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Company Name	Tata Consulting Engineers Ltd.	Project Title	Design Problem 1
Group/Team Name	Team 1	Subtitle	
Designer	Mazhar	Job Number	1
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Finplate	Pass
Finplate	
Connection Properties	
Connection	
Connection Title	Single Finplate
Connection Type	Shear Connection
Connection Category	
Connectivity	Column flange-Beam web
Beam Connection	Bolted
Column Connection	Welded
Loading (Factored Load)	·
Shear Force (kN)	160
Components	·
Column Section	ISSC 200
Material	Fe 410
Beam Section	ISMB 400
Material	Fe 410
Hole	STD
Plate Section	300X100X10
Thickness (mm)	10
Width (mm)	100
Depth (mm)	300
Hole	STD
Weld	
Туре	Double Fillet
Size (mm)	8
Bolts	·
Туре	HSFG
Grade	8.8
Diameter (mm)	20

Bolt Numbers	3	
Columns (Vertical Lines)	1	
Bolts Per Column	3	
Gauge (mm)	0	
Pitch (mm)	110	
End Distance (mm)	40	
Edge Distance (mm)	40	
Assembly		
Column-Beam Clearance (mm)	20	

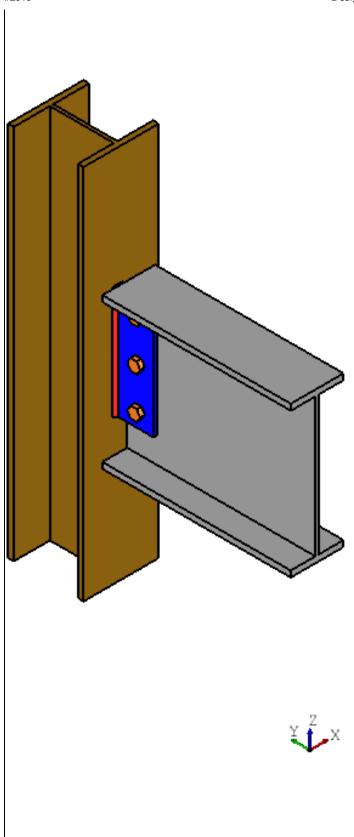
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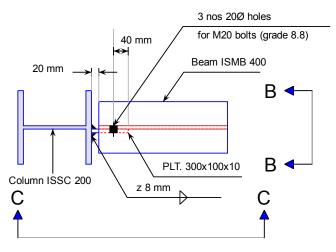
Design Check			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		V_{dsb} = (800*0.6126*20*20)/($\sqrt{3}$ *1.25*1000) = 90.529 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V _{dpb} = (2.5*0.508*20*8.9*410)/(1.25*1000) = 74.148 [cl. 10.3.4]	
Bolt capacity (kN)		Min (90.529, 74.148) = 74.148	
No. of bolts	160/74.148 = 2.2	3	Pass
No.of column(s)	≤ 2	1	
No. of bolts per column		3	
Bolt pitch (mm)	≥ 2.5* 20 = 50, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	110	Pass
Bolt gauge (mm)	≥ 2.5*20 = 50, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*22 = 37.4, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	40	Pass
Edge distance (mm)	≥ 1.7*22 = 37.4, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	40	Pass
Block shear capacity (kN)	≥ 160	V _{db} = 426	Pass
Plate thickness (mm)	(5*160*1000)/(300*250) = 10.67 [Owens and Cheal, 1989]	10	Pass
Plate height (mm)	≥ 0.6*400=240.0, ≤ 400-16-14- 10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002]	300	Pass
Plate width (mm)		100	
Plate moment	(2*90.529*110 ²)/(110*1000) =	$M_{\rm d} = (1.2*250*Z)/(1000*1.1) = 40.91$	Pass

capacity (kNm)	19.916	[cl. 8.2.1.2]	
Effective weld length (mm)		300-2*8 = 284	
Weld strength (kN/mm)	$\sqrt{[(19916*6)/(2*284^2)]^2}$ + $[160/(2*284)]^2$ = 0.793	$f_V = (0.7*8*410)/(\sqrt{3}*1.25)$ = 1.06 [cl. 10.5.7]	Pass
Weld thickness (mm)	Max($(0.793*1000*\sqrt{3}*1.25)/(0.7*410)$, $10*0.8$) = 8.0 [cl. 10.5.7, Insdag Detailing Manual, 2002]	8	Pass

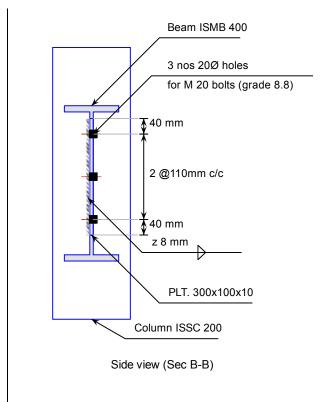
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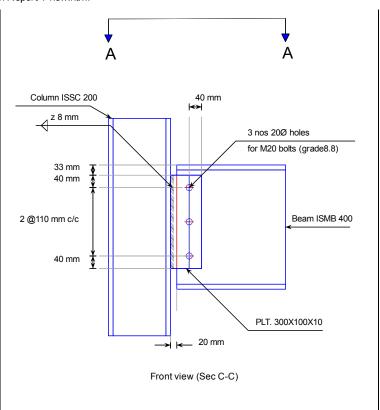
Views	





Top view (Sec A-A)





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Additional Comments	Comment for design problem 1
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