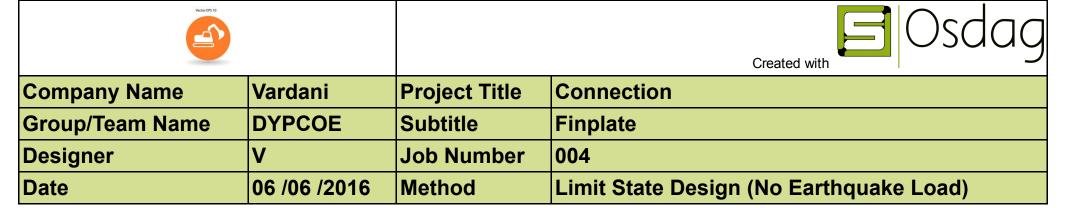


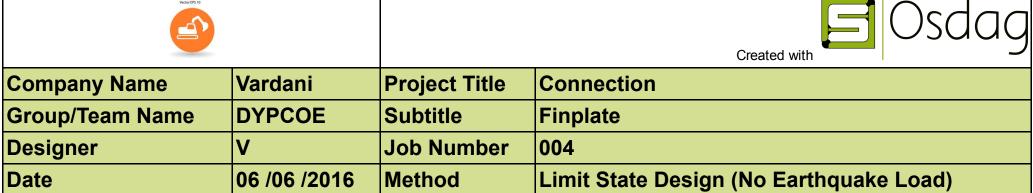
Pass
Single Finplate
Shear Connection
<u> </u>
Column flange-Beam web
Bolted
Welded
200
<u> </u>
ISSC 200
Fe 410
ISMB 400
Fe 410
STD
250X85X16
16
85
250
STD
Double Fillet
13
HSFG
8.8
12
7
1
7
0
31
30
30
·

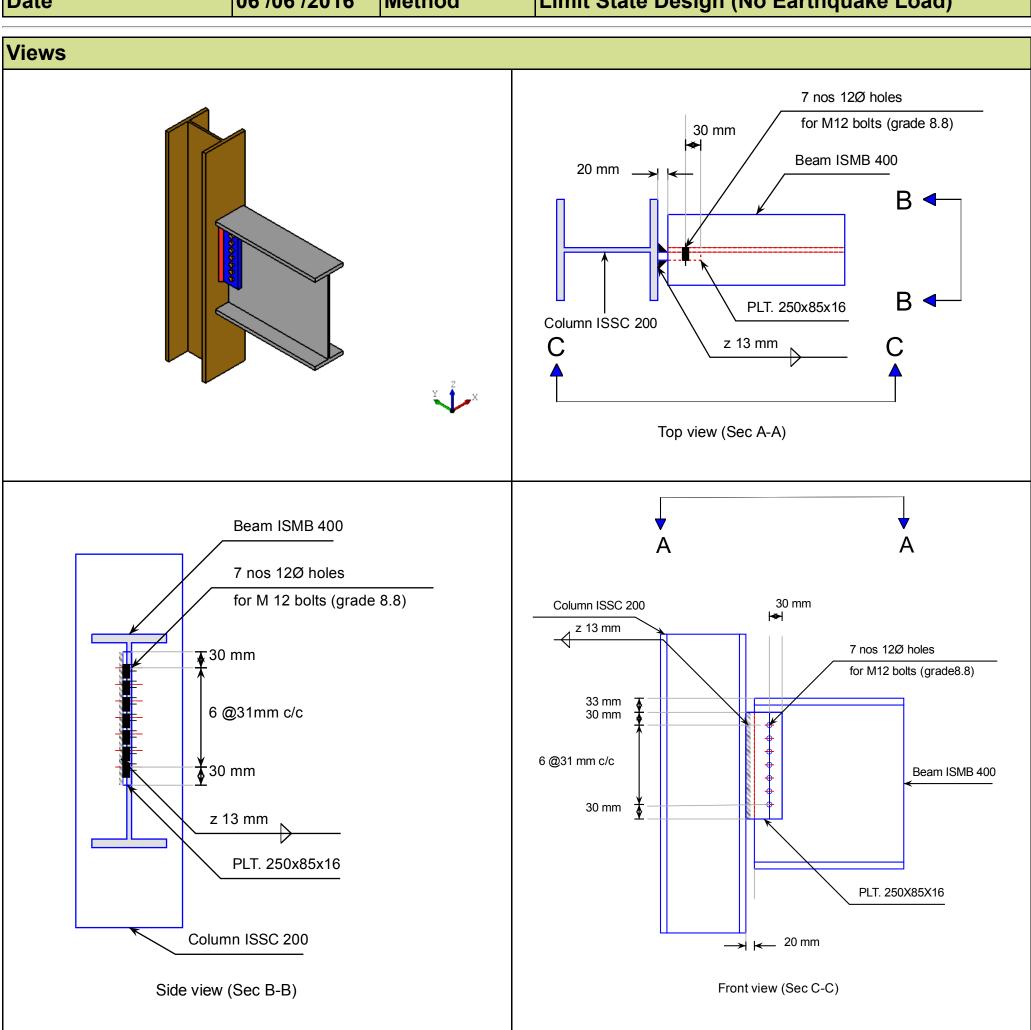
	,	



Design Check			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{\rm dsb}$ = (800*0.6126*12*12)/($\sqrt{3}$ *1.25*1000) = 31.223 [cl. 10.3.3]	
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.519*12*8.9*410)/(1.25*1000) = 45.452 [cl. 10.3.4]	
Bolt capacity (kN)		Min (31.223, 45.452) = 31.223	
No. of bolts	200/31.223 = 6.4	7	Pass
No.of column(s)	≤ 2	1	
No. of bolts per column		7	
Bolt pitch (mm)	≥ 2.5* 12 = 30, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	31	Pass
Bolt gauge (mm)	≥ 2.5*12 = 30, ≤ Min(32*8.9, 300) = 285 [cl. 10.2.2]	0	
End distance (mm)	≥ 1.7*13 = 22.1, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	30	Pass
Edge distance (mm)	≥ 1.7*13 = 22.1, ≤ 12*8.9 = 106.8 [cl. 10.2.4]	30	Pass
Block shear capacity (kN)	≥ 200	V _{db} = 467	Pass
Plate thickness (mm)	(5*200*1000)/(250*250) = 16.0 [Owens and Cheal, 1989]	16	Pass
Plate height (mm)	≥ 0.6*400=240.0, ≤ 400-16-14- 10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002]	250	Pass
Plate width (mm)		100	
Plate moment capacity (kNm)	(2*31.223*31 ²)/(31*1000) = 11.99	$M_{\rm d}$ = (1.2*250* Z)/(1000*1.1) = 45.45 [cl. 8.2.1.2]	Pass
Effective weld length (mm)		250-2*16 = 218	
Weld strength (kN/mm)	$\sqrt{[(11990*6)/(2*218^2)]^2} + [200/(2*218)]^2 = 0.885$	$f_V = (0.7*13*410)/(\sqrt{3}*1.25)$ = 2.121 [cl. 10.5.7]	Pass
Weld thickness (mm)	Max((0.885*1000*√3* 1.25)/(0.7 * 410),16* 0.8) = 12.8 [cl. 10.5.7, Insdag Detailing Manual,	13	Pass

2002]	





Victor PS 10			Created with OSdag
Company Name	Vardani	Project Title	Connection
Group/Team Name	DYPCOE	Subtitle	Finplate
Designer	V	Job Number	004
Date	06 /06 /2016	Method	Limit State Design (No Earthquake Load)

Additional Comments	