

bine of a convert and a convert according to
$$A(t) = \int 180t$$
 $t < 0.5$

Delay out ticket counter = area (AOAB)=38.56 hr. Delay blo counter & security = area (AOBC)=84.3 hr. Total delay = 38.96+84.3 = 123.26 hr.

Scanned with CamScanner

(b) If ticket counter capacity is increased

> to 180/hr, the curve 0+B+c defining departure from counter will be changed to 0 + A + C, as the counters will be able to serve everything.

However, the delay overall will still be area (ADAe) = 123.26 hr.

So, there won't be any change.

- (e) If we bump the security coupacity to 70/hr or byond (like 180/hr) as the question asks, the culve 0 -> c defining departure from eccurity changes to $0 \rightarrow B \rightarrow C$. Hence the net delay becomes area (A DAB). . The delay is reduced by the pentine delay of counter I security i.e. by 84.3 hr.
- (d) To gword complications, we keep the highet counter and socuring potrat we cosentialty see them is one single server
- (d) Tust by increasing the capacity of security, we can reduce delay by 84.3 ×100 = 68.39 %. To get better 123.26 results like to%, we will need to improve both Counter and security forther Scanned with CamScanner

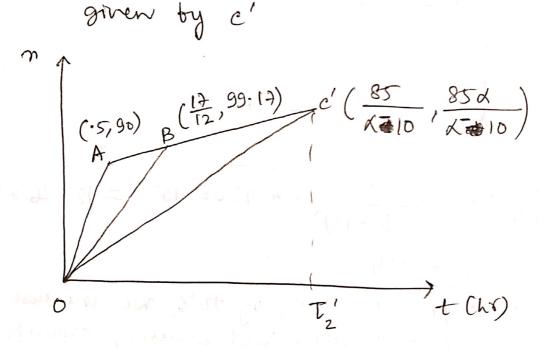
better is the cheapest method of bettering the system.

Theoreme

$$D_{2}'(t) = \int dt \qquad t < \tau_{2}'$$

85+10+ t > \tau_{2}'

So, the net departure curve is given by c'



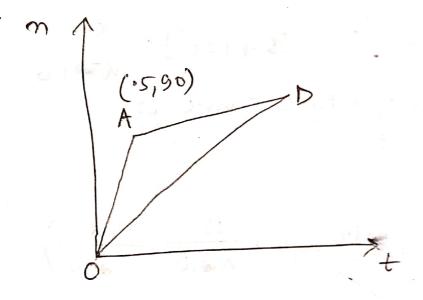
$$4\sqrt{(180-4)^{2}} = 7.4508 \times 10^{-9} = 123.26 \times 0.5$$

d ≈ 53.59 pers

So, to reduce delay by 50°%, we increase the capacity of security to 53.59 pm/hr without changes to ticket counter.

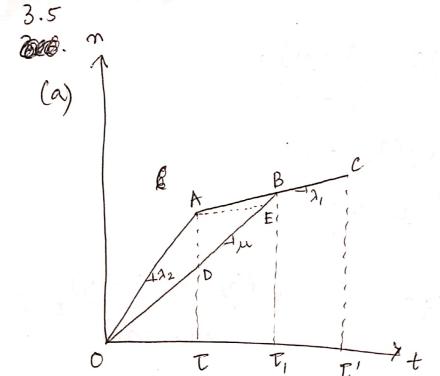
Apres

For 70% delay reduction, Both ticket counter and scenity capacity should be increased, to B.



$$4\sqrt{(180-\beta)^2 85^2} - 7.4508 \times 10^{-9} = 123.26 \times 0.3$$

so, to reduce delay by 70%, we increase both ticket counter and security capacity to 72.04 pays/hr.



line ABC
equation:

$$n = \frac{\lambda_{2}}{\lambda_{2}} + \lambda_{1}(t-t)$$

 $= (\lambda_{2} - \lambda_{1})t + \lambda_{1}t$

$$A(t) = \begin{cases} \lambda_2 t & t < T \\ (\lambda_2 - \lambda_1)T + \lambda_1 t & t > T \end{cases}$$

$$T_1 = \text{solve} \left((\lambda_2 - \lambda_1)t + \lambda_1 t = \mu t \right)$$

$$= \frac{\lambda_2 - \lambda_1}{\mu - \lambda_1} t$$

6000 Tota Maximum queue length = AD = (2-m)T

Longest delay to a customer $= \frac{\lambda_2 t}{\mu} - t = t (\lambda_2 - \mu)$ Duration of queue = $t_1 = \frac{\lambda_2 - \lambda_1}{\mu - \lambda_1} t$

Total delay = area (A OAB) = area (AOAD) + area (A ABD) $= \frac{1}{2} \left[AD \times T_{6} + AD \left(T_{1} - T \right) \right] = \left(\frac{\lambda_{2} \mu_{1}}{\mu_{1}} \right) TT_{6}$

Total delay
$$= \frac{(\lambda_2 - \mu) \tau}{2} \cdot \frac{(\lambda_2 - \lambda_1) \tau}{(\mu - \lambda_1)}$$

$$= \frac{(\lambda_2 - \mu)(\lambda_2 - \lambda_1) \tau}{2(\mu - \lambda_1)}$$

(b) Total cost = BM +
$$(\lambda_2-\mu)(\lambda_2-\lambda_1)$$
 $t^2 8$

$$\frac{dc}{d\mu} = 0 \Rightarrow \beta + 8t^{\gamma} \left[-\frac{(\lambda_2 - \lambda_1)^{\gamma}}{2(3\mu - \lambda_1)^{\gamma}} \right] = 0$$

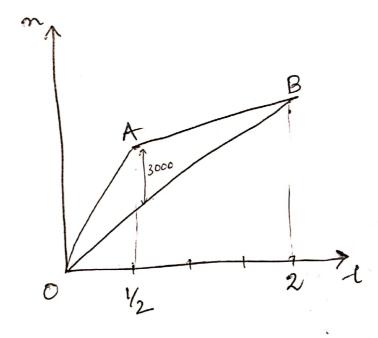
$$\Rightarrow \frac{(\mu - \lambda_1)^{\gamma}}{(\lambda_2 - \lambda_1)^{\gamma}} = \frac{t^{\gamma} \delta}{2\beta}$$

$$\Rightarrow \mu = \lambda_1 + \frac{t(\lambda_2 - \lambda_1)}{\sqrt{2\beta/\delta}}$$

$$A(t) = \begin{cases} 10000t & t < \frac{1}{2} \\ 2000t + 4000 & t > \frac{1}{2} \end{cases}$$

$$D(t) = \begin{cases} 4000 t & t < t \\ 2000t + 4000 & t > t \end{cases}$$

T = solve (4000t = 2000t + 4000) = 2 lr.



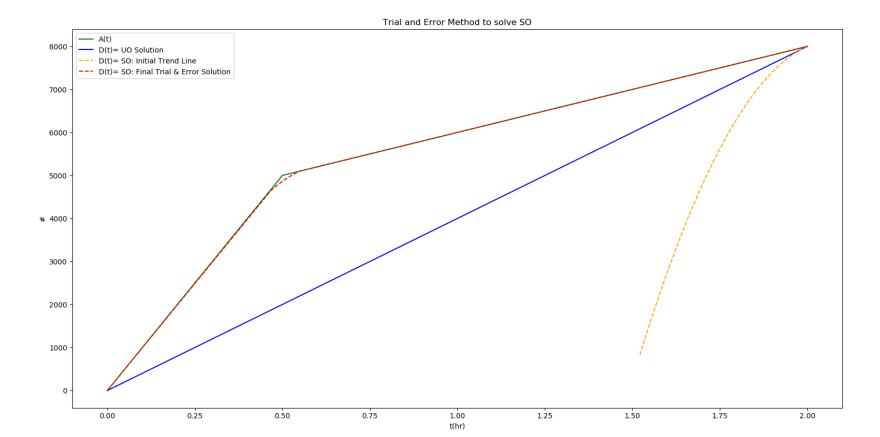
For User equilibrium, there won't ke any vehicle taking the off-ramp.

> so, the delay will be given by the area of \triangle 0AB = $\frac{1}{2} \times 3000 \times 2 = 3000$

3.6. (b) Assumption, Freeway speed = 80 Kph

(Plot win) So, taking each off-ramp causes extra time of 1 km - 1 km = parts 1/48 hr

(e) The attached plot shows overall reduction in delay agnificantly with so.



3.11.

$$A_{2}(t) = (6500 \times 15\%/hr)t$$

$$D_{2}(t) \int (500 \times 50\%/hr)t \qquad t < 20 \text{ mins}$$

$$\left(\frac{6500}{3} \times 50\%\right) \qquad 20 \text{ mins} < t < T_{0}$$

$$\frac{6500 \times 50\%/hr}{3} \qquad (6500 \times 15\%/hr)t \qquad t > T_{0}$$

$$\frac{6500 \times 15\%/hr}{3} \qquad t > T_{0}$$

where to is the time it takes to clear the OR Bottlenell.

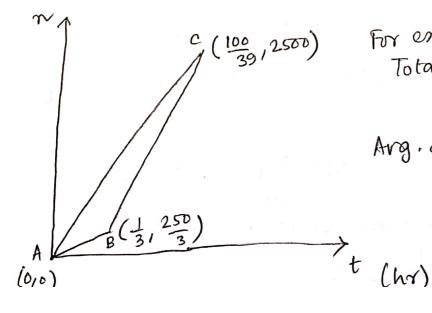
Queue formed in or in the 20 mins

$$= (6500 \times 15\% - 500 \times 50\%) \times \frac{20}{60} = \frac{725}{3} = 90$$
(say)

Queue clearing rate

$$= \left(\frac{6500}{3} \times 50\% - 6500 \times 15\right)/hr = \frac{325}{3 hr} = r$$
(Say)

$$T_0 = 20 \text{ mins} + \frac{9}{9} / \gamma = \frac{100}{39} \text{ hg}$$



For Freeway, initially it stays unaffected as the 200 m of OR gets filled up first at timestamp' t' (say). Then one lane gets blocked reducing the capacity of the Freeway to 23rd. So, 2 lanes carmy the load of non-existing vehicles till the time it takes for the off-ramp queue to be limited only to the 200m of or, which is denoted by timestamp 'T,' (say). As the third lane gets cleared, all the queues and new non-exiting vehicles get chared as the breway is sowed at full capacity. The timestamp for that clearance is denoted by 'Tz' (say).

A₁(t) =
$$(6500 \times 85\% / hr) \times t$$

D₁(t) = $\int (6500 \times 85\% / hr) t$
 $(6500 \times \frac{2}{3}) / hr \text{ slope}$
 $(6500 \times \frac{2}{3}) / hr \text{ slope}$
 $(6500 \times 85\% / hr) t$
 $(6500 \times 85\% / hr) t$
 $(6500 \times 85\% / hr) t$

Assumption! A car and allowable space behind it a queue altogether be assumed to 6m (say), so, the or holds $\frac{200}{6} = \frac{100}{3}$ relicles before affecting freeway lane at t = T

(6500 × 15% - 500 × 50%)
$$T = \frac{100}{3}$$
 $\Rightarrow T = 165.52.5$
Shalle formed in Freeway lane due to or $B_1 = 80 - \frac{100}{3} = \frac{625}{3}$

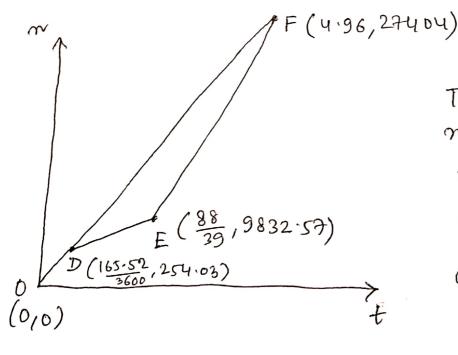
Time stamp at the which that gets decreed $E_1 = 20 \text{ mins} + \frac{g_1}{r} = \frac{88}{39} \text{ hr.}$

Total queue formed on the other 2-lanes from t = T to $t = T_1$ is given by

$$g_2 = (T_1 - T) \times \left[\frac{6500 \times 85\% - 6500 \times \frac{2}{3}}{\text{hy}} \right] \approx 2634.1$$

Rate of queue clearance at the freeway after t = T, is given by $\sigma_2 = [6500 - 6500 \times 85\%]/hr$

$$L_1 = L_1 + \frac{g_2}{r_2} \approx 4.96 \, \text{hr}.$$



Total delay for non-eniting vehicles