Detter education		Created with OSdac	
Company Name	KLE Institute of Technology Hubballi	Project Title	Assignment_04
Group/Team Name	Civil Dept.	Subtitle	
Designer	ISPatil	Job Number	04
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion		
Finplate	Pass	
Finplate		
Connection Properties		
Connection		
Connection Title	Single Finplate	
Connection Type	Shear Connection	
Connection Category		
Connectivity	Column flange-Beam web	
Beam Connection	Bolted	
Column Connection	Welded	
Loading (Factored Load)		
Shear Force (kN)	160	
Components		
Column Section	ISSC 200	
Material	Fe 410	
Beam Section	ISMB 400	
Material	Fe 410	
Hole	STD	
Plate Section	250X120X12	
Thickness (mm)	12	
Width (mm)	120	
Depth (mm)	250	
Hole	STD	
Weld		
Туре	Double Fillet	
Size (mm)	10	
Bolts	*	
Туре	HSFG	
Grade	8.8	
Diameter (mm)	12	
Bolt Numbers	6	
Columns (Vertical Lines)	1	
Bolts Per Column	6	

Gauge (mm)	0
Pitch (mm)	38
End Distance (mm)	30
Edge Distance (mm)	30
Assembly	
Column-Beam Clearance (mm)	20

TOSSEC e d u c a t i o n		Created with Created with	
Company Name	KLE Institute of Technology Hubballi	Project Title	Assignment_04
Group/Team Name	Civil Dept.	Subtitle	
Designer	ISPatil	Job Number	04
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Check			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		V_{dsb} = (800*0.6126*12*12)/($\sqrt{3}$ *1.25*1000) = 31.223 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V _{dpb} = (2.5*0.519*12*8.9*410)/(1.25*1000) = 45.452 [cl. 10.3.4]	
Bolt capacity (kN)		Min (31.223, 45.452) = 31.223	
No. of bolts	160/31.223 = 5.1	6	Pass
No.of column(s)	≤ 2	1	
No. of bolts per column		6	
Bolt pitch (mm)	≥ 2.5* 12 = 30, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	38	Pass
Bolt gauge (mm)	\geq 2.5*12 = 30, \leq Min(32*8.9, 300) = 285 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*13 = 22.1, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	30	Pass
Edge distance (mm)	≥ 1.7*13 = 22.1, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	30	Pass
Block shear capacity (kN)	≥ 160	V _{db} = 385	Pass
Plate thickness (mm)	(5*160*1000)/(250*250) = 12.8 [Owens and Cheal, 1989]	12	Pass
Plate height (mm)	≥ 0.6*400=240.0, ≤ 400-16-14- 10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002]	250	Pass
Plate width (mm)		100	
Plate moment capacity (kNm)	(2*31.223*38 ²)/(38*1000) = 14.4	$M_{\rm d}$ = (1.2*250*Z)/(1000*1.1) = 34.09 [cl. 8.2.1.2]	Pass
Effective weld		250-2*10 = 230	

length (mm) Weld strength (kN/mm)	$\sqrt{[(14400*6)/(2*230^2)]^2 + [160/(2*230)]^2}$ = 0.888	$f_V = (0.7*10*410)/(\sqrt{3}*1.25)$ = 1.326 [cl. 10.5.7]	Pass
Weld thickness (mm)	Max((0.888*1000*√3* 1.25)/(0.7 * 410),12* 0.8) = 9.6 [cl. 10.5.7, Insdag Detailing Manual, 2002]	10	Pass

ossec e t t e r e d u c a t i o n		Created with OSdac	
Company Name	KLE Institute of Technology Hubballi	Project Title	Assignment_04
Group/Team Name	Civil Dept.	Subtitle	
Designer	ISPatil	Job Number	04
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Views	

fossee e d u c a t i o n		Created with OSdag	
Company Name	KLE Institute of Technology Hubballi	Project Title	Assignment_04
Group/Team Name	Civil Dept.	Subtitle	
Designer	ISPatil	Job Number	04
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Additional Comments	