			Created with OSdag
<b>Company Name</b>	Aker Solutions	<b>Project Title</b>	Set 2.1
Group/Team Name	Aker Solutions	Subtitle	
Designer	Vishal Nalwar	Job Number	Set 2.1
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	<u> </u>
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	100.0
Components	<u> </u>
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 100X100X8
Thickness (mm)	8
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD
Bolts on Beam	
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	4
Columns (Vertical Lines)	1
Bolts Per Column	4
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	37
Edge Distance (mm)	37
Bolts on Column	
Туре	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	6

Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	62
Edge Distance (mm)	37
Assembly	
Column-Beam Clearance (mm)	20

			Created with OSdag
<b>Company Name</b>	Aker Solutions	Project Title	Set 2.1
Group/Team Name	Aker Solutions	Subtitle	
Designer	Vishal Nalwar	Job Number	Set 2.1
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Check: Secon	dary Beam Connectivity		
Check	Required	Provided	Remark
Bolt shear capacity (kN)	•	$V_{dsb}$ = ((2*400*0.6126*20*20)/( $\sqrt{3}$ *1.25*1000) = 90.529 [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*7.7*400)/(1.25*1000) = 62.586 [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*7.7*410)/(1.25*1000) = 64.15 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*8*410)/(1.25*1000) = 66.65 [cl. 10.3.4]	
Bearing capacity (kN)		Min (62.586, 64.15, 66.65) = 62.586	
Bolt capacity (kN)		Min (90.529, 62.586) = 62.586	
Critical bolt shear (kN)	≤ 62.586	22.66	Pass
No. of bolts		4	
No.of column(s)	≤ 2	1	
No. of bolts per column		4	
Bolt pitch (mm)	$\geq$ 2.5* 20 = 50, $\leq$ Min(32*7.7, 300) = 247 [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	$\geq$ ;2.5*20 = 50, $\leq$ Min(32*7.7, 300) = 247 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*7.7 = 92.4 [cl. 10.2.4]	37	Pass
Edge distance (mm)	≥ 1.7*22.0 = 37.4, ≤ 12*7.7 = 92.4 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥ 100.0	$V_{\rm db}$ = 217.254 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0-13.1- 14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment	(2*90.529*50 <sup>2</sup> )/(50*1000) = 3.15	$M_{\rm d}$ = (1.2*250* <i>Z</i> )/(1000*1.1) = 120.422	Pass

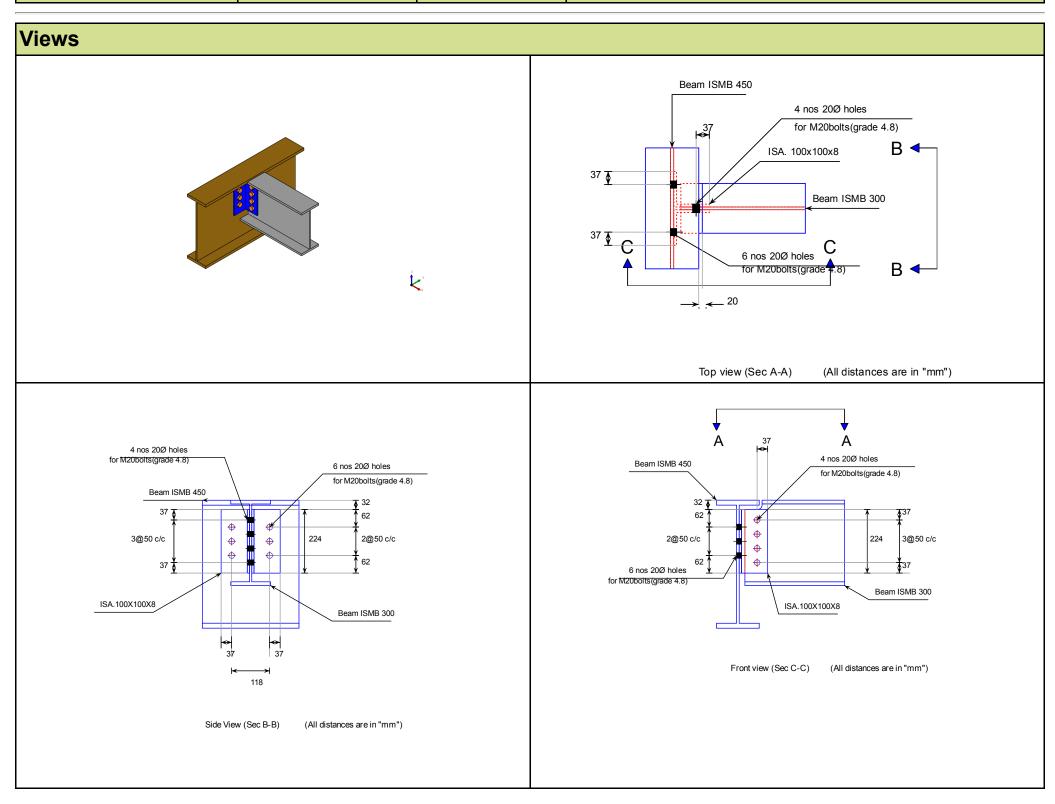
capacity (kNm)	[cl.	. 8.2.1.2]

			Created with OSdag
<b>Company Name</b>	Aker Solutions	<b>Project Title</b>	Set 2.1
Group/Team Name	Aker Solutions	Subtitle	
Designer	Vishal Nalwar	Job Number	Set 2.1
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Check: Primary	y Beam Connectivity		
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb}$ = ((400*0.6126*20*20)/( $\sqrt{3}$ *1.25*1000) = 45.264 [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*8.0*400)/(1.25*1000) = 65.024 [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*9.4*410)/(1.25*1000) = 78.313 [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{\text{dpb}}$ = (2.5*0.508*20*8*410)/(1.25*1000) = 66.65 [cl. 10.3.4]	
Bearing capacity (kN)		Min (65.024, 78.313, 66.65) = 66.65	
Bolt capacity (kN)		Min (45.264, 66.65) = 45.264	
Critical bolt shear (kN)	≤ 45.264	37.35	Pass
No. of bolts		6	
No.of column(s) per angle	≤ 2	1	
No. of bolts per column per angle		3	
Bolt pitch (mm)	≥ 2.5* 20 = 50, ≤ Min(32*8.0, 300) = 256 [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	$\geq$ 2.5*20 = 50, $\leq$ Min(32*8.0, 300) = 256 [cl. 10.2.2]	0	
End distance (mm)	$\geq 1.7*22.0 = 37.4, \leq 12*8.0 = 96.0$ [cl. 10.2.4]	62	Pass
Edge distance (mm)	≥1.7*22.0 = 37.4, ≤12*8.0 = 96.0 [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	≥100.0	$V_{db}$ = 213.164 [cl. 6.4.1]	Pass
Cleat height (mm)	≥ 0.6*300.0=180.0, ≤ 300.0-13.1- 14.0-17.4-15.0- 5=235.5 [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment		$M_{\rm d}$ = (1.2*250* $Z$ )/(1000*1.1) =	Pass

| capacity (kNm) |  $(2*45.264*50^2)/(50*1000) = 3.342$  | 120.422 | [cl. 8.2.1.2]

			Created with OSdag
Company Name	Aker Solutions	Project Title	Set 2.1
<b>Group/Team Name</b>	Aker Solutions	Subtitle	
Designer	Vishal Nalwar	Job Number	Set 2.1
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)



			Created with OSdag
<b>Company Name</b>	Aker Solutions	Project Title	Set 2.1
<b>Group/Team Name</b>	Aker Solutions	Subtitle	
Designer	Vishal Nalwar	Job Number	Set 2.1
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Additional Comments	Beam to beam cleat angle shear connection
---------------------	---