Heavyweight Concrete: Measuring, Mixing, Transporting, and Placing

Reported by ACI Committee 304

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Presents recommended methods and procedures for measuring, mixing, transporting, and placing heavyweight concretes that are used principally for radiation shielding in nuclear construction. Also covered are recommendations on cement, heavyweight aggregates, water, and admixtures. Mixture proportioning of heavyweight concrete is discussed. Recommendations for preplaced-aggregate heavyweight concrete are also included, together with suitable grout proportions. Mixing equipment, form construction, placing procedures, and methods of consolidation are described. Quality control, inspection, and testing are emphasized, and a list of references is included.

Keywords: admixtures; aggregates; barite; cements; concrete construction; **concretes;** consolidation; construction equipment; **conveying;** density (mass/volume); formwork (construction); grout; **heavyweight aggregates; heavyweight concretes;** ilmenite; limonite; magnetite; mass concrete; materials handling; **mix proportioning; mixing; placing;** quality control; preplaced-aggregate concrete; radiation shielding; segregation.

CONTENTS

Chapter 1—General considerations, p. 304.3R-2

Chapter 2—Materials, p. 304.3R-2

- 2.1—Cement
- 2.2—Aggregate
- 2.3—Mixing water

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- 2.4—Admixtures
- 2.5—Proprietary premixed mortar

Chapter 3—Concrete characteristics, p. 304.3R-4

James S. Pierce*

- 3.1—Physical properties
- 3.2—Mixture proportioning

Chapter 4—Equipment, p. 304.3R-5

Chapter 5—Formwork, p. 304.3R-5

- 5.1—Conventional method
- 5.2—Preplaced-aggregate method

Chapter 6—Placement, p. 304.3R-5

- 6.1—Conventional method
- 6.2—Preplaced-aggregate method

Chapter 7—Quality control, p. 304.3R-6

- 7.1—Samples and tests
- 7.2—Cement
- 7.3—Aggregate
- 7.4—Admixtures
- 7.5—Control tests
- 7.6—Inspection

Chapter 8—References, p. 304.3R-7

- 8.1—Specified and/or recommended references
- 8.2—Cited references

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Table 1— Typical aggregates

| | | | Specific Gravity [†] | | Specific Gravity [†] Percent by | | by weight |
|--------------------|-------------------------|--|-------------------------------|-----------|--|-------------|-----------|
| Heavy Aggregate | Source [‡] | Composition [§] | Coarse pieces | Fine sand | Iron | Fixed water | |
| Ilmenite | Que. | Fe, Ti, O, etc. | 4.50 | 4.60 | 40 | 0 | |
| Limonite-goethite | UT, MI | $2\text{Fe}_2\text{O}_3$ - $3\text{H}_2\text{O}$ | 3.45 | 3.70 | 55 | 11 | |
| Magnetite | NV, WY | Fe ₃ O ₄ , etc. | 4.50 | 4.55 | 60 | 1 | |
| Magnetite | MT | Hydrous iron | 4.30 | 4.34 | 60 | 2-5 | |
| Barite | TN | >92 percent BaSO ₄ | 4.20 | 4.24 | 1-10 | 0 | |
| Barite | NV | >90 percent BaSO ₄ | 4.28 | 4.31 | <1 | 0 | |
| Ferrophosphorous** | | Fe ₃ P, Fe ₂ P, FeP | 6.30 | 6.28 | 70 | 0 | |
| Steel aggregate | Punchings, sheared bars | | 7.78 | | 99 | 0 | |
| Iron shot | Cold chilled shot | | | 7.50 | 99 | 0 | |

^{*}Source: Reference 4.

CHAPTER 1—GENERAL CONSIDERATIONS

The procedures for measuring, mixing, transporting, and placing heavyweight concrete are similar to those used in conventional concrete construction; however, special expertise and thorough planning are necessary for the successful completion of this type of concrete work. The use of heavyweight concrete in construction is a specialized field, and it is advisable that the work be undertaken by qualified personnel.¹

Heavyweight concrete is used in counterweights of bascule and lift bridges, but it is generally used in radiation shielding structures and differs from normal weight concrete by having a higher density and special compositions to improve its attenuation properties. When the heavyweight concrete is used to absorb gamma rays, the density and materials costs are of prime importance. When heavyweight shielding concrete is used to attenuate neutrons, sufficient material of light atomic weight, which produces hydrogen, should be included in the concrete mixture. Some aggregates are used because of their ability to retain water of crystallization at elevated temperatures, which ensures a source of hydrogen not necessarily available in heavyweight aggregates.

CHAPTER 2—MATERIALS

2.1—Cement

Cements conforming to ASTM C 150, which would be suitable for conventional concrete and produce the required physical properties, are suitable for use in heavyweight concrete. Low-alkali cement should be used when alkali-reactive constituents are present in the aggregates and a moderate or low-heat cement should be used for massive members in accordance with Chapter 14, Massive Concrete, of ACI 301. To avoid high and rapid heat of hydration and resultant cracking, it is advisable not to use Type III cement or accelerators unless the concrete temperature is controlled by specially designed refrigeration systems. Blended hydraulic cements meeting the requirements of ASTM C 595 also may be used in lieu of portland cements. However, blended hydraulic cements should be used only if their use does not reduce the density of the concrete below the specified limit.

Storage of cementitious materials should be in accordance with ACI 304R.

2.2—Aggregate

Thorough examination and evaluation of heavyweight aggregate sources are necessary to obtain material suitable for the type of shielding required.³ These sources are limited, and a material survey should be conducted to determine availability, chemical and physical qualities, and service records. Samples for test should be taken under the supervision of the owner's representative to meet requirements of ASTM C 637. The supplier's sources should be inspected to evaluate rock composition, abrasion resistance, and density since these properties may vary from one location to another within a deposit. The purchaser must realize that mineral ores are not as uniform as normal weight concrete aggregates and make appropriate allowances. Purchase orders should specify strict limits on allowable deviations for each property and make specific provisions for remedial action in the event the tolerances are not met.

Density and composition—Aggregate density and composition for radiation shielding are described in ASTM C 638 and specified in ASTM C 637. Some typical properties for shielding aggregates are shown in Table 1.

Coarse metallic aggregate for preplaced-aggregate concrete should have 100 percent retained on a 9.5-mm ($^{3}/_{8}$ -in) sieve and be reasonably free of thin, flat, or elongated pieces of metal. Maximum particle size is usually limited to $1^{1}/_{2}$ -in. (37.5-mm).

Fine metallic aggregate should consist of commercial chilled-iron or steel shot or ground iron, meeting SAE J 444a. Metallic aggregates should have a specific gravity of 7.50 or greater and be clean and free from foreign coatings of grease, oil, machine shop compounds, zinc chromate, loose scale, and dirt.

Shipment and storage—Aggregate should be shipped, handled, and sorted in a manner that will insure little loss of fines, no contamination by foreign material, and no significant aggregate breakage or segregation.

Aggregates may be shipped in heavy wooden boxes, watertight bags, steel containers, or in bulk by railroad cars or

[†]Material water-saturated, with its surface dry.

[‡]Other sources may be available.

[§]In some instances the composition may be more important than the specific gravity if the aggregate's attenuation characteristics are good.

^{**}Ferrophosphorous when used in portland cement concrete will generate flammable and possibly toxic gases that can develop high pressures if confined.

Table 2 — Typical proportions for heavyweight concrete

| | | | (| Conventionally | placed concrete | ; | | | |
|---|---------------------------|---------------|--|----------------|----------------------------------|--|----------------------|---------------|---------------|
| Density (unit weight), lb/ft ³ | Compressive strength, | Cement, | Heavy aggregate, lb/ft ³ (kg/m ³) | | Mix water, lb/ft ³ | Water content* lb/ft ³ (kg/m ³) | | | |
| (kg/m^3) | age 3 months psi (MPa) | (kg/m^3) | Fir | ne | Coa | rse | (kg/m ³) | Minimum | Maximum |
| 300 (4810) | 5000 (34.5) | 23.5 (376) | Iron shot | 195 (3120) | | | 12.0 (192) | 3.5 (56) | 12.0 (192) |
| | | | Magnetite | 70 (1120) | | | | | |
| 300 (4810) | 4870 (33.6) | 24.1 (386) | Ferrophospho- rous | 92 (1470) | Ferrophospho- rous | 171 (2740) | 12.7 (203) | 3.6 (58) | 12.7 (203) |
| 262 (4200) | 5350 (36.9) | 23.7 (380) | Ferrophospho- rous | 70 (1120) | Ferrophospho- rous | 70 (1120) | 12.8 (205) | 3.6 (58) | 12.8 (205) |
| | | | Barite | 35 (560) | Barite | 50 (800) | | | |
| 232 (3720) | 6500 (44.8) | 24.3 (389) | Magnetite | 86 (1380) | Magnetite | 110 (1760) | 11.5 (184) | 5.7 (91) | 13.5 (216) |
| 222 (3560) | 6000 (41.4) | 19.3 (309) | Barite | 86 (1380) | Barite | 105 (1680) | 11.6 (186) | 2.9 (46) | 11.6 (186) |
| 219 (3510) | 6500 (44.8) | 24.9 (399) | Hydrous iron ore | 82 (1310) | Hydrous iron ore | 100 (1600) | 12.0 (192) | 9.2 (147) | 17.5 (280) |
| 190 (3040) | 5750 (39.6) | 20.9 (335) | Serpentine | 50 (800) | Magnetite | 106 (1700) | 13.0 (208) | 9.1 (146) | 19.0 (304) |
| | | | | | gregate method | | | | |
| 346 (5540) | 3000 (207) | 20.6 (330) | Magnetite | 44 (700) | Punchings | 270 (4330) | 11.3 (181) | 3.5 (56) | 11.8 (189) |
| 300 (4810) | 5000 (34.5) | 19.8 (317) | Magnetite | 42 (670) | Magnetite | 67 (1070) | 10.9 (175) | 4.1 (66) | 12.0 (192) |
| | | | | | Punchings | 160 (2560) | | | |
| 263 (4210) | 6000 (41.4) | 22.2 (356) | Limonite | 28 (450) | Limonite | 60 (960) | 12.2 (195) | 13.0 (208) | 21.9 (351) |
| | | | | | Punchings | 140 (2240) | | | |
| 262 (4200) | 4800 (33.1) | 19.8 (317) | Magnetite | 42 (670) | Magnetite | 122 (1950) | 10.9 (175) | 4.7 (75) | 12.6 (202) |
| | | | | | Punchings | 67 (1070) | | | |
| 245 (3920) | | 19.5 (312) | Serpentine | 23 (370) | Serpentine | 48 (769) | 9.8 (157) | | |
| | | | | | Punchings | 145 (2320) | | | |
| 244 (3910) | 5000 (34.5) | 17.5 (280) | Magnetite | 37 (590) | Magnetite | 180 (2880) | 9.7 (155) | 4.8 (77) | 11.9 (191) |
| 215 (3440) | 5000 (34.5) | 22.7 (364) | Limonite | 29 (460) | Limonite | 28 (450) | 12.5 (200) | 10.9 (175) | 20.0 (320) |
| | n water content is | | han concrete is we | t Minimum wa | Magnetite | 122 (1950) | as to constant wa | | C) Diffor |

^{*}Maximum water content is water weight when concrete is wet. Minimum water content is amount left after drying to constant weight at 185 F (85 C). Difference between the maximum water content and the amount of mix water added is the water of crystallization held by the aggregate. The difference between the minimum water content and the water of crystallization is the water retained by the hardened cement paste.

Table 3— Typical grout proportions

| | Sand-cement ratio by | Sand-cement ratio by Water-cement ratio by | | Wet weight | | |
|--------------|----------------------|--|--------------------|-------------------|--|--|
| Grout Sand | weight | weight | lb/ft ³ | kg/m ³ | | |
| Magnetite | 2.13 | 0.55 | 170 | 2720 | | |
| Barite | 1.49 | 0.54 | 155 | 2480 | | |
| Limonite | 1.28 | 0.55 | 146 | 2336 | | |
| Conventional | 1.00 | 0.42 | 129 | 2064 | | |

trucks. Storage should be as near the batch plant as possible, and protected from moisture. The net weight of aggregate in each container should be plainly marked on the container.

2.3—Mixing water

Water and ice to be used in heavyweight concrete should conform to the requirements of ACI 301 or ASTM C 94. In addition, water should be clean and free from injurious amounts of oil, acid, alkali, organic matter, or other deleterious substances.

2.4—Admixtures

Conventionally placed heavyweight concrete may contain a chemical admixture meeting ASTM C 494 requirements for Type A or D. Type F or G may also be used to improve the placeability and reduce the tendency for segregation. When Type F or G is used, caution must be used to be sure the mixture is not over-vibrated and the heavyweight particles segregated. Air-entraining admixtures are not generally used in concrete that is not exposed to freezing and thawing because their use would tend to decrease the density of the concrete. However, if the concrete mixtures have sufficient density to allow 4 percent entrained air, there are definite advantages to be realized: reduced bleeding, greater workability, and a more homogeneous concrete. Preplaced-aggregate grout should contain a fluidifier conforming to ASTM C 937. Admixtures should be stored in accordance with requirements of ACI 304R.

Fly ash meeting the requirements of ASTM C 618 may be used to enhance the workability of heavyweight mixtures. Other mineral admixtures such as natural pozzolans and silica fume also may be used. These admixtures should meet the requirements of ASTM C 618 and C 1240, respectively. However, their effect on the concrete density must be evaluated and the admixtures omitted if the resulting concrete density is lower than that specified.

2.5—Proprietary premixed mortar

Iron mortar concretes produced commercially by manufacturers for shielding concrete have been tested and found adequate. However, these materials should be tested prior to use for density, compressive strength, and the necessary properties for shielding.

CHAPTER 3—CONCRETE CHARACTERISTICS

3.1—Physical properties

High modulus of elasticity, low coefficient of thermal expansion, and low elastic and creep deformations are ideal properties for both conventional structural concrete and heavyweight concrete. Heavyweight concrete is used when

the thickness of the concrete is limited. High compressive strengths may be required if heavyweight concrete is to be subjected to high temperatures and stress levels. This strength requirement may necessitate the use of a low watercement ratio, and if placing conditions are difficult, a high cement content will also be required for workability, or a Type F or G chemical admixture may be used. Heavyweight concrete with high cement content and low water-cement ratio may exhibit increased creep and shrinkage, and in a massive concrete placement could generate high temperatures at early ages causing undesirable localized cracking from the thermally induced stresses and strains. 4 When structural considerations require this cracking potential to be eliminated, it will be necessary to use appropriate temperature control measures that could include precooling or postcooling the concrete, or both, as described in ACI 207.1R, ACI 207.2R, and ACI 224R.

3.2—Mixture proportioning

Procedures outlined in ACI 211.1 should be used for mixture proportioning. Typical proportions for heavyweight concrete are shown in Table 2.

Conventionally placed heavyweight concrete—The concrete mixture should be proportioned to provide the desired compressive strength, density, and adequate workability. Also, the chemical constituents and fixed water content of the resulting mixture must provide satisfactory shielding properties. ⁵ (Fixed water is water necessary to supply hydrogen for shielding.)

Preplaced-aggregate heavyweight concrete—Coarse aggregate for heavyweight preplaced-aggregate concrete may be a heavyweight mineral aggregate, a blend of mineral aggregates, or a blend of mineral aggregates and steel particles. Because of high cost, steel particles are rarely used alone as coarse aggregate. Unpublished laboratory data of contractorengineers employed by the U.S. Atomic Energy Commission (now the Nuclear Regulatory Commission) at Hanford, WA, indicate that if steel punchings or smooth cubical pieces of steel or iron are used alone as coarse aggregate, compressive strengths will not exceed 3000 psi (20.7 MPa), regardless of the grout mixture or water-cement ratio used. However, when sheared reinforcing bars are used as aggregate with similar grout mixtures, concrete of normal strength will be produced. The compressive strengths of typical preplaced-aggregate concrete mixtures are shown in Table 2. Coarse aggregates should conform with Grading 1 in Table 7.2 of ACI 304R. Form configuration and embedment limitations usually preclude the use of coarse aggregate larger than $1^{1}/_{2}$ in. (37.5) mm). Sand grading should be within the limits but on the finer side of the grading shown in Table 7.2 (ACI 304R) since the smaller particles show less tendency to segregate. Some heavyweight fine aggregate, e.g., ferrophosphorous, when used in grouts causes excessive bleeding and produces grouts that are extremely difficult to pump.

Grout used in heavyweight preplaced-aggregate concrete should contain more cement than that used in normal density preplaced-aggregate concrete. Preplaced coarse aggregate proportions will vary in accordance with requirements for strength, density, and fixed water. Typical grout proportions are shown in Table 3. Guidance on proportioning grout mixtures is also available in ASTM C 938.

Fluidifier admixtures as described in ACI 304R must be used in the grout to insure complete filling of the voids among coarse aggregate particles, to obtain controlled expansion of the matrix during its plastic stage for optimum coarse aggregate contact, to minimize bleeding, and to obtain satisfactory contact of the concrete with embedded items.

Fly ash, which is frequently used with normal density preplaced-aggregate concrete, is used in heavyweight concrete only when its use does not reduce the concrete density below the specified limit. In addition, the lime-pozzolan hydration reaction is not as effective as the portland cement hydration action in chemically fixing the water for neutron attenuation.

CHAPTER 4—EQUIPMENT

Standard mixing equipment is generally used to mix heavyweight concrete. Special care should be taken not to overload the equipment. In general, the allowable volume of heavyweight concrete mixed should be equivalent to the mix weight of normal density concrete rather than the volume capacity of the mixing equipment. Transporting heavyweight concrete without agitation tends to cause excessive consolidation or packing.

The use of long, rigid chutes or drop pipes should be avoided. Where concrete must be placed in narrow forms or through restricted areas, a short flexible type drop chute that tends to collapse and restrain the fall of heavyweight concrete should be employed.

CHAPTER 5—FORMWORK

5.1—Conventional method

Formwork should conform to ACI 347R. The form designer should be aware that formwork for conventionally placed heavyweight concrete must be necessarily stronger than comparable formwork for ordinary concrete simply by reason of increased concrete density.

Typical structures will be of a complex shape and will contain many penetrations through the formwork. The strutting and bracing system must be carefully designed to avoid unintentionally placing a load on penetrating members and to insure precise alignment of external fixtures corresponding to these penetrations. In a typically complex placement, consideration should be given to the use of permanent steel forms as sheeting.

Steel penetrations are often precisely machined and fabricated assemblies that may be subject to delays in delivery. It may be prudent to allow for such delays by providing for blockouts to receive these penetrations, the blockouts to be

provided with normal bends or step configuration to reduce the possibility of radiation streaming, or leakage that could occur with straight-line joints. The basic structure then may be completed around the blockouts. After the items to be embedded are placed, the blockouts are filled with heavyweight grout. Precautions should be taken to insure that penetrations and blockouts are tightly grouted.

5.2—Preplaced-aggregate method

The precautions normally required for formwork for preplaced-aggregate concrete of conventional density are applicable. Additionally, particular care must be taken to ensure a grout-tight seal at those points where penetrations or embedded items pass through the form. Concentrations of electrical conduits, in particular, will present a problem if due care is not exercised.

In those instances where voids within the preplaced-aggregate must be measured by flooding with water to determine precise void content, unusual care must be taken not only to make forms grout-tight but also watertight. It is usually impossible to attempt construction of watertight forms other than by using permanent steel liners as the sheeting material, in which case penetrations will be welded, threaded, or gasketed.

If grout is to be injected through permanent steel sheeting by means of horizontal insert piping, insert holes should be drilled, tapped, and provided with a simple compression packer to allow gradual withdrawal of the insert pipe during grout injection without objectionable loss of grout around the insert pipe.

CHAPTER 6—PLACEMENT

6.1—Conventional method

Placement of conventionally mixed heavyweight concrete is subject to the same considerations of quality control as normal density concrete, except that it is far more susceptible to variations in quality due to improper handling. It is particularly subject to segregation during placement. Segregation of heavyweight concrete results not only in variation of strength but, far more importantly, in variations in density that are intolerable for work of this type, since this adversely affects shielding properties.

The placement of heavyweight concrete is frequently required in areas that are congested with reinforcing steel, penetrations, and instrumentation. Placement conditions dictate the strictest observance of good placement practice. Because of the complexity of forms and embedments, it is usually necessary to avoid pump or "drop-pipe" placement techniques in areas that are inaccessible to direct observation by workmen. However, in other accessible areas, if the mixtures are proportioned properly, they are pumpable and will pump better at a lower slump than normal weight concrete. Ferrophosphorous concrete because of its porosity is especially difficult to pump; therefore, pumping is not recommended for this concrete. Heavyweight concrete usually will not "flow" in a form and must be placed in each discrete area and compacted in place with a minimum of vibration. Under no circumstances should an attempt be made to move heavyweight concrete with vibration equipment. Lifts should be limited to a maximum 12-in. (300-mm) thickness.

Consolidation—Consolidation should conform to ACI 309R, Chapter 14, and should be by internal vibrators to achieve uniform and optimum density. In heavyweight concrete, vibrators have a smaller effective area, or radius of action; therefore, greater care must be exercised to insure that the concrete is properly consolidated. Vibrators should be inserted at closely spaced intervals and only to a depth sufficient to cause complete intermixing of adjacent lifts.

Cleanup—Vibration and revibration to remove entrapped air and to establish aggregate-to-aggregate contact cause an excessive amount of grout to collect on the top of lift surfaces. This grout matrix (up to 3 in. [75 mm] thick) should be removed from the lift surface at the completion of each placement while the concrete is still in a plastic state.

6.2—Preplaced-aggregate method

The preplaced-aggregate method is particularly well-suited for use with concretes containing steel punchings or particles as part or all of the coarse aggregate matrix, because the steel particles tend to segregate easily when placed by conventional methods. Additional guidance on the preplaced-aggregate method may be found in ACI 304R and ACI 304.1R.

Washing, blending, and handling coarse aggregate—For heavyweight preplaced-aggregate concrete it is essential that coarse aggregate particles be thoroughly washed and be free of any undersized particles prior to placement in the forms to insure unrestricted grout flow through the coarse aggregate matrix. Many of the heavyweight minerals are more susceptible to breakage than normal density crushed rock or gravel. Washing is normally accomplished in a trommel type or rotary wash screen. Frequently it is desirable to blend two types of aggregates by stream blending them into a trommel washer, thus achieving the density, washing, and blending at minimum cost. Careful preliminary tests and repeated quality control tests during this operation must be conducted to make allowances for loss through breakage of each of the two types of aggregates.

A second method of blending can also be accomplished by washing the two coarse aggregate fractions separately and subsequently stream blending them onto the conveyor belt that is transferring them to the conveying mechanism. Although the equipment required in this method is more expensive, it has the advantage of permitting aggregate to be washed and stored in advance of placing requirements. Satisfactory results are obtained with this method if care is exercised in the operation of the blending gates.

Another coarse aggregate blending method involves washing the various fractions separately, weighing and batching them individually into a blending device such as a concrete mixer, mixing them briefly, and then discharging them through a second washing screen.

Steel punchings as delivered from the mill are usually coated with a heavy cutting oil. Degreasing is normally accomplished by tumbling the punchings in perchloroethylene using a transit mix truck. This is followed by rinsing over a washer. The solvent is saved and recirculated to the truck.

After the blending operation, the coarse aggregates are charged into concrete buckets or other means of conveyance, delivered to the forms, and placed in shallow lifts. Short, flexible drop pipes are ordinarily used to avoid breakage of the coarse aggregate and damage to embedded items, and to reduce hand placement of coarse aggregate within the forms. Frequently, hand packing coarse aggregate around embedded items and in severely restricted areas may be required. Coarse aggregate can be tamped lightly after placement, but heavy rodding should be avoided to prevent coarse aggregate breakage or damage to instrumentation. Movement of coarse aggregate particles into restricted areas may be accomplished by use of an air lance, using a short length of 3/8-in. (10-mm) pipe. Placement by this method should be limited insofar as possible to avoid segregation.

Grout—Procedures for mixing and placing heavyweight grout are the same as those employed with normal density grout. However, because of greater tendency for segregation, line blockages are more frequent. Therefore, ample preparations should be made for rapid clearing of grout hoses and pipes, and ample standby equipment should be available. Since some heavyweight grouts are difficult to pump, a trial is recommended prior to actual operation.

CHAPTER 7—QUALITY CONTROL

7.1—Samples and tests

Heavyweight concrete materials and heavyweight concrete should be sampled and tested prior to and during construction to insure conformance with applicable standards and specifications. Guidance presented in these standards as well as previous experience with the same materials will determine the required frequency of testing.

The complexity of structures in which heavyweight concrete is placed usually precludes the possibility of taking test cores. Furthermore, extraction of test cores from concrete containing coarse metallic aggregate particles is costly. Unless great care is exercised, the metallic particles will be torn loose from the matrix, destroying both the bit and the core sample. It is, therefore, of the utmost importance that a thorough quality control program be established prior to the start of construction and maintained throughout the duration of construction.

7.2—Cement

The cement should comply with the requirements of ASTM C 150 or C 595. On projects requiring the submission of mill tests, comparative tests should also be made on samples taken from the cement received at the batch plant. Optional requirements such as heat of hydration, alkali content, and false set properties are important design considerations for some types of construction.

7.3—Aggregate

The quality of the aggregates should comply with the requirements of ASTM C 33 for normal weight aggregates and ASTM C 637 for heavyweight aggregates. Other special requirements that may be requested by the designer include the fixed water content and limitations on specific chemical con-

stituents. When ferrophosphorous aggregates are used, tests should be made to determine if gases (nontoxic) might be released during construction. Aggregates should be carefully checked again after delivery to ensure that they conform completely with purchase specifications.

Other conditions that are frequently evaluated are the effects of deleterious substances or aggregate coatings on concrete strength or the promotion of corrosion in metallic aggregates or embedments.

7.4—Admixtures

Chemical admixtures used in the concrete should meet ASTM C 494 requirements. Detailed recommendations for controlling, storing, and handling of admixtures are given in ACI 212.3R. As stated previously, pozzolans and silica fume used in heavyweight concrete should meet the requirements of ASTM C 618 and C 1240, respectively. Additional guidance on the use of fly ash in concrete is given in ACI 226.3R.

7.5—Control tests

Conventional heavyweight concrete—The quality of the concrete produced should be controlled by an established program of sampling and testing. This program should conform to the requirements of national standards such as ACI 318 and to the requirements of the construction specifications. Quality control tests of freshly mixed concrete should include tests of density, temperature, slump, and air content made in accordance with appropriate ASTM test methods. The limits of rejection for heavyweight concrete should be established in the construction specifications to conform to the design parameters of the structures involved. These tests and control limits are used to avoid incorporating questionable or substandard concrete in the permanent structure.

Concrete uniformity tests to evaluate the mixing of the heavyweight concrete should be performed in accordance with procedures given in ASTM C 94.

Sampling of concrete and tests for compressive strength and density of hardened concrete should be made in accordance with accepted standards.

Preplaced-aggregate heavyweight concrete—Heavy-weight preplaced-aggregate concrete is adaptable to the use of sophisticated and exacting quality control tests. For example, density may be controlled to within a fixed range of values, the extent of control depending on the complexity and importance of the project.

To accomplish this type of control, coarse aggregate should be weighed, and unit density of coarse aggregate should be determined at frequent intervals prior to placement of the coarse aggregate within the forms. If the density is particularly critical, the coarse aggregate in the forms may be flooded with water through a water meter to precisely measure the volume of voids within the coarse aggregate matrix. Records of weight of the aggregate placed, together with measurement of form volume and void volume, will give a precise measure of the density of the coarse aggregate placed.

The pumpability of grout is determined with a standard flow cone in accordance with ASTM C 939, expansion and bleeding characteristics by ASTM C 940, water retentivity by ASTM C 941, and setting characteristics by ASTM C 953. The compressive strength of preplaced-aggregate concrete should be determined in accordance with ASTM C 943 and C 39.

The density of all concrete test cylinders should be determined. Where exact density is required, 4- x 4- x 10-in. (100- x 100- x 250-mm) prisms should be cut from each test cylinder, except when steel shot or punchings are used, and the density of prism and segments removed should be compared with that of the complete cylinder for determination of the "skin effect" on density. This "skin effect" results from the density difference between the formed surface when a cylinder specimen is cast and the "fresh" surface of a sawed specimen.

7.6—Inspection

ACI 318

Concrete

The inspection of heavyweight concrete should be in accordance with applicable ACI standards (ACI 311.1R). Special consideration should be given to the use and operation of equipment used in the production and handling of heavyweight concrete because of its greater density as compared with normal weight concrete. Reduced mixer loading and longer mixing time are usually required for satisfactory results.

Other than special modifications required by high density, those items emphasized as important in ACI 304R are also important inspection items for heavyweight concrete.

CHAPTER 8—REFERENCES

8.1—Specified and/or recommended references

The documents of the various standards-producing organizations referred to in this document are listed below with their serial designation. The documents listed were the latest effort at the time this document was revised. Since some of these documents are revised frequently, generally in minor detail only, the user of this document should check directly with the sponsoring group if reference to the latest revision is desired.

| American (| Concrete Institute |
|------------|---|
| ACI 207.1R | Mass Concrete |
| ACI 207.2R | Effect of Restraint, Volume Change, and Re- |
| | inforcement on Cracking of Mass Concrete |
| ACI 211.1 | Standard Practice for Selecting Proportions |
| | for Normal, Heavyweight, and Mass Concrete |
| ACI 212.3R | Chemical Admixtures for Concrete |
| ACI 224R | Control of Cracking in Concrete Structures |
| ACI 226.3R | Use of Fly Ash in Concrete |
| ACI 301 | Specifications for Structural Concrete for Build- |
| | ings |
| ACI 304R | Guide for Measuring, Mixing, Transporting, |
| | and Placing Concrete |
| ACI 304.1R | Guide for the Use of Preplaced Aggregate |
| | Concrete for Structural and Mass Concrete |
| | Applications |
| ACI 309R | Guide for Consolidation of Concrete |
| ACI 311.1R | ACI Manual of Concrete Inspection—SP-2 |

Building Code Requirements for Structural

| ACI 347R | Guide to Formwork for Concrete |
|-----------|---|
| ASTM Star | ndards |
| C 33 | Specification for Concrete Aggregates |
| C 39 | Test Method for Compressive Strength of Cy- |
| | lindrical Concrete Specimens |
| C 94 | Specification for Ready-Mixed Concrete |
| C 150 | Specification for Portland Cement |
| C 494 | Specification for Chemical Admixtures for |
| | Concrete |
| C 595 | Specification for Blended Hydraulic Cements |
| C 618 | Specification for Fly Ash and Raw or Cal- |
| | cined Natural Pozzolan for Use as a Mineral |
| | Admixture in Coal Concrete |
| C 637 | Specification for Aggregates for Radiation- |
| | Shielding Concrete |
| C 638 | Descriptive Nomenclature of Constituents of |
| | Aggregates for Radiation-Shielding Concrete |
| C 937 | Specification for Grout Fluidifier for Pre- |
| | placed-Aggregate Concrete |
| C 938 | Practice for Proportioning Grout Mixtures for |
| | Preplaced-Aggregate Concrete |
| C 939 | Test Method for Flow of Grout for Preplaced- |
| | Aggregate Concrete (Flow Cone Method) |
| C 940 | Test Method for Expansion and Bleeding of |
| | Freshly Mixed Grouts for Preplaced-Aggre- |
| | gate Concrete in the Laboratory |
| C 941 | Test Method for Water Retentivity of Grout |
| | Mixtures for Preplaced-Aggregate Concrete |
| | in the Laboratory |
| C 943 | Practice for Making Test Cylinders and |
| | Prisms for Determining Strength and Density |
| | of Preplaced-Aggregate Concrete in the Lab- |
| G 0.70 | oratory |
| C 953 | Test Method for Time of Setting of Grouts for |
| | Preplaced-Aggregate Concrete in the Labora- |

tory

C 1240 Specification for Silica Fume for Use in Hydraulic-Cement Concrete and Mortar

Society of Automotive Engineers

SAE J 4 Specification for Cast Shot and Grit for Peening and Cleaning

The above publications may be obtained from the following organizations:

American Concrete Institute P.O. Box 9094 Farmington Hills, Mich. 48333-9094

ASTM

100 Barr Harbor Drive

West Conshohocken, Pa. 19428-2959

Society of Automotive Engineers 400 Commonwealth Dr. Warrendale, Pa. 15096-0001

8.2—Cited References

- 1. Pihlajavaara, S. E., "Preliminary Recommendation for Design, Making, and Control of Radiation Shielding Structures," *Concrete for Nuclear Reactors*, SP-34, V. 1, American Concrete Institute, Farmington Hills, Mich., 1972, pp. 57-67.
- 2. Davis, Harold S., "Concrete for Radiation Shielding—In Perspective," *Concrete for Nuclear Reactors,* SP-34, V. 1, American Concrete Institute, Farmington Hills, Mich., 1972, pp. 3-13.
- 3. Browne, R. D., and Blundell, R., "Relevance of Concrete Property Research to Pressure Vessel Design," *Concrete for Nuclear Reactors*, SP-34, V. 1, American Concrete Institute, Farmington Hills, Mich., 1972, pp. 69-102
- Nucleonics, McGraw-Hill Publishing Co., New York, June 1955, pp. 60-65.
- 5. Davis, H. S., "Iron Serpentine Concrete," *Concrete for Nuclear Reactors*, SP-34, V. 3, American Concrete Institute, Farmington Hills, Mich., 1972, pp. 1195-1224.
- 6. Davis, H. S., "N-Reactor Shielding," *Concrete for Nuclear Reactors*, SP-34, V. 2, American Concrete Institute, Farmington Hills, Mich., 1972, pp. 1109-1161.