



Created with

<b>Company Name</b>	<b>PVPIT</b>	<b>Project Title</b>	<b>Project 03</b>
<b>Group/Team Name</b>	<b>OSDAG</b>	<b>Subtitle</b>	
<b>Designer</b>	<b>USER 03</b>	<b>Job Number</b>	<b>123</b>
<b>Date</b>	<b>05 /06 /2016</b>	<b>Method</b>	<b>Limit State Design (No Earthquake Load)</b>

### Design Conclusion

<b>Cleat Angle</b>	<b>Pass</b>
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### Cleat Angle

### Connection Properties

### Connection

Connection Title	Double Angle Web Cleat
Connection Type	Shear Connection

### Connection Category

Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Bolted

### Loading (Factored Load)

Shear Force (kN)	100.0
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### Components

<b>Column Section</b>	ISMB 450
Material	Fe 410
<b>Beam Section</b>	ISMB 300
Material	Fe 410
Hole	STD
<b>Cleat Section</b>	ISA 70X70X8
Thickness (mm)	8
Cleat Leg Size B (mm)	70
Cleat Leg Size A (mm)	70
Hole	STD

### Bolts on Beam

Type	Black Bolt
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Grade	4.8
Diameter (mm)	12
Bolt Numbers	6
Columns (Vertical Lines)	1
Bolts Per Column	6
Gauge (mm)	0
Pitch (mm)	30
End Distance (mm)	22
Edge Distance (mm)	22
<b>Bolts on Column</b>	
Type	Black Bolt
Grade	4.8
Diameter (mm)	12
Bolt Numbers	12
Columns (Vertical Lines)	1
Bolts Per Column	6
Gauge (mm)	0
Pitch (mm)	30
End Distance (mm)	22
Edge Distance (mm)	22
<b>Assembly</b>	
<b>Column-Beam Clearance (mm)</b>	20



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### Design Check: Secondary Beam Connectivity

Check	Required	Provided	Remark
<b>Bolt shear capacity (kN)</b>		$V_{dsb} = ((2*400*0.6126*12*12)/(\sqrt{3}*1.25*1000)) = 31.223$ [cl. 10.3.3]	
<b>Bolt bearing capacity (kN)</b>		$V_{dpb} = (2.5*0.519*12*7.7*400)/(1.25*1000) = 38.364$ [cl. 10.3.4]	
<b>Bearing capacity of beam web (kN)</b>		$V_{dpb} = (2.5*0.519*12*7.7*410)/(1.25*1000) = 39.324$ [cl. 10.3.4]	
<b>Bearing capacity of cleat (kN)</b>		$V_{dpb} = (2.5*0.519*12*8*410)/(1.25*1000) = 40.856$ [cl. 10.3.4]	
<b>Bearing capacity (kN)</b>		Min (38.364, 39.324, 40.856) = 38.364	
<b>Bolt capacity (kN)</b>		Min (31.223, 38.364) = 31.223	
<b>Critical bolt shear (kN)</b>	$\leq 31.223$	14.144	<b>Pass</b>
<b>No. of bolts</b>		6	
<b>No. of column(s)</b>	$\leq 2$	1	

<b>No. of bolts per column</b>		6	
<b>Bolt pitch (mm)</b>	$\geq 2.5 \times 12 = 30, \leq \text{Min}(32 \times 7.7, 300) = 247$ [cl. 10.2.2]	30	Pass
<b>Bolt gauge (mm)</b>	$\geq ; 2.5 \times 12 = 30, \leq \text{Min}(32 \times 7.7, 300) = 247$ [cl. 10.2.2]	0	
<b>End distance (mm)</b>	$\geq 1.7 \times 13.0 = 22.1, \leq 12 \times 7.7 = 92.4$ [cl. 10.2.4]	22	Pass
<b>Edge distance (mm)</b>	$\geq 1.7 \times 13.0 = 22.1, \leq 12 \times 7.7 = 92.4$ [cl. 10.2.4]	22	Pass
<b>Block shear capacity (kN)</b>	$\geq 100.0$	$V_{db} = 177.029$ [cl. 6.4.1]	Pass
<b>Cleat height (mm)</b>	$\geq 0.6 \times 300.0 = 180.0, \leq 300.0 - 13.1 - 14.0 - 17.4 - 15.0 - 5 = 235.5$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	194	Pass
<b>Cleat moment capacity (kNm)</b>	$(2 \times 31.223 \times 30^2) / (30 \times 1000) = 2.4$	$M_d = (1.2 \times 250 \times Z) / (1000 \times 1.1) = 90.326$ [cl. 8.2.1.2]	Pass



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### Design Check: Primary Beam Connectivity

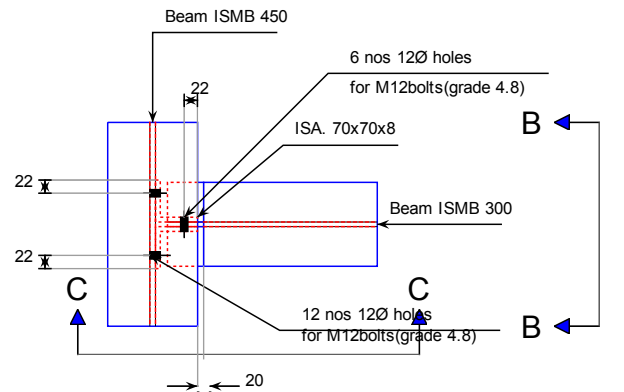
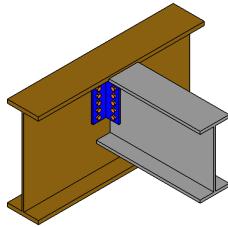
Check	Required	Provided	Remark
<b>Bolt shear capacity (kN)</b>		$V_{dsb} = ((400*0.6126*12*12)/(\sqrt{3}*1.25*1000))$ $= 15.611$ [cl. 10.3.3]	
<b>Bolt bearing capacity (kN)</b>		$V_{dpb} = (2.5*0.519*12*8.0*400)/(1.25*1000)$ $= 39.859$ [cl. 10.3.4]	
<b>Bearing capacity of beam web (kN)</b>		$V_{dpb} = (2.5*0.519*12*9.4*410)/(1.25*1000)$ $= 48.005$ [cl. 10.3.4]	
<b>Bearing capacity of cleat (kN)</b>		$V_{dpb} = (2.5*0.519*12*8*410)/(1.25*1000) = 40.856$ [cl. 10.3.4]	
<b>Bearing capacity (kN)</b>		Min (39.859, 48.005, 40.856) = 40.856	
<b>Bolt capacity (kN)</b>		Min (15.611, 40.856) = 15.611	
<b>Critical bolt shear (kN)</b>	$\leq 15.611$	14.895	<b>Pass</b>
<b>No. of bolts</b>		12	
<b>No. of column(s) per angle</b>	$\leq 2$	1	
<b>No. of bolts</b>			

<b>per column per angle</b>		6	
<b>Bolt pitch (mm)</b>	$\geq 2.5 \cdot 12 = 30, \leq \text{Min}(32 \cdot 8.0, 300) = 256$ [cl. 10.2.2]	30	<b>Pass</b>
<b>Bolt gauge (mm)</b>	$\geq 2.5 \cdot 12 = 30, \leq \text{Min}(32 \cdot 8.0, 300) = 256$ [cl. 10.2.2]	0	
<b>End distance (mm)</b>	$\geq 1.7 \cdot 13.0 = 22.1, \leq 12 \cdot 8.0 = 96.0$ [cl. 10.2.4]	22	<b>Pass</b>
<b>Edge distance (mm)</b>	$\geq 1.7 \cdot 13.0 = 22.1, \leq 12 \cdot 8.0 = 96.0$ [cl. 10.2.4]	22	<b>Pass</b>
<b>Block shear capacity (kN)</b>	$\geq 100.0$	$V_{db} = 177.029$ [cl. 6.4.1]	<b>Pass</b>
<b>Cleat height (mm)</b>	$\geq 0.6 \cdot 300.0 = 180.0, \leq 300.0 - 13.1 - 14.0 - 17.4 - 15.0 - 5 = 235.5$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	194	<b>Pass</b>
<b>Cleat moment capacity (kNm)</b>	$(2 \cdot 15.611 \cdot 30^2) / (30 \cdot 1000) = 2.592$	$M_d = (1.2 \cdot 250 \cdot Z) / (1000 \cdot 1.1) = 90.326$ [cl. 8.2.1.2]	<b>Pass</b>

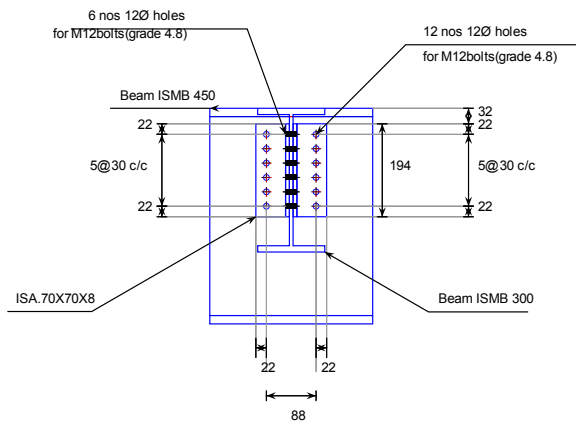


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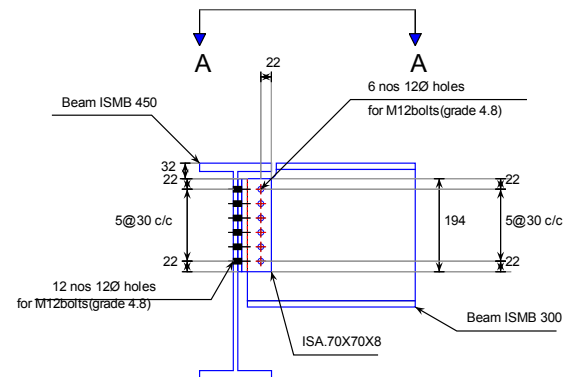
## Views



Top view (Sec A-A) (All distances are in "mm")



Side View (Sec B-B) (All distances are in "mm")



Front view (Sec C-C) (All distances are in "mm")



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<b>Additional Comments</b>	
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