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Company Name	ROhini	Project Title	cleat angle connection
Group/Team Name	ABC	Subtitle	
Designer	Khandelwal	Job Number	3
Date	05 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Cleat Angle	Pass
Cleat Angle	
Connection Properties	
Connection	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
Connection Category	
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	100.0
Components	
Column Section	ISMB 450
Material	Fe 410
Beam Section	ISMB 300
Material	Fe 410
Hole	STD
Cleat Section	ISA 100X100X10
Thickness (mm)	10
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD
Bolts on Beam	
Type	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	4
Columns (Vertical Lines)	1
Bolts Per Column	4
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	37
Edge Distance (mm)	37
Bolts on Column	
Type	Black Bolt
Grade	4.8
Diameter (mm)	20
Bolt Numbers	6

Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	50
End Distance (mm)	62
Edge Distance (mm)	37
<b>Assembly</b>	
Column-Beam Clearance (mm)	20

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Design Check: Secondary Beam Connectivity			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb} = ((2*400*0.6126*20*20)/(\sqrt{3}*1.25*1000)) = 90.529$ [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{dpb} = (2.5*0.508*20*7.7*400)/(1.25*1000) = 62.586$ [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{dpb} = (2.5*0.508*20*7.7*410)/(1.25*1000) = 64.15$ [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{dpb} = (2.5*0.508*20*10*410)/(1.25*1000) = 83.312$ [cl. 10.3.4]	
Bearing capacity (kN)		Min (62.586, 64.15, 83.312) = 62.586	
Bolt capacity (kN)		Min (90.529, 62.586) = 62.586	
Critical bolt shear (kN)	$\leq 62.586$	22.66	Pass
No. of bolts		4	
No.of column(s)	$\leq 2$	1	
No. of bolts per column		4	
Bolt pitch (mm)	$\geq 2.5* 20 = 50, \leq \text{Min}(32*7.7, 300) = 247$ [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	$\geq ;2.5*20 = 50, \leq \text{Min}(32*7.7, 300) = 247$ [cl. 10.2.2]	0	
End distance (mm)	$\geq 1.7*22.0 = 37.4, \leq 12*7.7 = 92.4$ [cl. 10.2.4]	37	Pass
Edge distance (mm)	$\geq 1.7*22.0 = 37.4, \leq 12*7.7 = 92.4$ [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	$\geq 100.0$	$V_{db} = 271.568$ [cl. 6.4.1]	Pass
Cleat height (mm)	$\geq 0.6*300.0=180.0, \leq 300.0-13.1-14.0-17.4-15.0- 5=235.5$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass
Cleat moment	$(2*90.529*50^2)/(50*1000) = 3.15$	$M_d = (1.2*250*Z)/(1000*1.1) = 150.528$	Pass

capacity (kNm)		[cl. 8.2.1.2]	
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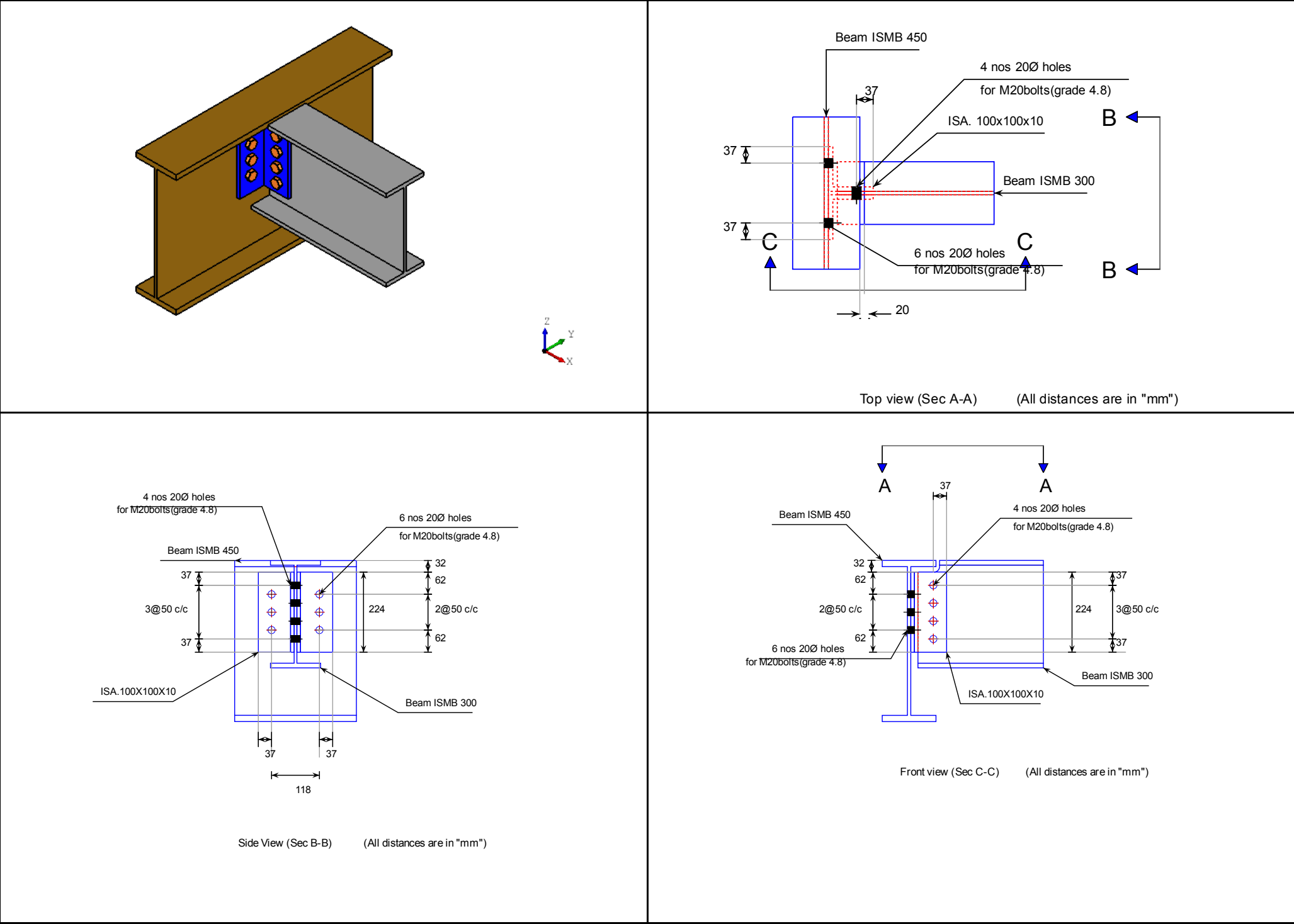
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Design Check: Primary Beam Connectivity			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb} = ((400 \times 0.6126 \times 20 \times 20) / (\sqrt{3} \times 1.25 \times 1000)) = 45.264$ [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{dpb} = (2.5 \times 0.508 \times 20 \times 9.4 \times 400) / (1.25 \times 1000) = 76.403$ [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{dpb} = (2.5 \times 0.508 \times 20 \times 9.4 \times 410) / (1.25 \times 1000) = 78.313$ [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{dpb} = (2.5 \times 0.508 \times 20 \times 10 \times 410) / (1.25 \times 1000) = 83.312$ [cl. 10.3.4]	
Bearing capacity (kN)		Min (76.403, 78.313, 83.312) = 83.312	
Bolt capacity (kN)		Min (45.264, 83.312) = 45.264	
Critical bolt shear (kN)	$\leq 45.264$	37.35	Pass
No. of bolts		6	
No.of column(s) per angle	$\leq 2$	1	
No. of bolts per column per angle		3	
Bolt pitch (mm)	$\geq 2.5 \times 20 = 50, \leq \text{Min}(32 \times 9.4, 300) = 300$ [cl. 10.2.2]	50	Pass
Bolt gauge (mm)	$\geq 2.5 \times 20 = 50, \leq \text{Min}(32 \times 9.4, 300) = 300$ [cl. 10.2.2]	0	
End distance (mm)	$\geq 1.7 \times 22.0 = 37.4, \leq 12 \times 9.4 = 112.8$ [cl. 10.2.4]	62	Pass
Edge distance (mm)	$\geq 1.7 \times 22.0 = 37.4, \leq 12 \times 9.4 = 112.8$ [cl. 10.2.4]	37	Pass
Block shear capacity (kN)	$\geq 100.0$	$V_{db} = 266.455$ [cl. 6.4.1]	Pass
Cleat height (mm)	$\geq 0.6 \times 300.0 = 180.0, \leq 300.0 - 13.1 - 14.0 - 17.4 - 15.0 - 5 = 235.5$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	224	Pass

Cleat moment capacity (kNm)	$(2 \cdot 45.264 \cdot 50^2) / (50 \cdot 1000) = 3.342$	$M_d = (1.2 \cdot 250 \cdot Z) / (1000 \cdot 1.1) = 150.528$ [cl. 8.2.1.2]	Pass
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Additional Comments			