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Company Name	INSDAG	Project Title	PROB 2
Group/Team Name	M M GHOSH	Subtitle	
Designer	M M GHOSH	Job Number	
Date	04 /06 /2016	Method	Limit State Design (No Earthquake Load)

Design Conclusion	
Endplate	Pass
Endplate	
Connection Properties	
Connection	
Connection Title	Flexible Endplate
Connection Type	Shear Connection
Connection Category	•
Connectivity	Column flange-Beam web
Beam Connection	Welded
Column Connection	Bolted
Loading (Factored Load)	
Shear Force (kN)	160
Components	·
Column Section	ISSC 250
Material	Fe 410
Beam Section	ISMB 400
Material	Fe 410
Hole	STD
Plate Section	240X174X10
Thickness (mm)	10
Width (mm)	174
Depth (mm)	240
Hole	STD
Weld	
Туре	Double Fillet
Size (mm)	8
Bolts	
Туре	HSFG
Grade	8.8
Diameter (mm)	20
Bolt Numbers	6
Columns (Vertical Lines)	2
Bolts Per Column	3

Gauge (mm)	0	
Pitch (mm)	50	
End Distance (mm)	70	
Edge Distance (mm)	37	
Assembly		
Column-Beam Clearance (mm)	10	

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Bolt shear capacity (kN)	
Bolt bearing capacity (kN) (2.5*0.508*20*10.0*410)/(1.25 = 83.312 [cl. 10.3.4] Bolt capacity (kN) Min (52.694, 83.312) = 52.694 Critical bolt shear (kN) ≤ 52.694 No. of bolts 6 No.of column(s) ≤ 2	
Critical bolt shear (kN) ≤ 52.694 48.074 No. of bolts 6 No.of column(s) ≤ 2 2	5*1000)
(kN) ≤ 52.694 48.074 No. of bolts 6 No.of column(s) ≤ 2 2	Pass
No.of column(s) ≤ 2 2	Pass
No. of bolts per	
column per side of end plate	
Bolt pitch (mm) $\geq 2.5*20 = 50, \leq$ Min(32*8.9, 300) = 285 [cl. 10.2.2] 50	Pass
Bolt gauge (mm) $\geq 2.5*20 = 50, \leq$ Min(32*8.9, 300) = 285 [cl. 10.2.2]	
End distance (mm) ≥ 1.7*22.0 = 37.4, ≤ 12*8.9	Pass
Edge distance (mm) ≥ 1.7*22.0 = 37.4, ≤ 12*8.9 37 [cl. 10.2.4]	Pass
Block shear capacity (kN) ≥ 160 $V_{db} = 203$ [cl. 6.4.1]	
Plate thickness (mm) ≥ 8	Pass
Plate height (mm) ≥ 0.6*400.0=240.0, ≤ 400.0-16.0-14.0-16.0-14.0-10=330.0 [cl. 10.2.4, Insdag Detailing Manual, 2002] 240	Pass

Plate Width (mm)	≥ 174, ≤ 250.0	174	Pass
Effective weld length (mm)		240-2*8 = 224	
Weld strength (kN/mm)	0.357	$f_{\rm V}$ =(0.7*8*410)/($\sqrt{3}$ *1.25*1000) = 1.06 [cl. 10.5.7]	Pass

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Views	

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Additional Comments	