

Fundamental Theorems of Limit Analysis (of frames - Calcul de rupture)

• Static quantities:

Bending moments $M(x)$
that are admissible
(statically and plastically)

λ , to which $M(x)$ is in equilibrium.
sets the family of possible λ
(collapse load multiplier λ_c is a λ^-)

• Kinematic quantities:

Kinematic mechanism displacements $u(x)$
and rotation $\varphi(x)$ that are admissible
(kinematically and plastically)

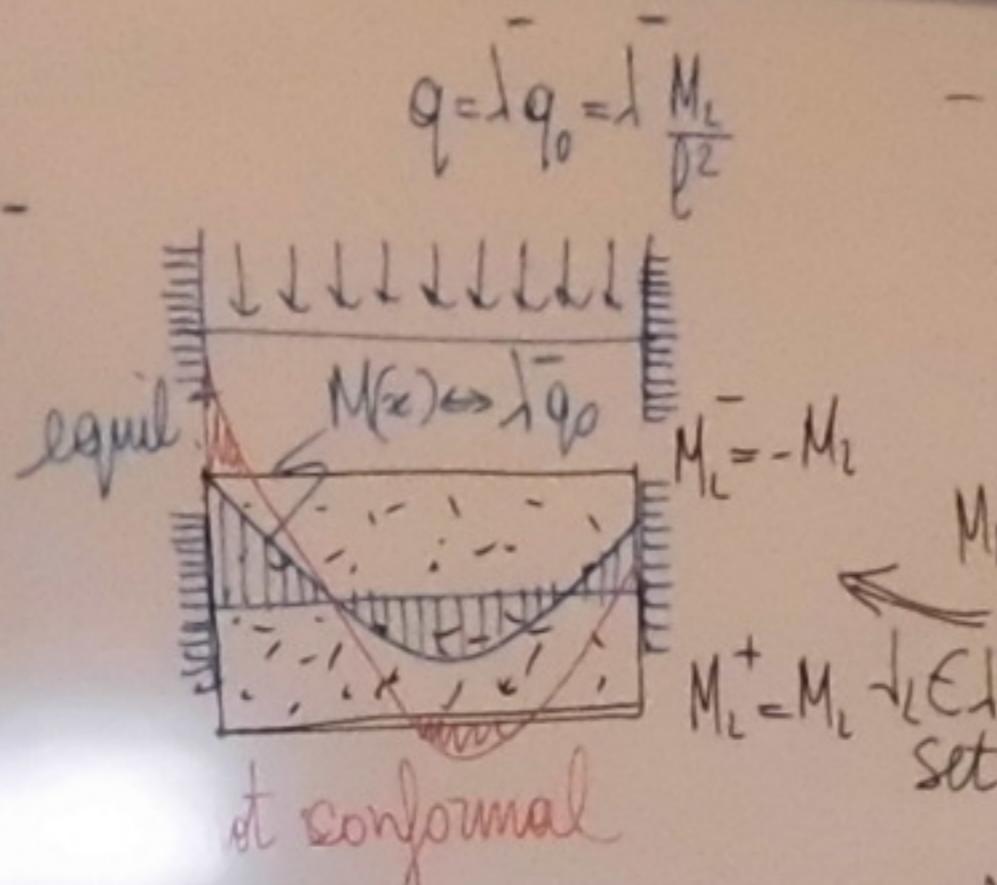
λ^+ , to which a potential collapse mechanism is
associated sets the family of possible λ^+
(collapse load multiplier λ_c is a λ^+)

- statically admissible, namely equilibrated,
with "live" loads affected by load multiplier λ

$$M(x) \leftarrow \lambda P_{ax}, \lambda q_0$$

- plastically admissible, namely conformal:

$$M^- \leq M(x) \leq M^+$$



- As said, the true collapse internal actions
and collapse mechanism are both statically
and kinematically admissible.

$$\begin{aligned} M_L &= M_L \\ M_L^+ &= M_L \\ \lambda &= \lambda_c \end{aligned}$$

associated to such a mechanism, can be evaluated as

$$\lambda^+ = \frac{D = \lambda L_p}{\lambda L_p} = \frac{\sum_i M_L^+ \varphi_i + M_L^- \varphi_i}{\sum_i P_{ax} y_i + \int q(x) y(x) dx}$$

Here, the associated λ is
calculated by energy balance as if this would be
the collapse mode

$$\Delta E = \sum_i \lambda P_{ax} y_i + \int \lambda q(x) y(x) dx = \sum_i M_L^+ \varphi_i + M_L^- \varphi_i = D = \lambda L_p > 0$$

State theorem
Kinematic mechanism
Statically admissible

Kinematic theorem
Kinematically admissible

Mixed theorem

Any statically admissible distribution being
to the same $\lambda = \lambda^+$ of a kinematically admissible
mechanism yields the true equilibrium. $D = \lambda L_p$

Fundamental Theorems of Limit Analysis (or Frame - Global response)

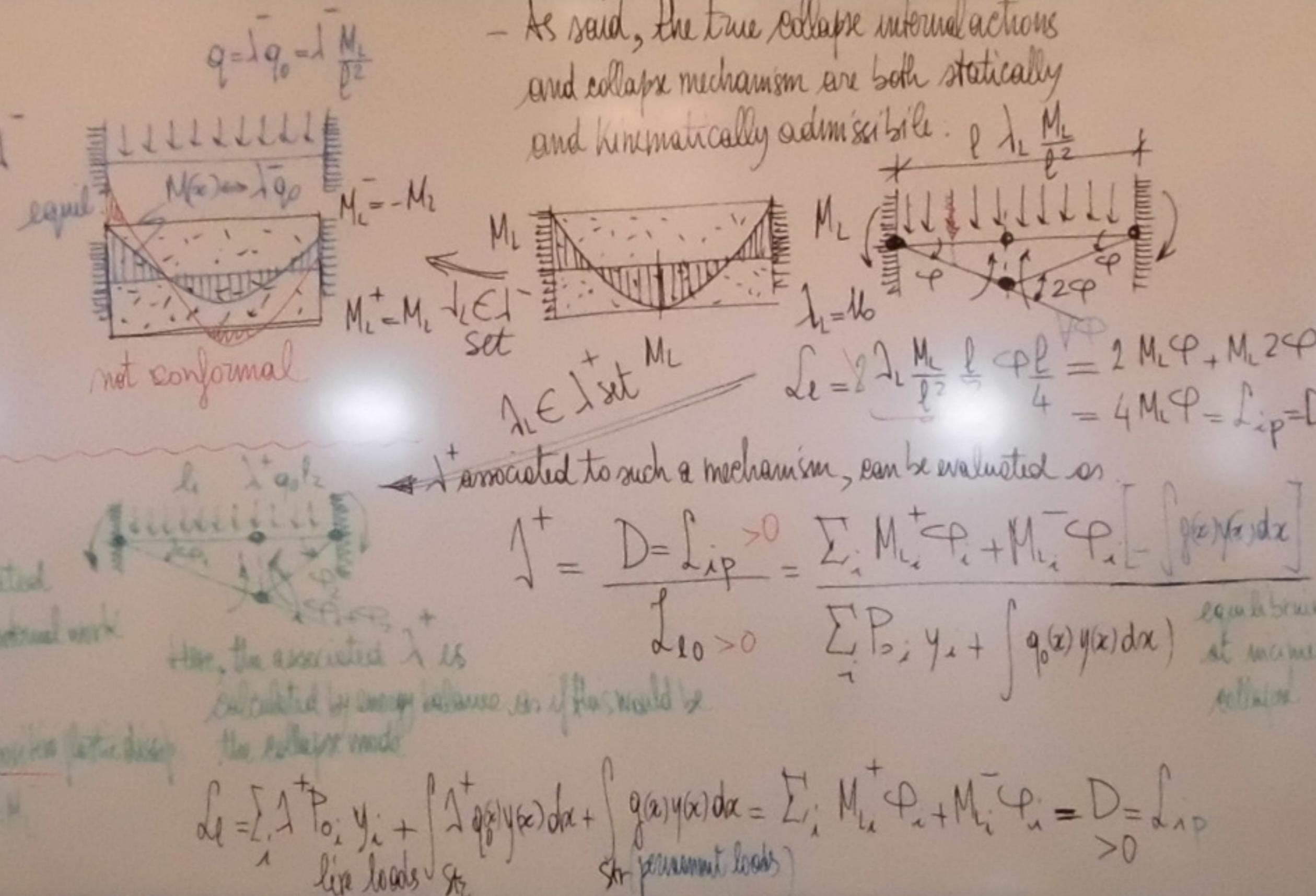
- Static equilibrium
bounding moment M_{eq}
that are admissible
statically and kinematically

λ to which M_{eq} is equal.

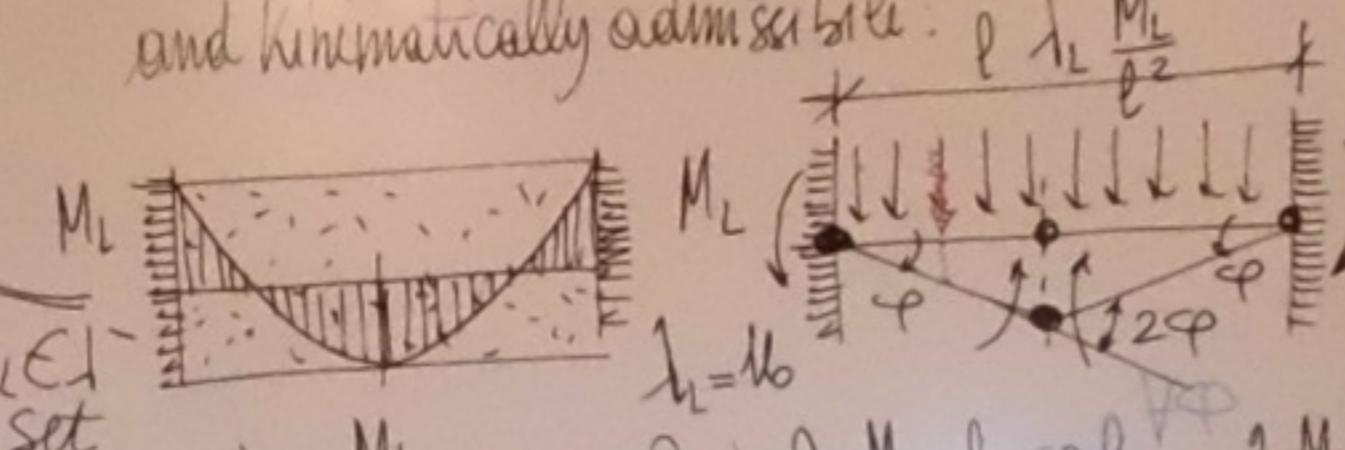
sets the family of possible λ
(collapse load multiplier $\lambda_{\text{col}} = \lambda^+$)

- statically admissible, namely equilibrated,
with live loads affected by load multiplier λ
 $M(\lambda) = \lambda P_{\text{ax}} + \lambda g$

- plastically admissible, namely conformal
 $M_{\text{pl}}^- \leq M(\lambda) \leq M_{\text{pl}}^+$



- As said, the true collapse internal actions and collapse mechanism are both statically and kinematically admissible.



$$\lambda^+ = \frac{D = \int_{x=0}^{x=l} P_{ip} dx}{\int_{x=0}^{x=l} M_{\text{pl}}^+ - M_{\text{pl}}^- dx} = \frac{\sum_i M_{\text{pl}}^+ \varphi_i + M_{\text{pl}}^- \varphi_i - \int g(x) y(x) dx}{\sum_i P_{ip} y_i + \int q(x) y(x) dx}$$

equilibrium at moment collapse

$$D = \sum_i \lambda^+ P_{ip} y_i + \int \lambda^+ g(x) y(x) dx + \int q(x) y(x) dx = \sum_i M_{\text{pl}}^+ \varphi_i + M_{\text{pl}}^- \varphi_i = D = \int_{x=0}^{x=l} P_{ip} dx > 0$$

live loads set for permanent loads

- Static theorem (lower-bound theorem)

The collapse load multiplier is the max of all λ :

$$S = \lambda_c = \max \{ \lambda^+ \}$$

"Safe Distance"

$\lambda_c \leq \lambda_c - S$
 λ_c lower bound
(conservative estimate of λ)

- Kinematic theorem (upper-bound theorem)

use load multiplier in the min. of all λ :

$$S = \lambda_c = \min \{ \lambda^+ \}$$

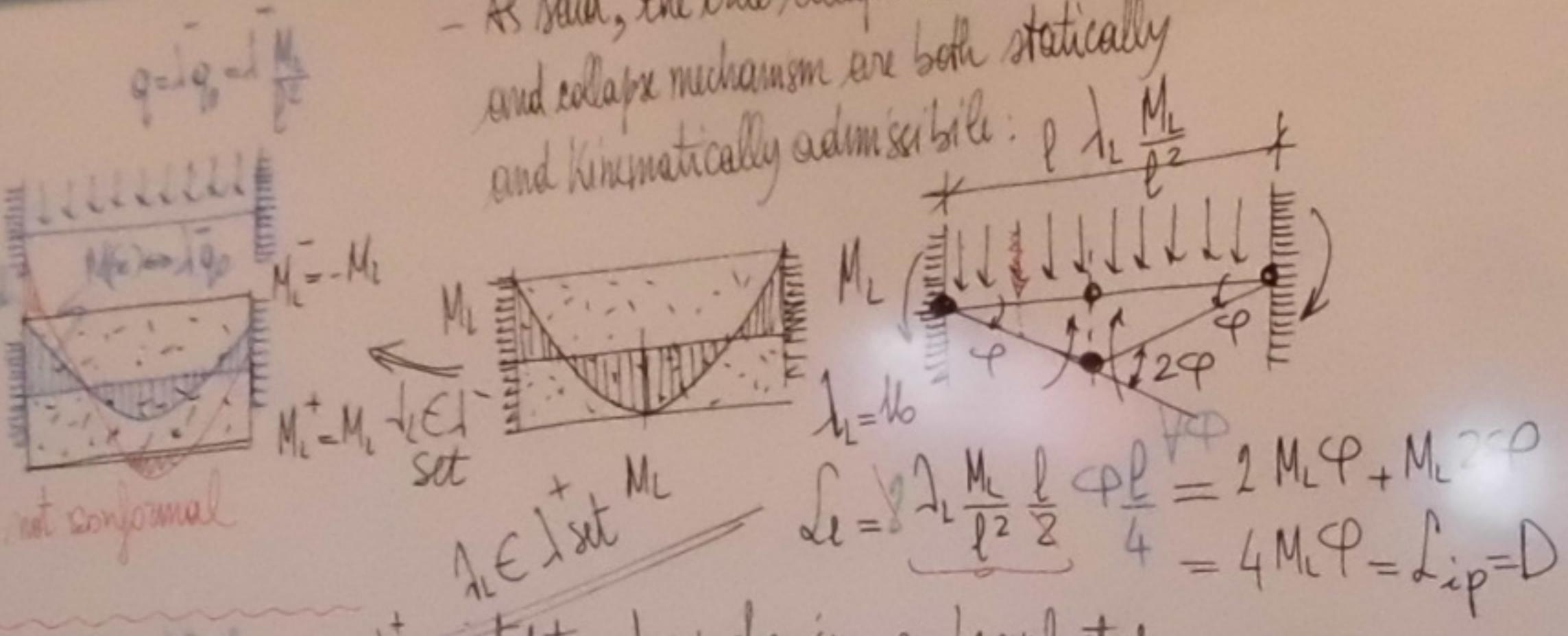
$S = \lambda_c \leq \lambda^+$
upper-bound
(non-conservative)
estimate of λ

- Mixed theorem

Any statically admissible distribution leading to the same $\lambda = \lambda^+$ of a kinematically admissible mechanism provides the true collapse multiplier: $\lambda = \lambda^+ = \lambda_c = S$

In fact, if λ is one of the λ 's and one of the λ^+ 's, then in general the best result is found of values around λ (intermediate), when the deflection minimizes the value of the collapse multiplier.

- As said, the true collapse interactions and collapse mechanism are both statically and kinematically admissible: $\lambda \geq \frac{M_L}{P_L}$



Associated to such a mechanism, can be evaluated as:

$$\lambda^+ = \frac{D = \lambda_{lip}}{\lambda_{lo} > 0} = \frac{\sum_i M_{li}^+ \varphi_i + M_{li}^- \varphi_i - \int g(x) y(x) dx}{\sum_i P_i y_i + \int q_0(x) y(x) dx}$$

equilibrium
at moment
collapse

$$D = \sum_i \lambda^+ P_i y_i + \int \lambda^+ g(x) y(x) dx + \int q(x) y(x) dx = \sum_i M_{li}^+ \varphi_i + M_{li}^- \varphi_i = D = \lambda_{lip} > 0$$

live loads $\int g(x) y(x) dx$
dead loads $\int q(x) y(x) dx$

- Static theorem (lower-bound theorem)

The collapse load multiplier is the max of all λ^- :

$$s = \lambda_L^- = \max \{ \lambda^- \}$$

"Safe theorem"

$$\lambda^- \leq \lambda_L - s$$

lower-bound
(conservative)
estimate of λ_L

- Kinematic theorem (upper-bound theorem)

The collapse load multiplier is the min. of all λ^+ :

$$s = \lambda_L^+ = \min \{ \lambda^+ \}$$

$$s = \lambda_L^+ \leq \lambda^+$$

upper-bound
(non-conservative)
estimate of λ_L

- Mixed theorem

Any statically admissible distribution leading to the same $\lambda^- = \lambda^+$ of a kinematically admissible mechanism provides the true collapse multiplier: $\lambda^- = \lambda^+ = \lambda_L = s$

In fact, if λ_L is one of the λ^- and one of the λ^+ , though in general they shall make a field of values around λ_L (when distinct), when they do coincide they provide the evaluation of the collapse multiplier

$$0^+ \quad \lambda^- \quad \lambda_L = s \quad \lambda^+ \quad \lambda_L$$

Principles

S. Th.

Kinematic quantities at collapse

① static: collapse

$$\sum_i \lambda_i^+ P_{oi} y_i = \sum_i M_{li}^+ \varphi_i + M_{ri}^- \varphi_i \quad L_{ip} = D$$

This is the equation that one would use to get λ_c if collapse mode would be known

② static: stability

$$\sum_i \lambda_i^+ P_{oi} y_i = \sum_i M_{li}^+ \varphi_i + M_{ri}^- \varphi_i$$

$$\{\lambda \leq \lambda_L\} \Leftrightarrow \lambda_L - \lambda \geq 0$$

$$\Leftrightarrow (\lambda_L - \lambda) \sum_i P_{oi} y_i = \sum_i (\underbrace{M_{li}^+ - M_i}_> \varphi_i + \underbrace{(M_r^- - M_i)}_> \varphi_i) \geq 0$$

and since λ_c is a λ , λ_c is the max of all λ

K. Th.

Kinematic quantities at kinematically admissible collapse

① static: Kinematically admissible

$$\sum_i \lambda_i^+ P_{oi} y_i = \sum_i M_{li}^+ \varphi_i + M_{ri}^- \varphi_i \quad \text{This is the equation that one would use to get } \lambda^+ \text{ for the given admissible mechanism}$$

② static collapse mode

$$\sum_i \lambda_L^+ P_{oi} y_i = \sum_i M_{li}^+ \varphi_i + M_{ri}^- \varphi_i$$

$$\Leftrightarrow \lambda^+ - \lambda_L \geq 0 \Leftrightarrow (\lambda^+ - \lambda_L) \sum_i P_{oi} y_i = \sum_i (\underbrace{M_{li}^+ - M_i}_> \varphi_i + \underbrace{(M_r^- - M_i)}_> \varphi_i) \geq 0$$

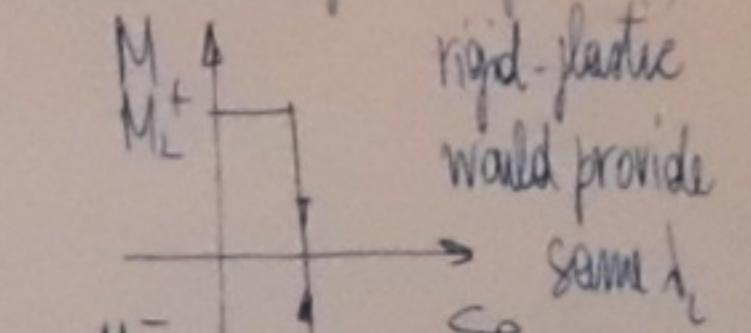
and since λ_c is a λ^+ , λ_c is the min of all λ^+

PVW

D

Corollaries:

- Elastic structural properties are relevant (for the determination of the collapse load)



(though of course this matter to the real executive elasto-plastic response)

- Self-equilibrated imposed actions (like temperature variations, settlements) occurring prior to the growing live loads afforded by load multipliers do not matter in assessing λ_c

The application of the theorem allows for "simple methods" of calculation of the collapse resistance for frame structures. These rules are generalised to other frame and roof components (e.g. by the superposition method).

Example

$$O = \frac{1}{2} M_L$$

$$T(x) = \frac{1}{2} - \frac{1}{2}x \rightarrow z = \frac{1}{2} - \frac{1}{2}z$$

$$M_L = \frac{1}{2} M_L$$

$$M_r^+ = M_r^- = M_r$$

$$M_r^- = M_r^+ = M_r$$

$$M_r = M_r$$

$\sum \Delta \varphi = \sum \varphi_+ - \sum \varphi_-$
 $\sum \Delta \varphi = \sum \varphi_+ + \sum \varphi_-$

This is the equation that one would
 use to get λ^+ if collapse mode
 would be known

$\sum \Delta P_i \varphi_i = \sum M_i \varphi_i + M_L \varphi_i$
 $\sum \Delta P_i \varphi_i = \sum M_i \varphi_i + M_L \varphi_i$

This is the equation that one would
 use to get λ^+ for the given
 admissible mechanism

$\sum \Delta P_i \varphi_i = \sum M_i \varphi_i + M_L \varphi_i$

$\sum \Delta P_i \varphi_i = \sum M_i \varphi_i + (M_L - M_0) \varphi_i$

This is the equation that one would
 use to get λ^- for the given
 admissible mechanism

Corollaries:

- Elastic structural properties are irrelevant (for the determination of the collapse load)
- Example: $q = \frac{1}{l^2} M_L$

rigid-plastic wall provides same λ^+

Hinge of course has nothing in the real admissible mechanism

- Self-equilibrated impressed co-actions (like temperature variations, settlements) occurring prior to the growing live loads affected by load multiplier λ do not matter in assessing λ_L

- The application of the theorems allows for "manual methods" of calculation of λ_L and attached collapse mechanism for frame structures. These could be generalized by systematic programming methods and solved computationally (e.g. by the simplex method).

$M(x) = V_B x - \frac{q l^2}{2} = -\frac{M_L}{l} x + \frac{1}{2} \frac{M_L}{l^2} x(l-x)$
 $T(x) = V_B - \lambda q_0 x = 0 \Rightarrow x_m = \frac{V_B}{\lambda q_0} = \frac{\lambda - 2}{2\lambda} l$

At $\lambda = 12$, $M^{\max} = \frac{25}{24} M_L = c_m M_L$

Then, this λ^+ is not a λ^- , because conformity is violated.

I could scale the loads by $1/c_m$ so that $M = M_L$ at x_m

$\lambda = \frac{\lambda^+}{c_m} = 11.52$

$\bar{\lambda} = 11.52 \leq \lambda^- \leq 12 = \lambda^+$

Reinforce and set plastic hinge at $x_m \rightarrow$ new estimate $\lambda^+ = 11.6567$

New c_m and delimitation $11.656736 \leq \lambda_L \leq 11.65674162$