



Created with

<b>Company Name</b>	<b>Vasudev Raghunath Upadhye</b>	<b>Project Title</b>	<b>Numerical 3</b>
<b>Group/Team Name</b>	<b>MIT</b>	<b>Subtitle</b>	
<b>Designer</b>	<b>VRU</b>	<b>Job Number</b>	<b>Numerical 3</b>
<b>Date</b>	<b>05 /06 /2016</b>	<b>Method</b>	<b>Limit State Design (No Earthquake Load)</b>

<b>Design Conclusion</b>	
<b>Cleat Angle</b>	<b>Pass</b>
<b>Cleat Angle</b>	
<b>Connection Properties</b>	
<b>Connection</b>	
Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection
<b>Connection Category</b>	
Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted
<b>Loading (Factored Load)</b>	
Shear Force (kN)	100.0
<b>Components</b>	
<b>Column Section</b>	ISMB 450
Material	Fe 410
<b>Beam Section</b>	ISMB 300
Material	Fe 410
Hole	STD
<b>Cleat Section</b>	ISA 100X100X8
Thickness (mm)	8
Cleat Leg Size B (mm)	100
Cleat Leg Size A (mm)	100
Hole	STD
<b>Bolts on Beam</b>	
Type	Black Bolt

Grade	4.8
Diameter (mm)	8
Bolt Numbers	18
Columns (Vertical Lines)	2
Bolts Per Column	9
Gauge (mm)	20
Pitch (mm)	20
End Distance (mm)	18
Edge Distance (mm)	18
<b>Bolts on Column</b>	
Type	Black Bolt
Grade	4.8
Diameter (mm)	8
Bolt Numbers	36
Columns (Vertical Lines)	2
Bolts Per Column	9
Gauge (mm)	20
Pitch (mm)	20
End Distance (mm)	18
Edge Distance (mm)	18
<b>Assembly</b>	
<b>Column-Beam Clearance (mm)</b>	20



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### Design Check: Secondary Beam Connectivity

Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb} = \frac{((2*400*0.6126*8*8))/(\sqrt{3}*1.25*1000)}{}$ $= 14.477$ [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{dpb} = \frac{(2.5*0.356*8*7.7*400))/(1.25*1000)}{}$ $= 17.544$ [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{dpb} = \frac{(2.5*0.356*8*7.7*410))/(1.25*1000)}{}$ $= 17.982$ [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{dpb} = \frac{(2.5*0.356*8*8*410))/(1.25*1000)}{}$ $= 18.683$ [cl. 10.3.4]	
Bearing capacity (kN)		Min (17.544, 17.982, 18.683) = 17.544	
Bolt capacity (kN)		Min (14.477, 17.544) = 14.477	
Critical bolt shear (kN)	$\leq 14.477$	6.782	Pass
No. of bolts		18	
No. of column(s)	$\leq 2$	2	
No. of bolts per column		9	

<b>Bolt pitch (mm)</b>	$\geq 2.5 \cdot 8 = 20, \leq \text{Min}(32 \cdot 7.7, 300) = 247$ [cl. 10.2.2]	20	Pass
<b>Bolt gauge (mm)</b>	$\geq 2.5 \cdot 8 = 20, \leq \text{Min}(32 \cdot 7.7, 300) = 247$ [cl. 10.2.2]	20	
<b>End distance (mm)</b>	$\geq 1.7 \cdot 11.0 = 18.7, \leq 12 \cdot 7.7 = 92.4$ [cl. 10.2.4]	18	Pass
<b>Edge distance (mm)</b>	$\geq 1.7 \cdot 11.0 = 18.7, \leq 12 \cdot 7.7 = 92.4$ [cl. 10.2.4]	18	Pass
<b>Block shear capacity (kN)</b>	$\geq 100.0$	$V_{db} = 184.304$ [cl. 6.4.1]	Pass
<b>Cleat height (mm)</b>	$\geq 0.6 \cdot 300.0 = 180.0, \leq 300.0 - 13.1 - 14.0 - 17.4 - 15.0 - 5 = 235.5$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	196	Pass
<b>Cleat moment capacity (kNm)</b>	$(2 \cdot 14.477 \cdot 20^2) / (20 \cdot 1000) = 3.6$	$M_d = (1.2 \cdot 250 \cdot Z) / (1000 \cdot 1.1) = 92.198$ [cl. 8.2.1.2]	Pass



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### Design Check: Primary Beam Connectivity

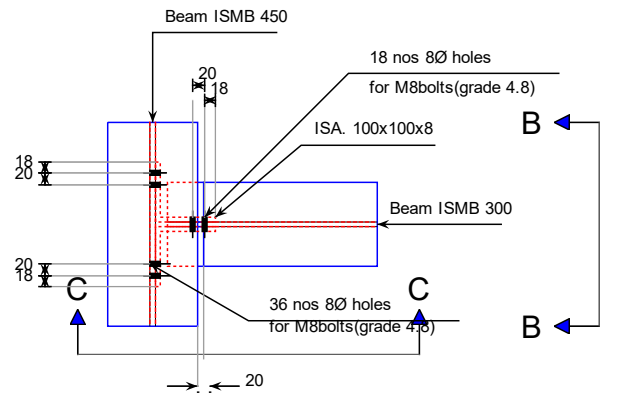
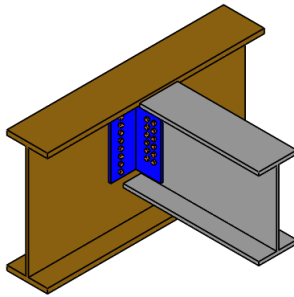
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb} = ((400 \times 0.6126 \times 8 \times 8) / (\sqrt{3} \times 1.25 \times 1000)) = 7.239$ [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{dpb} = (2.5 \times 0.356 \times 8 \times 8.0 \times 400) / (1.25 \times 1000) = 18.227$ [cl. 10.3.4]	
Bearing capacity of beam web (kN)		$V_{dpb} = (2.5 \times 0.356 \times 8 \times 9.4 \times 410) / (1.25 \times 1000) = 21.952$ [cl. 10.3.4]	
Bearing capacity of cleat (kN)		$V_{dpb} = (2.5 \times 0.356 \times 8 \times 8 \times 410) / (1.25 \times 1000) = 18.683$ [cl. 10.3.4]	
Bearing capacity (kN)		Min (18.227, 21.952, 18.683) = 18.683	
Bolt capacity (kN)		Min (7.239, 18.683) = 7.239	
Critical bolt shear (kN)	$\leq 7.239$	7.069	Pass
No. of bolts		36	
No. of column(s) per angle	$\leq 2$	2	
No. of bolts			

per column per angle		9	
<b>Bolt pitch (mm)</b>	$\geq 2.5 \cdot 8 = 20$ , $\leq \text{Min}(32 \cdot 8.0, 300) = 256$ [cl. 10.2.2]	20	<b>Pass</b>
<b>Bolt gauge (mm)</b>	$\geq 2.5 \cdot 8 = 20$ , $\leq \text{Min}(32 \cdot 8.0, 300) = 256$ [cl. 10.2.2]	20	
<b>End distance (mm)</b>	$\geq 1.7 \cdot 11.0 = 18.7$ , $\leq 12 \cdot 8.0 = 96.0$ [cl. 10.2.4]	18	<b>Pass</b>
<b>Edge distance (mm)</b>	$\geq 1.7 \cdot 11.0 = 18.7$ , $\leq 12 \cdot 8.0 = 96.0$ [cl. 10.2.4]	18	<b>Pass</b>
<b>Block shear capacity (kN)</b>	$\geq 100.0$	$V_{db} = 184.304$ [cl. 6.4.1]	<b>Pass</b>
<b>Cleat height (mm)</b>	$\geq 0.6 \cdot 300.0 = 180.0$ , $\leq 300.0 - 13.1 - 14.0 - 17.4 - 15.0 - 5 = 235.5$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	196	<b>Pass</b>
<b>Cleat moment capacity (kNm)</b>	$(2 \cdot 7.239 \cdot 20^2) / (20 \cdot 1000) = 3.792$	$M_d = (1.2 \cdot 250 \cdot Z) / (1000 \cdot 1.1) = 92.198$ [cl. 8.2.1.2]	<b>Pass</b>

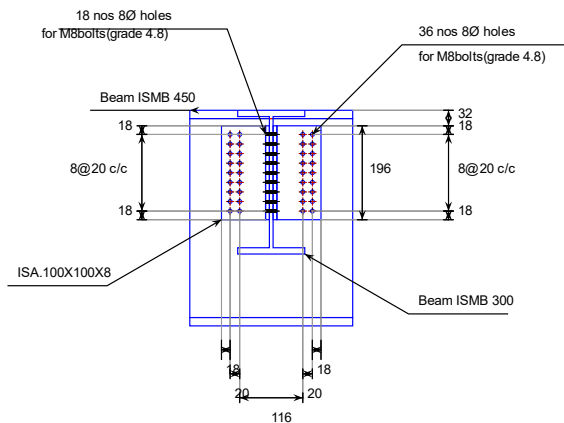


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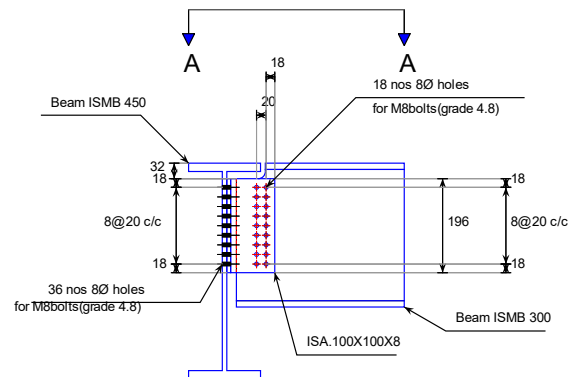
## Views



Top view (Sec A-A) (All distances are in "mm")



Side View (Sec B-B) (All distances are in "mm")



Front view (Sec C-C) (All distances are in "mm")



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<b>Additional Comments</b>	Ok. The connection is checked for various combinations of bolt diameter and cleat section dimensions and current combination is found satisfactory.
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