Company Name AkerSolution Project Title Group/Team Name Subtitle Designer Amar Gajjam Job Number

Date 05 /06 /2016 Method Limit State Design (No Earthquake Load)

**Design Conclusion** 

Finplate Pass

Finplate

**Connection Properties** 

Connection

Connection Title Single Finplate **Shear Connection** Connection Type

**Connection Category** 

Connectivity Column flange-Beam web

**Bolted Beam Connection** Column Connection Welded

Loading (Factored Load)

Shear Force (kN) 200

Components

Column Section **ISSC 200** Material Fe 410 Beam Section **ISMB 400** Material Fe 410 Hole STD Plate Section 330X80X16

Thickness (mm) 16 Width (mm) 80 Depth (mm) 330 STD Hole

Weld

Type Double Fillet

Size (mm) 13

**Bolts** 

Type **HSFG** Grade 8.8 Diameter (mm) 12 **Bolt Numbers** 7 Columns (Vertical Lines) 1 Bolts Per Column 7 Gauge (mm) 0 Pitch (mm) 45 End Distance (mm) 30 30 Edge Distance (mm)

Assembly

Column-Beam Clearance (mm) 20

Created with

Company Name AkerSolution Project Title Subtitle Group/Team Name Designer Amar Gajjam Job Number

05 /06 /2016 Method Date Limit State Design (No Earthquake Load)

Design Check

Bolt shear capacity (kN)

Bolt bearing capacity (kN)

Check Required Provided Remark

 $V_{\rm dsb} = (800*0.6126*12*12)/(\sqrt{3}*1.25*1000) =$ 

[cl. 10.3.3]

 $V_{\rm dpb} = (2.5*0.519*12*8.9*410)/(1.25*1000) =$ 

45.452 [cl. 10.3.4]

31.223

Bolt capacity (kN) Min (31.223, 45.452) = 31.223

No. of bolts 200/31.223 = 6.4**Pass** 

| No.of column(s)             | $\leq 2$   | 1   |      |
|-----------------------------|--|---|------|
| No. of bolts per column     |  | 7   |      |
| Bolt pitch (mm)             | $\geq 2.5* 12 = 30, \leq Min(32*8.9, 300) = 285$ [cl. 10.2.2]  | 45  | Pass |
| Bolt gauge (mm)             | $\geq 2.5*12 = 30, \leq Min(32*8.9, 300) = 285$ [cl. 10.2.2]   | 0   |      |
| End distance (mm)           | $\geq 1.7*13 = 22.1, \leq 12*8.9 = 106.8$ [cl. 10.2.4]   | 30  | Pass |
| Edge distance (mm)          | $\geq 1.7*13 = 22.1, \leq 12*8.9 = 106.8$ [cl. 10.2.4]   | 30  | Pass |
| Block shear capacity (kN)   | $\geq$ 200   | $V_{\rm db} = 696$  | Pass |
| Plate thickness (mm)        | (5*200*1000)/(330*250) = 12.12<br>[Owens and Cheal, 1989]  | 16  | Pass |
| Plate height (mm)           | ≥ 0.6*400=240.0, ≤ 400-16-14-10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002]                          | 330   | Pass |
| Plate width (mm)            |  | 100   |      |
| Plate moment capacity (kNm) | $(2*31.223*45^2)/(45*1000) = 16.86$  | $M_{\rm d} = (1.2*250*Z)/(1000*1.1) = 79.2$ [cl. 8.2.1.2]             | Pass |
| Effective weld length (mm)  |  | 330-2*16 = 298  |      |
| Weld strength (kN/mm)       | $\sqrt{[(16860*6)/(2*298^2)]^2 + [200/(2*298)]^2}$<br>= 0.661  | $f_{\rm V} = (0.7*13*410)/(\sqrt{3}*1.25)$<br>= 2.121<br>[cl. 10.5.7] | Pass |
| Weld thickness (mm)         | Max( $(0.661*1000*\sqrt{3}*1.25)/(0.7*410),16*0.8$ ) = 12.8<br>[cl. 10.5.7, Insdag Detailing Manual, 2002] | 13  | Pass |

## Created with

Company Name AkerSolution Project Title
Group/Team Name Subtitle
Designer Amar Gajjam Job Number

Date 05 /06 /2016 Method Limit State Design (No Earthquake Load)

Views

Created with

Company Name AkerSolution Project Title
Group/Team Name Subtitle
Designer Amar Gajjam Job Number

Date 05 /06 /2016 Method Limit State Design (No Earthquake Load)

**Additional Comments**