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Company Name	Nandadeep designers and valures Pvt.Ltd.	Project Title	Cleat angle
Group/Team Name	NDVPL	Subtitle	
Designer	Priyanka	Job Number	123
Date	05 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	100.0
Components	
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 100X100X8
Thickness (mm)	8
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD
Bolts on Beam	<u> </u>
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	4
Columns (Vertical Lines)	1
Bolts Per Column	4
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	37
Edge Distance (mm)	37
Bolts on Column	•
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	6

Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	62
Edge Distance (mm)	37
Assembly	
Column-Beam Clearance (mm)	20

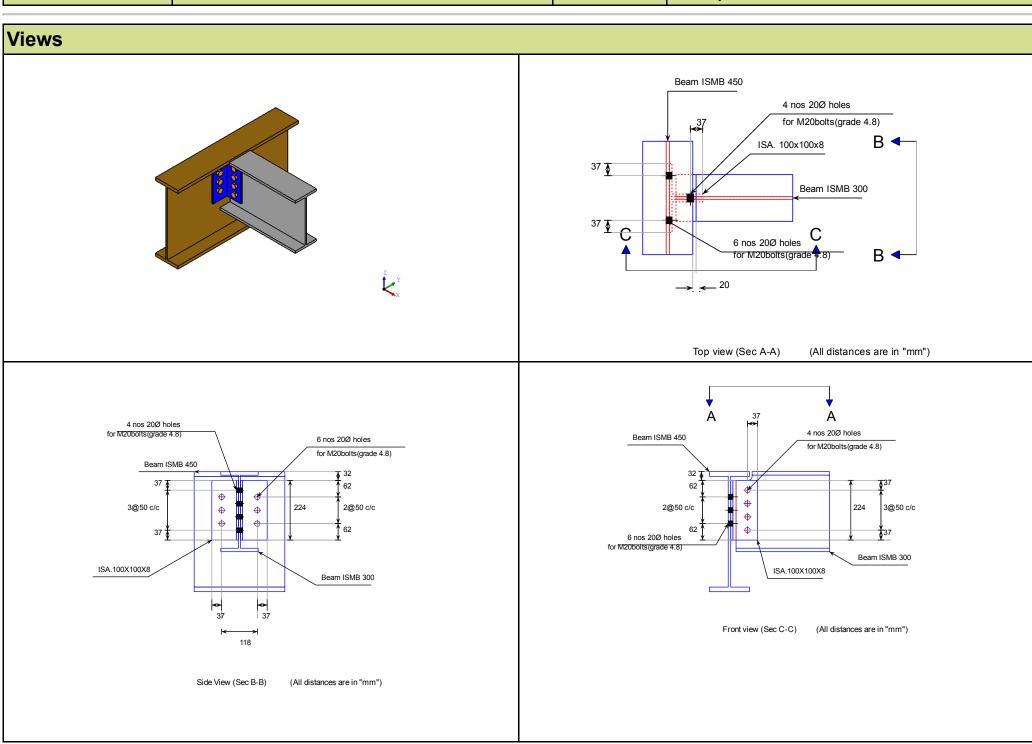
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Design Check: Secondary Beam Connectivity				
Check	Required	Provided	Remark	
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((2*400*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 90.529 [cl. 10.3.3]		
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.508*20*7.7*400)/(1.25*1000) = 62.586 [cl. 10.3.4]		
Bearing capacity of beam web (kN)		V_{dpb} = (2.5*0.508*20*7.7*410)/(1.25*1000) = 64.15 [cl. 10.3.4]		
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.508*20*8*410)/(1.25*1000) = 66.65 [cl. 10.3.4]		
Bearing capacity (kN)		Min (62.586, 64.15, 66.65) = 62.586		
Bolt capacity (kN)		Min (90.529, 62.586) = 62.586		
Critical bolt shear (kN)	≤ 62.586	22.66	Pass	
No. of bolts		4		
No.of column(s)	≤ 2	1		
No. of bolts per column		4		
Bolt pitch (mm)	\geq 2.5* 20 = 50, \leq Min(32*7.7, 300) = 247 [cl. 10.2.2]	50	Pass	
Bolt gauge (mm)	≥ ;2.5*20 = 50, ≤ Min(32*7.7, 300) = 247 [cl. 10.2.2]	0		
End distance (mm)	$\geq 1.7*22.0 = 37.4, \leq 12*7.7 = 92.4$ [cl. 10.2.4]	37	Pass	
Edge distance (mm)	$\geq 1.7*22.0 = 37.4, \leq 12*7.7 = 92.4$ [cl. 10.2.4]	37	Pass	
Block shear capacity (kN)	≥ 100.0	V_{db} = 217.254 [cl. 6.4.1]	Pass	
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0-13.1-14.0- 17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass	
Cleat moment capacity (kNm)	(2*90.529*50 ²)/(50*1000) = 3.15	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 120.422 [cl. 8.2.1.2]	Pass	

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Design Check: Primary Beam Connectivity			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{\rm dsb}$ = ((400*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 45.264 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.508*20*8.0*400)/(1.25*1000) = 65.024 [cl. 10.3.4]	
Bearing capacity of beam web (kN)		V_{dpb} = (2.5*0.508*20*9.4*410)/(1.25*1000) = 78.313 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		V_{dpb} = (2.5*0.508*20*8*410)/(1.25*1000) = 66.65 [cl. 10.3.4]	
Bearing capacity (kN)		Min (65.024, 78.313, 66.65) = 66.65	
Bolt capacity (kN)		Min (45.264, 66.65) = 45.264	
Critical bolt shear (kN)	≤ 45.264	37.35	Pass
No. of bolts		6	
No.of column(s) per angle	≤ 2	1	
No. of bolts per column per angle		3	
Bolt pitch (mm)	\geq 2.5* 20 = 50, \leq Min(32*8.0, 300) = 256 [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	\geq 2.5*20 = 50, \leq Min(32*8.0, 300) = 256 [cl. 10.2.2]	0	
End distance (mm)	$\geq 1.7*22.0 = 37.4, \leq 12*8.0 = 96.0$ [cl. 10.2.4]	62	Pass
Edge distance (mm)	≥1.7*22.0 = 37.4, ≤12*8.0 = 96.0 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥100.0	V _{db} = 213.164 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0-13.1-14.0- 17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment capacity (kNm)	$(2*45.264*50^2)/(50*1000) = 3.342$	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 120.422 [cl. 8.2.1.2]	Pass

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Additional Comments	