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Company Name	ca	Project Title	cleat angle
Group/Team Name	caa	Subtitle	
Designer		Job Number	1235435
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	•
Shear Force (kN)	100.0
Components	
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 120X120X10
Thickness (mm)	10
Cleat Leg Size B (mm)	120
Cleat Leg Size A (mm)	120
Hole	STD
Bolts on Beam	
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	4
Columns (Vertical Lines)	1
Bolts Per Column	4
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	37

Edge Distance (mm)	37
Bolts on Column	
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	6
Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	62
Edge Distance (mm)	53.85
Assembly	
Column-Beam Clearance (mm)	20

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Design Check: Se	Design Check: Secondary Beam Connectivity			
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		V_{dsb} = ((2*400*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 90.529 [cl. 10.3.3]		
Bolt bearing capacity (kN)		V _{dpb} = (2.5*0.508*20*7.7*400)/(1.25*1000) = 62.586 [cl. 10.3.4]		
Bearing capacity of beam web (kN)		V_{dpb} = (2.5*0.508*20*7.7*410)/(1.25*1000) = 64.15 [cl. 10.3.4]		
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.508*20*10*410)/(1.25*1000) = 83.312 [cl. 10.3.4]		
Bearing capacity (kN)		Min (62.586, 64.15, 83.312) = 62.586		
Bolt capacity (kN)		Min (90.529, 62.586) = 62.586		
Critical bolt shear (kN)	≤ 62.586	27.861	Pass	
No. of bolts		4		
No.of column(s)	≤ 2	1		
No. of bolts per column		4		
Bolt pitch (mm)	\geq 2.5* 20 = 50, \leq Min(32*7.7, 300) = 247 [cl. 10.2.2]	50	Pass	
Bolt gauge (mm)	\geq ;2.5*20 = 50, \leq Min(32*7.7, 300) = 247 [cl. 10.2.2]	0		
End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*7.7 = 92.4 [cl. 10.2.4]	37	Pass	
Edge distance	≥ 1.7*22.0 = 37.4, ≤ 12*7.7 = 92.4	37	Pass	

(mm)	[cl. 10.2.4]		
Block shear capacity (kN)	≥ 100.0	V _{db} = 271.568 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	(2*90.529*50 ²)/(50*1000) = 4.15	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 150.528 [cl. 8.2.1.2]	Pass

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Design Check: Pri	Design Check: Primary Beam Connectivity			
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((400*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 45.264 [cl. 10.3.3]		
Bolt bearing capacity (kN)		V _{dpb} = (2.5*0.508*20*9.4*400)/(1.25*1000) = 76.403 [cl. 10.3.4]		
Bearing capacity of beam web (kN)		V _{dpb} = (2.5*0.508*20*9.4*410)/(1.25*1000) = 78.313 [cl. 10.3.4]		
Bearing capacity of cleat (kN)		V _{dpb} = (2.5*0.508*20*10*410)/(1.25*1000) = 83.312 [cl. 10.3.4]		
Bearing capacity (kN)		Min (76.403, 78.313, 83.312) = 83.312		
Bolt capacity (kN)		Min (45.264, 83.312) = 45.264		
Critical bolt shear (kN)	≤ 45.264	38.766	Pass	
No. of bolts		6		
No.of column(s) per angle	≤ 2	1		
No. of bolts per column per angle		3		
Bolt pitch (mm)	$\geq 2.5^* \ 20 = 50, \leq Min(32^*9.4, 300) = 300$ [cl. 10.2.2]	50	Pass	
Bolt gauge (mm)	≥ 2.5*20 = 50, ≤ Min(32*9.4, 300) = 300 [cl. 10.2.2]	0		
End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*9.4 = 112.8 [cl. 10.2.4]	62	Pass	
	≥1.7*22.0 = 37.4, ≤12*9.4 =			

Edge distance (mm)	112.8 [cl. 10.2.4]	53.85	Pass
Block shear capacity (kN)	≥100.0	$V_{\rm db}$ = 304.751 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0- 13.1-14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	(2*45.264*50 ²)/(50*1000) = 3.5	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 150.528 [cl. 8.2.1.2]	Pass

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Additional Comments	