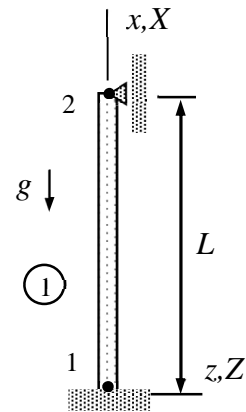


Name \_\_\_\_\_ Student number \_\_\_\_\_

## Assignment 4

A beam is loaded by its own weight as shown in the figure. Assume that displacement is confined to the  $XZ$  – plane. Derive the equilibrium equations for buckling analysis giving the axial displacement and the critical density  $\rho_{cr}$  of the material. Start with the virtual work density and approximations to the axial and transverse displacements. The cross-section properties  $A$ ,  $I$  and material properties  $E$ ,  $\rho$  are constants.



### Solution template

Virtual work expressions for the buckling analysis of a beam in  $xz$  – plane consist of the internal parts for the bar and bending modes, coupling (stability expression) between them, and virtual work of the external point force. Altogether ( $f_x = -\rho Ag$ )

$$\delta w_{\Omega} = -\frac{d^2 \delta w}{dx^2} EI_{yy} \frac{d^2 w}{dx^2} - \frac{d \delta u}{dx} EA \frac{du}{dx} - \frac{d \delta w}{dx} N \frac{dw}{dx} + \delta u f_x, \text{ where } N = EA \frac{du}{dx}.$$

In terms of the non-zero displacement/rotation components of the structural system, approximations to the axial displacement  $u$ , transverse displacement  $w$ , and the axial force  $N$  simplify to

$$u(x) = \begin{Bmatrix} 1-x/L \\ x/L \end{Bmatrix}^T \begin{Bmatrix} 0 \\ u_{X2} \end{Bmatrix} = \frac{x}{L} u_{X2},$$

$$w(x) = \begin{Bmatrix} (1-x/L)^2(1+2x/L) \\ L(1-x/L)^2 x/L \\ (3-2x/L)(x/L)^2 \\ L(x/L)^2(x/L-1) \end{Bmatrix}^T \begin{Bmatrix} 0 \\ 0 \\ 0 \\ -\theta_{Y2} \end{Bmatrix} = \frac{1}{L^2} (Lx^2 - x^3) \theta_{Y2},$$

$$N = EA \frac{du}{dx} = \frac{EA}{L} u_{X2}.$$

When the approximations are substituted there, virtual work density simplifies to (substitute the expression for the axial force  $N$  and distributed force  $f_x$ )

$$\delta w_{\Omega} = -\delta \theta_{Y2} \frac{EI}{L^4} (2L-6x)^2 \theta_{Y2} - \delta u_{X2} \frac{EA}{L^2} u_{X2} - \delta \theta_{Y2} (2Lx-3x^2)^2 u_{X2} \frac{EA}{L^5} \theta_{Y2} - \delta u_{X2} \frac{x}{L} \rho Ag.$$

Integration over the length of the beam gives

$$\delta W = \int_0^L \delta w_{\Omega} dx = -\delta\theta_{Y2} 4 \frac{EI}{L} \theta_{Y2} - \delta u_{X2} \frac{EA}{L} u_{X2} - \delta\theta_{Y2} \frac{2}{15} EA u_{X2} \theta_{Y2} - \delta u_{X2} \frac{1}{2} L \rho Ag \quad \Leftrightarrow$$

$$\delta W = - \left\{ \begin{matrix} \delta u_{X2} \\ \delta \theta_{Y2} \end{matrix} \right\}^T \left\{ \begin{matrix} \frac{EA}{L} u_{X2} + \frac{1}{2} L \rho Ag \\ (4 \frac{EI}{L} + \frac{2}{15} EA u_{X2}) \theta_{Y2} \end{matrix} \right\}.$$

Principle of virtual work and the fundamental lemma of variation calculus imply equilibrium equations

$$\left\{ \begin{matrix} \frac{EA}{L} u_{X2} + \frac{1}{2} L \rho Ag \\ (4 \frac{EI}{L} + \frac{2}{15} EA u_{X2}) \theta_{Y2} \end{matrix} \right\} = 0.$$

The first equation is linear and can be solved for the axial displacement

$$\frac{EA}{L} u_{X2} + \frac{1}{2} L \rho Ag = 0 \quad \Leftrightarrow \quad u_{X2} = - \frac{L^2}{2} \frac{\rho g}{E}.$$

When the solution to the axial displacement is substituted there, the second (non-linear) equation simplifies to

$$(4 \frac{EI}{L} - \frac{1}{15} L^2 A \rho g) \theta_{Y2} = 0.$$

The remaining task is to deduce the possible solutions: If the expression in parenthesis is non-zero, the equation implies that  $\theta_{Y2} = 0$ . If the expression in parenthesis is zero, the equation is satisfied no matter the non-zero value of  $\theta_{Y2}$ . Therefore, buckling may occur when (here density  $\rho$  stands for the loading parameter)

$$4 \frac{EI}{L} - \frac{1}{15} L^2 A \rho g = 0 \quad \Leftrightarrow \quad \rho_{cr} = 60 \frac{EI}{AgL^3}. \quad \leftarrow$$