

Principal coordinates (modal dynamic analysis)

- $\ddot{\mathbf{M}}\ddot{\mathbf{q}} + \mathbf{K}\mathbf{q} = \mathbf{Q}(t)$ [in MDOF system]

\mathbf{q} : generalized coordinates (Lagrange's) representing the physical defls

- After solution of the eigenvalue problem

$$\mathbf{K}\Phi = \mathbf{M}\Phi\Omega \quad (2)$$

Eigenvector matrix $[\Phi]$ Eigenvalue matrix $[\omega^2]$

- Change of variables:

$$\mathbf{q} = \Phi \mathbf{P} = \sum_i \Phi_i p_i \quad \text{principal coordinates}$$

$$\mathbf{P} = \Phi^{-1} \mathbf{q} \quad \{ \Phi \text{ is invertible}$$

granted for linearly indep. eigenvectors, like orthonormal eigenvectors associated to distinct eigenvalues

(1)

To get explicit inverse relation

$$\Phi^T \mathbf{M} \mathbf{q} = \sum_i \Phi_i^T \mathbf{M} \Phi_i p_i = M_r p_r$$

$$p_r = \frac{\Phi_r^T \mathbf{M} \mathbf{q}}{\Phi_r^T \mathbf{M} \Phi_r} \quad (\text{depends on the number of } \Phi_r) \quad \mathbf{p}_r \leftarrow \Phi$$

or:

$$\Phi^T \mathbf{M} \mathbf{q} = \sum_i \mathbf{M}_i \Phi_i P \quad \mathbf{M}_i = \text{diag}[M_i = \Phi_i^T \mathbf{M} \Phi_i]$$

$$\mathbf{P} = \mathbf{M}^{-1} \Phi^T \mathbf{M} \mathbf{q} = \mathbf{K}^{-1} \Phi^T \mathbf{K} \mathbf{q} \quad \mathbf{K} = \text{diag}[K_i = \Phi_i^T \mathbf{K} \Phi_i]$$

- Decoupling of the eqns. of motion in principal coordinates

$$\ddot{\mathbf{M}}\ddot{\mathbf{P}} + \mathbf{K}\mathbf{P} = \Phi^T \mathbf{Q} = \mathbf{P}(t) \rightarrow n \text{ eqns } M_i \ddot{p}_i + K_i p_i = \frac{\Phi_i^T \mathbf{Q}}{M_i} = \frac{\Phi_i^T \mathbf{P}(t)}{M_i} \rightarrow \ddot{p}_i(t) - \omega_i^2 p_i(t) = 0 \quad (3)$$

$M_i \ddot{u} + K_i u = F_i(t)$ SDOF

Response to pure initial conditions (free vibrations)

$$\text{i.e. } \begin{cases} q(t=0) = q_0 \\ \dot{q}(t=0) = \dot{q}_0 \end{cases} \quad Q=0$$

$q = \sum_i \Phi_i (A_{\text{sinus}} + B_{\text{cosine}}) \quad A, B \text{ amplitude coefficients}$
Linear combination of harmonic motions with different frequencies ω_i to t)

Set the i.e. $[B_i]$

$$\begin{cases} q(0) = \sum_i \Phi_i B_i = \Phi B = q_0 \Rightarrow B = \Phi q_0 = b_i \\ \dot{q}(0) = \sum_i \Phi_i \omega_i A_i = \Phi \Omega A = \dot{q}_0 \Rightarrow A = [\Phi \Omega]^{-1} \dot{q}_0 \end{cases}$$

$$\begin{cases} \text{Notice that if } q_0 \approx \Phi_r \rightarrow B_r = 1 \\ \dot{q}_0 = 0 \rightarrow A_r = 0 \end{cases} \quad \ddot{q}_i = \Omega_i^2 \Phi_i q_i$$

$$\ddot{q}_i = \Omega_i^2 \Phi_i q_i \quad \text{solution resulting to mode } i$$

Procedure of modal analysis

- Set the eqns. of motion (\mathbf{M} and \mathbf{K})

- Solve the eigenvalue problem

- Get the natural frequencies and the associated eigenvectors

- Set the initial conditions and solve the decoupled equations in principle

- Find the pure motion in terms of natural frequencies

- Get final response in terms of physical variables

- $q(t) = \Phi \ddot{q}(t)$

Procedure of modal analysis :

(1) - Set the eqns of motion
(write M and K)

(2) Solve the eigenvalue problem
(numerical iterative methods)

(3) Shift to principal coordinates and solve the
(decoupled) eqns of motion in principal coord.
[direct step-by-step integration, e.g. Newmark
or numerical evaluation of Duhamel's integral]

(4) Get final response in terms of physical coordinates
 $q(t) = \Phi p(t)$

Response to pure initial conditions (free vibrations)

$$\text{i.e. } \begin{cases} q(t=0) = q_0 \\ \dot{q}(t=0) = \dot{q}_0 \end{cases}$$

$\Rightarrow \sum_i \Phi(A_{i,i} \sin \omega_i t + B_{i,i} \cos \omega_i t)$ A_i, B_i amplitude
linear combination of harmonic motions
with different frequencies ω_i
coefficients (linked to Φ_i)

Set the i.e. $[B_i]$

$$\begin{cases} q(0) = \sum_i \Phi_i B_i = \Phi B = q_0 \Rightarrow B = \Phi q_0 = \Phi p_0 \\ \dot{q}(0) = \sum_i \Phi_i \omega_i A_i = \Phi \Omega A = \dot{q}_0 \Rightarrow A = (\Phi \Omega)^{-1} \dot{q}_0 \end{cases}$$

Notice that if $q_0 \approx \Phi_r \rightarrow B_r = 1$

$$[A] \quad B_{i,r} = 0 \quad (q_0 = 0)$$

$$= \Phi^{-1} \dot{\Phi} q_0$$

Vibration according to mode r only

Principal coordinates

$\ddot{M}q + \ddot{K}q = \ddot{Q}$ [a MDF system]
generalized coordinates (displacements)
representing the physical defls

or $\ddot{\Phi}^T M q = \ddot{\Phi}^T K \ddot{Q}$

$M = diag[M_{ii} = \Phi_i^T K \Phi_i]$

General solution

$q = \Phi \ddot{q} = \sum_i \Phi_i \ddot{q}_i$

Decoupling of the eqns of motion in principal coordinates

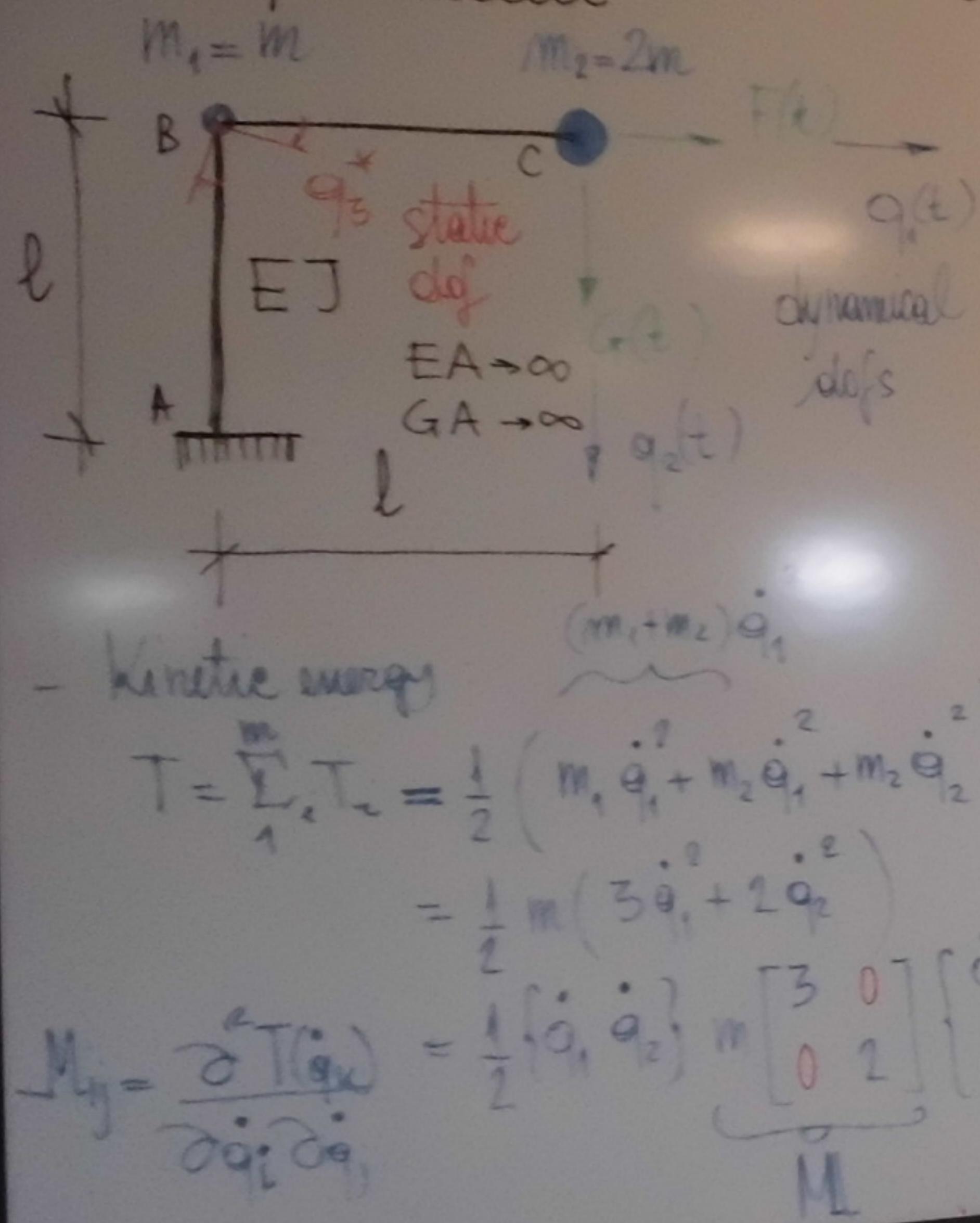
$\ddot{M}p + \ddot{K}p = \ddot{\Phi}^T Q = P(t) \rightarrow n$ eqns

related to n degrees of freedom, n independent equations associated to distinct frequencies

$\ddot{M}_r p_r + \ddot{K}_r p_r = \Phi_r^T Q = P_r(t) \rightarrow p_r(t) \sim \alpha_r(t)$

$\ddot{M}_r p_r + \ddot{K}_r p_r = F_r(t)$ SDOF

Example (2-DOF)



Partition of d.o.f. (and attachment of E and K)

$$q = \begin{bmatrix} q_1 \\ q_2 \\ q_3 \end{bmatrix} = \begin{bmatrix} q_d \\ \dots \\ q_s \end{bmatrix}$$

$$E = \begin{bmatrix} E_d \\ E_s \end{bmatrix} = \begin{bmatrix} K_{dd} & K_{ds} \\ K_{sd} & K_{ss} \end{bmatrix} \begin{bmatrix} q_d \\ q_s \end{bmatrix}$$

$$K = \frac{E}{l^3} \begin{bmatrix} 12 & 0 & -6l \\ 0 & 3 & -3l \\ -6l & -3l & 7 \end{bmatrix}$$

$$K = M^{-1}$$

Explicitly:

$$\begin{cases} E_d = K_{dd} q_d + K_{ds} q_s \\ E_s = K_{sd} q_d + K_{ss} q_s \end{cases}$$

static combination of q_3

$$\begin{aligned} \dot{E}_d &= K_{dd} \dot{q}_d + K_{ds} \dot{q}_s \\ \dot{E}_s &= K_{sd} \dot{q}_d + K_{ss} \dot{q}_s \end{aligned}$$

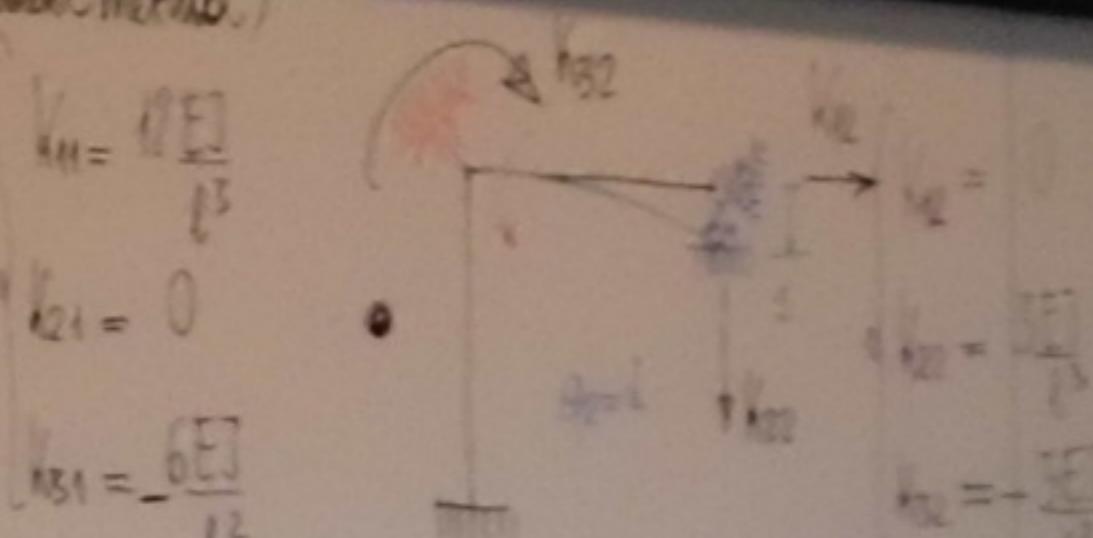
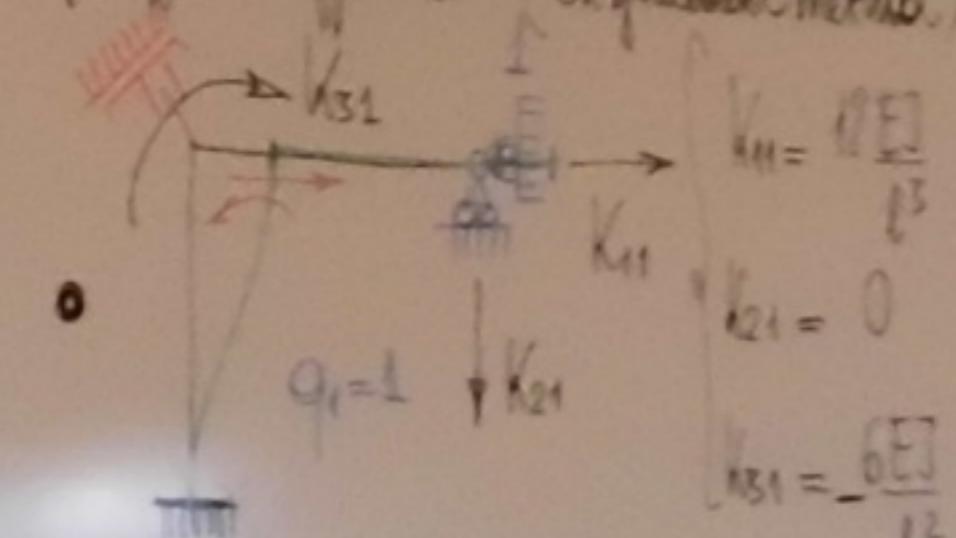
$\dot{q}_3 = -K_{ss}^{-1} K_{sd} q_d + K_{ss}^{-1} E_s$

$$\begin{aligned} \ddot{E}_d &= K_{dd} \ddot{q}_d + K_{ds} (-K_{ss}^{-1} K_{sd} \dot{q}_d + K_{ss}^{-1} E_s) \\ &= (K_{dd} - K_{ds} K_{ss}^{-1} K_{sd}) \ddot{q}_d + K_{ds} K_{ss}^{-1} E_s \end{aligned}$$

$$\ddot{E}_s = \ddot{E}_d - K_{ds} K_{ss}^{-1} E_s = [K_{dd}] q_d$$

coupled stiffness matrix
linked to dynamic d.o.f. only

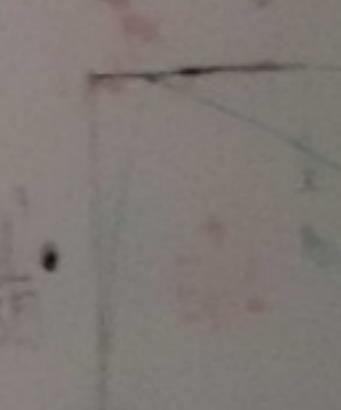
Determination of stiffness components: displacement method.



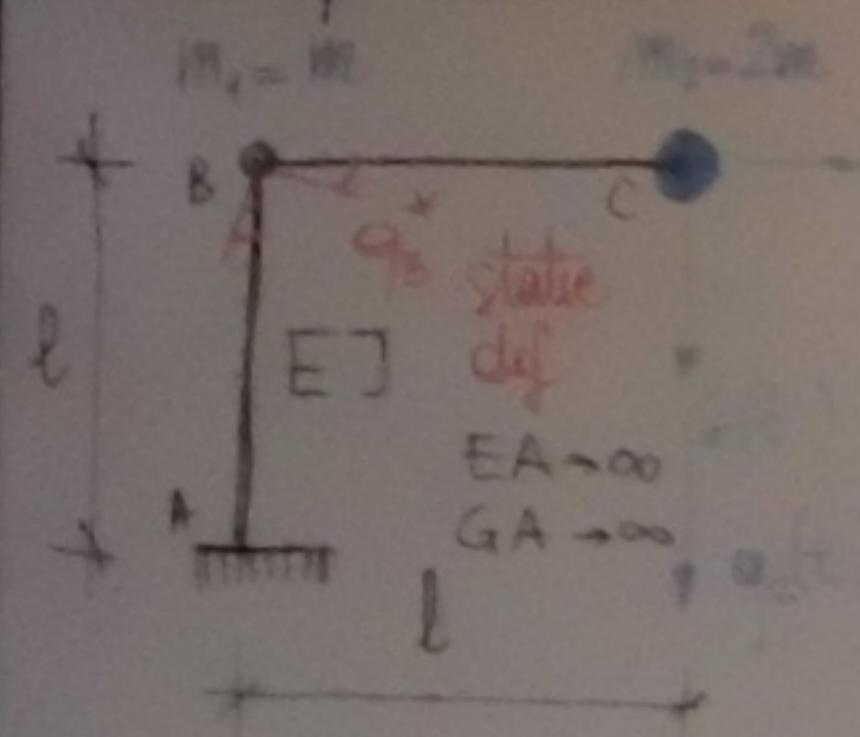
$$K_{dd} = \frac{E}{l^3} \left[\begin{bmatrix} 12 & 0 & -6l \\ 0 & 3 & -3l \\ -6l & -3l & 7 \end{bmatrix} \right]^{-1}$$

(Compliance)
Influence coeff. (force method)

$$\begin{aligned} K_{11} &= \frac{1}{3} E \\ K_{12} &= \frac{1}{3} E \\ K_{21} &= \frac{1}{3} E \\ K_{22} &= \frac{1}{3} E \end{aligned}$$



Example (2-DOF)



- Partition of dofs (and attachment of F and K)

$$E = \begin{bmatrix} EJ \\ EA \end{bmatrix}, \begin{bmatrix} q_1 \\ q_2 \end{bmatrix} = \begin{bmatrix} q_d \\ q_s \end{bmatrix}$$

$$K = \frac{EJ}{l^3} \begin{bmatrix} 12 & 0 & -6l \\ 0 & 3 & -3l \\ -6l & -3l & 7 \end{bmatrix}$$

Explicitly:

$$\begin{cases} F_d = K_{dd} q_d + K_{ds} q_s \\ F_s = K_{sd} q_d + K_{ss} q_s \end{cases}$$

$$q_d = -K_{dd}^{-1} K_{sd} q_s - K_{dd}^{-1} K_{ss} F_s$$

condensed stiffness matrix
linked to dynamic dofs only

- Determination of stiffness coefficients (displacement method)

$$\begin{array}{c} q_1 = 1 \\ q_2 = L \\ q_3 = 1 \end{array} \quad \begin{array}{c} K_{11} = \frac{12EJ}{l^3} \\ K_{21} = 0 \\ K_{31} = -\frac{6EJ}{l^2} \end{array}$$

$$\begin{array}{c} K_{12} = 0 \\ K_{22} = \frac{3EJ}{l^3} \\ K_{32} = -\frac{3EJ}{l^2} \end{array}$$

$$\begin{array}{c} K_{13} = 0 \\ K_{23} = -\frac{3EJ}{l^2} \\ K_{33} = \frac{4EJ}{l^3} + \frac{3EJ}{l^2} \end{array}$$

$$= \frac{7EJ}{l^3}$$

(Compliance)

- Influence coeff. (force method)

$$K_{dd} = \frac{EJ}{l^3} \begin{bmatrix} 12 & 0 & -6l \\ 0 & 3 & -3l \\ -6l & -3l & 7 \end{bmatrix} = \frac{1}{6} EJ$$

$$= \frac{1}{6} \frac{l^3}{EJ}$$

• Influence Coefficients and stiffness coefficients (displacement method)

$$K = \eta^{-1}$$

$$E = E_d$$

$$K_{dd} = \frac{EJ}{l^3} \begin{bmatrix} 12 & 0 & -6l \\ 0 & 3 & -3l \\ -6l & -3l & 7 \end{bmatrix}$$

$$q_1 = 1 \quad q_2 = l$$

$$K_{11} = \frac{12E}{l^3} \quad K_{21} = 0 \quad K_{31} = -\frac{6E}{l^2}$$

$$K_{12} = 0 \quad K_{22} = \frac{3E}{l^3} \quad K_{32} = -\frac{3E}{l^2}$$

$$K_{13} = -\frac{6E}{l^2} \quad K_{23} = -\frac{3E}{l^2} \quad K_{33} = \frac{4E}{l_c} + \frac{3E}{l_b} = 7 \frac{E}{l}$$

(Compliance)

• Influence Coeff. (force method)

$$\eta_{dd} = \frac{EJ}{l^3} \begin{pmatrix} 12 & 0 & \{-6l - 3l\} \\ 0 & 3 & l \\ -6l & -3l & 7 \end{pmatrix} = \frac{6E}{7l^3} \begin{bmatrix} 8 & -3 \\ -3 & 2 \\ 7 & 18 \end{bmatrix} = \frac{1}{6} \frac{l^3}{E} \eta_{dd}^{-1}$$

$$\eta_{11} = \frac{1}{3} \frac{l^3}{EJ} \quad \eta_{21} = \eta_{31} l = \frac{1}{2} \frac{l^3}{EJ} \quad \eta_{31} = \frac{1}{2} \frac{l}{EJ}$$

$$\eta_{12} = \frac{1}{2} \frac{l^3}{EJ} \quad \eta_{22} = \eta_{32} l + \frac{1}{3} \frac{l^3}{EJ} = \frac{4}{3} \frac{l^3}{EJ} \quad \eta_{32} = \frac{l^2}{EJ}$$

$$\eta_{13} = \frac{1}{2} \frac{l^2}{EJ} \quad \eta_{23} = \frac{l}{EJ} \quad \eta_{33} = \frac{l}{EJ}$$

$$E_d = K_{dd} q_d + K_{ds} q_s \rightarrow q_d = -K_{dd}^{-1} K_{ds} q_s + K_{ds} E_s$$

$$E_s = K_{sd} q_d + K_{ss} q_s \rightarrow q_s = -K_{ss}^{-1} K_{sd} q_d + K_{sd} E_s$$

$$K_{ds} = K_{dd} - K_{ds} K_{ss}^{-1} K_{sd}$$

Observe stiffness matrix linked to determine stiffness only