 <small>Created with</small>	
Company Name	abc	Project Title	
Group/Team Name	xyz	Subtitle	
Designer	amit	Job Number	
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)


Design Conclusion	
Endplate	Pass
Endplate	
Connection Properties	
Connection	
Connection Title	Flexible Endplate
Connection Type	Shear Connection
Connection Category	
Connectivity	Column web-Beam web
Beam Connection	Welded
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	160
Components	
Column Section	ISSC 250
Material	Fe 410
Beam Section	ISMB 400
Material	Fe 410
Hole	STD
Plate Section	240X180X20
Thickness (mm)	20
Width (mm)	180
Depth (mm)	240
Hole	STD
Weld	
Type	Double Fillet
Size (mm)	6
Bolts	
Type	HSFG
Grade	8.8
Diameter (mm)	20
Bolt Numbers	6
Columns (Vertical Lines)	2
Bolts Per Column	3

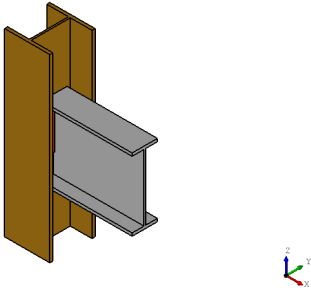
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	70
Edge Distance (mm)	37
Assembly	
Column-Beam Clearance (mm)	20


Company Name	abc	Project Title	
Group/Team Name	xyz	Subtitle	
Designer	amit	Job Number	
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Check			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb} = ((800.0 \times 0.6126 \times 20 \times 20) / (\sqrt{3} \times 1.25 \times 1000)) = 52.694$ [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{dpb} = (2.5 \times 0.508 \times 20 \times 10.0 \times 410) / (1.25 \times 1000) = 83.312$ [cl. 10.3.4]	
Bolt capacity (kN)		Min (52.694, 83.312) = 52.694	Pass
Critical bolt shear (kN)	≤ 52.694	50.089	Pass
No. of bolts		6	
No. of column(s)	≤ 2	2	
No. of bolts per column per side of end plate		3	
Bolt pitch (mm)	$\geq 2.5 \times 20 = 50, \leq \text{Min}(32 \times 8.9, 300) = 285$ [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	$\geq 2.5 \times 20 = 50, \leq \text{Min}(32 \times 8.9, 300) = 285$ [cl. 10.2.2]	0	
End distance (mm)	$\geq 1.7 \times 22.0 = 37.4, \leq 12 \times 8.9 = 106.8$ [cl. 10.2.4]	70	Pass
Edge distance (mm)	$\geq 1.7 \times 22.0 = 37.4, \leq 12 \times 8.9 = 106.8$ [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥ 160	$V_{db} = 407$ [cl. 6.4.1]	
Plate thickness (mm)	≥ 8	20	Pass
Plate height (mm)	$\geq 0.6 \times 400.0 = 240.0, \leq 400.0 - 16.0 - 14.0 - 16.0 - 14.0 - 10 = 330.0$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	240	Pass

Plate Width (mm)	$\geq 174, \leq 160.0$	180	Pass
Effective weld length (mm)		$240 - 2 \cdot 6 = 228$	
Weld strength (kN/mm)	0.351	$f_v = (0.7 \cdot 6 \cdot 410) / (\sqrt{3} \cdot 1.25 \cdot 1000)$ $= 0.795$ [cl. 10.5.7]	Pass

		<div> <div>Created with</div> <div>  <div>Osdag</div> </div> </div>	
Company Name	abc	Project Title	
Group/Team Name	xyz	Subtitle	
Designer	amit	Job Number	
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

Views	
	

		 Created with Osdag	
Company Name	abc	Project Title	
Group/Team Name	xyz	Subtitle	
Designer	amit	Job Number	
Date	04 /06 /2016	Metdod	Limit State Design (No Earthquake Load)
Additional Comments			