Company Name	**rohini**	**Project Title**	**problem 4**
Group/Team Name	**abc**	**Subtitle**	
Designer	**khandelwal**	**Job Number**	
Date	**05 /06 /2016**	**Method**	**Limit State Design (No Earthquake Load)**

Design Conclusion	
Finplate	**Pass**
Finplate	
Connection Properties	
Connection	
Connection Title	Single Finplate
Connection Type	Shear Connection
Connection Category	
Connectivity	Column flange-Beam web
Beam Connection	Bolted
Column Connection	Welded
Loading (Factored Load)	
Shear Force (kN)	200
Components	
Column Section	ISSC 200
Material	Fe 410
Beam Section	ISMB 400
Material	Fe 410
Hole	STD
Plate Section	250X80X16
Thickness (mm)	16
Width (mm)	80
Depth (mm)	250
Hole	STD
Weld	
Type	Double Fillet
Size (mm)	13
Bolts	
Type	HSFG
Grade	8.8
Diameter (mm)	16
Bolt Numbers	4
Columns (Vertical Lines)	1
Bolts Per Column	4
Gauge (mm)	0
Pitch (mm)	63
End Distance (mm)	30
Edge Distance (mm)	30
Assembly	
Column-Beam Clearance (mm)	20

Company Name	rohini	Project Title	problem 4
Group/Team Name	abc	Subtitle	
Designer	khandelwal	Job Number	
Date	05 /06 /2016	Method	Limit State Design (No Earthquake Load)

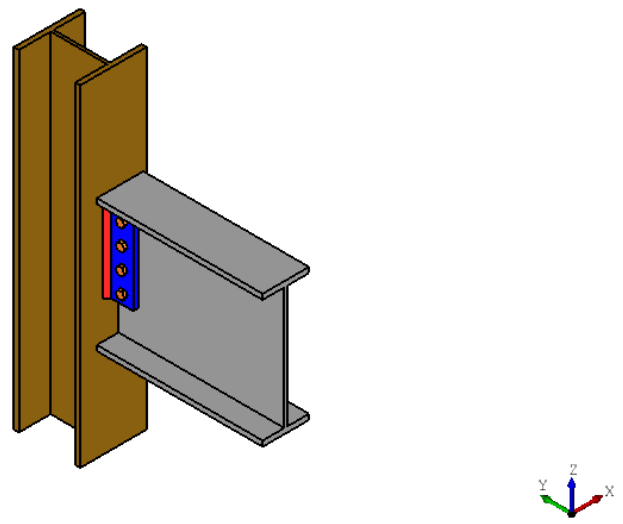
Design Check			
Check	**Required**	**Provided**	**Remark**
Bolt shear capacity (kN)		V_{dsb} = (800*0.6126*16*16)/(√3*1.25*1000) = 58.012 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.491*16*8.9*410)/(1.25*1000) = 57.333 [cl. 10.3.4]	
Bolt capacity (kN)		Min (58.012, 57.333) = 57.333	
No. of bolts	200/57.333 = 3.5	4	**Pass**
No.of column(s)	≤ 2	1	
No. of bolts per column		4	
Bolt pitch (mm)	≥ 2.5* 16 = 40, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	63	**Pass**
Bolt gauge (mm)	≥ 2.5*16 = 40, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*18 = 30.6, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	30	**Pass**
Edge distance (mm)	≥ 1.7*18 = 30.6, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	30	**Pass**
Block shear capacity (kN)	≥ 200	V_{db} = 534	**Pass**
Plate thickness (mm)	(5*200*1000)/(250*250) = 16.0 [Owens and Cheal, 1989]	16	**Pass**
Plate height (mm)	≥ 0.6*400=240.0, ≤ 400-16-14-10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002]	250	**Pass**
Plate width (mm)		100	
Plate moment capacity (kNm)	(2*58.012*63^2)/(63*1000) = 14.735	M_d = (1.2*250*Z)/(1000*1.1) = 45.45 [cl. 8.2.1.2]	**Pass**
Effective weld length (mm)		250-2*16 = 218	
Weld strength (kN/mm)	√[(14735*6)/(2*218^2)]^2 + [200/(2*218)]^2 = 1.037	f_v= (0.7*13*410)/(√3*1.25) = 2.121 [cl. 10.5.7]	**Pass**
Weld thickness (mm)	Max((1.037*1000*√3* 1.25)/(0.7 * 410),16* 0.8) = 12.8 [cl. 10.5.7, Insdag Detailing Manual,	13	**Pass**

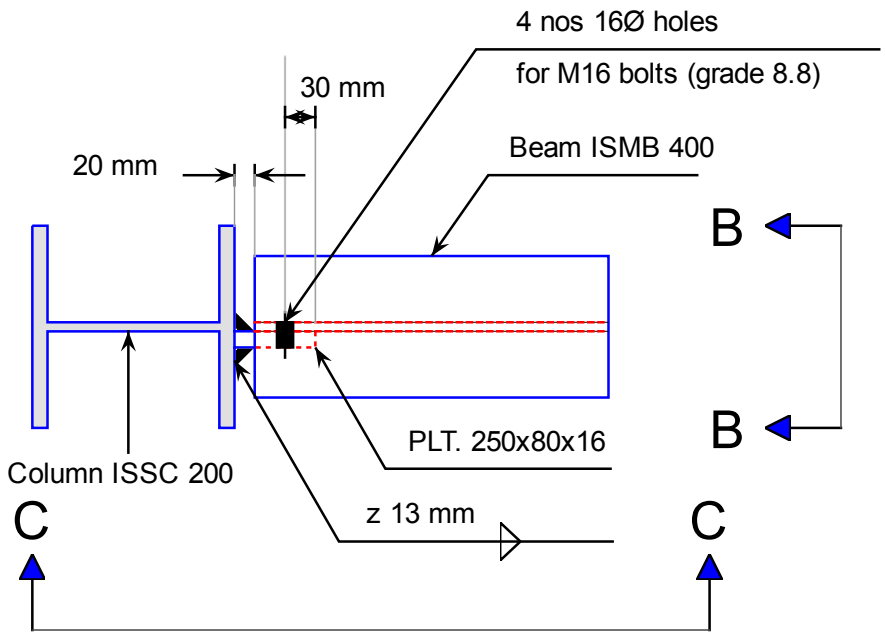
	2002]		

Created with Osdag

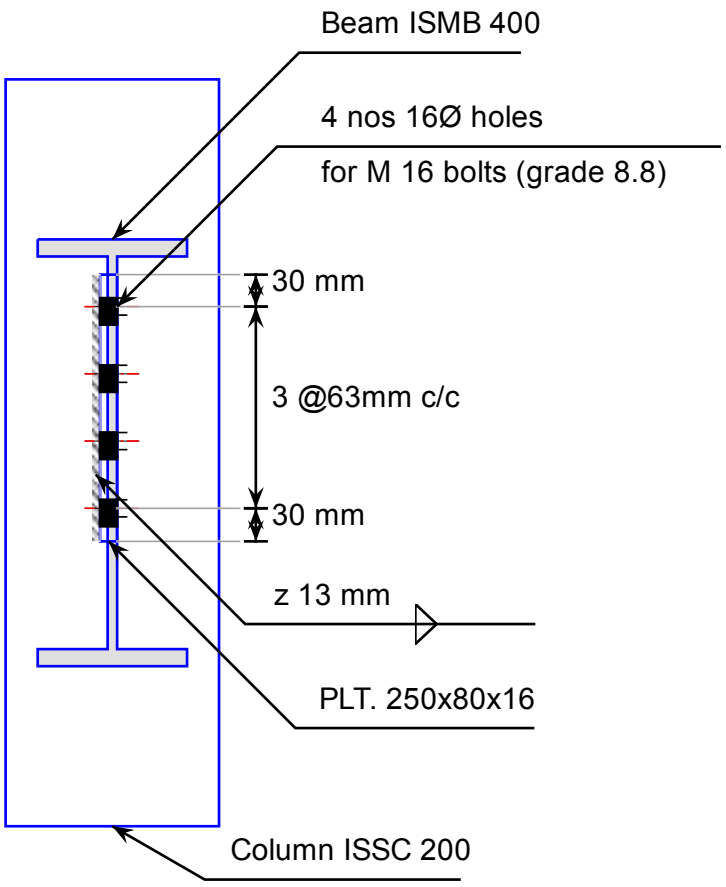
Company Name	**rohini**	**Project Title**	**problem 4**
Group/Team Name	**abc**	**Subtitle**	
Designer	**khandelwal**	**Job Number**	
Date	**05 /06 /2016**	**Method**	**Limit State Design (No Earthquake Load)**

Views

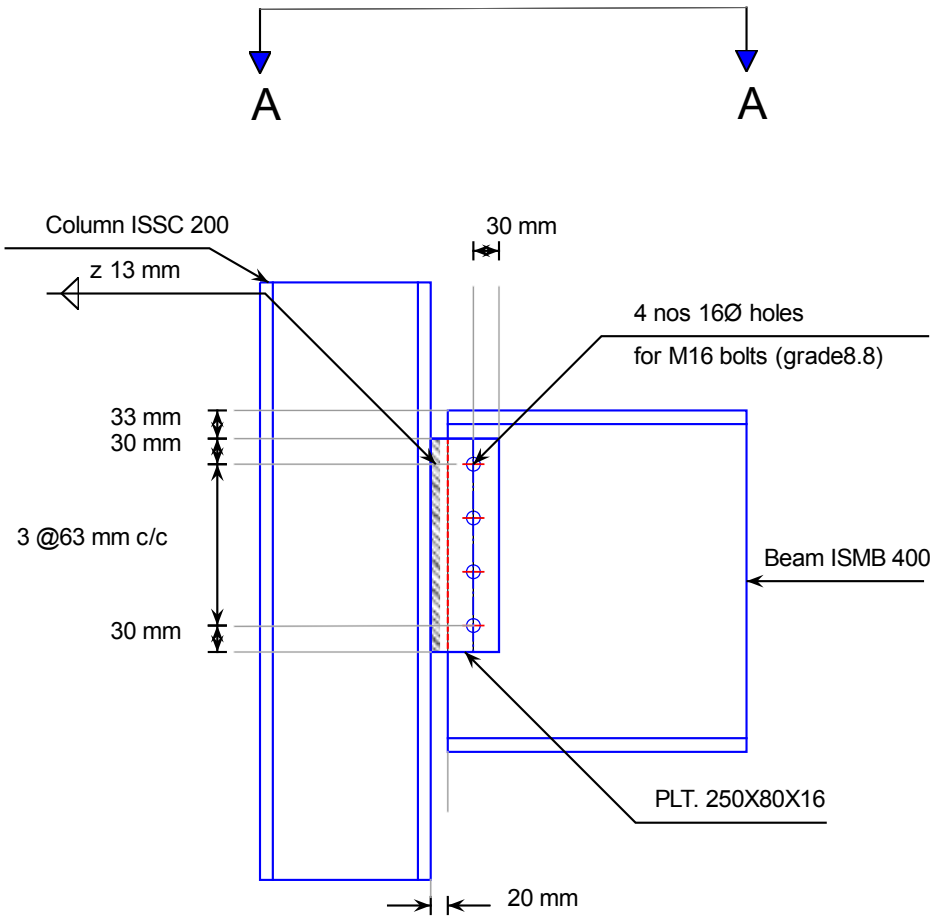




Top view (Sec A-A)



Side view (Sec B-B)



Front view (Sec C-C)

Company Name	rohini	Project Title	problem 4
Group/Team Name	abc	Subtitle	
Designer	khandelwal	Job Number	
Date	05 /06 /2016	Method	Limit State Design (No Earthquake Load)

Additional Comments	