

# 1 # Doubly Symmetric I Shape Wale Design

316000

```
1 # Input Geometry
2 begin
3
4     # Material & Total Geometry
5
6     E=29000 # ksi Elastic Modulus
7     G=11200 # ksi Shear Modulus
8     F_y=50 # ksi Yield Strength
9     L_cx=18*12 # in
10    L_cy=18*12 # in
11    L_cz=18*12 # in
12
13    # Cross Section Geometry
14
15    A=72.5 # in^2 Area
16    d=36.7 # in Total Depth
17
18    t_w=0.8 # in Web Thickness
19
20    b_f=16.5 # in Flange Width
21    t_f=1.35 # in Flange Thickness
22
23    k=2.3 # in Corner, h=d-2*k for web depth
24
25    γ=247/100 # klf Unit Weight over Length
26
27    λ_f= 6.11 # Flange Ratio
28    λ_w=40.1 # Web Ratio, can also be determined as (d-2*k)/t_w
29
30    I_x=16700
31    I_y=1010
32
33    S_x=913
34    S_y=123
35
36    r_x=15.2
37    r_y=3.74
38
39    Z_x=1030
40    Z_y=190
41
42    r_ts=4.42
43    h_0=35.4
44
45    J=34.7
46    C_w=316000
47 end
```

```

1  # Compact Check
2  begin
3
4       $\lambda_{c\_rf} = 0.56 \sqrt{E/F_y}$  # B4.1(a)
5
6       $\lambda_{c\_rw} = 1.49 \sqrt{E/F_y}$  # B4.1(a)
7
8       $\lambda_{lr} = \max(L_{cx}/r_x, L_{cy}/r_y)$  # E3-2, E3-3
9
10     # Determine F_e (Flexural Buckling / Torsional Buckling)
11
12     #  $C_w = I_y h_0^2 / 4$  # E4(c)
13      $F_e = \min(\pi^2 E / (\lambda_{lr})^2, (\pi^2 E C_w / (L_{cz}^2 + G J)) / (I_x + I_y))$  # E3-4, E4-2
14
15     # Determine F_cr (From F_e)
16
17     if  $\lambda_{lr} \leq 4.71 \sqrt{E/F_y}$  # E3(a)
18          $F_{cr} = 0.658 (F_y / F_e) F_y$  # E3-2
19     else # E3(b)
20          $F_{cr} = 0.877 F_e$  # E3-3
21     end
22
23     # Determine A_ge (Non-Slender / Slender)
24
25     if  $\lambda_f \leq \lambda_{c\_rf}$  &&  $\lambda_w \leq \lambda_{c\_rw}$  # E3, E4, E7
26          $A_{ge} = A$ 
27     else
28
29          $h = \lambda_w t_w$ 
30
31          $A_{flange} = 2 b_f t_f$ 
32          $A_{web} = h t_w$ 
33          $A_{res} = A - A_{flange} - A_{web}$  # Area except Web and Flange
34
35         # Flange
36
37         if  $\lambda_f \leq \lambda_{c\_rf} \sqrt{F_y / F_{cr}}$  # E7(a)
38              $A_{flange} = A_{flange}$  # E7-2
39         else # E7(b)
40              $c_{1\_f} = 0.22$  # E7.1
41              $c_{2\_f} = (1 - \sqrt{1 - 4 c_{1\_f}}) / 2 c_{1\_f}$  # E7-4
42              $F_{el\_f} = (c_{2\_f} \lambda_{c\_rf} / \lambda_f)^2 F_y$  # E7-5
43
44              $A_{flange} = A_{flange} * (1 - c_{1\_f} \sqrt{F_{el\_f} / F_{cr}}) \sqrt{F_{el\_f} / F_{cr}}$  # E7-3
45         end
46
47         # Web
48
49
50         if  $\lambda_w \leq \lambda_{c\_rw} \sqrt{F_y / F_{cr}}$  # E7(a)
51              $A_{web} = A_{web}$  # E7-2
52         else
53              $c_{1\_w} = 0.18$  # E7.1
54              $c_{2\_w} = (1 - \sqrt{1 - 4 c_{1\_w}}) / 2 c_{1\_w}$  # E7-4

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55         F_el_w = (c_2_w*λ_c_rw/λ_w)^2*F_y # E7-5
56
57         A_web = A_web * (1-c_1_w*sqrt(F_el_w/F_cr))*sqrt(F_el_w/F_cr) # E7-3
58     end
59
60     A_ge = A_res + A_flange + A_web
61
62 end
63
64 P_n = F_cr*A_ge # E3-1, E4-1, E7-1
65 print(A_ge)

```

72.5



```

1 begin
2     a=1
3     b=2
4     if a<=1 && b<=1
5         print(1)
6     else
7         print(0)
8     end
9 end

```

0



3

```

1 begin
2     c=2
3     c=c+1
4 end

```