connection Category	•
Connectivity	Column flange-Beam web
Beam Connection	Welded
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	160
Components	
Column Section	ISSC 250
Material	Fe 410
Beam Section	ISMB 400
Material	Fe 410
Hole	STD
Plate Section	240X174X10
Thickness (mm)	10
Width (mm)	174
Depth (mm)	240
Hole	STD
Weld	
Туре	Double Fillet
Size (mm)	8
Bolts	
Туре	HSFG

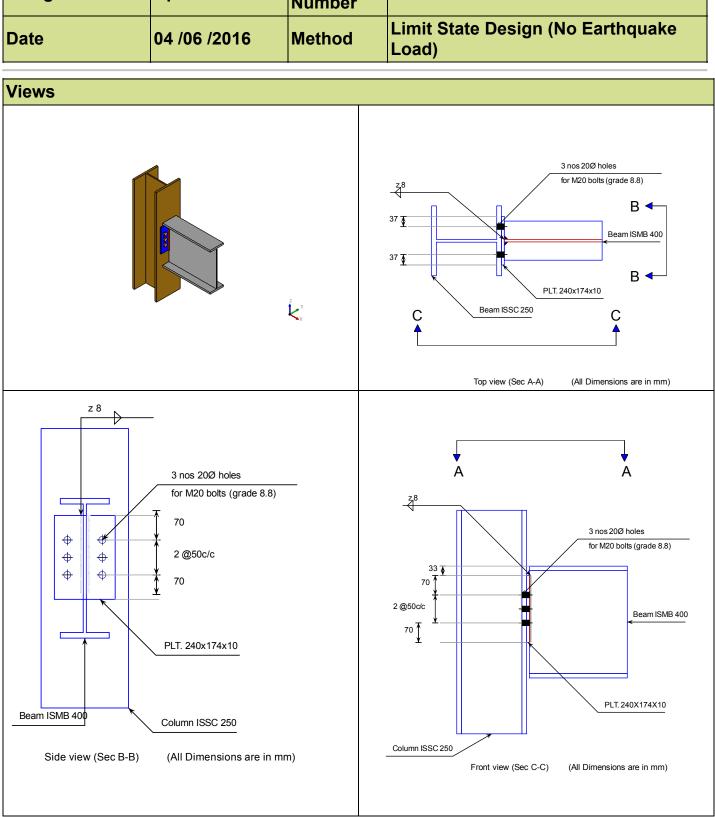
Grade	8.8	
Diameter (mm)	20	
Bolt Numbers	6	
Columns (Vertical Lines)	2	
Bolts Per Column	3	
Gauge (mm)	0	
Pitch (mm)	50	
End Distance (mm)	70	
Edge Distance (mm)	37	
Assembly		
Column-Beam Clearance (mm)	10	

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Design Check			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((800.0*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 52.694 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.508*20*10.0*410)/(1.25*1000) = 83.312 [cl. 10.3.4]	
Bolt capacity (kN)		Min (52.694, 83.312) = 52.694	Pass
Critical bolt shear (kN)	≤ 52.694	48.074	Pass
No. of bolts		6	
No.of column(s)	≤ 2	2	
No. of bolts per column per side of end plate		3	
Bolt pitch (mm)	\geq 2.5*20 = 50, \leq Min(32*8.9, 300) = 285 [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	\geq 2.5*20 = 50, \leq Min(32*8.9, 300) = 285 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	70	Pass
Edge distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥ 160	V _{db} = 203 [cl. 6.4.1]	

Plate thickness (mm)	≥ 8	10	Pass
Plate height (mm)	≥ 0.6*400.0=240.0, ≤ 400.0-16.0-14.0-16.0- 14.0- 10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002]	240	Pass
Plate Width (mm)	≥ 174, ≤ 250.0	174	Pass
Effective weld length (mm)		240-2*8 = 224	
Weld strength (kN/mm)	0.357	$f_{V} = (0.7*8*410)/(\sqrt{3}*1.25*1000)$ = 1.06 [cl. 10.5.7]	Pass

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Additional Comments	
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